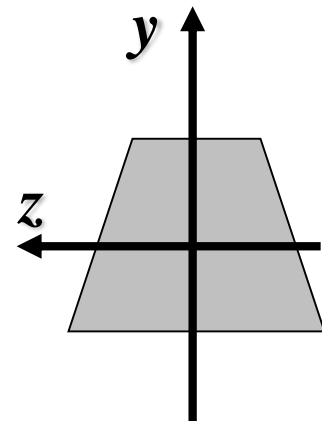
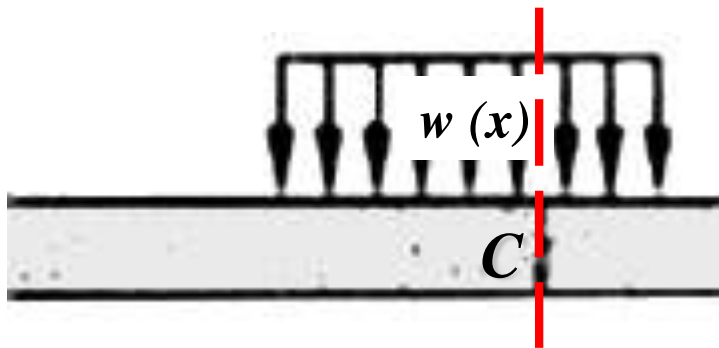


# Shear Stresses in Transversely Loaded Beams

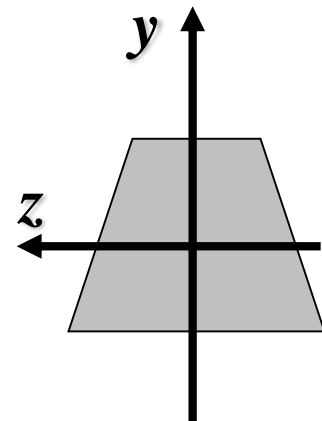
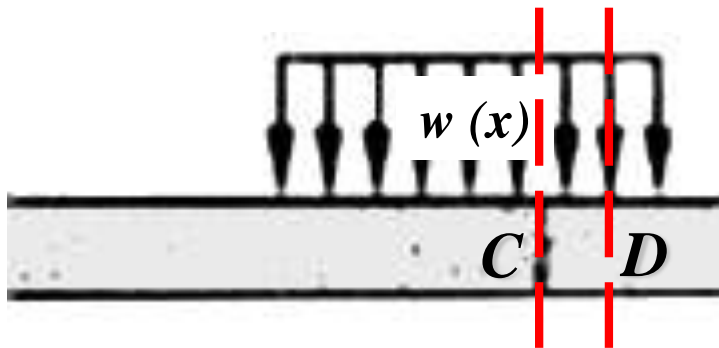
# Shear Stresses in Beams

- We will consider a beam with a distributed load in the neighbourhood of point C as shown
- The transverse section of the beam at C and the coordinate system is shown in the picture on the right.



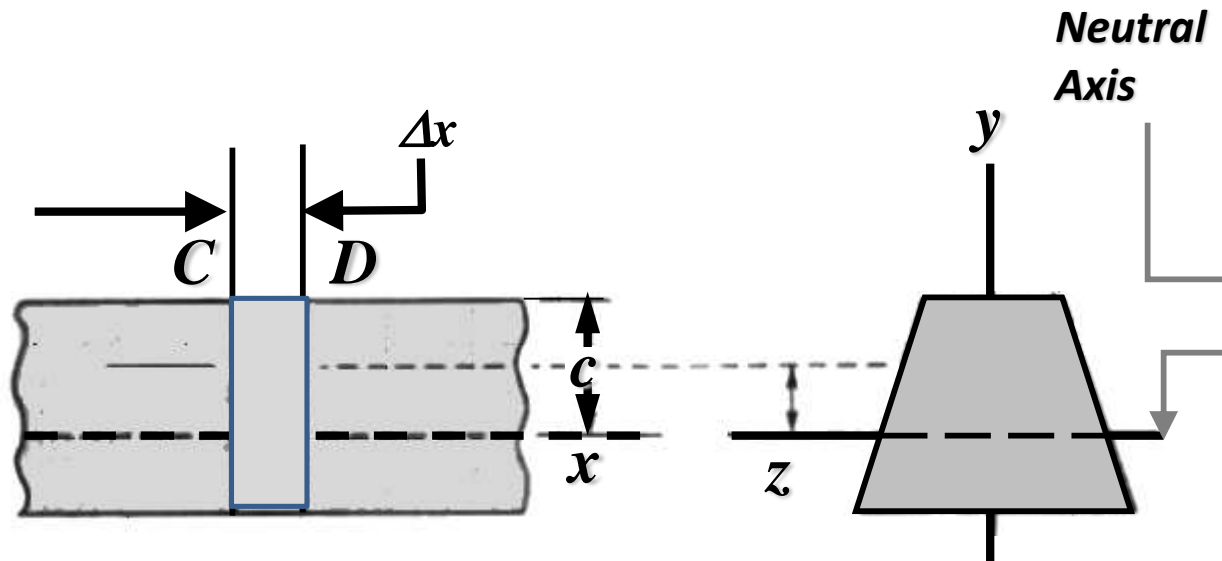
# Shear Stresses in Beams

- Now we will consider another point D, very close to C.
- Since the point is close to C, the nature of load, material properties and cross section will change very little.



