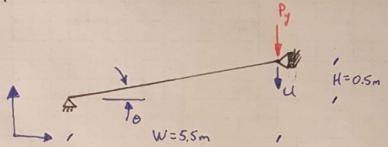
1-1



EA = 2100 KN

to 
$$E = \frac{L - L}{L}$$

$$E = \frac{1}{2} \frac{L^2 - L^2}{L^2}$$

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$$\tilde{E} = \frac{1}{2} \ln \left( \frac{L^2}{L^2} \right)$$

1. Derive force-displacement relation for all possible combinations (8 options) of kinematic relations and equilibrium on the undeformed and the deformed configuration and compare them in a single plot for OSUS2.5 H. Include the linear solution in the same plat

$$L = \begin{cases} 5.5 \\ 0.5 \end{cases}$$
  $L = \begin{cases} 5.5 \\ 0.5 - u \end{cases}$ 

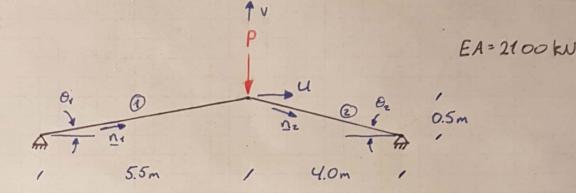
$$L^2 = L \cdot L = 30.5$$
  $L^2 = L \cdot L = 30.5 + u^2 - u$   
 $f = EA \cdot E$   $P_y = f \cdot \sin(\theta)$   $\sin(\theta) = \frac{H - u}{L}$ 

Undeformed

Deformed

$$P_y = \frac{EA}{2} \ln \left( \frac{L^2}{L^2} \right) \frac{H-u}{L}$$





1. Using Herkey strain, a linear relation  $T = E \cdot E$  A = const. and equilibrium on the deformed system, derive the relationship betweend displacement and forces on the free node i.e.

$$\begin{cases} P_x \\ P_y \end{cases} = \begin{cases} F_x(u,v) \\ F_y(u,v) \end{cases}$$

$$\epsilon = \frac{1}{2} \ln(\lambda^2) = \frac{1}{2} \ln(\frac{L^2}{L^2})$$
 (Herkey strain)

$$=30.5+u^2+v^2+11u+v$$

Also written as

2. Final the tangent stiffness matrix of the system. Keep it simple and don't substitute long expressions into your answer

Tangent stiffness for element is written as

$$[K^e] = \begin{bmatrix} k^e & k^e \end{bmatrix}$$

where

where f is force in element

Our global stiffness matrix is then

1 R' - R' O with nodes 1 & 3 being supported 2 -k1 k1+k2 -k2 node 2 un supported

Us & Us = 0, we are therefor only interested in K#= k'+k2

$$k^{1} = \frac{EA}{L_{1}} \underbrace{n_{1} \otimes n_{1}} + \frac{f_{1}}{l_{1}} \left( \underbrace{1 - n_{1} \otimes n_{1}} \right)$$

$$= \begin{cases} Cos(\theta_{1}) \\ sin(\theta_{1}) \end{cases} = \begin{cases} C_{1} \\ s_{1} \end{cases}$$

$$= \underbrace{EA}_{l_{1}} \begin{bmatrix} C_{1}^{2} & C_{1}s_{1} \\ C_{1}s_{1} & s_{1}^{2} \end{bmatrix} + \underbrace{EAE_{1}}_{l_{1}} \left[ \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix} - \begin{bmatrix} C_{1}^{2} & C_{1}s_{1} \\ C_{1}s_{1} & s_{1}^{2} \end{bmatrix} \right]$$

$$= \underbrace{EA(1 - E_{1})}_{l_{1}} \begin{bmatrix} C_{1}^{2} & C_{1}s_{1} \\ C_{1}s_{1} & s_{1}^{2} \end{bmatrix} + \underbrace{EAE_{1}}_{l_{1}} \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix}}_{l_{1}} = \underbrace{EAE_{1}}_{l_{2}} \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix}$$

