

CE394M: Stress paths and invariants

Krishna Kumar

University of Texas at Austin

krishnak@utexas.edu

Overview

1 Stresses / strains in typical geotechnical lab tests

2 Stress paths and Pi-plane

3 Stress invariants

4 References

Stresses / strains

1D consolidation / simple shear

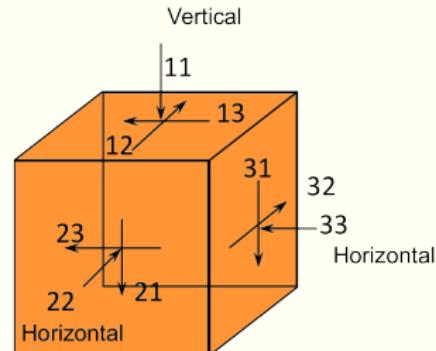
- Zero lateral strain ($\varepsilon_{22} = \varepsilon_{33} = 0$)
- Stresses: σ and τ
- Strains: $\varepsilon_{11} = \varepsilon_v$ and γ

2D plane strain

- Zero lateral strain ($\varepsilon_{22} = \gamma_{12} = \gamma_{23} = 0$)
- Stresses: s and t
- Strains: ε_v and ε_γ

3D general (axi-symmetric as a special case)

- Stresses: p and q
- Strains: ε_v and ε_s



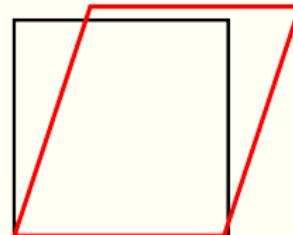
1D simple shear

- ① No lateral strain
- ② Constant normal effective stress σ'_v
- ③ Increasing shear strain γ
- ④ Measure shear resistance τ
- ⑤ Measure volumetric strain ε_v or void ratio $e = e_0 - (1 + e_0)\varepsilon_v$
- ⑥ **No information for the lateral direction**

Normal effective stress σ'_v



σ'_v ↓ $\Delta\tau_{12}$ → $\Delta\gamma_{12}$ $\Delta\varepsilon_v$ ↑



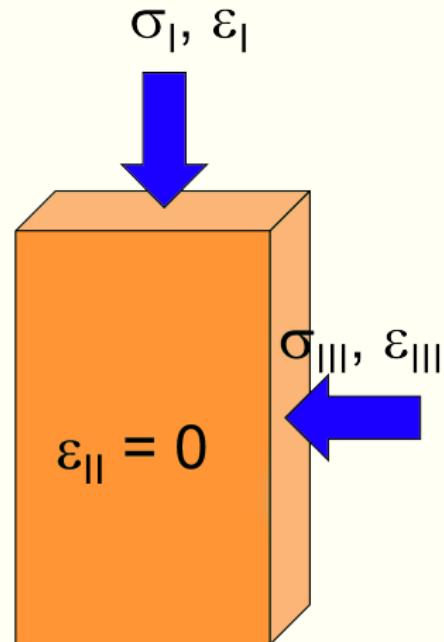
2D plane strain / Mohr-Coulomb model

Stresses and strains: independent components

- ① Mean stress: $s = (\sigma_I + \sigma_{III})/2$ and $s' = s - u$
- ② Volumetric strain: $\varepsilon_v = (\varepsilon_I + \varepsilon_{III})$
- ③ Deviatoric / shear stress: $t = (\sigma_I - \sigma_{III})/2$ and $t' = t$.
- ④ Deviatoric / shear strain: $\varepsilon_\gamma = (\varepsilon_I - \varepsilon_{III})$
- ⑤ Work increment:

$$\delta W = \sigma'_I \delta \varepsilon_I + \sigma'_{III} \delta \varepsilon_{III} = s' \delta \varepsilon_v + t \delta \varepsilon_\gamma$$

- ⑥ s and t are often used to derive parameters for Mohr-Coulomb model because it only considers σ_I and σ_{III} and not σ_{II} .