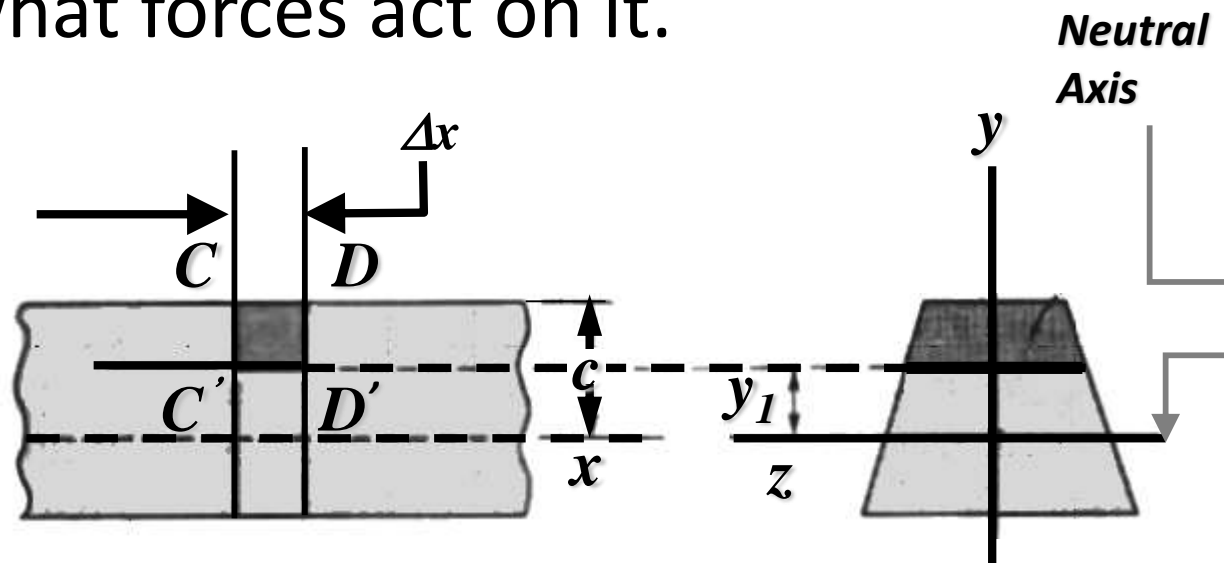
# Shear Stresses in Beams

- Next we take a magnified view of the beam around CD. Our  $x$  axis is positive from C to D.
- The distance between C and D is  $\Delta x$ .
- The neutral axis is at  $c$  from top



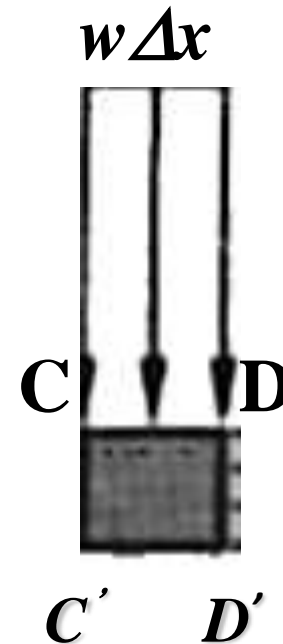
# Shear Stresses in Beams

- We now consider two points  $C'$  and  $D'$  right below  $C$  and  $D$  and at a distance  $y_1$  from the neutral axis.
- We will take this chunk out of the beam and see what forces act on it.



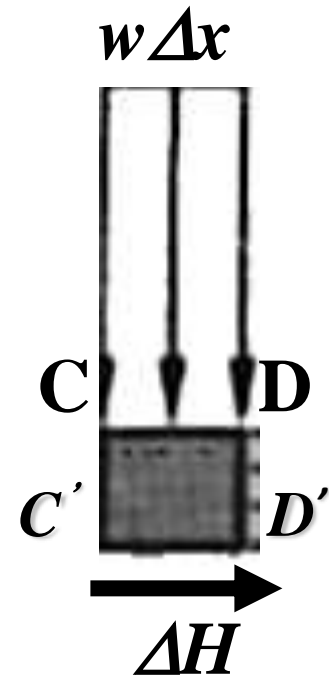
# Shear Stresses in Beams

- Here is how things look like when the chunk is still attached to the beam
- We see a load  $w\Delta x$  acting vertically downward on the exposed surface  $CD$



# Shear Stresses in Beams

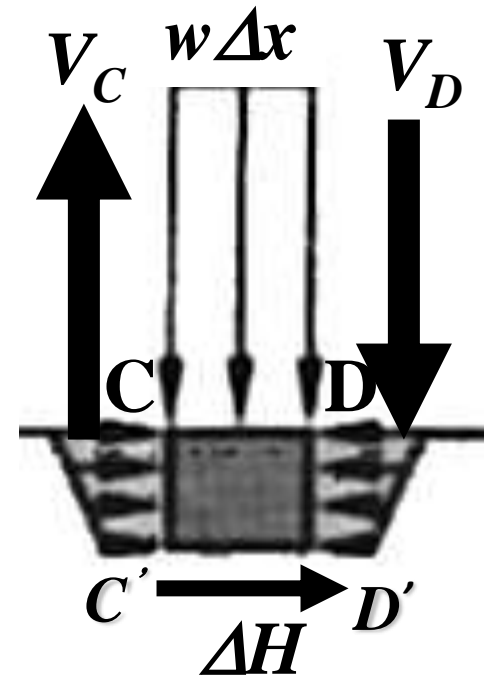
- We now remove the part of the beam attached to the bottom  $C'D'$
- We will now see the shear force  $\Delta H$  which was holding the bottom of the chunk to the rest of the beam and preventing it from sliding in the process of bending.
- The obvious question is what about vertical forces ?
- We will answer that next.



*$H$  = Horizontal shear force*

# Shear Stresses in Beams

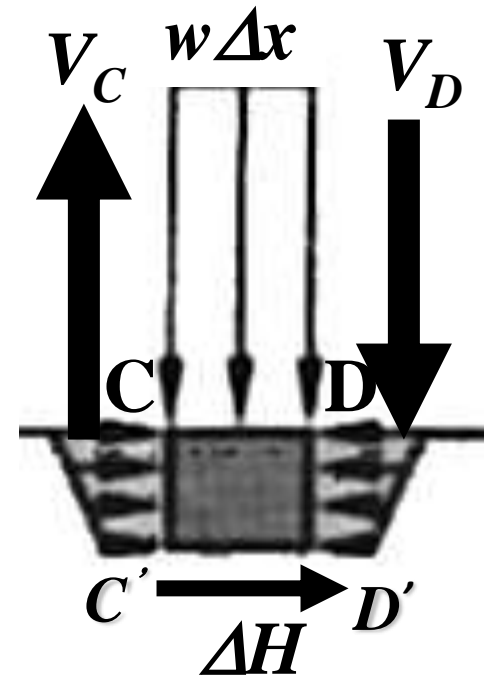
- We now detach the side walls i.e. CC' and DD' from the beam.
- Now the chunk is completely free of the beam
- We get to see the vertical shear forces acting on the side walls.
- These are the forces that were preventing the chunk from popping out of the beam while bending.
- Vertical normal stresses are much smaller. So we are ignoring them. We have already encountered these vertical forces when drawing SFD for beams.





