

10/6 Local Form of the Balance of Linear Momentum

Result global form: $\int_{\mathcal{B}} \rho (\ddot{\mathbf{x}} - \mathbf{b}) = \int_{\partial \mathcal{B}} \hat{\mathbf{f}} dA$

Then, for an arbitrary subbody $E \subset \mathcal{B}$
we have:

$$\int_E \rho (\ddot{\mathbf{x}} - \mathbf{b}) dv = \int_{\partial E} \underline{\mathbf{t}} dA = \int_{\partial E} \underline{\underline{\sigma}} \cdot \underline{n} dA$$

$$= \int_E \operatorname{div}(\underline{\underline{\sigma}}) dv$$

↑
divergence
theorem

∴ $\int_E (\operatorname{div}(\underline{\underline{\sigma}}) + \rho \ddot{\mathbf{b}} - \rho \ddot{\mathbf{x}}) dv = 0 \quad \forall E \subset \mathcal{B}$

already
written
using
stress
strain
relation

This must be true $\forall E \subset \mathcal{B}$, therefore the
integrand must vanish:

$$\operatorname{div}(\underline{\underline{\sigma}}) + \rho \ddot{\mathbf{b}} = \rho \ddot{\mathbf{x}}_j$$

$$\sigma_{ij,j} + \rho b_i = \rho \ddot{x}_i$$

in $x \in \mathcal{B}$

Finally, for static problems:

$$\operatorname{div}(\underline{\sigma}) + \rho \underline{b} = 0 ;$$

$$\sigma_{ij,j} + \rho b_i = 0$$

$$\text{in } \underline{x} \in \Omega$$

Balance of angular momentum

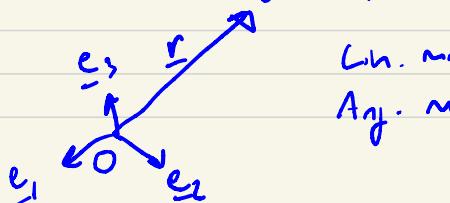
The principle of conservation of angular momentum states:

$$\frac{D}{Dt} \underline{H}_0 = \underline{M}_0^{\text{ext}} ; \quad \left\{ \begin{array}{l} \underline{H}_0 \equiv \text{angular momentum at the sys about } o \\ \underline{M}_0^{\text{ext}} \equiv \text{total external moment about } o \end{array} \right.$$

For a system of N particles we have:

$$\underline{H}_0 = \sum_{d=1}^N \underline{r}^d \times (\underline{m}^d \underline{v}^d) ;$$

$$\underline{M}_0^{\text{ext}} = \sum_{d=1}^N \underline{r}^d \times \underline{f}^d ;$$



$$\text{Lin. mom: } \underline{m}^d \underline{v}^d$$

$$\text{Ang. mom: } \underline{r}^d \times \underline{m}^d \underline{v}^d$$

These expressions can be generalized for a continuum!

For a subbody $E \subset B$ we have:

$$\underline{H}_0(E) = \int_E \underline{x} \times (\underline{dm} \dot{\underline{x}}) = \int_E \underline{x} \times (\rho \dot{\underline{x}}) d\underline{v}$$

$$\underline{M}_0^{\text{ext}}(E) = \underbrace{\int_E \underline{x} \times (\rho \underline{b}) d\underline{v}}_{\text{external}} + \underbrace{\int_{\partial E} \underline{x} \times \underline{t} dA}_{\text{traction}}$$

Resulting into:

$$\frac{D}{Dt} \int_E \underline{x} \times (\rho \dot{\underline{x}}) d\underline{v} = \int_E \underline{x} \times \rho \underline{b} d\underline{v} + \int_{\partial E} \underline{x} \times \underline{t} dA$$

In indicial notation:

$$\frac{D}{Dt} \int_E \epsilon_{ijk} x_j \dot{x}_k \rho dV = \int_E \epsilon_{ijk} x_j b_k \rho + \int_{\partial E} \epsilon_{ijk} x_j t_k dA$$

As previously done, by applying conservation of mass and $d\underline{v} = \int dV$, we can express the left term in the reference configuration, introduce the time derivative inside the integral, and then go back to the deformed configuration

Additionally, we know that $t_k = \sigma_{km} n_m$,
 leading to:

$$\int_E \frac{\partial}{\partial t} (\epsilon_{ijk} x_j \dot{x}_k) p \, dv = \int_E \epsilon_{ijk} x_j b_k p \, dv + \int_{\partial E} \epsilon_{ijk} x_j \sigma_{km} n_m \, dA$$

