

CE394M: Introduction to the Finite Element Method

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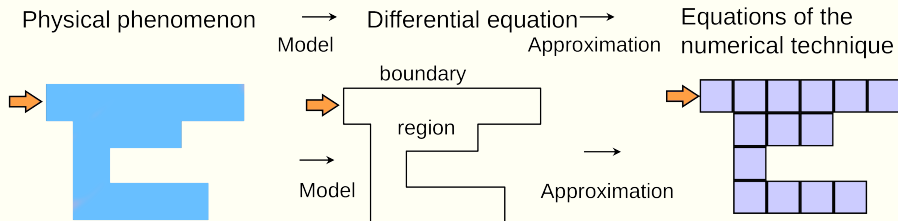
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Overview

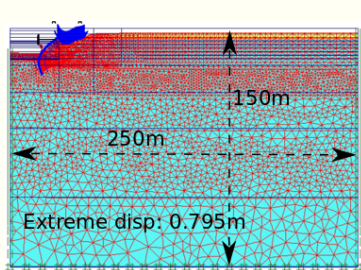
- 1 Numerical analysis of engineering problems
- 2 Galerkin methods
- 3 Strong form
- 4 Weak form
- 5 Finite Element formulation
- 6 Shape functions

Numerical analysis of engineering problems

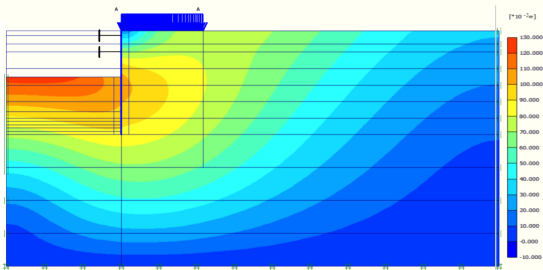
- Conceptualize the system
 - Geometry
 - Properties
 - Processes
- Describe it mathematically
 - Select the relevant differential equations
- Solve the equations (numerically)
 - Discretize the system
 - Settle for approximations (numerical techniques)
- Interpret the results



Finite Element Analysis



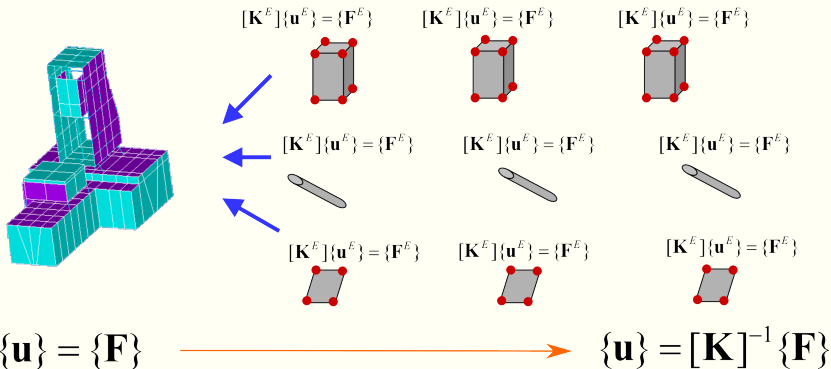
(a) FE Mesh



(b) Displacement profile

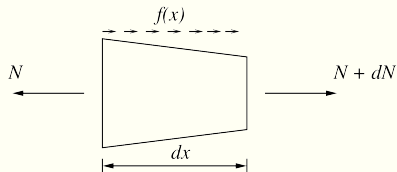
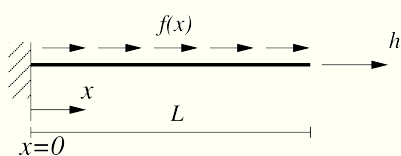
Singapore Nicoll highway excavation FE analysis

Finite Element Analysis



Finite Element Approximations

Strong form of the equilibrium equation for a 1-D bar



where f is a distributed force and h as a force applied at the end of the bar

The equilibrium equation can be derived by considering an infinitesimal bar:

where N is the normal force in the bar and f is the distributed force along the bar.