# Shear Stresses in Beams

- We also get to see the internal normal stresses that arise due to bending.
- These are the stresses we have already encountered while studying bending of beams.
- Since the external forces are downwards hence the beam will be compressed at the top.  
Hence the stresses are compressive.

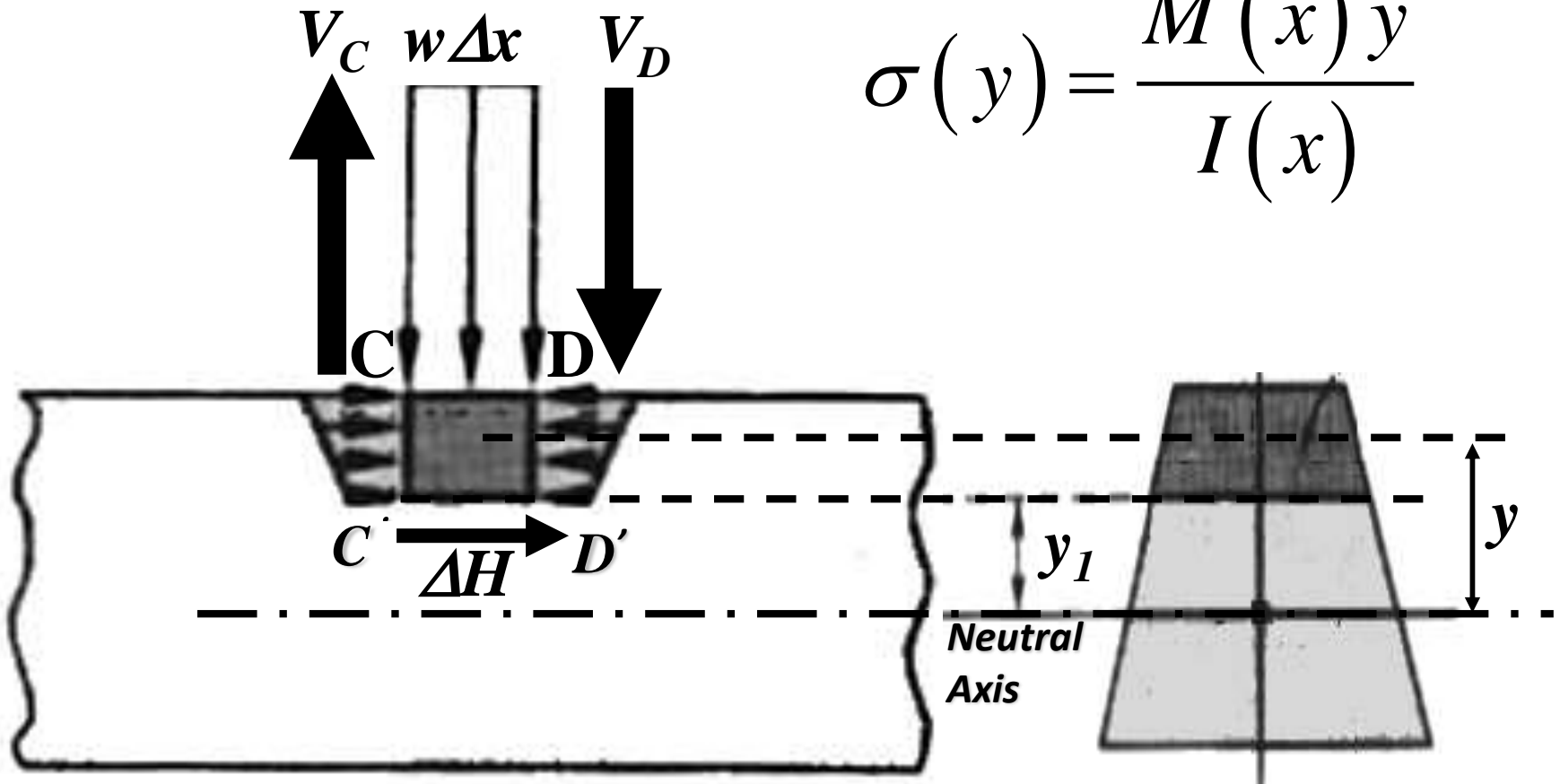




# Shear Stresses in Beams

- The normal stresses on the walls CC' and DD' at any point a distance  $y$  from the neutral axis are given by

