

Marine Hydrodynamics

1. Dynamic pressure : the total pressure p_T may be written as:

$$p_T = -\rho \left[\frac{\partial \phi}{\partial t} + \frac{1}{2} (\nabla \phi) \cdot (\nabla \phi) + g z \right] \dots (1.1)$$

Now $\phi = \frac{ag}{\omega} e^{kz} \sin(kx - \omega t) \dots (1.2)$

[For simplicity, considering deep water case]

Then from (1.2)

$$\frac{\partial \phi}{\partial t} = -ag e^{kz} \cos(kx - \omega t) \dots (1.2)$$

$$\nabla \phi = \left(i \frac{\partial \phi}{\partial x} + k \frac{\partial \phi}{\partial z} \right) \phi \quad [2D - case]$$

$$= i \frac{\partial \phi}{\partial x} + i \frac{\partial \phi}{\partial z}$$

$$\left(\frac{\partial \phi}{\partial x} \right)^2 + \left(\frac{\partial \phi}{\partial z} \right)^2$$

$$\therefore \nabla \phi \cdot \nabla \phi =$$

$$= \left[a\omega e^{kz} \cos(kx - \omega t) \right]^2 +$$

$$\left[a\omega e^{kz} \sin(kx - \omega t) \right]^2$$

$$= \frac{a^2 \omega^2 e^{2kz}}{2} \rightarrow (1.3)$$

See previous lecture note for expression of $u = \frac{\partial \phi}{\partial x}$ & $w = \frac{\partial \phi}{\partial z}$

\therefore from equation (1.1), we get

$$p = -\rho \left[-ag e^{kz} \cos(kx - \omega t) + \frac{1}{2} a^2 \omega^2 e^{2kz} + g z \right]$$

$$\dots (1.4)$$

Now, the dynamic pressure $- \rho \frac{\partial \phi}{\partial t}$ is driven by e^{kz} as z decreases from $z=0$ to $z=-h$. That is why the term $- \rho \frac{\partial \phi}{\partial t}$ decreases exponentially as depth decreases linearly from $z=0$ to $z=-h$.

at $z=0$, the dynamic pressure

$$p_d = - \rho \frac{\partial \phi}{\partial t} = + \rho a g e^{kz} \Big|_{z=0} \left[\cos(kx - at) = 1 \text{ at wave crest} \right]$$

$$= + \rho a g$$

\therefore at $z=0$, the dynamic pressure is $\rho a g$. Now, under the assumption of linearity, the dynamic pressure at $z=0$ should be equal to $z=a$ also. [at wave crest $|\eta| = a$]

\therefore the figure (2.1) is justified

2.1
300 Hydrostatic pressure distribution at wave crest

Figure 2.2 ~~Figure~~ shows the distribution of hydrostatic force from $z=a$ to $z=-h$, the graph is varying linearly from $z=a$ to $z=-h$, and thus looks like as given in (2.2)

