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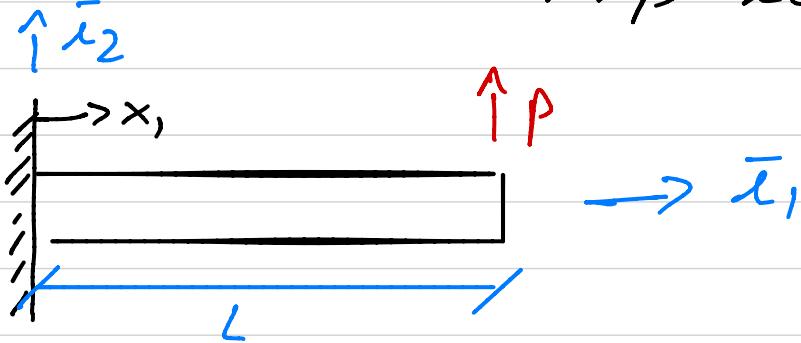
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Example: Cantilevered beam w/  
a tip load



$$\frac{d}{dx_1} \left( H_{33}^C \frac{d\phi_3(x_1)}{dx_1} \right) + K_{22} \left( \frac{dU_2(x_1)}{dx_1} - \phi_3(x_1) \right) = -\phi_3(x_1) \quad (1)$$

$$\frac{d}{dx_1} \left[ K_{22} \left( \frac{dU_2(x_1)}{dx_1} - \phi_3(x_1) \right) \right] = -P_2(x_1) \quad (2)$$

B.C.'s      @  $x_1 = 0$        $U_2 = \phi_3 = 0$

@  $x_1 = L$        $U_3 = 0, V_2 = P$

$$H_{33}^C \frac{d\phi_3}{dx_1} \Big|_L = 0, K_{22} \left( \frac{dU_2}{dx_1} - \phi_3 \right) \Big|_L = P$$

Integrating (2)

$$K_{22} \left( \frac{dU_2}{dx_1} - \phi_3 \right) = C_1 = P$$

Applying  $V_2 = P$  @  $x_1 = L$   
 $\Rightarrow C_1 = P$

(Combining w/ (1))

$$H_{33}^C \frac{d^2 \phi_3}{dx_1^2} + P = 0$$

$$H_{33}^C \frac{d\phi_3}{dx_1} = -Px_1 + C_2$$

Applying  $M_3 = 0$  @  $x_1 = L$

$$0 = -PL + C_2 \Rightarrow C_2 = PL$$

$$H_{33}^C \frac{d\phi_3}{dx_1} = P(L - x_1)$$

$$H_{33}^C \phi_3 = P \left( Lx_1 - \frac{x_1^2}{2} \right) + \cancel{C_2}$$

Applying  $\phi_3 = 0$  @  $x_1 = 0$

$$\rightarrow C_2 = 0$$

$$K_{22} \left( \frac{dU_2}{dx_1} - \frac{P}{H_{33}^C} \left( Lx_1 - \frac{x_1^2}{2} \right) \right) = P$$

$$\frac{dU_2}{dx_1} = \frac{P}{K_{22}} + \frac{P}{H_3^C} \left( Lx_1 - \frac{x_1^2}{2} \right)$$

$$U_2 = \frac{P}{K_{22}} x_1 + \frac{P}{H_3^C} \left( \frac{Lx_1^2}{2} - \frac{x_1^3}{6} \right) + C_3$$

Applying  $U_2 = 0$  @  $x_1 = 0$   
 $\rightarrow C_3 = 0$

$$U_2 = \frac{P}{K_{22}} x_1 + \frac{P}{H_3^C} \left( \frac{Lx_1^2}{2} - \frac{x_1^3}{6} \right)$$

Shear  
Contribution

EB Bending  
Solution.