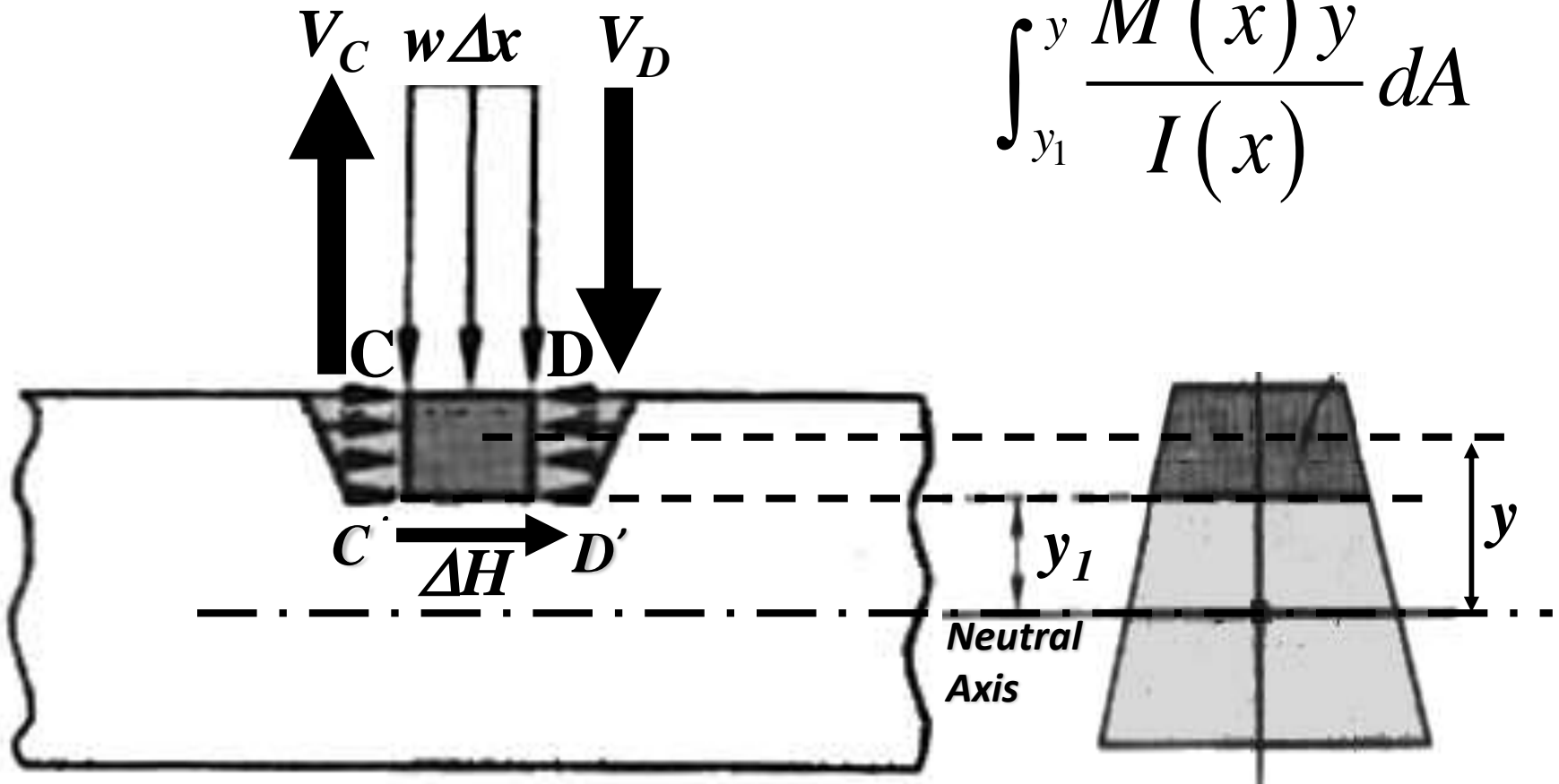
$$\sigma(y) = \frac{M(x)y}{I(x)}$$



# Shear Stresses in Beams

- Hence the total force due to these stresses on the side walls up to a distance  $y$  is

$$\int_{y_1}^y \frac{M(x) y}{I(x)} dA$$



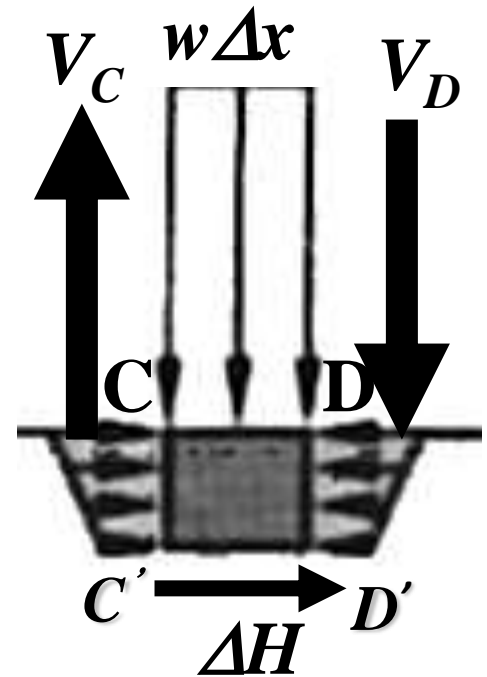
# Shear Stresses in Beams

- Let us now do the force balance in the y direction.
- Since  $\Delta x$  is small we can ignore change in area of cross section between C and D.
- We get

$$\sum F_x = 0$$

$$\Rightarrow \int_A (-\sigma_C) dA - \int_A (-\sigma_D) dA + \Delta H = 0$$

$$\Rightarrow \Delta H = \int_a (\sigma_D - \sigma_C) dA$$



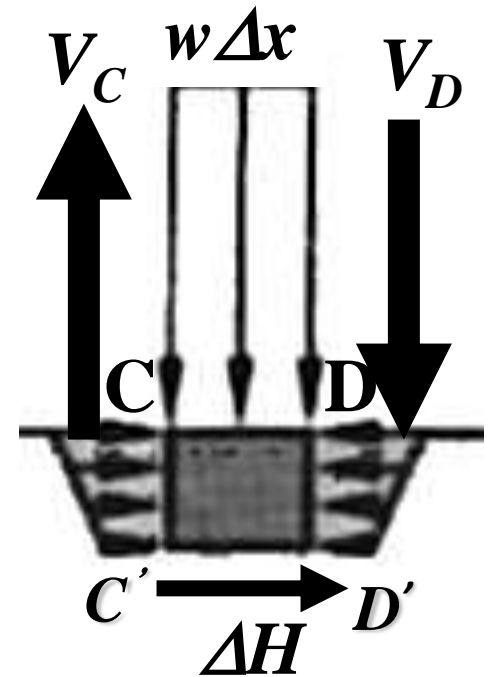
# Shear Stresses in Beams

- We can now substitute for the stresses using the formula  $\sigma = \frac{My}{I}$

$$\therefore \Delta H = \int_a \left( \frac{M_D y_D}{I_D} - \frac{M_C y_C}{I_C} \right) dA$$

