

# CIV\_ENV 410 HW#3

Hao Yin

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Problem 1. Derive the governing equation for circular cylindrical shells.

Solution:

## Assumptions

- 1) Normal segments remain straight.
- 2) Segments orthogonal to the mid-plane remain orthogonal after deformation.
- 3) Normal segment has the same length after deformation.
- 4)  $\sigma_z \approx 0$
- 5) Linear elastic behavior, first order theory.

## Kinematics

The coordinate system used for circular cylindrical shell is shown in Figure 1. From symmetry we conclude that the component  $v$  of the displacement in the circumferential direction vanishes (here both  $v$  and  $\varphi$  represent the circumferential direction, the subscription  $v$  and  $\varphi$  are interchangeable in following sections). Thus, we only need to consider the components  $u$  and  $w$  in the  $x$  and  $z$  directions, respectively.

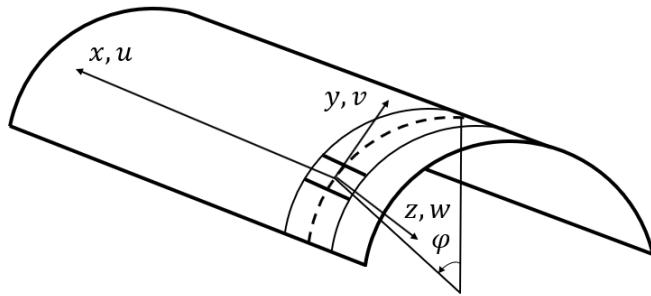


Figure 1: Coordinate system used for circular cylindrical shell

$$\begin{aligned}u(x, y, z) &= u \\v(x, y, z) &= 0 \\w(x, y, z) &= w\end{aligned}$$

### Strains

$$\varepsilon_x = \frac{du}{dx}$$

$$\varepsilon_\varphi = \frac{ds' - ds}{ds} = \frac{rd\varphi - wd\varphi - r\varphi}{rd\varphi} = -\frac{w}{r}$$

### Stresses

$$\sigma_x = \frac{E}{1-\nu^2}(\varepsilon_x + \nu\varepsilon_\varphi)$$

$$\sigma_\varphi = \frac{E}{1-\nu^2}(\varepsilon_\varphi + \nu\varepsilon_x) = E\varepsilon_\varphi = -E\frac{w}{r}$$

### Stress Resultants

The diagram of the sign convention of the stress resultants is shown in Figure 2.

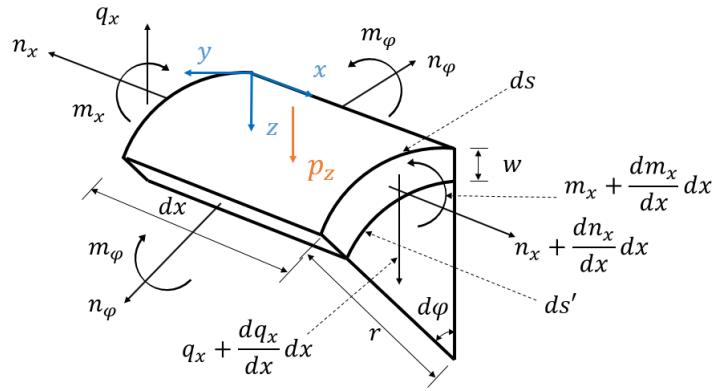


Figure 2: Sign convention of the stress resultants

$$n_x = \int_{-\frac{h}{2}}^{\frac{h}{2}} \sigma_x dz = \frac{Eh}{1-\nu^2}(\varepsilon_x + \nu\varepsilon_\varphi)$$

$$n_\varphi = \int_{-\frac{h}{2}}^{\frac{h}{2}} \sigma_\varphi dz = \frac{Eh}{1-\nu^2}(\varepsilon_\varphi + \nu\varepsilon_x) = -E\frac{w}{r}h$$

Considering the bending moments, we conclude from symmetry that there is no change in curvature in the circumferential direction (membrane behavior only).