$$\sigma' = \sigma - u$$

CE394M: Stresses - paths & invariants

└ Stresses / strains in typical geotechnical lab tests

└ 2D plane strain / Mohr-Coulomb model

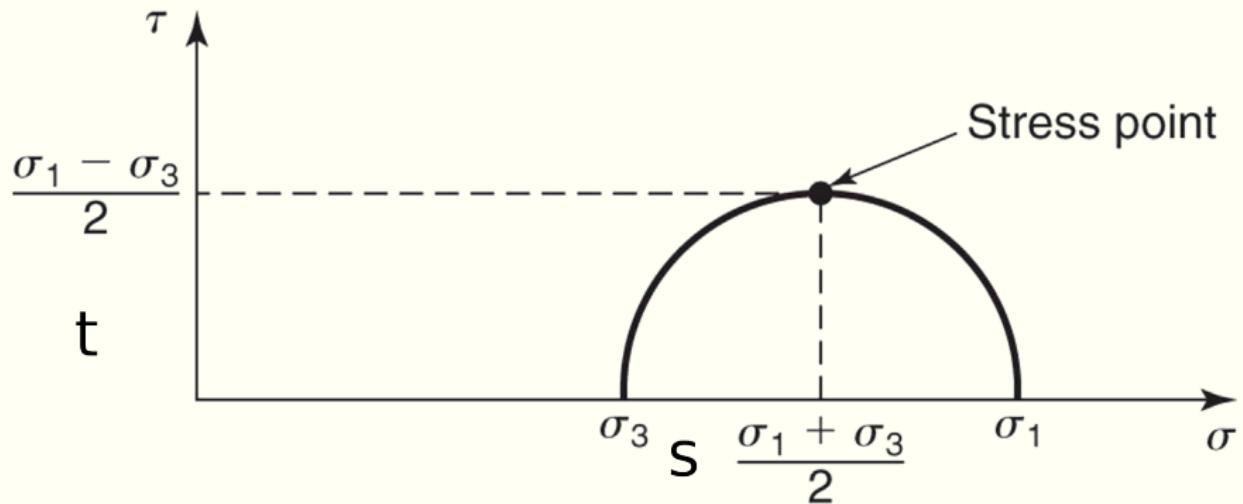
Stresses and strains: independent components

- ➊ Mean stress: $s = (\sigma_I + \sigma_{II})/2$ and $s' = s - u$
 - ➋ Volumetric strain: $\varepsilon_v = (\varepsilon_I + \varepsilon_{II})$
 - ➌ Deviatoric / shear stress: $t = (\sigma_I - \sigma_{II})/2$ and $t' = t$.
 - ➍ Deviatoric / shear strain: $\varepsilon_t = (\varepsilon_I - \varepsilon_{II})$
 - ➎ Work increment:
- $$\delta W = \sigma'_I \delta \varepsilon_I + \sigma'_{II} \delta \varepsilon_{II} = s' \delta \varepsilon_v + t \delta \varepsilon_t$$
- s and t are often used to derive parameters for Mohr-Coulomb model because it only considers σ_I and σ_{II} and not σ_{III} .**
-
- $\sigma_{III} = 0$
- $\varepsilon_{III} = 0$
- σ'_I, σ'_{II}

Principal stresses $\sigma_I > \sigma_{II} > \sigma_{III}$. This equation holds good and is the definition of σ in principal stress notations, i.e., $I > II > III$.