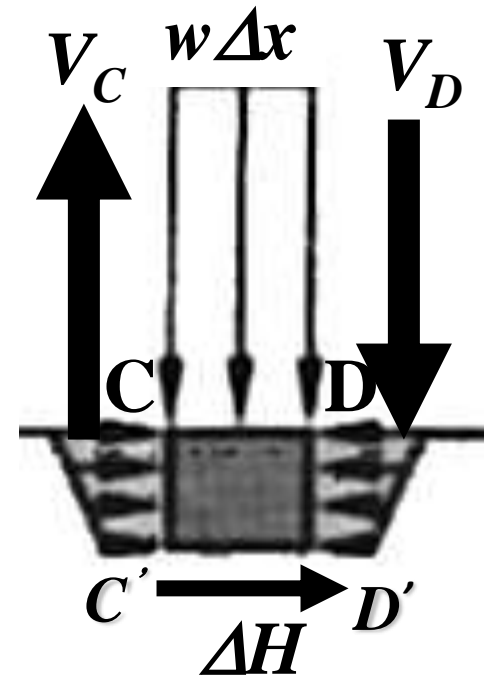
- If  $\Delta x$  is small then  $y_C = y_D, I_C = I_D = I$

$$\therefore \Delta H = \int_a \left[ \frac{M_C y}{I} - \frac{M_D y}{I} \right] dA = \frac{M_C - M_D}{I} \int_a y dA$$



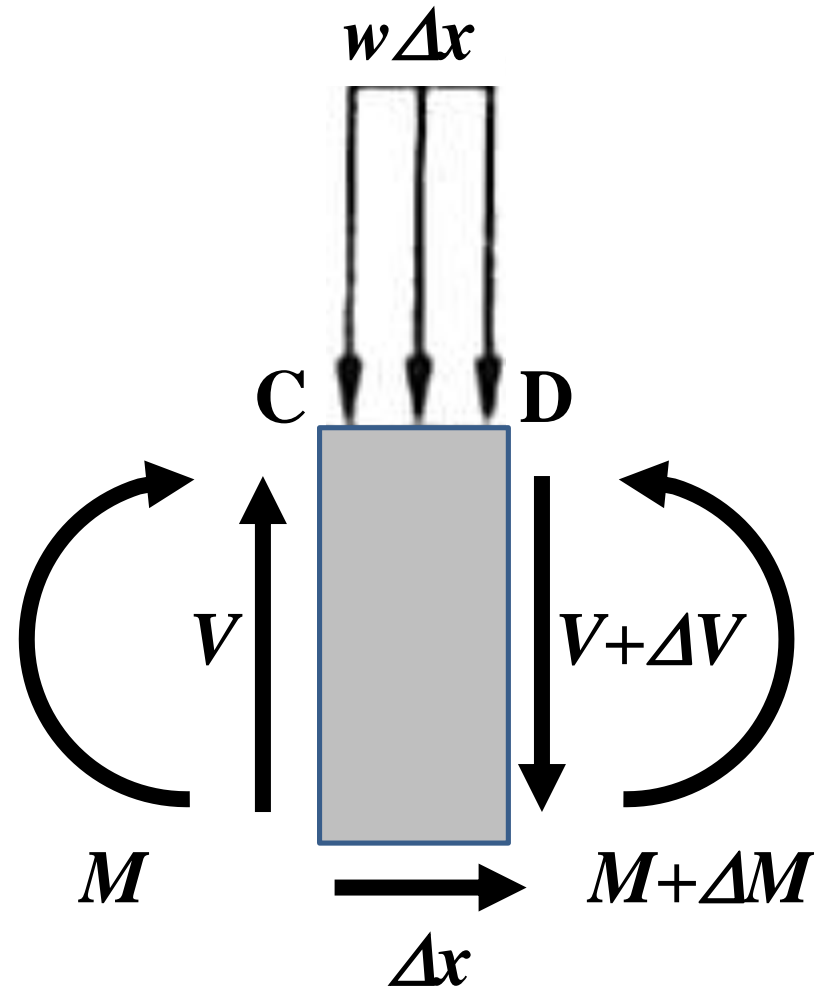
# Shear Stresses in Beams

- We need to note some important points regarding how what we have done so far differs from what we usually do in our normal stress problem solving ?
- First, we are not integrating over the entire cross section of the beam but from  $C'$  to  $C$ .
- But the  $I$  that we are using is for the entire cross section.
- It is not the second moment of the area between  $C$  and  $C'$  or  $D$  and  $D'$ .
- This is because the formula for stress at any  $y$  uses the  $I$  for the entire cross section only. The  $I$  does not depend on  $y$  but on  $x$ .



# Shear Stresses in Beams

- Let us now recall a few points about moments and shear forces in beams.
- We consider a section of the beam in the neighbourhood of C extending across the full depth of the beam.
- The directions of the moments have been set such that the top part of the beam is in compression
- The internal forces and moments change by  $\Delta V$  and  $\Delta M$  respectively as we shift by  $\Delta x$ .





