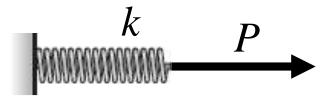
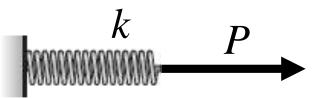
Energy Methods

Strain Energy

- We will consider a spring which has extended by an amount x very slowly over a long period of time under the action of a force P which was gradually increased from zero to its final value P very slowly over that long time.
- Essentially it means that the spring was almost at equilibrium at every stage of expansion



 The spring is uniform and linear. So the stiffness does not vary across the length. Also the force developed due to an extension x is kx and not kx³ or kx^{0.5} or ksin(x) or some other function of x.



• Consider that the spring was extended by an amount ξ and comes to an equilibrium with the external force. So the force felt by the spring is k ξ . We are not talking f the external force here, but what we will see if we cut the spring just before the free end.



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- Now the spring is further extended by a very small amount $d\xi$.
- Since this amount is very small, we may assume that the force still remains $k\xi$.
- The work done will therefore be $(k\xi)d\xi$.



- So what will be the total work done by this internal force once the final extension x is achieved?
- We can find that out by integrating from 0 to x.
- Do keep in mind that x has nothing to do with the length of the spring but only the extension

$$\int_{0}^{x} k\xi d\xi = \frac{1}{2}kx^{2}$$

$$k\xi$$

$$k\xi$$

$$k\xi$$

- This is the energy that is now stored inside the spring.
- It is a very simple type of strain energy

$$U = \int_0^x k\xi d\xi = \frac{1}{2}kx^2$$

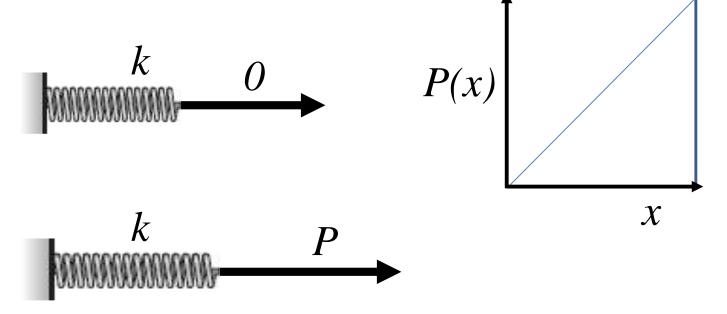
$$k\xi$$

$$k\xi$$

$$k\xi$$

- We now look at the other player in this game the force P
- Keep in mind that the force increased from 0 to its final value P.
- Also the force at each incremental deformation $d\xi$ was equal to $(k\xi)$. So we can safely say that P was increasing linearly

 So if we plot a graph of P vs x we will get a straight line



 From the point of view of the external force therefore, work done is

$$W = \frac{1}{2}Px$$



- Now we will do something that will appear very trivial.
- We will equate the work done with the strain energy
- The results appear very very trivial and not worth the derivations.

$$W = P$$

$$\Rightarrow \frac{1}{2}Px = \frac{1}{2}kx^{2}$$

$$\Rightarrow P = kx$$

 We will now modify the strain energy expression by using the force relation

$$P = kx \Longrightarrow x = \frac{P}{k}$$

Substituting in the strain energy expression

$$U = \frac{1}{2}kx^{2} = \frac{1}{2}k\left(\frac{P}{k}\right) = \frac{1}{2}\frac{P^{2}}{k}$$

 We use this new expression and equate work done with strain energy

$$U = W \Rightarrow \frac{1}{2} \frac{P^2}{k} = \frac{1}{2} Px$$
$$\Rightarrow x = \frac{P}{k}$$

- This still looks trivial
- However note that now our strain energy expression contains only the external force and a property of the spring (stiffness) only

$$U = \frac{1}{2} \frac{P^2}{k}$$

 Both of these are known to us, when we face the problem of finding the deformation of a spring pulled by a force

In the work done expression, we have only one unknown

$$Q = \frac{1}{2}Px$$

 So when we equate the two we have an equation with one unknown only, which is good

$$\frac{1}{2}Px = \frac{1}{2}\frac{P^2}{k}$$

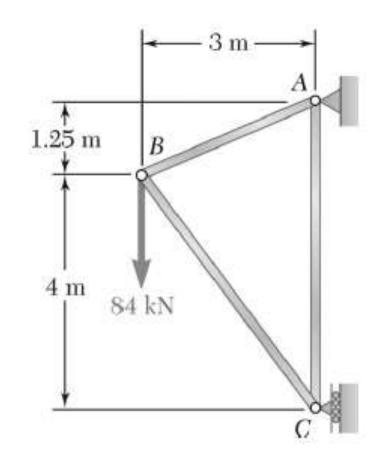
- Still, the problem we have tackled being simple does not really explain how useful this approach can be.
- So we will solve a tougher problem by skipping derivations which will be explained in details later.

- We have already discussed how a rod of length L and area of cross section A acted upon by an axial force P can be treated as a spring.
- The stiffness of such a rod was derived as

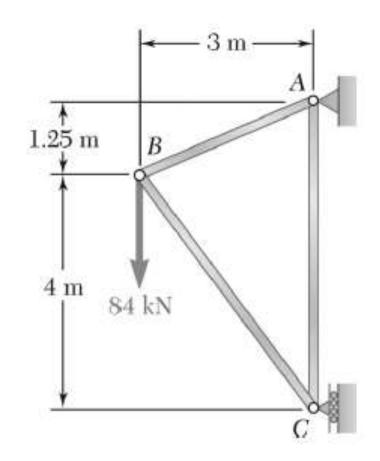
$$k = \frac{EA}{L}$$

 We will now consider a problem involving a truss keeping in mind that a truss is made up of rods carrying only axial forces.

