

ME 202
SPRING 2023
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Previously,

- Equilibrium of stresses/tractions
- Kinematics / Strain-Displacement Relationship
- Stress-strain relationship / Hooke's Law
- Boundary conditions

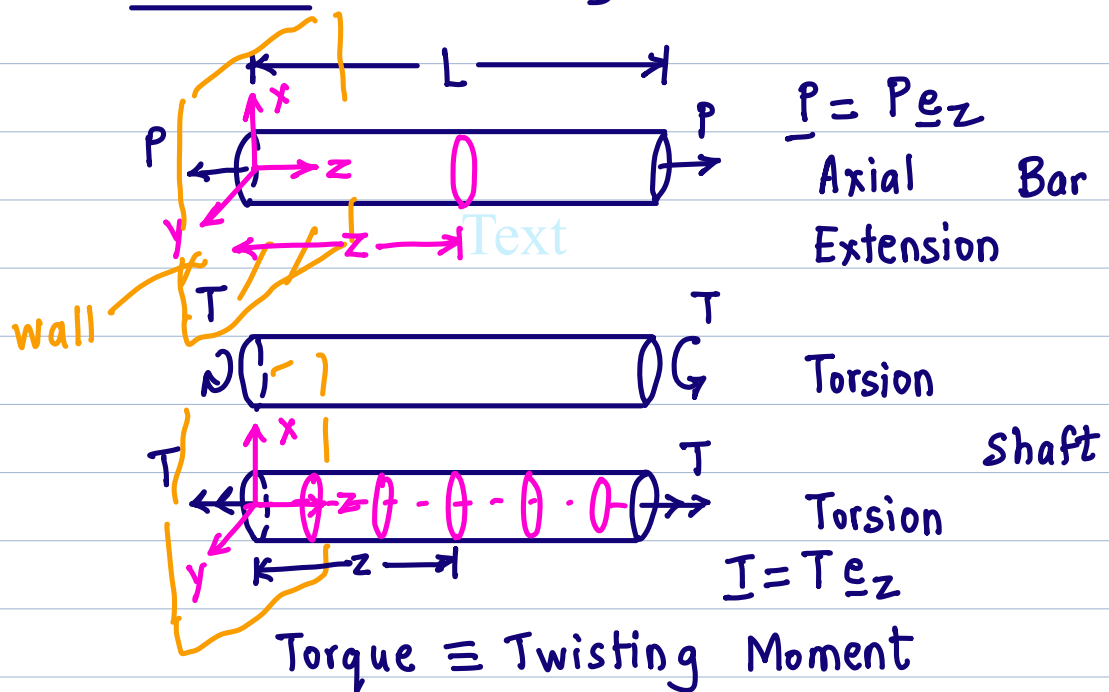
Now

- Axial deformation review
- Torsion

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TORSION

Twisting



$$\underline{e}_x = \begin{pmatrix} 1 \\ 0 \\ 0 \end{pmatrix}, \quad \underline{e}_y = \begin{pmatrix} 0 \\ 1 \\ 0 \end{pmatrix}, \quad \underline{e}_z = \begin{pmatrix} 0 \\ 0 \\ 1 \end{pmatrix}$$

$\hat{i} \qquad \hat{j} \qquad \hat{k}$

Review of Axial Extension

$$\left. \begin{matrix} u \\ v \\ w \end{matrix} \right\} \text{ disp. along } \begin{matrix} \underline{e}_x, x \text{ axis} \\ \underline{e}_y, y \text{ axis} \\ \underline{e}_z, z \text{ axis} \end{matrix}$$

Assumption $u = 0$

Kinematics $v = 0$

$$w = c_1 z$$

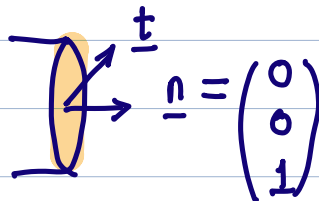
Ansatz

Strain $\epsilon_{zz} = \frac{dw}{dz} = c_1$

Hook's Law

Stress $\sigma_{zz} = E c_1$

At $z = L,$



$$\underline{t} = \begin{pmatrix} 0 \\ 0 \\ P/A \end{pmatrix} \text{ applied traction}$$