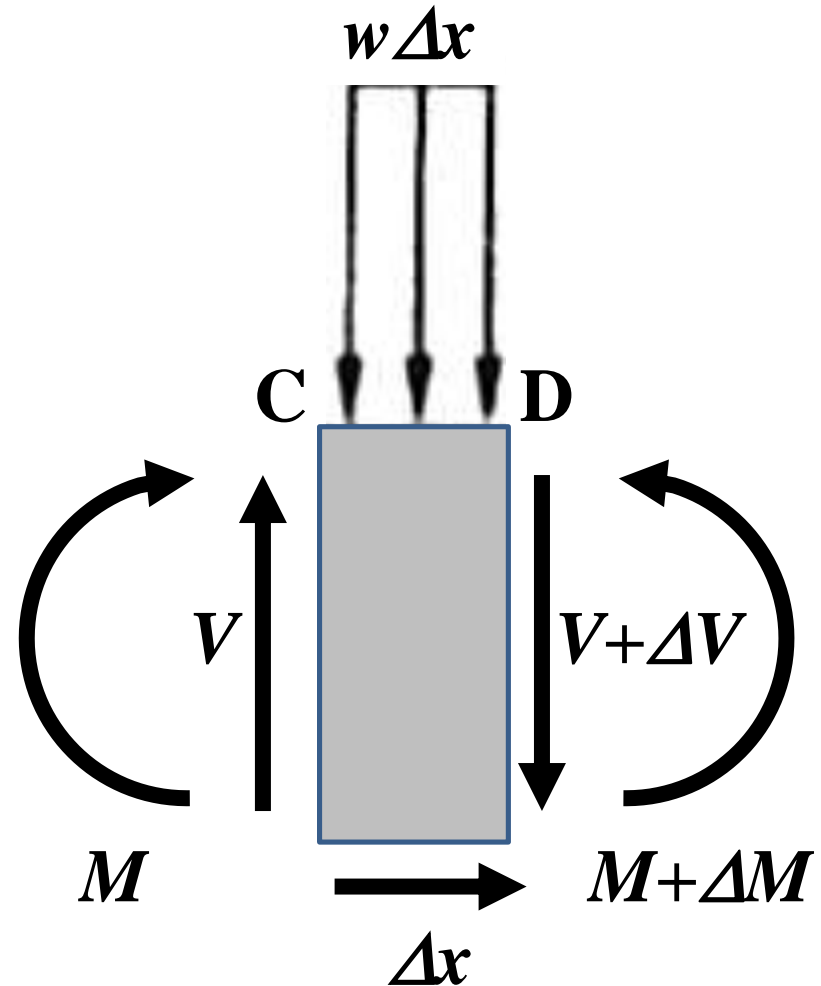
# Shear Stresses in Beams

- First we consider the equilibrium in y direction

$$\sum F_y = 0$$

$$\Rightarrow V = V + \Delta V + w\Delta x$$

$$\Rightarrow \Delta V = -w\Delta x$$



# Shear Stresses in Beams

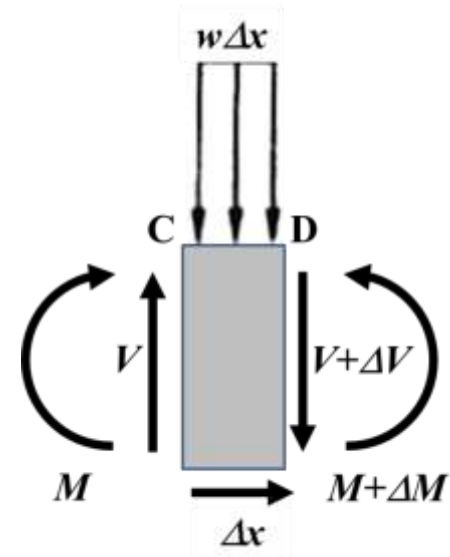
- Next we consider moment equilibrium about C

$$\sum M_C = 0 \Rightarrow M + \Delta M - M - (V + \Delta V) \Delta x - (w \Delta x) \frac{\Delta x}{2} = 0$$

$$\Rightarrow \Delta M = V \Delta x + \Delta V \Delta x + \frac{w}{2} \Delta x^2$$

$$\Rightarrow \frac{\Delta M}{\Delta x} = V + \Delta V + \frac{w}{2} \Delta x$$

$$\Rightarrow \frac{dM}{dx} = \lim_{\Delta x \rightarrow 0} \frac{\Delta M}{\Delta x} = V$$



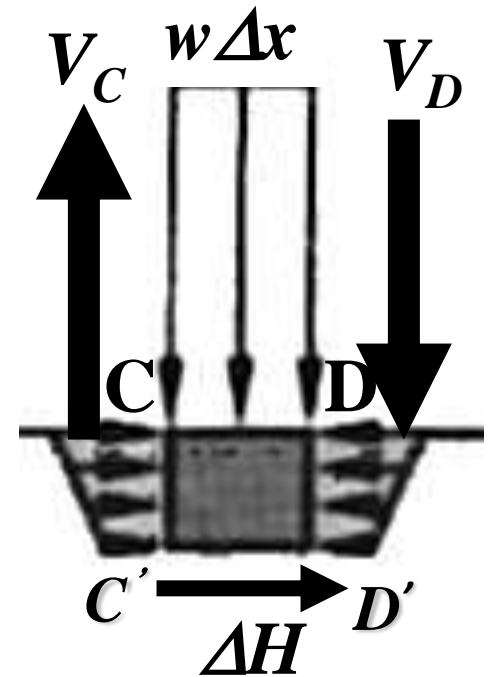
# Shear Stresses in Beams

- In case of the present problem we have already obtained

$$\Delta H = \frac{M_C - M_D}{I} \int_a y dA$$

- If  $\Delta x$  is small then in the limit

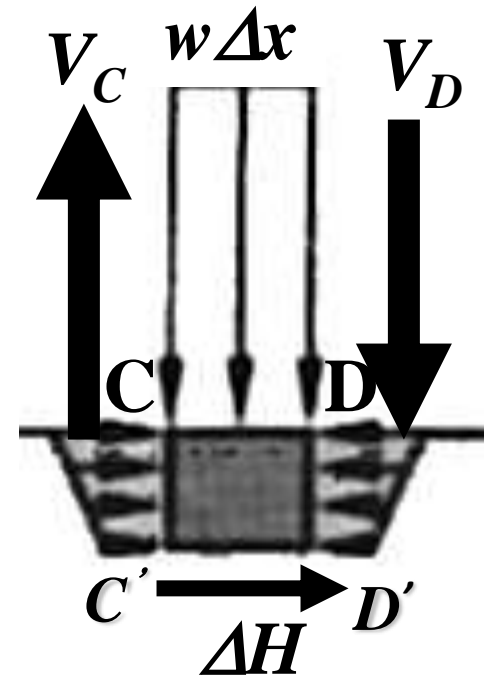
$$M_D - M_C = \Delta M = \frac{dM}{dx} \Delta x$$



# Shear Stresses in Beams

- We also know from our previous experience with SFD and BMD and our revision of bending in beams that