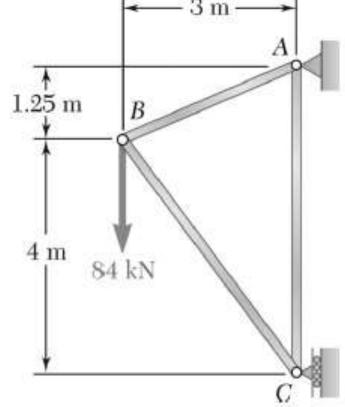
- We wish to find out the downward deflection of the point B.
- We will assume that the rods are made of the same material with modulus of elasticity E and have the same area of cross section A.



- Using basic knowledge of trusses we can solve for the forces in each member
- Using geometry we can find out the unknown lengths.



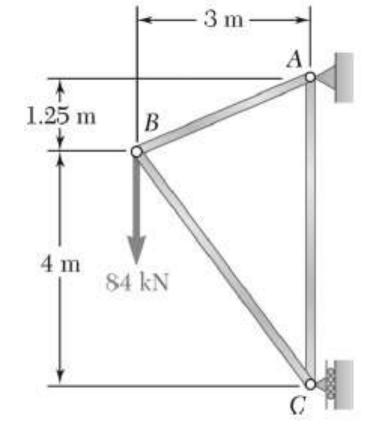
• The solutions are as follows



$$L_{AB} = 3.25m, L_{BC} = 5m, L_{CA} = 5.25m$$

 $F_{AB} = 52kN, F_{BC} = -80kN, F_{CA} = 64kN$

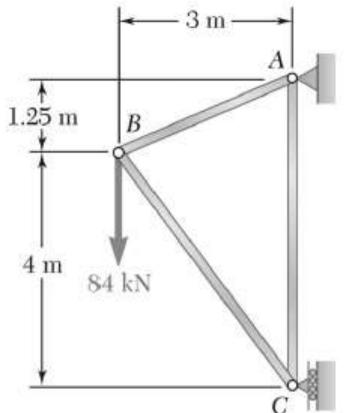
- We will treat the external force as an unknown force P as a variable for the time being.
- Hence



$$F_{AB} = \frac{52}{84}P, F_{BC} = -\frac{80}{84}P, F_{CA} = \frac{64}{84}P$$

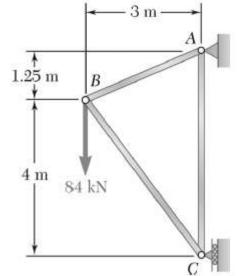
- Now force P is acting vertically. Hence the work done by the force will involve only the vertical deflection at P. The horizontal deflection, if any will not contribute to the work done
- Hence

$$W = \frac{1}{2} P \delta_{B,v}$$



 The equivalent stiffness of each member AB,BC, CA for axial loading will be

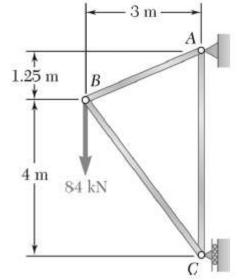
$$k_{AB} = \frac{EA}{L_{AB}}, k_{BC} = \frac{EA}{L_{BC}}, k_{CA} = \frac{EA}{L_{CA}}$$



We will now apply the strain energy expression for a spring

$$U = \frac{P^2}{2k}$$

 The strain energies of each member AB,BC, CA due to the forces developed in them will be



$$U_{AB} = \frac{F_{AB}^{2}}{2\frac{EA}{L_{AB}}}, U_{BC} = \frac{F_{BC}^{2}}{2\frac{EA}{L_{BC}}}, U_{CA} = \frac{F_{CA}^{2}}{2\frac{EA}{L_{CA}}}$$

The total strain energy of the truss will therefore be

$$F_{AB} = \frac{52}{84}P, F_{BC} = -\frac{80}{84}P, F_{CA} = \frac{64}{84}P$$

$$U = \frac{F_{AB}^{2}L_{AB}}{2EA} + \frac{F_{BC}^{2}L_{BC}}{2EA} + \frac{F_{CA}^{2}L_{CA}}{2EA}$$

$$= \frac{P}{EA} \left(\frac{169L_{AB}}{882} + \frac{400L_{BC}}{882} + \frac{64L_{CA}}{882} \right)$$

 Since this strain energy comes from the work done by the force P, hence

$$\frac{1}{2}P\delta_{B,v} = \frac{P}{EA} \left(\frac{169L_{AB}}{882} + \frac{400L_{BC}}{882} + \frac{64L_{CA}}{882} \right)$$

$$\Rightarrow \delta_{B,v} = \frac{1}{EA} \left(\frac{169L_{AB}}{441} + \frac{400L_{BC}}{441} + \frac{64L_{CA}}{441} \right)$$

 Everything on the RHS is known to us and hence we can calculate the deflection

- Note that only one force has been considered
- Also only deflection in the direction of that force has been obtained.
- However given the simplicity of the theory even this can be considered a big benefit
- We are not having to calculate individual deformations and then apply geometrical conditions to find the deflection in the vertical direction.
- With improved energy methods even multiple forces and deflections in any directions can be handled.