From equilibrium at $z = L,$

resultant of tractions $\int_{\Omega} \underline{t} \, da = \underline{P}$

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Recall,

$$\underline{t} = \underline{\sigma} \underline{n} = \begin{pmatrix} 0 \\ 0 \\ \sigma_{zz} \end{pmatrix} = \sigma_{zz} \underline{e}_z$$

$$\underline{\sigma} = \begin{pmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & \sigma_{zz} \end{pmatrix} \leftarrow \text{uniaxial loading}$$

$$\int_{\Omega} \sigma_{zz} da \underline{e}_z = P \underline{e}_z$$

$$\int_{\Omega} \sigma_{zz} da = P$$

$$\int_{\Omega} E c_1 da = P$$

$$E c_1 A = P \quad A \text{ area c/s}$$

$$c_1 = \frac{P}{AE}$$

$$w(z) = \frac{Pz}{AE}, \quad \delta = w(L) = \frac{PL}{AE}$$

$$\sigma_{zz} = \frac{P}{A}$$

Kinematic Assumptions



Strains $\xrightarrow{\text{Hooke's Law}}$ Stresses $\xrightarrow{\text{Equilibrium}}$ Ext. Force

Same logic for torsion.

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P.S.

check static equilibrium equation in 1D

$$\frac{d}{dz} \tau_{zz} + b = 0 \quad \text{along } z \text{ direction}$$

$$\frac{d}{dz} (Ec_1) + 0 = 0 \quad \checkmark \quad \underline{\nabla \cdot \underline{\sigma}} + \underline{b} = \underline{0}$$

This is the 1D approx solution to the 3D problem.

Theory of Failure

If brittle, $\tau_{\max} = \sigma^*$ max normal/principal stress

If ductile, $\gamma_{\max} = \tau^*$ max shear stress

$$\text{Say ductile (metal)} \quad \gamma_{\max} = \frac{\tau_{zz}}{2} = \frac{P}{2A} = \tau^*$$

from analysis \uparrow from expts

Max load that can be applied $P^* = 2A\tau^*$

$$\text{Note: } \gamma_{\max} = \max \left\{ \frac{|\sigma_1 - \sigma_2|}{2}, \frac{|\sigma_2 - \sigma_3|}{2}, \frac{|\sigma_3 - \sigma_1|}{2} \right\}$$

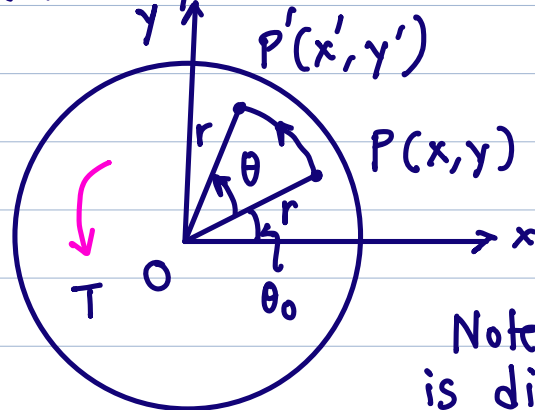
where $\sigma_1, \sigma_2, \sigma_3$ are three principal stresses (eigenvalues) of $\underline{\sigma}$

Questions

1. How much does the shaft deflect?
Angular deflection
2. What is the max torque that can applied?

Kinematic Assumptions

1. Circular c/s



Rigid Body
Rotation for
each c/s

Note: $\theta = \alpha z$
is diff for every c/s

c/s of shaft at some z coordinate

2. Let α unit angle of twist for the entire shaft $\alpha \equiv \text{rad/m}$
 $\theta(z) = \text{angle of twist for c/s at } z$
 $= \alpha z$

Total angle of twist $= \theta(L) = \alpha L$

3. Each c/s twists as if it undergoes a rigid body rotation about z-axis through angle $\theta(z)$.

$$x = r \cos \theta_0, \quad y = r \sin \theta_0$$
$$x' = r \cos(\theta + \theta_0), \quad y' = r \sin(\theta + \theta_0)$$

$$u = x' - x = \cancel{r \cos \theta \cos \theta_0} - \cancel{r \sin \theta \sin \theta_0} - \cancel{r \cos \theta_0}$$
$$v = y' - y = \cancel{r \sin \theta \cos \theta_0} + \cancel{r \cos \theta \sin \theta_0} - \cancel{r \sin \theta_0}$$
$$w = 0 \quad \text{assume, no warping}$$