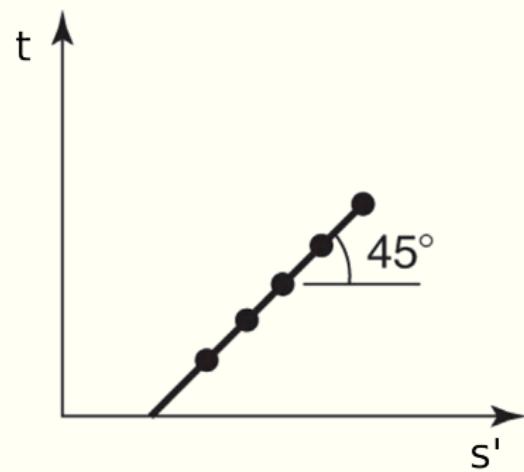
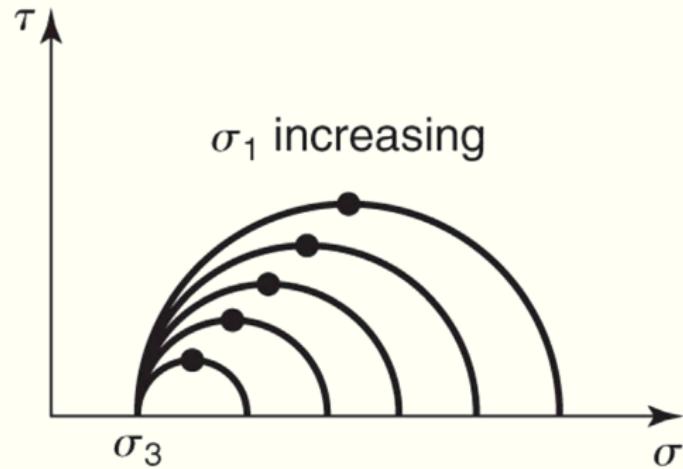
No shear stresses on these planes.

2D Mohr circle



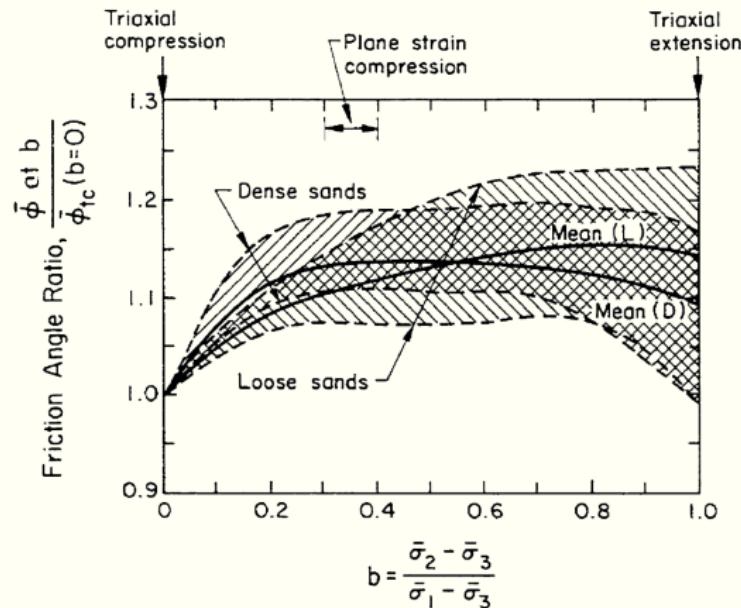
Mohr circle to 2D stress components

Successive Mohr circles and stress path for constant σ_3 and increasing σ_1 :



Effect of σ_{II}

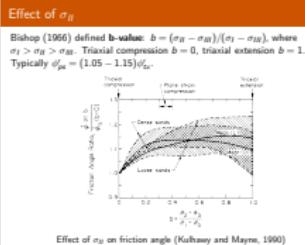
Bishop (1966) defined **b-value**: $b = (\sigma_{II} - \sigma_{III}) / (\sigma_I - \sigma_{III})$, where $\sigma_I > \sigma_{II} > \sigma_{III}$. Triaxial compression $b = 0$, triaxial extension $b = 1$. Typically $\phi'_{ps} = (1.05 - 1.15)\phi'_{tx}$.



Effect of σ_{II} on friction angle (Kulhawy and Mayne, 1990)

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└ Stresses / strains in typical geotechnical lab tests

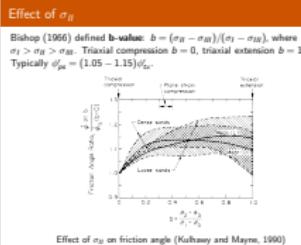
└ Effect of σ_{II} 

σ_{II} do have an effect on soil behavior. For example, the friction angle depends on the loading condition: triaxial compression, plane-strain, triaxial extension and others, The effect of σ_{II} can be measured using true triaxial apparatus or hollow cylinder torsional shear apparatus. In general, the peak friction angle increases 10 to 15 percent from $b = 0$ (triaxial compression) to $b = 0.3$ to 0.4 (plane strain), and it stays constant or slightly decreases as b reaches 1 (tri-axial extension).