The curvature in the x direction is equal to  $-\frac{d^2w}{dx^2}$  (flexural behavior only). Using the same equations as for plates, we then obtain

$$m_x = -D \frac{d^2w}{dx^2}$$

$$m_\varphi = \nu m_x$$

where  $D = \frac{E}{1-\nu^2} \cdot \frac{h^3}{12}$

### Equilibrium

Assuming that the external forces consist only of a pressure normal to the surface, taking the equilibrium of force in x-direction and in z-direction and moment about y-axis.

(1) Force in x-direction

$$\Sigma F_x = (n_x + \frac{dn_x}{dx} dx) rd\varphi \cdot w - n_x rd\varphi \cdot w = 0$$

Solve the equation

$$\frac{dn_x}{dx} = 0$$

(2) Moment about  $\varphi$ -axis

$$\begin{aligned} \Sigma M_\varphi = & m_x rd\varphi - (m_x + \frac{dm_x}{dx} dx) rd\varphi + p_z rd\varphi \cdot dx \cdot \frac{dx}{2} \\ & + 2(n_\varphi \frac{d\varphi}{2}) dx \cdot \frac{dx}{2} + (q_x + \frac{dq_x}{dx} dx) rd\varphi dx = 0 \end{aligned}$$

Solve the equation

$$q_x = \frac{dm_x}{dx}$$

(3) Force in z-direction

$$\begin{aligned} \Sigma F_z = & -q_x rd\varphi + (q_x + \frac{dq_x}{dx} dx) rd\varphi \\ & + z(n_\varphi \frac{d\varphi}{2}) dx + p_z dx \cdot rd\varphi = 0 \end{aligned}$$

Solve the equation

$$\frac{dq_x}{dx} + \frac{n_\varphi}{r} + p_z = 0$$

From the first the equation,  $n_x$  is constant, and we take them equal to zero in our further discussion. Since  $n_x = \frac{Eh}{1-\nu^2}(\varepsilon_x + \nu\varepsilon_\varphi) = 0$ , we obtain

$$\varepsilon_x = -\nu\varepsilon_\varphi = \nu \frac{w}{r}$$

and

$$\sigma_x = 0$$

### Governing Equations

By solving the equations

$$\left\{ \begin{array}{l} q_x = \frac{dm_x}{dx} \\ \frac{dq_x}{dx} + \frac{n_\varphi}{r} + p_z = 0 \\ m_x = -D \frac{d^2 w}{dx^2} \end{array} \right.$$

we can get

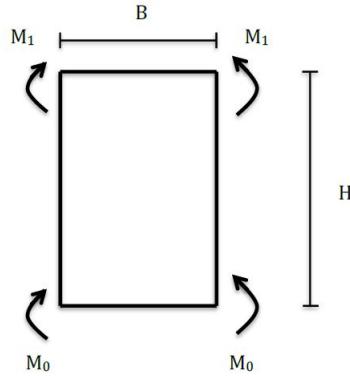
$$D \frac{d^4 w}{dx^4} + \frac{Eh}{r^2} w = p_z$$

which is the governing equation for circular cylindrical shells.

Problem 2. Compute displacement, rotation, bending moment, and shear force distributions for a cylindrical shell subject to uniform edge moments at both edges by using

- (1) the general exact solution (with 4 integration constants and coupled BCs at the two edges), and
- (2) the approximated solution with uncoupled BCs at the two edges. Provide the analytical equations of the solution as well as their graphical representation.

Use  $M_0 = 2 \text{ kNm/m}$ ,  $M_1 = 3 \text{ kNm/m}$ ,  $B = 2.4 \text{ m}$ ,  $H = 4 \text{ m}$ , thickness  $h = 0.1 \text{ m}$ , modulus of elasticity  $E_c = 30000 \text{ MPa}$ , Poisson ratio  $\nu = 0.2$ . Compare and comment on the two solutions.