Focus on tip displacement  $U_2 (x_1 = L)$

$$U_2 = \frac{PL^3}{3H_3^C} \left( 1 + \frac{3H_3^C}{K_{22}L^2} \right)$$

Tip deflection  
due to  
shear,  $\delta_s$

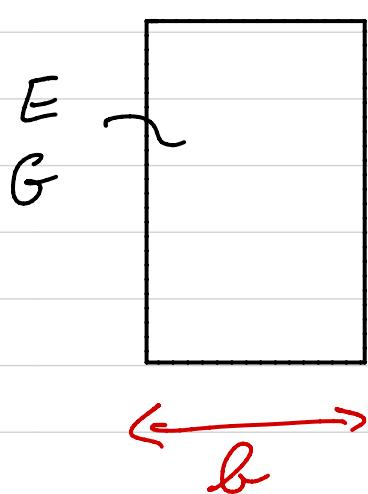
$$\frac{\delta_s}{\delta_B} = \frac{3H_3^C}{K_{22}L^2}$$

Tip deflection due to  
bending,  $\delta_B$

Now we need  $H_{33}^c$  &  $K_{22}$

## Solid Homogeneous Rectangular Bar

Bar


$$H_{33}^c = \frac{E \cancel{\ell} h^3}{12}$$
$$K_{22} = \frac{5}{G} G \cdot A \rightarrow \text{From previous equilibrium analysis.}$$

$$\frac{H_{33}^c}{K_{22} L^2} = \frac{E \cancel{\ell} h^3}{12} \frac{6}{5} \frac{1}{G \cancel{\ell} h} \frac{1}{L^2} = \frac{1}{10} \left( \frac{E}{G} \right) \left( \frac{h}{L} \right)^2$$

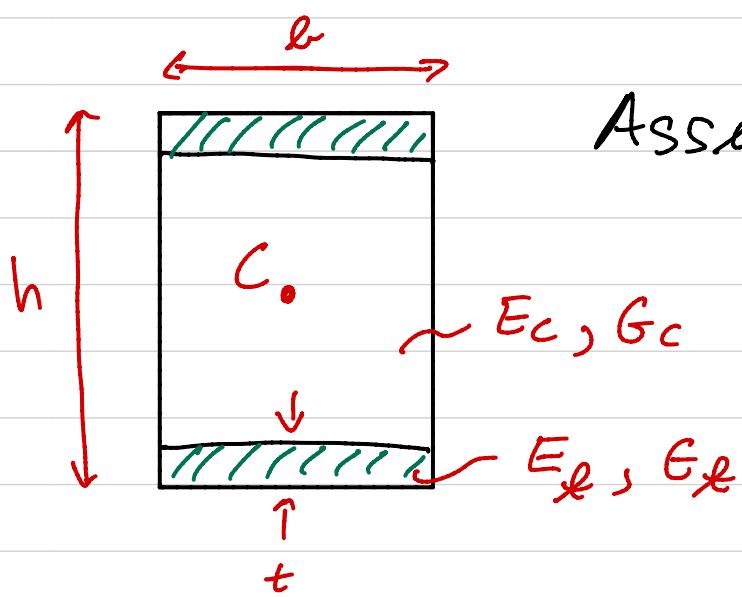
Rough Estimate for  $E/G$ ?

$$\frac{E}{G} = 2(1+\nu) \quad \nu \approx 0.3 \quad (\text{Al, Ti})$$

$$\frac{H_{33}^c}{K_{22} L^2} = 0.2 G \left( \frac{h}{L} \right)^2$$

$\rightarrow$  The importance of shear deformations decreases quadratically w/ beam slenderness.

# Sandwich Beam



Assume  $\rightarrow t \ll h$

$$\rightarrow E_c \ll E_f$$

$$E_c \approx 0 \quad (G_c \neq 0)$$

$$H_{33}^C = 2 \cdot E_f \left[ \frac{b \cdot t^3}{12} + (b \cdot t) \left( \frac{h}{2} \right)^2 \right] \stackrel{\approx 0}{\sim}$$

$$H_{33}^C = \frac{1}{2} E_f b \cdot t \cdot h^2$$

$\rightarrow$  Efficient since  $H_{33}^C \sim h^2$  if core increases  $h$ .