The variation of measured friction angle with changes in σ_{II} can be attributed to the effects of different mean stress and stress anisotropy on the dilatancy and particle rearrangement contributions to the total strength. For given maximum and minimum principal stresses, the TXE conditions have the largest mean effective stress, whereas the triaxial compression conditions have the smallest mean effective stress. The higher confinement for triaxial extension and plane strain conditions contributes to the increasing friction angle for these conditions. (Mitchell and Soga., 2005)

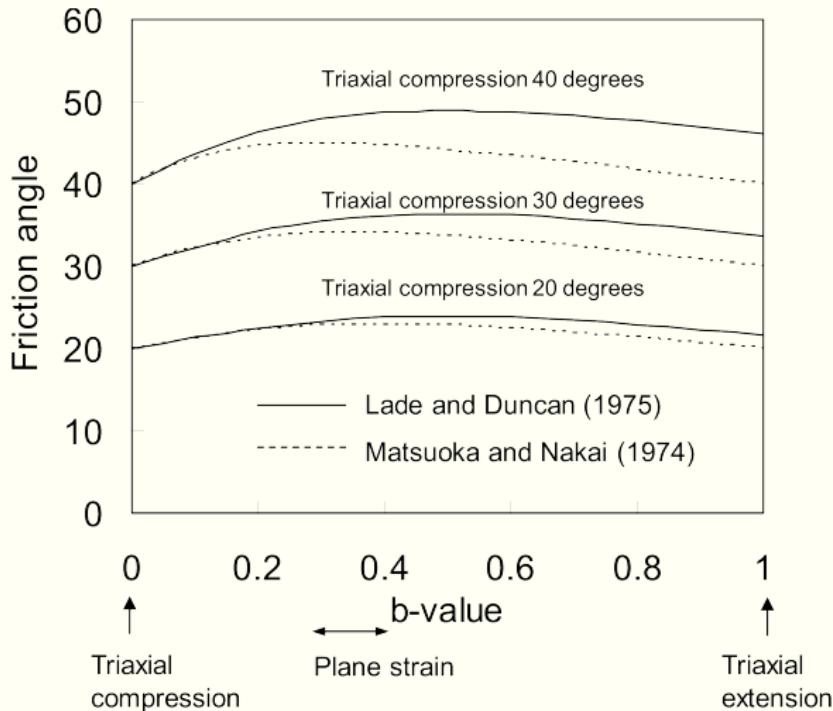
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└ Stresses / strains in typical geotechnical lab tests

└ Effect of σ_{II} 

In TXC major principal stress, σ_1 acts vertically, compared to triaxial extension mode of loading, where σ_1 acts horizontally. The effect of the intermediate principal stress is comparatively smaller, and is often assessed using the parameter $b = (\sigma_{II} - \sigma_{III})/(\sigma_1 - \sigma_{III})$, which is zero in triaxial compression, and one in triaxial extension. In TXE $\sigma_1 = \sigma_{II}$ (radial), while σ_{III} is the vertical stress.

Effect of σ_{II} on friction



Chapter 11., Mitchell and Soga, 2005

Effect of σ_{II} on friction

Various models fit the experimental data showing the intermediate stress effect. Lade and Duncan (1975)

$$\frac{I_1^3}{I_3} = \text{const}$$

Matsuoka and Nakai (1974)

$$\frac{I_1 I_2}{I_3} = \text{const}$$

$$I_1 = \sigma_1 + \sigma_2 + \sigma_3$$

$$I_2 = \sigma_1\sigma_2 + \sigma_2\sigma_3 + \sigma_3\sigma_1$$

$$I_3 = \sigma_1\sigma_2\sigma_3$$

Matsuoka and Nakai's model gives the same friction angle for compression and extension, whereas Lade and Duncan's model gives the ratio of the TXE friction angle (ϕ'_{TXE}) to the TXC friction angle (ϕ'_{TXC}) to be 1.08 at $\phi'_{TXC} = 20^\circ$ to 1.15 at $\phi'_{TXC} = 40^\circ$.