$$V = \frac{dM}{dx} \Rightarrow \Delta M = V \Delta x$$

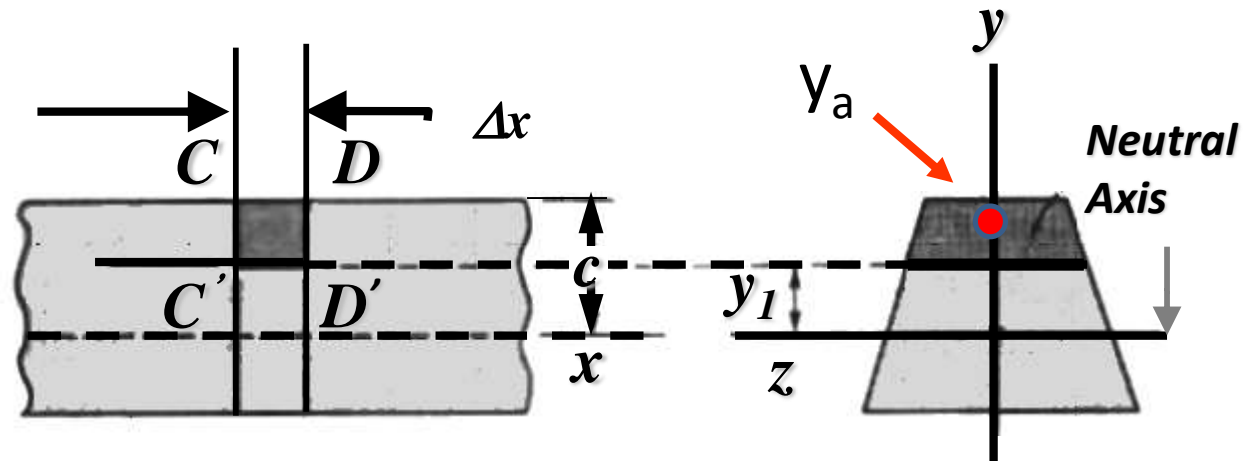


- Hence

$$\Delta H = \frac{M_C - M_D}{I} \int_a y dA = \frac{V \Delta x}{I} \int_a y dA = \frac{VQ}{I} \Delta x$$

# Shear Stresses in Beams

- Let us try to understand this new quantity  $Q = \int_a y dA$
- Let us look at our initial diagrams of the beam.  $Q$  is the first moment of the dark shaded area in the second figure. Hence if that area is  $A_a$  and the centroid of that area has a  $y$  coordinate  $y_a$  then  $Q = y_a A_a$

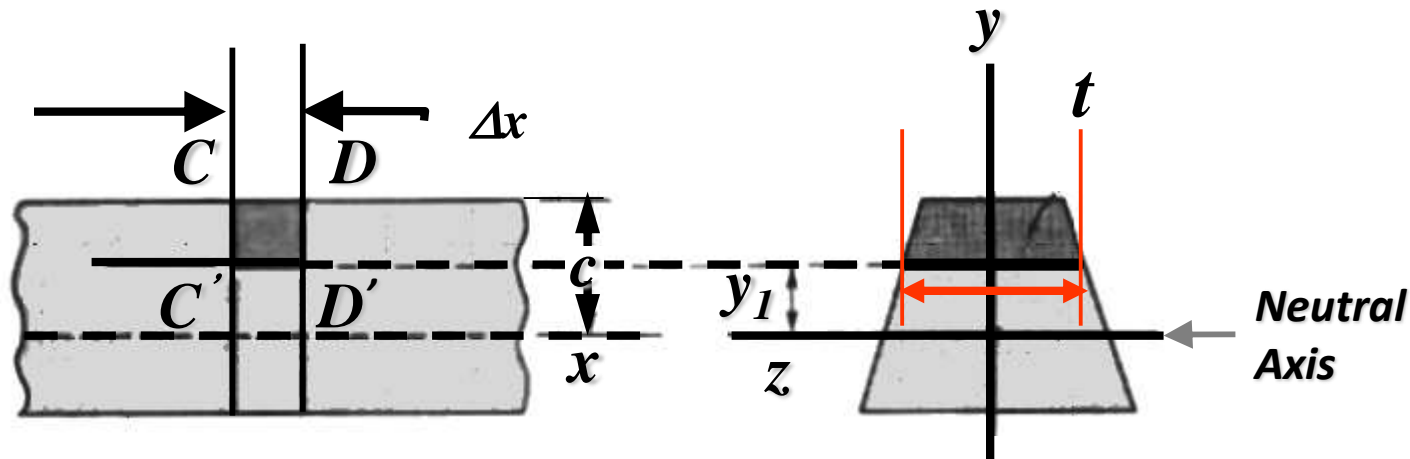
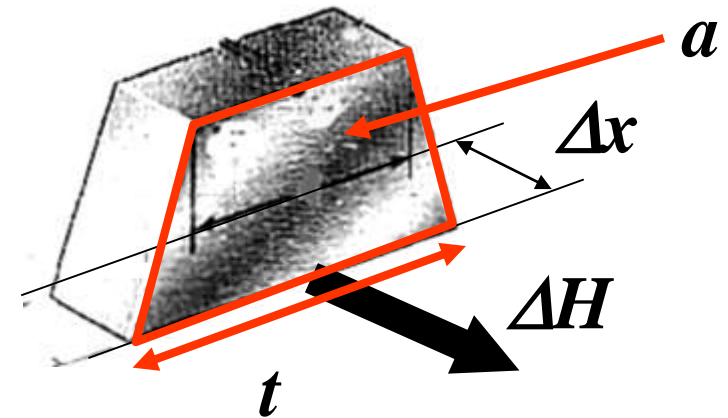




# Shear Stresses in Beams

- We can now define a quantity called shear flow by considering the width of the shaded area.