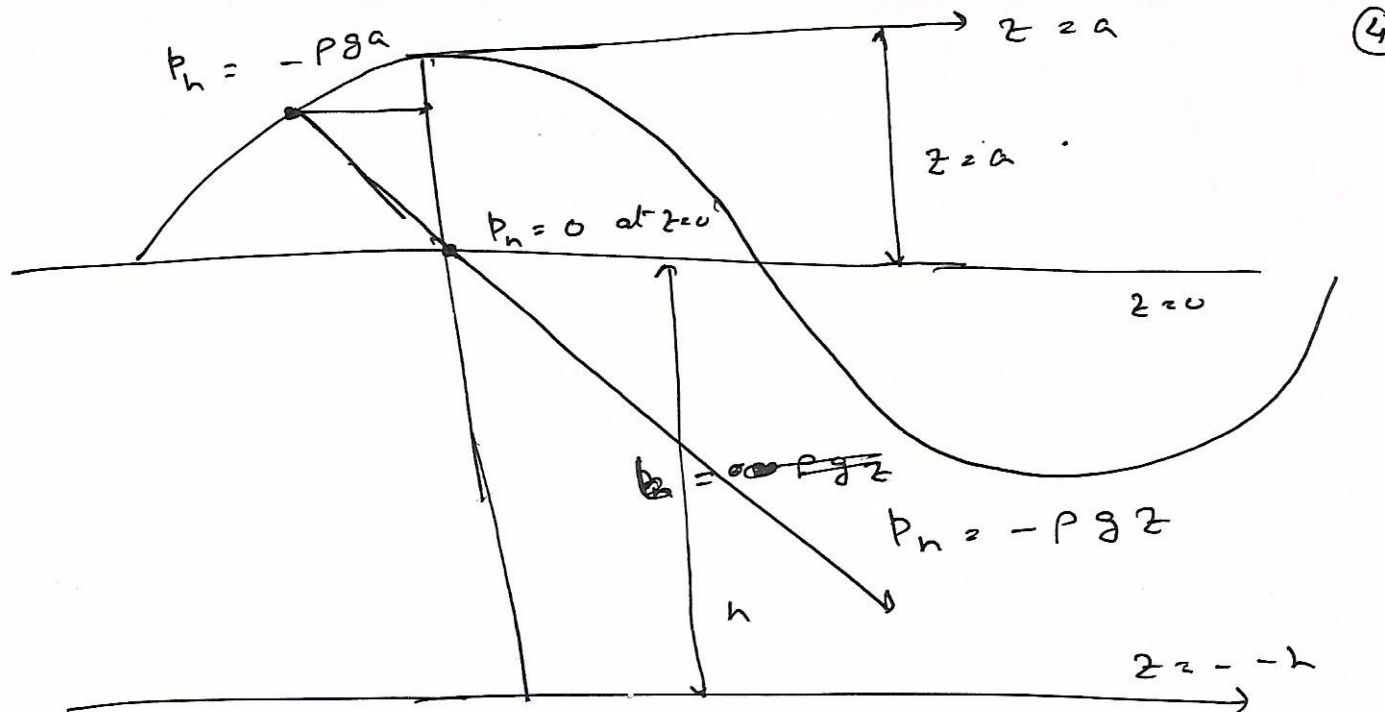


Figure 2.2.

From the expression of hydrostatic pressure,
 $p_h = 0$ at $z=0$, $p_h = \rho g h$ at $z = -h$,

$$p_h = -\rho g a \text{ at } z = a.$$

now, this two expression is automatically satisfied
 the dynamic free surface boundary condition.
 as p_T at $z=a$ [which is free surface at
 wave crest]

$$p_T = -\rho \left[\frac{\partial \phi}{\partial t} + g z \right] \Big|_{z=a}$$

$$= -\rho [a g - a g] = 0$$

$\therefore p_T = 0$ at wave crest :

which confirms that, under the assumption
 of the linear theory, dynamic free surface
 boundary condition is automatically satisfied
 at wave crest.

