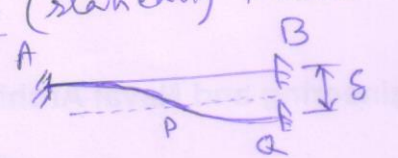
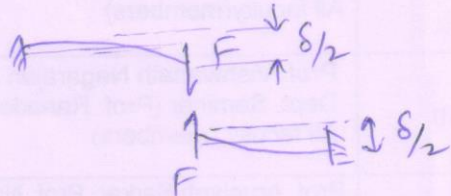



Some applications (statically indeterminate)

support settlement  support B is sunk by  $\delta$ .

We can split the problem into 2 part due to symmetry as shown  
 Since there is a point of inflection, BM must be  $= 0$  at that point.  
 $\therefore$  we can split at P into 2 cantilever beams with shear acting between them.

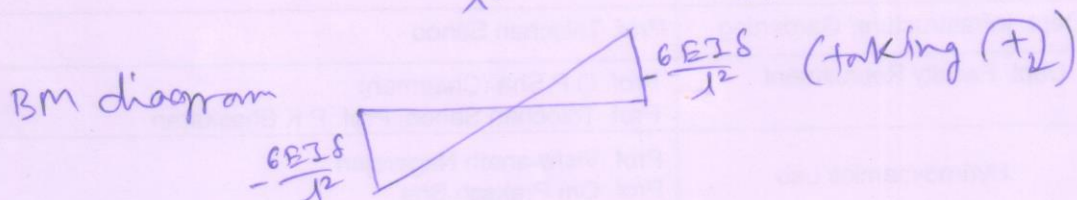


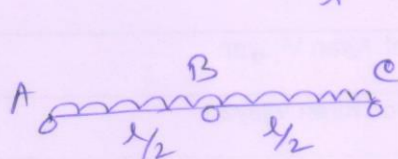
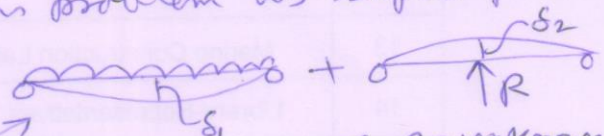
Previously, we got,   $= \frac{Pl^3}{3EI}$

$$\therefore \text{here we get, } \delta/2 = \frac{F(l/2)^3}{3EI} = \frac{Fl^3}{24EI}$$

$$\therefore \delta = \frac{Fl^3}{12EI} \quad \sim F = \text{Shear force at P} = \frac{12EI\delta}{l^3}$$

$$\text{BM at support} = \frac{12EI\delta}{l^3} \times \frac{l}{2} = \frac{6EI\delta}{l^2}, \quad \text{shear at support} = \frac{12EI\delta}{l^3}$$

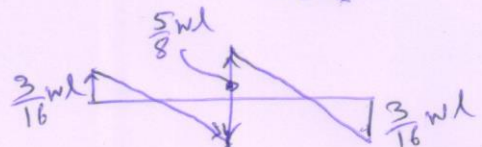
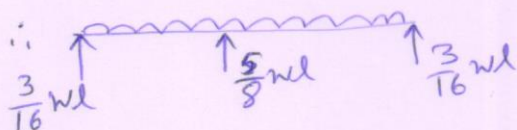


 We consider this problem as superimposition of 2 problems,   $\delta_1$   $\delta_2$

We can write  $\delta_1 = \frac{5}{384} \frac{wl^4}{EI}$  for  $\delta_1$ ,  $\delta_2 = \frac{Rl^3}{48EI}$  for  $\delta_2$   $\rightarrow R = \text{unknown}$

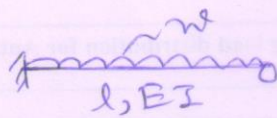
$$\therefore \frac{5}{384} \frac{wl^4}{EI} - \frac{Rl^3}{48EI} = 0 \quad \text{since deflection at B} = 0 \quad (\text{due to given boundary condition})$$

$$\therefore R = \frac{5}{8} wl$$





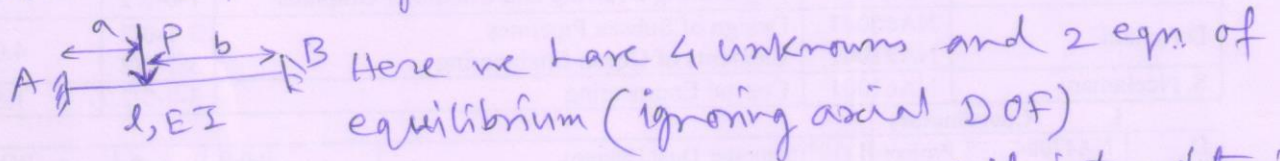
Solve the problem



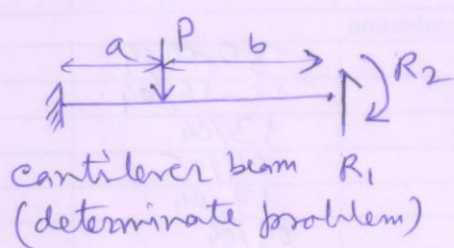
Solution of statically indeterminate problems:

general approach — (i) calculate the degree of indeterminacy ( $n$ )  
 (ii) Convert it into a statically determinate problem by removing reactions  
 " $n$ " number of constraints. (iii) Put " $n$ " no. of unknowns in  
 place of the constraints. (iv) At each constraint, write the  
 equation of resultant deflection/deformation involving those " $n$ "  
 unknown reactions. Thus we get " $n$ " no. of linear simultaneous  
 equations involving " $n$ " unknown reactions. We solve them  
 using standard approach. Thus we find the unknown  
 reactions and the problem is solved.