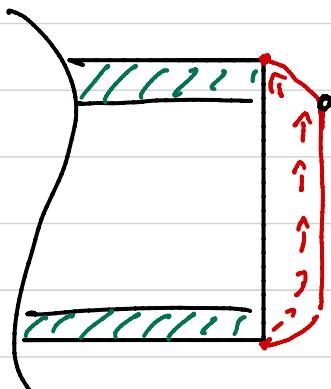
Recall from equilibrium analysis

$$\mathcal{Z}_{12} = \frac{E}{2} \left( x_2^2 - \left( \frac{h}{2} \right)^2 \right) \frac{d^2 \phi_3}{dx_1^2}$$

$$\mathcal{Z}_{12}^* = \frac{E*}{2} \left( x_2^2 - \left( \frac{h+t}{2} \right)^2 \right) \frac{d^2 \phi_3}{dx_1^2}$$

Since the core has no axial stiffness ( $E_c \approx 0$ )  $\rightarrow \sigma_1 = 0$

$$\cancel{\frac{d\sigma_1}{dx_1}} + \frac{d\mathcal{Z}_{12}}{dx_2} = 0 \rightarrow \underline{\mathcal{Z}_{12}^c = \text{constant}}$$



$$\mathcal{Z}_{12}^c = \mathcal{Z}_{12}^* \left( x_2 = \frac{h-\epsilon}{2} \right)$$

$$\mathcal{Z}_{12}^c = \frac{E*}{2} \left( \left( \frac{h-\epsilon}{2} \right)^2 - \left( \frac{h+t}{2} \right)^2 \right) \frac{d^2 \phi_3}{dx_1^2}$$

$$\mathcal{Z}_{12}^c = -\frac{1}{2} E* h t \frac{d^2 \phi_3}{dx_1^2}$$

$$V_2 = \int_{-h/2}^{h/2} \mathcal{Z}_{12}^c dA = -\frac{1}{2} E* h^2 t b \frac{d^2 \phi_3}{dx_1^2}$$

$$T_{12}^c = \frac{V_2}{hb} \rightarrow \text{CORE CARRIES ALL SHEAR!}$$

$$dE = \frac{1}{2} \int \frac{I_{12}^2}{G_c} dA dx_1 = \frac{1}{2} \frac{V_2^2}{(hb)^2} \frac{(hb)}{G_c} dx_1$$

$$dE = \frac{1}{2} \frac{V_2^2}{\underline{G_c b \cdot h}} \rightarrow K_{22} = G_c b \cdot h$$

FOR A SANDWICH BEAM

$$\frac{\sigma_s}{\sigma_b} = \frac{H_{33}^c}{K_{22} L^2} = \frac{1}{2} E_f t h^2 \cdot \frac{1}{G_c b h} \cdot \frac{1}{L^2}$$

$$\frac{\sigma_s}{\sigma_b} = \frac{1}{2} \left( \frac{E_f}{G_c} \right) \left( \frac{t}{h} \right) \left( \frac{h}{L} \right)^2$$



No intrinsic relationship between these!

Ex: Aluminium faces,  $E_t = 73 \text{ GPa}$

Aluminium Honeycomb Core,  $G_c = 1 \text{ GPa}$

$\frac{E_t}{G_c} = 73$ , compared to  $\frac{E}{G} = 2.6$  for pure aluminium.

Comparison: Solid:  $E/G = 2.6$

Sandwich:  $E_t/G_c = 73$ ,  $t/h = 1/10$

	SOLID		SANDWICH	
$h/t$	$1/10$	$1/5$	$1/10$	$1/5$
$\frac{\sigma_s}{\sigma_B}$	$0.78\%$	$3.1\%$	$10.5\%$	$42\%$