3. Total pressure under the wave trough

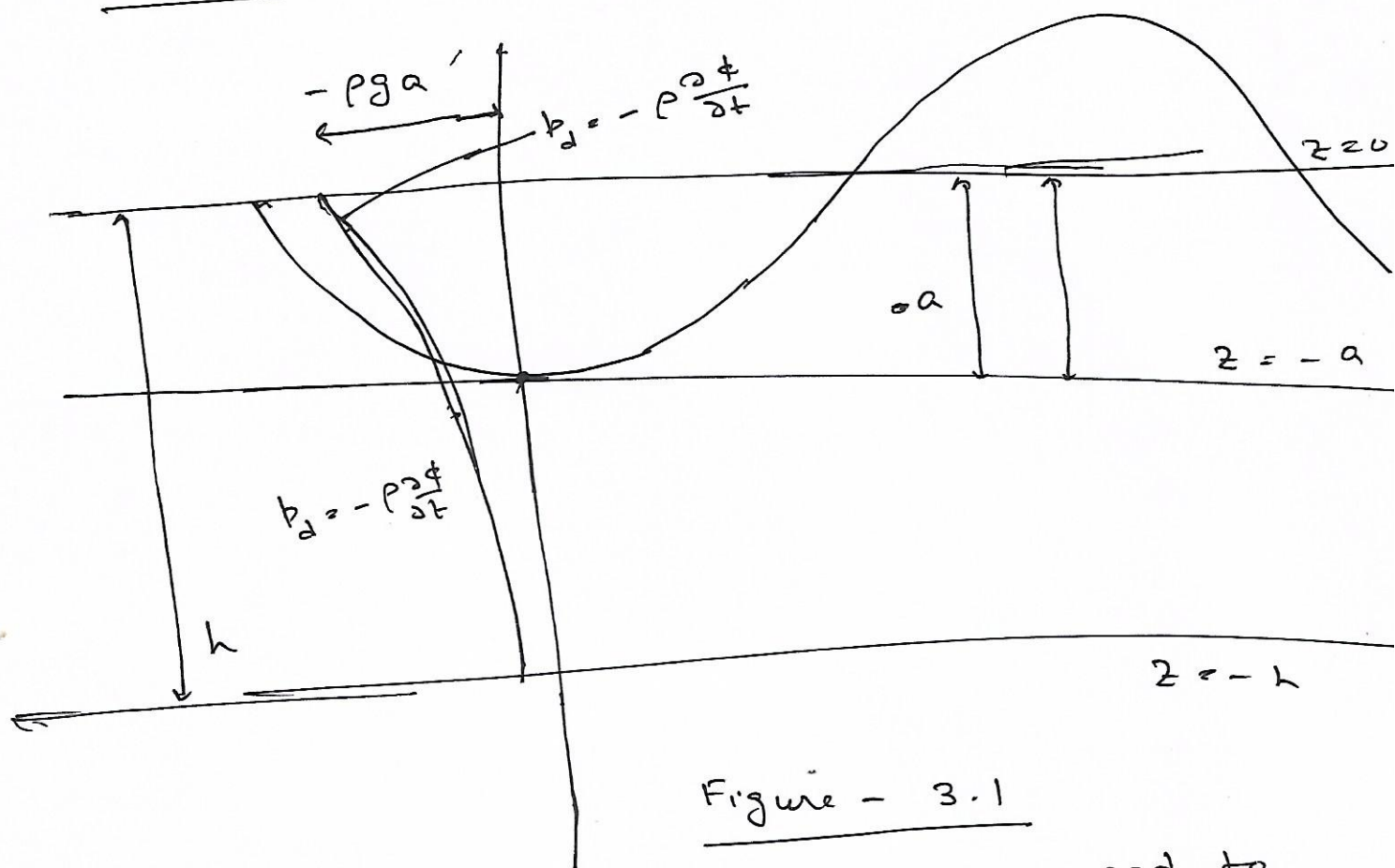


Figure - 3.1

In case of wave trough, we need to remember that here, $\eta = -a$ as $\cos(kx - at) = -1$ at wave trough. In that case the dynamic pressure

$$p = -\rho \frac{\partial \phi}{\partial t} = -\rho \left[-a g e^{kz} \cos(kx - at) \right]$$

$$= -\rho a g \left[\text{as } \cos(kx - at) = -1 \right]$$

\therefore the dynamic pressure at $z=0$ is $-\rho a g$ and then it decreases exponentially from $z=0$ to $z=-h$. Therefore the figure 3.1 is justified.