Solve the following problem.



$\therefore$  we must remove 2 constraints to convert it into a determinate problem. We put unknown reaction forces in those constraints.



(+)  $\downarrow$   
 we can write equations of slope and deflection at B.



slope at B = 0

$$\Rightarrow \frac{Pa^2}{2EI} + \frac{R_2 l}{EI} - \frac{R_1 l^2}{2EI} = 0$$

$$\therefore 3Pa^2 + 2R_2 l - 2R_1 l^2 = 0 \quad \text{--- (i)}$$

$$\text{deflection at B} = 0 \Rightarrow \frac{Pa^3}{3EI} + \frac{Pa^2}{2EI}(b) + \frac{R_2 l^2}{2EI} - \frac{R_1 l^3}{3EI} = 0$$

$$2Pa^3 + 3Pa^2 b + 3R_2 l^2 - 2R_1 l^3 = 0 \quad \text{--- (ii)}$$

solve (i) and (ii) to get  $R_1$  and  $R_2$



From ①  ~~$lPa^2 + 2R_2l$~~   $2Pa^2l + 4R_2l^2 = 2R_1l^3$

Use in ②  $2Pa^3 + 3Pa^2b + 3R_2l^2 = 2R_1l^3 = 2Pa^2l + 4R_2l^2$

$$-2Pa^2l + 2Pa^3 + 3Pa^2b = R_2l^2$$

$$\therefore R_2 = \frac{Pa^2}{l^2} \left( \frac{2a+3b}{-2l} \right) = \frac{Pa^2(2l+b-2l)}{l^2} = \frac{Pba^2}{l^2}$$

$$\therefore R_1 = \frac{1}{l^2} (Pa^2 + 2R_2l) = \frac{1}{l^2} \left( Pa^2 + 2 \frac{Pba^2}{l^2} l \right) = \frac{1}{l^2} \left( \frac{Pa^2l + 2Pba^2}{l} \right)$$

$$= \frac{Pa^2}{l^2} \left( \frac{l+2b}{l} \right) = \frac{Pa^2}{l^3} (l+2b) = \frac{Pa^2}{l^3} (a+3b)$$

$$\therefore \text{Shear at A} = P - \frac{Pa^2}{l^3} (a+3b) = \frac{P}{l^3} [l^3 - a^2(a+3b)]$$

using  $l = a+b$ , we get shear at A =  $\frac{Pb^2}{l^3} (b+3a)$

Moment at A =

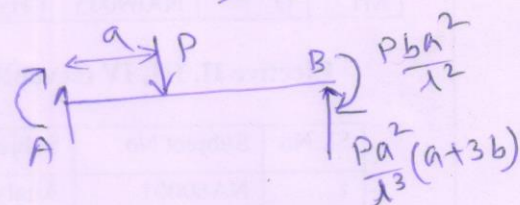
$$\frac{Pba^2}{l^2} + Pa - \frac{Pa^2}{l^3} (a+3b) \cdot l$$

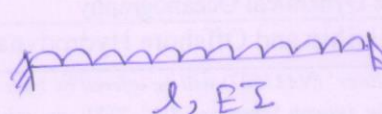
$$= \frac{P}{l^2} [al^2 + ba^2 - a^2(a+3b)] = \frac{P}{l^2} [a(a+b)^2 + ba^2 - a^3 - 3ba^2]$$

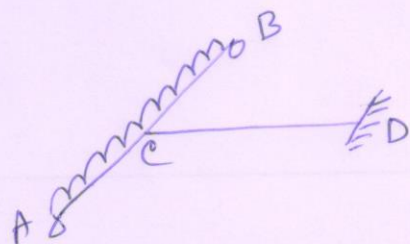
$$= \frac{P}{l^2} [a^3 + 2a^2b + ba^2 + \cancel{ba^2} - a^3 - 3ba^2] = \frac{P}{l^2} [a^2b - 2ba^2] = \frac{P}{l^2} [ab^2]$$

$$= \frac{Pab^2}{l^2}$$

$$\therefore \text{Reaction at A} = -\frac{Pab^2}{l^2}$$



Solve this problem  Find the support reactions and draw the SFD and BMD.



Solve this problem using the same method and draw the SFD and BMD.

Note: these shear force diagram are the vertical shear force diagram.