But $\epsilon_{ijk} x_j \sigma_{km} \equiv \Gamma_{im}$ (from free indices)
 so the divergence theorem gives

$$\int_{\partial E} \Gamma_{im} n_m \, dA = \int_E (\Gamma_{im})_{,m} \, dA$$

$$\int_E \epsilon_{ijk} (x_j \dot{x}_k + x_j \ddot{x}_k) p \, dv = \int_E \epsilon_{ijk} x_j b_k p \, dv + \int_E [\epsilon_{ijk} x_j \sigma_{km}]_{,m} \, dv$$

$$\epsilon_{ijk} \dot{x}_j \dot{x}_k = (\underline{\dot{x}} \times \underline{\dot{x}})_i = 0$$

$$(\epsilon_{ijk} x_j \sigma_{km})_{,m} = \epsilon_{ijk} \underbrace{\frac{\partial x_j}{\partial x_m} \sigma_{km}}_{\delta_{jm}} + \epsilon_{ijk} x_j \underbrace{\frac{\partial \sigma_{km}}{\partial x_m}}_{\delta_{km}}$$

$$= \epsilon_{ijk} \delta_{kj} + \epsilon_{ijk} x_j \sigma_{km, m}$$

Resulting into:

$$\int_E G_{ijk} x_j \underbrace{\left(p_{ik} - \rho b_k - \delta_{km} v_m \right)}_{=0} dv = \int_E G_{ijk} \delta_{kj} dv = 0 \quad \forall E \subset B$$

(balance of linear momentum)

Since the last equation must hold $\forall E \subset B$,
then the integrand must vanish pointwise:

$$G_{ijk} \delta_{kj} = 0$$

Expanding this expression, we get:

$$\delta_{12} = \delta_{21}, \quad \delta_{23} = \delta_{32}, \quad \delta_{13} = \delta_{31}$$

* { That is, balance of linear momentum implies that
the Cauchy stress tensor is symmetric: $\delta_{ij} = \delta_{ji}$ }

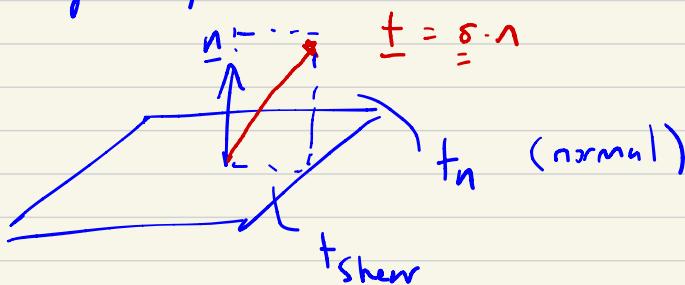
The symmetry of $\underline{\underline{\sigma}}$ has the following implications:

- (1) $\underline{\underline{\sigma}}$ has three real eigenvalues, which are called principal stresses: $\sigma_1 \geq \sigma_2 \geq \sigma_3$
- (2) one can find a basis consisting entirely of eigenvectors of $\underline{\underline{\sigma}}$ $\{\underline{v}_1, \underline{v}_2, \underline{v}_3\}$ with $\underline{v}_i \cdot \underline{v}_j = \delta_{ij}$
- so: $\underline{\underline{\sigma}} \cdot \underline{v}_i = \sigma_i \underline{v}_i$

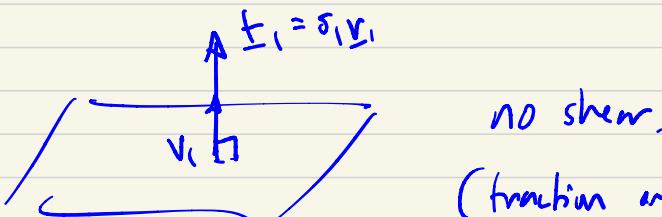
$$\underline{\underline{\sigma}} \cdot \underline{v}_2 = \sigma_2 \underline{v}_2$$

$$\underline{\underline{\sigma}} \cdot \underline{v}_3 = \sigma_3 \underline{v}_3$$

In general,



but, if we pick a face w/ normal \underline{v}



(traction only normal)

That is, for surfaces \underline{v}_i , the corresponding traction is purely normal.

(3) In the basis $\{\underline{v}_1, \underline{v}_2, \underline{v}_3\}$ the components of $\underline{\sigma}$ form a diagonal matrix:

$$[\underline{\sigma}] = \begin{bmatrix} \sigma_1 & 0 & 0 \\ 0 & \sigma_2 & 0 \\ 0 & 0 & \sigma_3 \end{bmatrix}$$

σ_i gives the normal stress in the direction \underline{v}_i , there are no shear stresses in those planes.

(4) The largest shear stress is $\frac{1}{2}(\sigma_1 - \sigma_3)$ and corresponds to directions $\frac{1}{\sqrt{2}}(\underline{v}_1 + \underline{v}_3)$

and

$$\frac{1}{\sqrt{2}}(\underline{v}_1 - \underline{v}_3)$$

(5) If $\sigma_1 = \sigma_2 = \sigma_3 = \sigma$ then, in any basis we have $\underline{\sigma} = \sigma \underline{I}$ (no shear in any direction)