3.1 Hydrostatic pressure variation at wave trough. ⑥

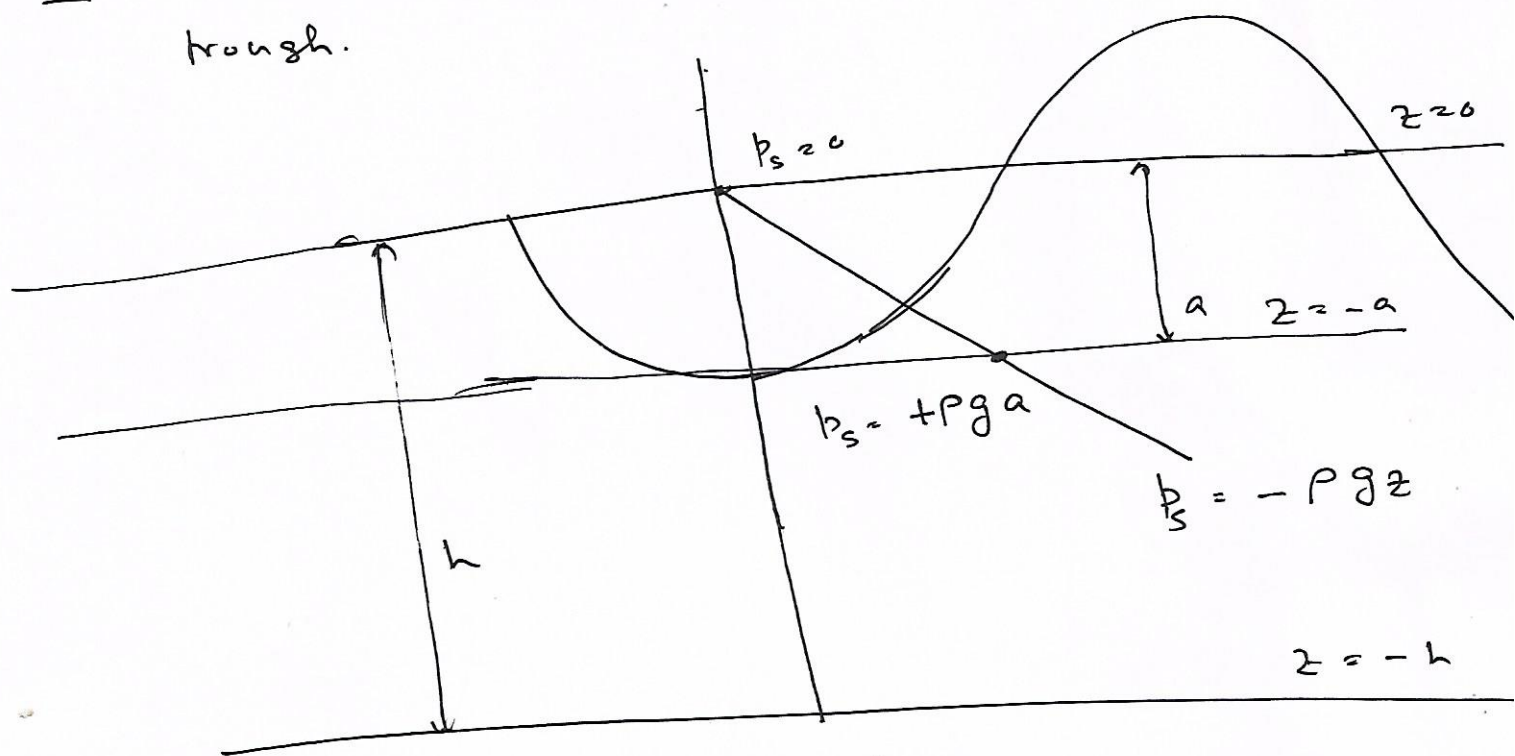


Figure 3.2

Now, Figure the hydrostatic pressure $p_s = 0$ at $z = 0$, $p_s = p_g h$ at $z = -h$ and $p_s = p_g a$ at $z = -a$. and the static hydrostatic pressure p_s is varying linearly from $z = 0$ to $z = -h$. Thus the figure 3.2 is justified.

Now: interestingly, one can note from figure (3.1) and (3.2), qualitatively, at $z = -a$

~~total~~ Total pressure

$$p_T = -\rho \left[\frac{\partial \phi}{\partial t} + g z \right] \text{ at } z = -a \neq 0$$

since $p - p_g z = p_g a$ at $z = -a$ but

$$-\rho \frac{\partial \phi}{\partial t} \Big|_{z=-a} \neq -p_g a \text{ as it is}$$

the value at $z = 0$ and then it decreases

exponentially from $z = 0$ to $z = -a$. ⑦

This is an anomaly of the linear theory. The dynamic free surface condition is satisfied at $z = a$, i.e. at wave crest but it is not zero at wave trough, i.e. at $z = -a$.

The possible reason would be: to drop the quadratic term $\frac{1}{2} a^2 \omega^2 e^{2kz}$ from the total dynamic pressure equation.

Now, collecting these concepts, the pressure variation (total pressure) ~~from~~ at wave crest and wave trough may be schematically given as follows (only qualitative) (see Fig. 3.3)