3. small angular deflections

$$\theta \text{ small, } \cos \theta \simeq 1, \sin \theta \simeq \theta$$

$$u = -r \sin \theta_0 \cdot \theta = -y \alpha z = -\alpha y z$$
$$v = r \cos \theta_0 \cdot \theta = +x \cdot \alpha z = +\alpha x z$$
$$w = 0$$

Strains from displacements

2D \rightarrow 3D extend

$$\epsilon_{xx} = \frac{\partial u}{\partial x} = 0, \quad \epsilon_{xy} = \frac{1}{2} \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) = 0$$

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$$\epsilon_{yy} = \frac{\partial v}{\partial y} = 0, \quad \epsilon_{zz} = \frac{\partial w}{\partial z} = 0$$

$$\epsilon_{zx} = \frac{1}{2} \left(\frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \right) = \frac{1}{2} (-\alpha y + 0) = -\frac{\alpha y}{2}$$

$$\epsilon_{zy} = \frac{1}{2} \left(\frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \right) = \frac{1}{2} (\alpha x + 0) = \frac{\alpha x}{2}$$

strain at any point in shaft

$$\underline{\underline{\epsilon}} = \begin{pmatrix} 0 & 0 & -\alpha y/2 \\ 0 & 0 & \alpha x/2 \\ -\frac{\alpha y}{2} & \frac{\alpha x}{2} & 0 \end{pmatrix}$$

Get stresses from Hooke's Law

Isotropic. $\sigma_{xx} = 0, \sigma_{yy} = 0, \sigma_{zz} = 0$

$$\sigma_{xy} = 2G \epsilon_{xy} = 0 \quad \mu = G = \frac{E}{2(1+\nu)}$$

$$\sigma_{xz} = 2G \epsilon_{xz} = -2G \frac{\alpha y}{2} = -G \alpha y$$

$$\sigma_{yz} = 2G \epsilon_{yz} = 2G \frac{\alpha x}{2} = G \alpha x$$

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Stress / Stress tensor at any point
in the shaft

$$\underline{\underline{\sigma}} = \begin{pmatrix} 0 & 0 & -G\alpha y \\ 0 & 0 & +G\alpha x \\ -G\alpha y & +G\alpha x & 0 \end{pmatrix}$$

Note: α unknown as of now.

Note: Hooke's Law for isotropic body in 3D

$$\epsilon_{xx} = \frac{\sigma_{xx}}{E} - \frac{\nu}{E} \sigma_{yy} - \frac{\nu}{E} \sigma_{zz}$$

$$\epsilon_{yy} = \frac{\sigma_{yy}}{E} - \frac{\nu}{E} \sigma_{xx} - \frac{\nu}{E} \sigma_{zz}$$

$$\epsilon_{zz} = \frac{\sigma_{zz}}{E} - \frac{\nu}{E} \sigma_{xx} - \frac{\nu}{E} \sigma_{yy}$$

$$\epsilon_{xy} = \frac{\sigma_{xy}}{2G} = \frac{1+\nu}{E} \sigma_{xy}$$

$$\epsilon_{yz} = \frac{\sigma_{yz}}{2G} = \frac{1+\nu}{E} \sigma_{yz}$$

$$\epsilon_{zx} = \frac{\sigma_{zx}}{2G} = \frac{1+\nu}{E} \sigma_{zx}$$

Note: We use tensor shear strains in this course and not engineering shear strains

$$\epsilon_{xy} = \frac{1}{2} \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) = \frac{\gamma_{xy}}{2}$$

$$\epsilon_{yz} = \frac{1}{2} \left(\frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \right) = \frac{\gamma_{yz}}{2}$$

$$\epsilon_{zx} = \frac{1}{2} \left(\frac{\partial w}{\partial x} + \frac{\partial u}{\partial z} \right) = \frac{\gamma_{zx}}{2}$$