Solution: The boundary conditions for this problem

$$\begin{aligned} m_x(x = 0) &= M_0 = 2 \times 10^3 \text{ N}\cdot\text{m/m} \\ m_x(x = 4) &= -M_1 = -3 \times 10^3 \text{ N}\cdot\text{m/m} \\ q_x(x = 0) &= \frac{dm_x}{dx}(x = 0) = 0 \\ q_x(x = 4) &= \frac{dm_x}{dx}(x = 4) = 0 \end{aligned}$$

- (1) The general exact solution (with 4 integration constants and coupled BCs at the two edges)

$$w_0 = e^{-\alpha x} [C_2 e^{i\alpha x} + C_3 e^{-i\alpha x}] + e^{\alpha x} [C_1 e^{i\alpha x} + C_4 e^{-i\alpha x}]$$

where

$$\alpha = \sqrt[4]{\frac{Eh}{4Dr^2}} = \frac{\sqrt[4]{3(1-\nu^2)}}{\sqrt{hr}} = \frac{\sqrt[4]{3(1-0.2^2)}}{\sqrt{(0.1)(2.4/2)}} = 3.76$$

Substituting the BCs into the equation above, we obtain

$$\begin{aligned} C_1 &= -8.3836 \times 10^{-12} + 1.0096i \times 10^{-12} \\ C_2 &= -1.3576 \times 10^{-5} - 1.3576i \times 10^{-5} \\ C_3 &= -1.3576 \times 10^{-5} + 1.3576i \times 10^{-5} \\ C_4 &= -8.3836 \times 10^{-12} - 1.0096i \times 10^{-12} \end{aligned}$$

Thus, the displacement

$$\Delta = -w_0 = e^{-\alpha x}[C_2 e^{i\alpha x} + C_3 e^{-i\alpha x}] + e^{\alpha x}[C_1 e^{i\alpha x} + C_4 e^{-i\alpha x}]$$

The rotation

$$\gamma = \frac{dw_0}{dx}$$

The bending moment

$$\begin{aligned} m_x &= -D \frac{d^2 w_0}{dx^2} \\ m_\varphi &= \nu m_x \end{aligned}$$

The shear force

$$q_x = \frac{dm_x}{dx} = -D \frac{d^3 w_0}{dx^3}$$

The expressions of the rotation, bending moment and the shear force from the exact solution are too long, please see the Appendix I for the full expressions.

- (2) The approximated solution with uncoupled BCs at the two edges

Considering  $M_0$  only,

$$w_{0b} = C_b e^{-\alpha x} \sin(\alpha x + \psi_b)$$

Substituting the BCs into the equation above, we obtain

$$\begin{aligned} C_b &= 3.840 \times 10^{-5} \text{ or } -3.840 \times 10^{-5} \\ \psi_b &= -\frac{\pi}{4} \text{ or } \frac{3\pi}{4} \end{aligned}$$

Considering  $M_1$  only,

$$w_{0t} = C_t e^{-\alpha(-x)} \sin(\alpha(-x) + \psi_t)$$

Substituting the BCs into the equation above, we obtain

$$\begin{aligned} C_t &= -1.689 \times 10^{-11} \text{ or } 1.689 \times 10^{-11} \\ \psi_t &= -\frac{\pi}{4} + 15.04 \text{ or } \frac{3\pi}{4} + 15.04 \end{aligned}$$

Here we take the values

$$C_b = 3.840 \times 10^{-5}$$

$$\psi_b = -\frac{\pi}{4}$$

$$C_t = -1.689 \times 10^{-11}$$

$$\psi_t = -\frac{\pi}{4} + 15.04$$

By superposing these two solutions,

$$\begin{aligned} w_0 &= w_{0b} + w_{0t} \\ &= 3.840 \times 10^{-5} e^{-\alpha x} \sin(\alpha x - \frac{\pi}{4}) \\ &\quad - 1.689 \times 10^{-11} e^{-\alpha(-x)} \sin(\alpha(-x) - \frac{\pi}{4} + 15.04) \end{aligned}$$

Thus, the displacement

$$\begin{aligned} \Delta &= -w_0 \\ &= -3.840 \times 10^{-5} e^{-\alpha x} \sin(\alpha x - \frac{\pi}{4}) \\ &\quad + 1.689 \times 10^{-11} e^{-\alpha(-x)} \sin(\alpha(-x) - \frac{\pi}{4} + 15.04) \end{aligned}$$