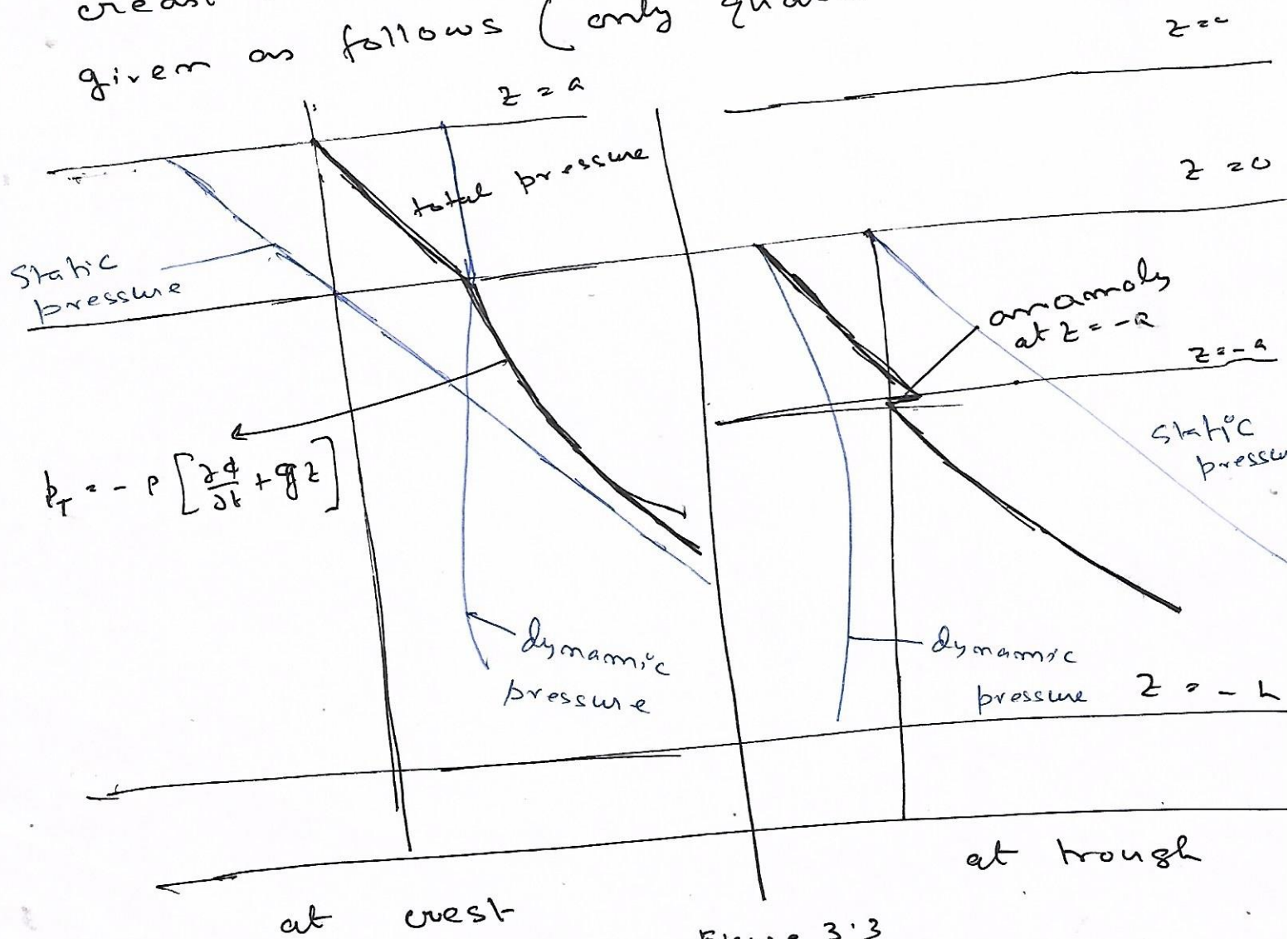


Figure 3.3

4. Froude-Krylov Force on submerged structure

we know that, if $\eta = a \cos(kx - \omega t)$ is the equation of the wave elevation. Then velocity potential of the fluid particle can be written as

$$\phi = \frac{ag}{\omega} e^{kz} \sin(kx - \omega t)$$

[Deep water case]

and the dynamic pressure due to wave may be written as

$$p_d = -\rho \frac{\partial \phi}{\partial t} = -\rho \left[-ag e^{kz} \cos(kx - \omega t) \right]$$

$$\text{or } p_d = \rho a g e^{kz} \cos(kx - \omega t) \quad \dots (4.1)$$

where 'z' varies from $z = -h$ to $z = 0$

\therefore if ' S_F ' be the surface of the submerged portion of the body, then the total force due to p_d is called Froude-Krylov or F-K force, and the expression of this force is

$$F_K = \iint_{S_F} p \cdot n \, dS_F \quad \dots (4.2)$$

$$F_K = \iint \left[\rho a g e^{kz} \cos(kx - \omega t) \right] \cdot \vec{n} \, dS. \quad \dots (4.3)$$

(4.3) represents the F-K force on the body at any instant of time 't' due to wave elevation $\eta = a \cos(kx - \omega t)$