1-2 2. continued

$$R^{2} = \frac{EA}{L_{1}} \underbrace{n_{1} \otimes n_{2} + \frac{f_{1}}{L_{1}} \left( \frac{1}{2} - n_{2} \otimes n_{2} \right)}_{C_{2}}$$

$$n_{2} = \left\{ \frac{Cos(\Theta_{2})}{-sin(\Theta_{2})} \right\} = \left\{ \frac{C_{2}}{-S_{2}} \right\} \qquad \left\{ \frac{C_{2}}{-S_{2}} + \frac{EAG_{2}}{L_{2}} \left( \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix} + \begin{bmatrix} C_{1}^{2} & -C_{2}S_{2} \\ -C_{2}S_{1} & S_{2}^{2} \end{bmatrix} \right)$$

$$= \underbrace{EA \left( 1 - G_{2} \right)}_{L_{2}} \begin{bmatrix} C_{2}^{2} & -C_{2}S_{1} \\ -C_{2}S_{2} & S_{2}^{2} \end{bmatrix} + \underbrace{EAG_{2}}_{L_{2}} \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix}}_{C_{2}}$$

$$= \underbrace{EA \left( 1 - G_{2} \right)}_{L_{2}} \begin{bmatrix} C_{2}^{2} & -C_{2}S_{1} \\ -C_{2}S_{2} & S_{2}^{2} \end{bmatrix} + \underbrace{EAG_{2}}_{L_{2}} \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix}}_{C_{2}}$$

Kff then be comes

Kff = [Kft K12]

Kff = [Ket K22]

where

$$K_{11} = \frac{EA(1-\epsilon_1)}{c_2} C_1^2 + \frac{EA\epsilon_1}{c_2} + \frac{EA(1-\epsilon_2)}{c_2} C_2^2 + \frac{EA\epsilon_2}{c_2}$$

$$= EA\left(\frac{C_1^2(1-\epsilon_1) + \epsilon_1}{c_2} + \frac{C_1^2(1-\epsilon_2) + \epsilon_2}{c_2}\right)$$

$$K_{21} = K_{12} = \frac{EA(1-E_1)C_1S_1}{L_1} + \frac{EA(1-E_2)(-C_2S_2)}{L_2}$$

$$= EA\left(\frac{C_1S_1(1-E_1)}{L_1} - \frac{C_2S_2(1-E_2)}{L_2}\right)$$

$$K_{22} = \frac{EA(1-\epsilon_1)}{l_1} S_1^2 + \frac{EA\epsilon_1}{l_1} + \frac{EA(1-\epsilon_2)}{l_2} S_2^2 + \frac{EA\epsilon_2}{l_2}$$

$$= EA\left(\frac{S_1^2(1-\epsilon_1) + \epsilon_1}{l_1} + \frac{S_2^2(1-\epsilon_2) + \epsilon_2}{l_2}\right)$$

Terms E, I have been written out on previous pages.

1-2

3. Formulate the condition Px=0 to the system of equations. Will this allow you to eliminate the horizontal displacement from the system of equations? Why? Why not?

From 1-21. we have

$$\frac{EA \in_2}{l_2} \left\{ \begin{array}{c} 4.0 - U \\ -0.5 - V \end{array} \right\} - \frac{EA \in_4}{l_4} \left\{ \begin{array}{c} 5.5 + U \\ 0.5 + V \end{array} \right\} = \left\{ \begin{array}{c} P_x \\ P_y \end{array} \right\}$$

Now setting Px = 0 gives

Looking only at force in x direction

$$EA\left(\frac{\epsilon_{1}}{l_{1}}(4.0-u)-\frac{\epsilon_{1}}{l_{1}}(5.5+u)\right)=0$$

$$=0 \quad \text{to fulfill conclition}$$

solving for a gives

$$U = \frac{5.5 \cdot \frac{\epsilon_1}{l_1} - 4.0 \cdot \frac{\epsilon_2}{l_2}}{-\left(\frac{\epsilon_1}{l_1} + \frac{\epsilon_2}{l_2}\right)}$$

But by, be, Es, Es are all functions of U, we can therefor not eliminate U.

Side note, if system was symmetric and  $l_1 = {a \brace b} l_2 = {a \brace -b}$  we would get

$$\frac{\varepsilon_2}{l_2}\left(\alpha-u\right)-\frac{\varepsilon_1}{l_1}\left(\alpha+u\right) \quad \frac{\omega}{l_2}=\varepsilon_1=\varepsilon$$

which would only be fulfilled at U=0 and we could eliminate U