The rotation

$$\begin{aligned} \gamma &= \frac{dw_0}{dx} \\ &= (-\sqrt{2}\alpha)3.840 \times 10^{-5} e^{-\alpha x} \sin(\alpha x - \frac{\pi}{2}) \\ &\quad + (\sqrt{2}\alpha)1.689 \times 10^{-11} e^{-\alpha(-x)} \sin(\alpha(-x) - \frac{\pi}{2} + 15.04) \end{aligned}$$

The bending moment

$$\begin{aligned} m_x &= -D \frac{d^2 w_0}{dx^2} \\ &= (2D\alpha^2)3.840 \times 10^{-5} e^{-\alpha x} \sin(\alpha x - \frac{3\pi}{4}) \\ &\quad - (2D\alpha^2)1.689 \times 10^{-11} e^{-\alpha(-x)} \sin(\alpha(-x) - \frac{3\pi}{4} + 15.04) \end{aligned}$$

The bending moment

$$\begin{aligned} m_\varphi &= \nu m_x \\ &= (2D\nu\alpha^2)3.840 \times 10^{-5} e^{-\alpha x} \sin(\alpha x - \frac{3\pi}{4}) \\ &\quad - (2D\nu\alpha^2)1.689 \times 10^{-11} e^{-\alpha(-x)} \sin(\alpha(-x) - \frac{3\pi}{4} + 15.04) \end{aligned}$$

The shear force

$$\begin{aligned} q_x &= \frac{dm_x}{dx} = -D \frac{d^3 w_0}{dx^3} \\ &= (-2\sqrt{2}D\alpha^3)3.840 \times 10^{-5} e^{-\alpha x} \sin(\alpha x - \pi) \\ &\quad + (2\sqrt{2}D\alpha^3)1.689 \times 10^{-11} e^{-\alpha(-x)} \sin(\alpha(-x) - \pi + 15.04) \end{aligned}$$

The plots of the rotation, bending moment and the shear force calculated from the exact solution and the approximate solution are shown in Figure 3-7.

We can observe that the exact solution and the approximate solution are nearly the same, calculating the mean squared error (MSE) over all height span of the exact solution and the approximate solution

$$\begin{aligned} \text{MSE}_{\Delta} &= 6.7790 \times 10^{-24} \\ \text{MSE}_{\gamma} &= 1.5149 \times 10^{-22} \\ \text{MSE}_{m_x} &= 8.0236 \times 10^{-9} \\ \text{MSE}_{m_{\varphi}} &= 1.5149 \times 10^{-22} \\ \text{MSE}_{q_x} &= 4.4530 \times 10^{-7} \end{aligned}$$

All MSE are small numbers, which means that the approximation we used for this problem is accurate and practical enough to capture the behavior of circular cylindrical shells.