Boundary value problem of a 1-D bar

For linear elasticity

where $A(x)$ is the area of the bar, $E(x)$ is Young's modulus u is the displacement and $\varepsilon = du/dx$ is the strain.

which is a second-order differential equation. BCs:

Weak form of the equilibrium equations of a 1D bar

The general derivation of the weak form of any equation from the strong form follows a standard procedure:

- 1 Multiply the strong equation by a weight function v which is equal to zero where Dirichlet (displacement) boundary conditions are applied, but is otherwise arbitrary (Another condition is that it must be sufficiently continuous. The degree of continuity required depends on the properties of the equation being considered.)
- 2 Use integration by parts to 'shift' derivatives to the weight function
- 3 Insert the Neumann (force) boundary conditions

We then want to find a solution u to the weak form that holds for all v . The weight function is also known as the '*test*' function.

Weak form of the equilibrium equations of a 1D bar

Multiplying equilibrium equation by an arbitrary weight function v and integrating along the bar:

$$-\int_0^L v \frac{dN}{dx} dx. = \int_0^L v f dx.$$

we require that $v(0) = 0$ because of the displacement boundary condition at $x = 0$.

$$\int_0^L \frac{dv}{dx} N dx. = \int_0^L v f dx + vN \Big|_{x=0}^{x=L}.$$

Since $v(0) = 0$, inserting the constitutive relationship and taking into account the force boundary condition at $x = L$.

$$\int_0^L \frac{dv}{dx} EA \frac{du}{dx} dx. = \int_0^L v f dx + v(L)h.$$

The task is to find u with $u(0) = 0$ such that this equation is satisfied for all v .

FE formulation of a 1-D bar

Approximates the PDE by replacing the unknown function (the displacement u of the elastic bar) and the weight function (v) by approximate fields u_h and v_h . Inserting these fields into the weak equilibrium equation for a bar:

We now allow only a limited number of possibilities for v_h and u_h . Therefore, it is now unlikely that u_h will be equal to the exact solution.

Basis functions

The approximate displacement field u_h is represented by a set of '*basis functions*' $N_i(x)$:

x_i : discrete number of points (known as nodes)

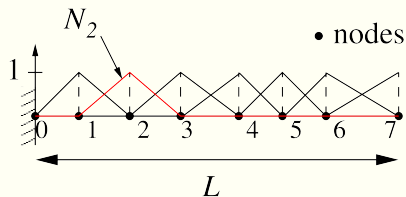
a_i : degrees of freedom

n : number of nodes

The approximate strain field:

FE shape functions

The simplest finite element basis functions in 1D hat-like continuous, piece-wise linear polynomials.



FE shape functions

For a bar divided into three elements, the displacement and strain fields could have the form

FE formulation of a 1-D bar

Weak form:

$$\int_0^L EA \frac{dv_h}{dx} \frac{du_h}{dx} dx = \int_0^L v_h f dx + v_h(L)h.$$

Using basis functions for u_h and v_h :

since a_i^* and a_j are not a function of x , we take it out.

$$\sum_i^n a_i^* \left(\sum_j^n a_j \int_0^L EA \frac{dN_i}{dx} \frac{dN_j}{dx} dx \right) = \sum_i^n a_i^* \int_0^L N_i f dx + \sum_i^n N_i(L) a_i^* h$$

FE formulation of a 1-D bar

Since a_i^* is arbitrary for each i we set $a_{k=i}^* = 1$ and $a_{k \neq i}^* = 0$. Then for each i we have an equation with n unknowns (the values of a_j):

$$i = 1 : \quad \sum_j^n a_j \int_0^L EA \frac{dN_1}{dx} \frac{dN_j}{dx} dx = \int_0^L N_1 f dx + N_1(L)h,$$

$$i = 2 : \quad \sum_j^n a_j \int_0^L EA \frac{dN_2}{dx} \frac{dN_j}{dx} dx = \int_0^L N_2 f dx + N_2(L)h,$$

\vdots

$$i = n : \quad \sum_j^n a_j \int_0^L EA \frac{dN_n}{dx} \frac{dN_j}{dx} dx = \int_0^L N_n f dx + N_n(L)h,$$

FE formulation of a 1-D bar

A system of linear equations is most conveniently expressed as a matrix:

Stiffness matrix:

right-hand side vector:

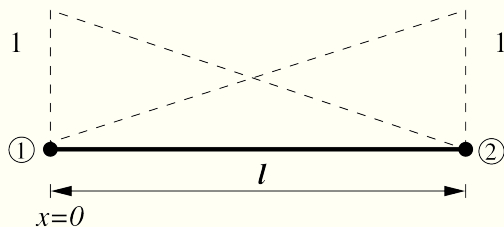
Finite Element Method: Formulation

$$[K] \mathbf{u} = F$$

$$\mathbf{u} = [K]^{-1} F$$

	Property $[K]$	Behavior $\{\mathbf{u}\}$	Action $\{\mathbf{F}\}$
Elastic	stiffness	displacement	force
Thermal	conductivity	temperature	heat source
Fluid	viscosity	velocity	body force

Linear shape functions



The displacement field inside the element is given by

Linear shape functions: displacements

Writing displacement field using matrices and vectors:

where the matrix N has the shape functions:

Matrix a_e contains the degrees of freedom for an element:

Linear shape functions: strains

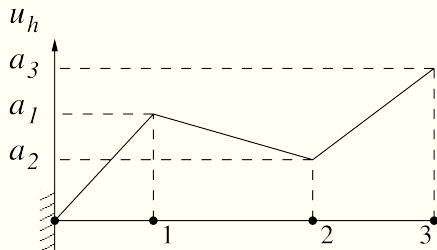
The strain field is written as:

Strain inside an element:

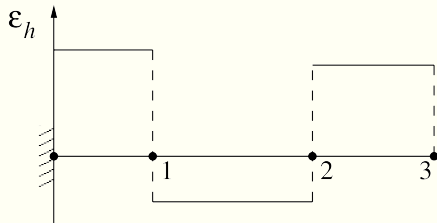
where the matrix B is the derivatives of the shape functions:

Continuity of finite element functions

For a bar divided into three elements, the displacement and strain fields could have the form:



(a) displacement field



(b) strain field

Quadratic element: shape functions

The shape functions:

$x_1 = -1$, $x_2 = 1$ and $x_3 = 0$:

$$N_1 = a_1 + b_1x + c_1x^2,$$

$$N_2 = a_2 + b_2x + c_2x^2,$$

$$N_3 = a_3 + b_3x + c_3x^2$$

$$N_1 = \frac{x^2}{2} - \frac{x}{2},$$

$$N_2 = \frac{x^2}{2} + \frac{x}{2},$$

$$N_3 = -x^2 + 1$$

Quadratic element: shape functions

The shape functions must satisfy three conditions and will be cubic polynomials of the form $N_i = a_i + b_i x + c_i x^2$. The SF must be equal to one at their node and zero at all others:

$$\begin{bmatrix} 1 & x_1 & x_1^2 \\ 1 & x_2 & x_2^2 \\ 1 & x_3 & x_3^2 \end{bmatrix} \begin{bmatrix} a_i \\ b_i \\ c_i \end{bmatrix} = \begin{bmatrix} 1 & -1 & 1 \\ 1 & 0 & 0 \\ 1 & 1 & 1 \end{bmatrix} \begin{bmatrix} a \\ b \\ c \end{bmatrix} = \begin{bmatrix} N_i(x_1) \\ N_i(x_2) \\ N_i(x_3) \end{bmatrix}$$

Inverting,

$$\begin{bmatrix} 0 & 1 & 0 \\ -0.5 & 0 & 0.5 \\ 0.5 & -1 & 0.5 \end{bmatrix} \begin{bmatrix} N_i(x_1) \\ N_i(x_2) \\ N_i(x_3) \end{bmatrix} = \begin{bmatrix} a_i \\ b_i \\ c_i \end{bmatrix}$$

For node one, $[N_i(x_1) \ N_i(x_2) \ N_i(x_3)]^T = [1 \ 0 \ 0]^T$, and for node two $[0 \ 0 \ 1]^T$, for node three $[0 \ 1 \ 0]^T$. This leads to

$$N_1 = \frac{x^2}{2} - \frac{x}{2}, \quad N_2 = \frac{x^2}{2} + \frac{x}{2}, \quad N_3 = -x^2 + 1$$