

## KEY IDEAS IN SOLID MECHANICS

① Equilibrium  
Static

↙ body force

$$\underline{\nabla} \cdot \underline{\sigma} + \underline{b} = \underline{0} \quad \text{Force eqm}$$

$$\underline{\sigma} = \underline{\sigma}^T \quad \text{Moment eqm}$$

$$\sigma_{xy} = \sigma_{yx}, \quad \sigma_{xz} = \sigma_{zx}, \quad \sigma_{yz} = \sigma_{zy}$$

No body moments

$$\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} + \frac{\partial \sigma_{xz}}{\partial z} + b_x = 0$$

$$\frac{\partial \sigma_{yx}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} + \frac{\partial \sigma_{yz}}{\partial z} + b_y = 0$$

$$\frac{\partial \sigma_{zx}}{\partial x} + \frac{\partial \sigma_{zy}}{\partial y} + \frac{\partial \sigma_{zz}}{\partial z} + b_z = 0$$

## ② Strain Displacement Relation SDR Kinematics . Small/Linearized Strain

$$\underline{\underline{\epsilon}} = \text{symm } \underline{\nabla} \otimes \underline{u}$$
$$= \frac{1}{2} \left( \underline{\nabla} \otimes \underline{u} + (\underline{\nabla} \otimes \underline{u})^T \right)$$

$$\underline{u} = \begin{pmatrix} u(x, y, z) \\ v(x, y, z) \\ w(x, y, z) \end{pmatrix}$$

$$\underline{\nabla} \otimes \underline{u} = \begin{pmatrix} \frac{\partial u}{\partial x} & \frac{\partial u}{\partial y} & \frac{\partial u}{\partial z} \\ \frac{\partial v}{\partial x} & \frac{\partial v}{\partial y} & \frac{\partial v}{\partial z} \\ \frac{\partial w}{\partial x} & \frac{\partial w}{\partial y} & \frac{\partial w}{\partial z} \end{pmatrix}$$

$$\epsilon_{xx} = \frac{\partial u}{\partial x}, \quad \epsilon_{yy} = \frac{\partial v}{\partial y}, \quad \epsilon_{zz} = \frac{\partial w}{\partial z}$$

$$\epsilon_{xy} = \frac{1}{2} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) = \frac{1}{2} \gamma_{xy}$$

$$\epsilon_{yz} = \frac{1}{2} \left( \frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \right) = \frac{1}{2} \gamma_{yz}$$

$$\epsilon_{zx} = \frac{1}{2} \left( \frac{\partial w}{\partial x} + \frac{\partial u}{\partial z} \right) = \frac{1}{2} \gamma_{zx}$$

### ③ Stress-Strain Relationship / Constitutive Law/ Generalized Hooke's Law

$$\underline{\underline{\sigma}} = \underline{\underline{C}} \underline{\underline{\epsilon}}, \quad \underline{\underline{\epsilon}} = \underline{\underline{S}} \underline{\underline{\sigma}}$$

$\uparrow$  stiffness tensor                       $\uparrow$  compliance tensor

81  $\rightarrow$  2 constants for isotropic materials

note: isotropy vs homogeneity

$E, \nu$  or  $E, G$  or  $G, \nu$   
↓ Young's modulus      ↗ shear modulus      ↘ Poisson's ratio

$$\epsilon_{xx} = \frac{\sigma_{xx}}{E} - \frac{\nu}{E} (\sigma_{yy} + \sigma_{zz}), \quad \epsilon_{xy} = \frac{1+\nu}{E} \sigma_{xy} = \frac{\sigma_{xy}}{2G}$$

$$\epsilon_{yy} = \frac{\sigma_{yy}}{E} - \frac{\nu}{E} (\sigma_{xx} + \sigma_{zz}), \quad \epsilon_{yz} = \frac{1+\nu}{E} \sigma_{yz} = \frac{1}{2G} \sigma_{yz}$$

$$\epsilon_{zz} = \frac{\sigma_{zz}}{E} - \frac{\nu}{E} (\sigma_{xx} + \sigma_{yy}), \quad \epsilon_{zx} = \frac{1+\nu}{E} \sigma_{zx} = \frac{1}{2G} \sigma_{zx}$$

#### ④ Boundary Conditions

Displacement BCs DBCs  $\underline{u} = \underline{u}^*$  on some boundary

Traction BCs TBCs

$\underline{t} = \underline{\sigma} \underline{n} = \underline{t}^*$  on some other boundary

↑ given

↙ given



Pointwise application. Strong Form.

$$\underline{t} = \underline{\sigma} \underline{n} = \underline{t}^* \quad \text{@ every point on boundary}$$

Integral application. Weak form

$$\int_{\partial\Omega} \underline{t} \, da = \int_{\partial\Omega} \underline{t}^* \, da$$

Resultant of  
force

Easier to work with.

## ⑤ "Theory" of Failure

Material is considered to have failed when this condition is met at a point in the solid

$$f(\underline{\sigma}, \sigma_c) = 0$$

↑ critical stress

$$f(\sigma_{xx}, \sigma_{xy}, \dots, \sigma_{zz}, \sigma_c) = 0$$

Yield stress Ductile

Fracture stress Brittle