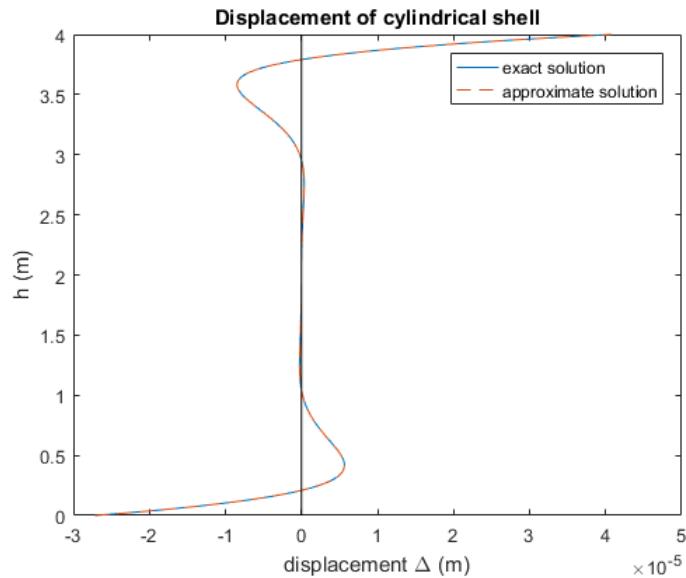


Figure 3: Displacement  $\Delta$  of the cylindrical shell

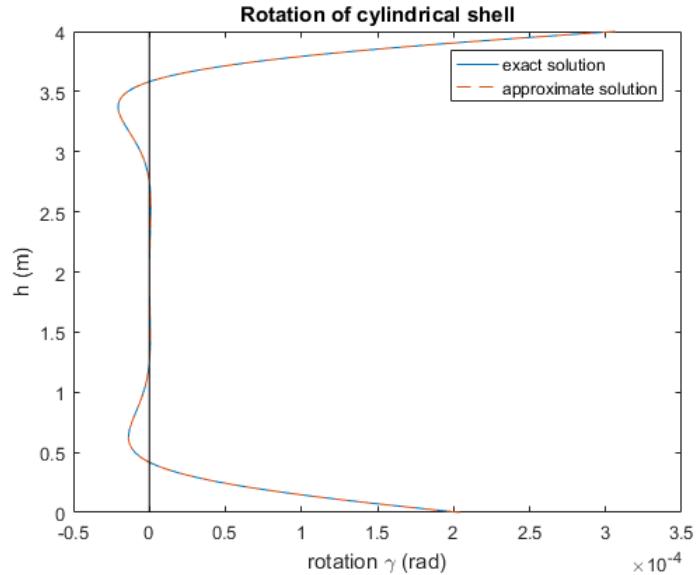


Figure 4: Rotation  $\gamma$  of the cylindrical shell

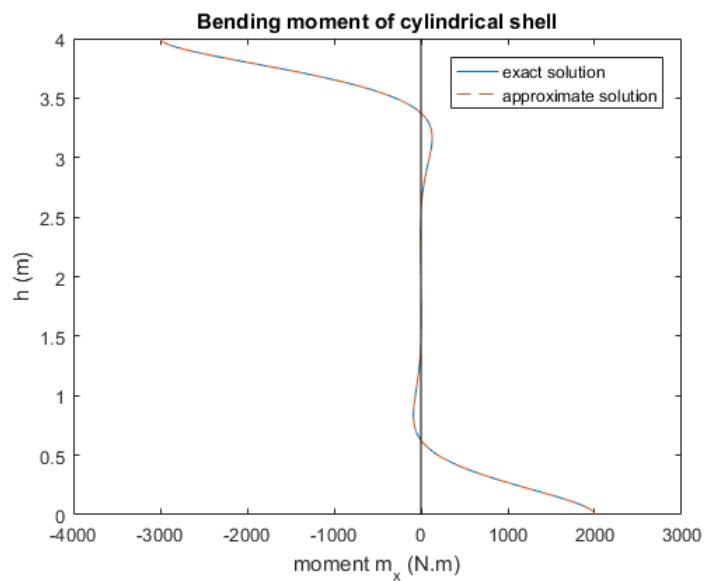


Figure 5: Bending moment  $m_x$  of the cylindrical shell

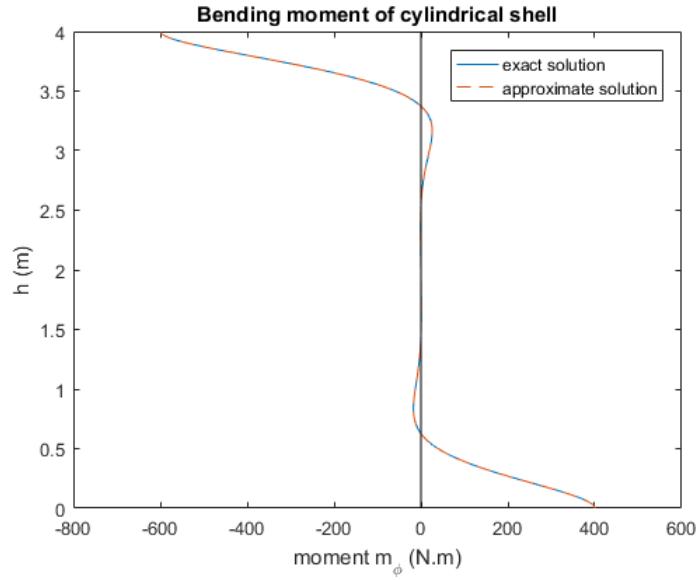


Figure 6: Bending moment  $m_\varphi$  of the cylindrical shell

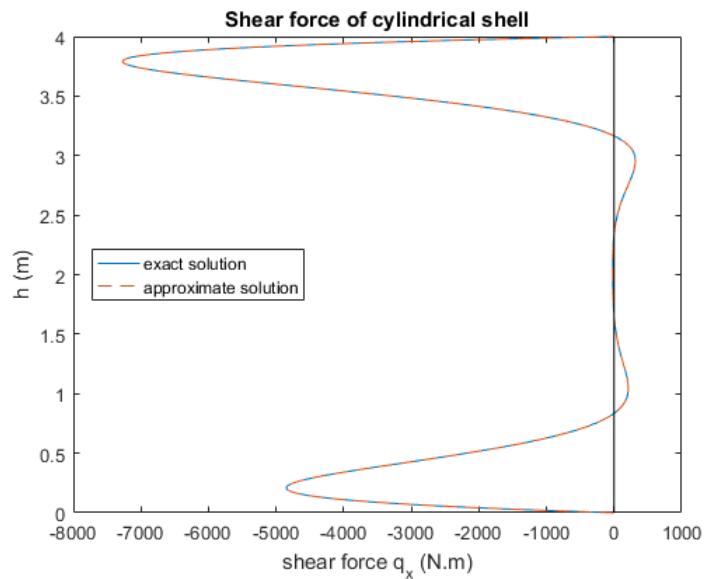


Figure 7: Shear force  $q_x$  of the cylindrical shell

## Appendix I: Full Expressions

For the exact solution, the displacement:

$$\Delta = e^{-\alpha x} [C_2 e^{i\alpha x} + C_3 e^{-i\alpha x}] + e^{\alpha x} [C_1 e^{i\alpha x} + C_4 e^{-i\alpha x}]$$

The rotation:

$$\begin{aligned}\gamma &= e^{\alpha x} (C_1 \alpha e^{\alpha x i} i - C_4 \alpha e^{-\alpha x i} i) + e^{-\alpha x} (C_2 \alpha e^{\alpha x i} i - C_3 \alpha e^{-\alpha x i} i) \\ &\quad + \alpha e^{\alpha x} (C_1 e^{\alpha x i} + C_4 e^{-\alpha x i}) - \alpha e^{-\alpha x} (C_2 e^{\alpha x i} + C_3 e^{-\alpha x i})\end{aligned}$$

The bending moment:

$$\begin{aligned}m_x &= D (e^{\alpha x} (C_1 \alpha^2 e^{\alpha x i} + C_4 \alpha^2 e^{-\alpha x i}) + e^{-\alpha x} (C_2 \alpha^2 e^{\alpha x i} + C_3 \alpha^2 e^{-\alpha x i})) \\ &\quad - 2 \alpha e^{\alpha x} (C_1 \alpha e^{\alpha x i} i - C_4 \alpha e^{-\alpha x i} i) + 2 \alpha e^{-\alpha x} (C_2 \alpha e^{\alpha x i} i - C_3 \alpha e^{-\alpha x i} i) \\ &\quad - \alpha^2 e^{\alpha x} (C_1 e^{\alpha x i} + C_4 e^{-\alpha x i}) - \alpha^2 e^{-\alpha x} (C_2 e^{\alpha x i} + C_3 e^{-\alpha x i})\end{aligned}$$