$$q = \frac{\Delta H}{\Delta x} = \frac{VQ}{I}$$

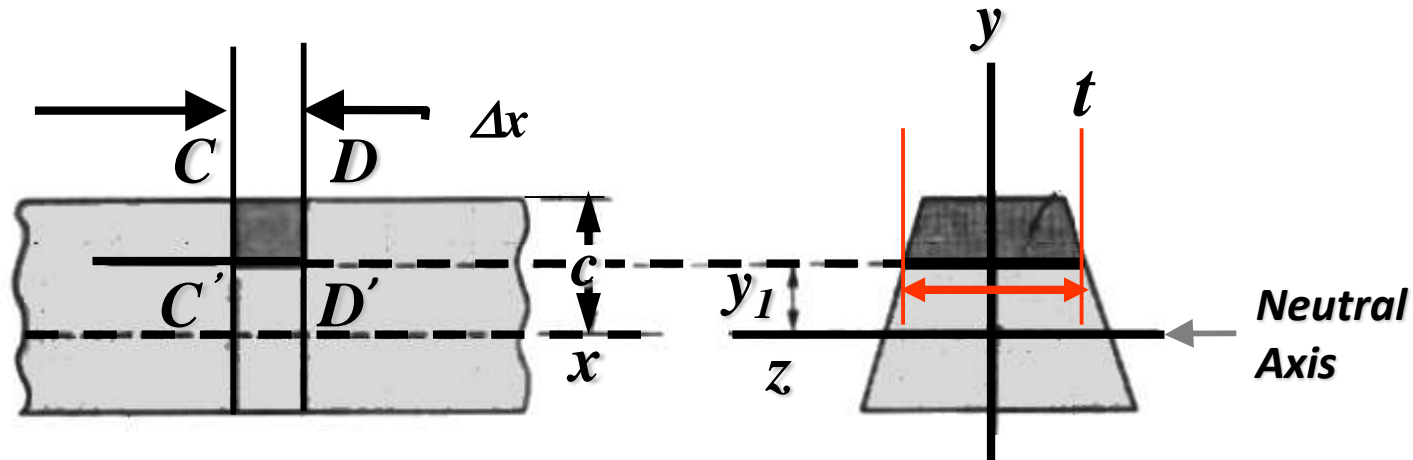
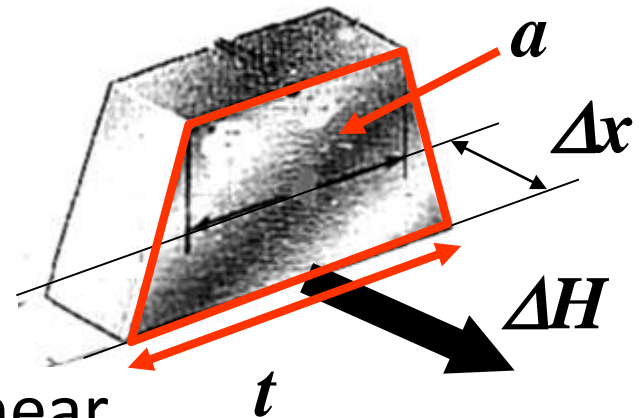


# Shear Stresses in Beams

- The shear stress is simply  $\Delta H$  divided by the area on which it is acting i.e.  $t\Delta x$ .

$$\tau = \frac{\Delta H}{t\Delta x} = \frac{VQ}{It}$$

- This should also answer the question why we call these vertical forces in beams as shear forces. They cause shear stress.



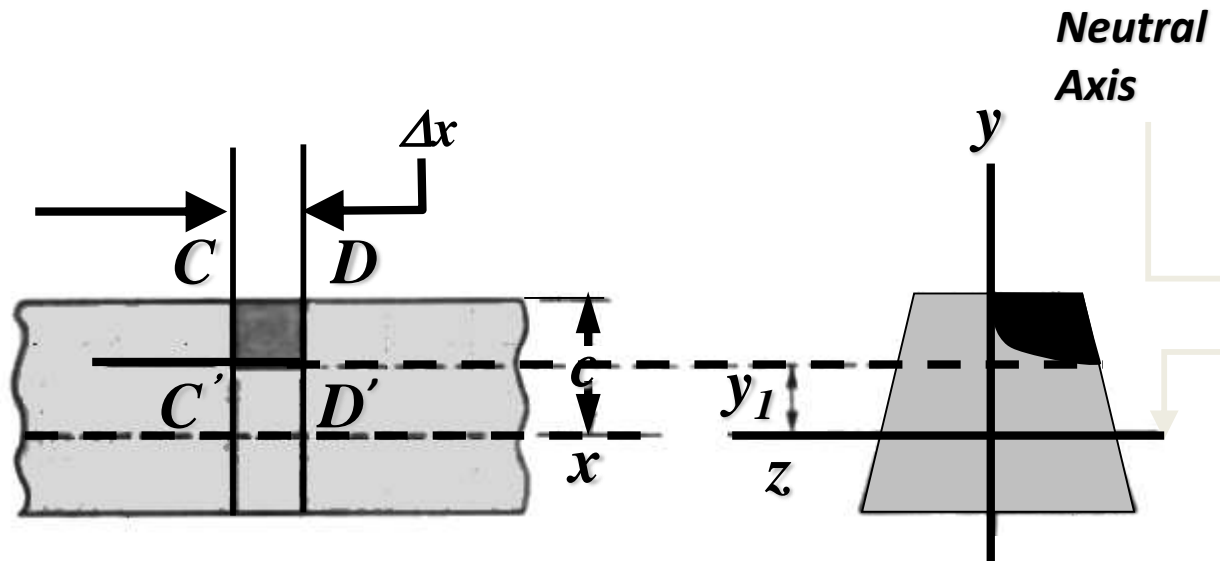






# Shear Stresses in Beams

- An alternate situation
- Magnified view of a small slice which is now obtained by scooping out from one side rather than by a straight transverse cut



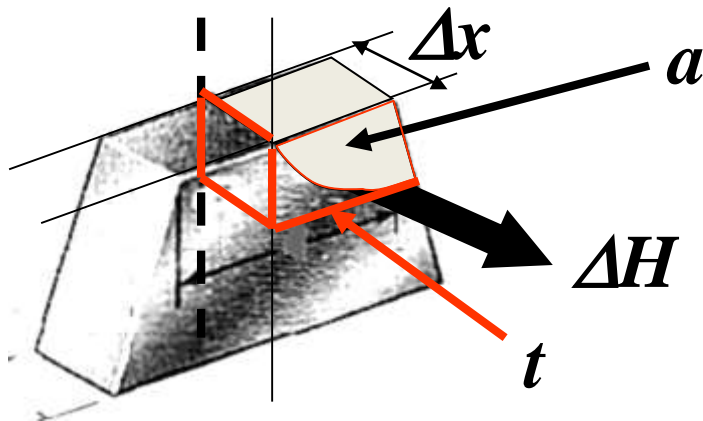
# Shear Stresses in Beams

- Here we consider the area  $a$  as shown and  $t$  is the projected width. Rest of the derivations and expressions remain the same. Hence, as before, we get

$$\Delta H = \frac{VQ}{I} \Delta x$$

$$Q = \int_a y dA = y_{centroid,a} A_a$$

$$q = \frac{\Delta H}{\Delta x} = \frac{VQ}{I}$$



$$\tau = \frac{\Delta H}{t \Delta x} = \frac{VQ}{It}$$