The shear force:

$$\begin{aligned}q_x &= D (e^{\alpha x} (C_1 \alpha^3 e^{\alpha x i} i - C_4 \alpha^3 e^{-\alpha x i} i) + e^{-\alpha x} (C_2 \alpha^3 e^{\alpha x i} i - C_3 \alpha^3 e^{-\alpha x i} i)) \\ &\quad - 3 \alpha^2 e^{\alpha x} (C_1 \alpha e^{\alpha x i} i - C_4 \alpha e^{-\alpha x i} i) - 3 \alpha^2 e^{-\alpha x} (C_2 \alpha e^{\alpha x i} i - C_3 \alpha e^{-\alpha x i} i) \\ &\quad - \alpha^3 e^{\alpha x} (C_1 e^{\alpha x i} + C_4 e^{-\alpha x i}) + \alpha^3 e^{-\alpha x} (C_2 e^{\alpha x i} + C_3 e^{-\alpha x i}) \\ &\quad + 3 \alpha e^{\alpha x} (C_1 \alpha^2 e^{\alpha x i} + C_4 \alpha^2 e^{-\alpha x i}) - 3 \alpha e^{-\alpha x} (C_2 \alpha^2 e^{\alpha x i} + C_3 \alpha^2 e^{-\alpha x i})\end{aligned}$$

## Appendix II: Matlab Code

```

1 clear ; clc
2
3 M0=2000;
4 M1=3000;
5 B=2.4;
6 H=4;
7 h=0.1;
8 Ec=30000*10^6;
9 nu=0.2;
10 r=B/2;
11 D=Ec*h^3/(12*(1-nu^2));
12 alpha=((3*(1-nu^2))^0.25)/(h*r)^0.5;
13
14 %% exact solution
15 syms x C2 C1 C3 C4
16 w0_syms=exp(-alpha*x)*(C2*exp(1i*alpha*x)+C3*exp(-1i*
    alpha*x))+exp(alpha*x)*(C1*exp(1i*alpha*x)+C4*exp(-1i*
    alpha*x));
17 dw1_syms=diff(w0_syms,x); % dw0/dx
18 dw2_syms=diff(dw1_syms); % d2w/dx2
19 dw3_syms=diff(dw2_syms); % d3w/dx3
20
21 mx1_syms=subs(-D*dw2_syms,x,0);
22 mx2_syms=subs(-D*dw2_syms,x,H);
23 qx1_syms=subs(-D*dw3_syms,x,0);
24 qx2_syms=subs(-D*dw3_syms,x,H);
25
26 [C1,C2,C3,C4]=solve(mx1_syms==M0,mx2_syms==M1,qx1_syms
    ==0,qx2_syms==0);
27 C1_double = double(C1);
28 C2_double = double(C2);
29 C3_double = double(C3);
30 C4_double = double(C4);
31 w0=exp(-alpha*x)*(C2_double*exp(1i*alpha*x)+C3_double*exp
    (-1i*alpha*x))+exp(alpha*x)*(C1_double*exp(1i*alpha*x)
    +C4_double*exp(-1i*alpha*x));
32 dw1=diff(w0,x);
33 dw2=diff(dw1);
34 dw3=diff(dw2);
35
36 w_func_exact = matlabFunction(w0);
37 gamma_func_exact = matlabFunction(diff(w0,x));
38 mx_func_exact = matlabFunction(-D*dw2);
39 qx_func_exact = matlabFunction(-D*dw3);
40
41 y = 0:0.01:4;
42 w_exact = w_func_exact(y);

```

```

43 gamma_exact = gamma_func_exact(y);
44 mx_exact = mx_func_exact(y);
45 qx_exact = qx_func_exact(y);
46
47 %% approximate solution
48 syms x Cb psib
49 w0b = Cb*exp(-alpha*x)*sin(alpha*x+psib);
50 dw1b_syms=diff(w0b,x);
51 dw2b_syms=diff(dw1b_syms);
52 dw3b_syms=diff(dw2b_syms);
53 mx1b_syms=subs(-D*dw2b_syms,x,0);
54 mx2b_syms=subs(-D*dw2b_syms,x,H);
55 qx1b_syms=subs(-D*dw3b_syms,x,0);
56 qx2b_syms=subs(-D*dw3b_syms,x,H);
57 [Cb, psib]=solve(mx1b_syms==M0, qx1b_syms==0);
58
59 syms x Ct psit
60 w0t = Ct*exp(-alpha*(-x))*sin(alpha*(-x)+psit);
61 dw1t_syms=diff(w0t,x);
62 dw2t_syms=diff(dw1t_syms);
63 dw3t_syms=diff(dw2t_syms);
64 mx1t_syms=subs(-D*dw2t_syms,x,0);
65 mx2t_syms=subs(-D*dw2t_syms,x,H);
66 qx1t_syms=subs(-D*dw3t_syms,x,0);
67 qx2t_syms=subs(-D*dw3t_syms,x,H);
68 [Ct, psit]=solve(mx2t_syms==M1, qx2t_syms==0);
69
70 % two set of C and psi here
71 Cb_double = double(Cb(1));
72 psib_double = double(psib(1));
73 Ct_double = double(Ct(1));
74 psit_double = double(psit(1));
75 %Cb_double = double(Cb(2));
76 %psib_double = double(psib2));
77 %Ct_double = double(Ct(2));
78 %psit_double = double(psit(2));
79
80 w_0 = Cb_double*exp(-alpha*x)*sin(alpha*x+psib_double);
81 w_1 = Ct_double*exp(alpha*x)*sin(-alpha*x+psit_double);
82 gamma_0 = diff(w_0);
83 gamma_1 = diff(w_1);
84 mx_0 = -D*diff(diff(w_0));
85 mx_1 = -D*diff(diff(w_1));
86
87 qx_func = -D*diff(diff(diff(w_0+w_1)));
88 mx_func = matlabFunction(mx_0+mx_1);
89 w_func = matlabFunction(w_0+w_1);
90 qx_func = matlabFunction(qx_func);
91 gamma_func = matlabFunction(gamma_0+gamma_1);
92

```

```

93 y = 0:0.01:4;
94 w_approx = w_func(y);
95 gamma_approx = gamma_func(y);
96 mx_approx = mx_func(y);
97 qx_approx = qx_func(y);
98
99
100 MSE_disp = mean((w_exact - w_approx).^2)
101 MSE_gamma = mean((gamma_exact - gamma_approx).^2)
102 MSE_mx = mean((-mx_exact + mx_approx).^2)
103 MSE_mphi = mean((gamma_exact - gamma_approx).^2)
104 MSE_qx = mean((qx_exact - qx_approx).^2)
105
106 figure()
107 plot(w_exact,y,w_approx,y,'—')
108 line([0 0],[0 4], 'Color', 'k')
109 xlabel('displacement \Delta (m)')
110 ylabel('h (m)')
111 legend('exact solution','approximate solution')
112 title('Displacement of cylindrical shell')
113
114 figure()
115 plot(gamma_exact,y,gamma_approx,y,'—')
116 line([0 0],[0 4], 'Color', 'k')
117 xlabel('gamma_func \gamma (rad)')
118 ylabel('h (m)')
119 legend('exact solution','approximate solution')
120 title('gamma_func of cylindrical shell')
121
122 figure()
123 plot(-mx_exact,y,-mx_approx,y,'—')
124 line([0 0],[0 4], 'Color', 'k')
125 xlabel('moment m_x (N.m)')
126 ylabel('h (m)')
127 legend('exact solution','approximate solution')
128 title('Bending moment of cylindrical shell')
129
130 figure()
131 plot(-nu*mx_exact,y,-nu*mx_approx,y,'—')
132 line([0 0],[0 4], 'Color', 'k')
133 xlabel('moment m_{\phi} (N.m)')
134 ylabel('h (m)')
135 legend('exact solution','approximate solution')
136 title('Bending moment of cylindrical shell')
137
138 figure()
139 plot(qx_exact,y,qx_approx,y,'—')
140 line([0 0],[0 4], 'Color', 'k')
141 xlabel('qx_func force q_x (N.m)')
142 ylabel('h (m)')

```

```
143 legend('exact solution','approximate solution','Location'  
144 , 'West')  
title('qx_func force of cylindrical shell')
```