CE394M: Stresses - paths & invariants

└ Stresses / strains in typical geotechnical lab tests

└ Effect of σ_{II} on frictionEffect of σ_R on friction

Various models fit the experimental data showing the intermediate stress effect. Lade and Duncan (1975)

$$\frac{I_2^2}{I_3} = \text{const}$$

Matsuoka and Nakai (1974)

$$\frac{I_2 I_3}{I_1} = \text{const}$$

$$I_1 = \sigma_1 + \sigma_2 + \sigma_3$$

$$I_2 = \sigma_1 \sigma_2 + \sigma_2 \sigma_3 + \sigma_3 \sigma_1$$

$$I_3 = \sigma_1 \sigma_2 \sigma_3$$

Matsuoka and Nakai's model gives the same friction angle for compression and extension, whereas Lade and Duncan's model gives the ratio of the TXE friction angle (ϕ'_{TXE}) to the TXC friction angle (ϕ'_{TXC}) to be 1.08 at $\phi'_{TXC} = 20^\circ$ to 1.15 at $\phi'_{TXC} = 40^\circ$.

Given the large scatter in the published experimental data, it is not possible to conclude that one model is better than the other.

Triaxial stresses and strains: independent components

- We split the stress system into:

- purely volumetric deformation 'p'
- purely distortional deformation 'q'

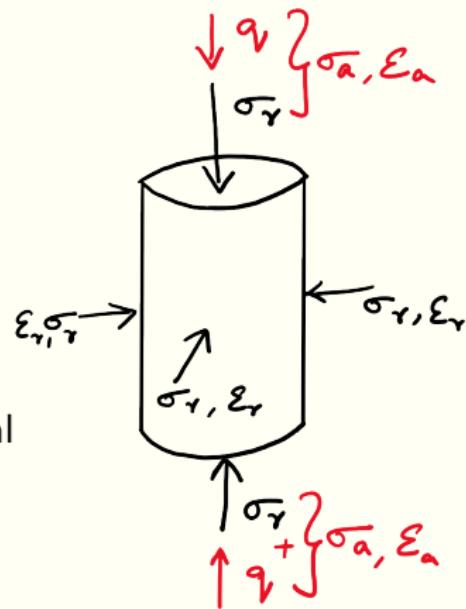
- Mean stress:

$$p = (\sigma_a + 2\sigma_r)/3 \quad p' = p - u$$

- Volumetric strain: $\varepsilon_v = (\varepsilon_a + 2\varepsilon_r)$
- Deviatoric / shear stress (purely distortional deformation):

$$q = (\sigma_a - \sigma_r) \quad q' = q$$

- Deviatoric / shear strain: $\varepsilon_s = 2/3(\varepsilon_a - \varepsilon_r)$



Triaxial deviatoric strain

- Deviatoric / shear strain: $\varepsilon_s = 2/3(\varepsilon_a - \varepsilon_r)$
- Why 2/3?:
- Work equation: $\delta W = p'\delta\varepsilon_v + q'\delta\varepsilon_s$

$$\delta W = \sigma'_1\delta\varepsilon_1 + \sigma'_2\delta\varepsilon_2 + \sigma'_3\delta\varepsilon_3 = \sigma'_a\delta\varepsilon_a + 2\sigma'_r\delta\varepsilon_r$$

$$\begin{aligned}\delta W &= (1/3\sigma'_a + 2/3\sigma'_r)(\delta\varepsilon_a + 2\delta\varepsilon_r) + (\sigma'_a - \sigma'_r)(2/3)(\delta\varepsilon_a - \delta\varepsilon_r) \\ &= \sigma'_a\delta\varepsilon_a + 2\sigma'_r\delta\varepsilon_r\end{aligned}$$

Triaxial stress - strain relationship

Relationship in terms of axial and radial direction:

$$\begin{bmatrix} \varepsilon_a \\ \varepsilon_r \end{bmatrix} = \frac{1}{E} \begin{bmatrix} 1 & -2\nu \\ -\nu & (1-\nu) \end{bmatrix} \begin{bmatrix} \sigma_a \\ \sigma_r \end{bmatrix} \quad (1)$$

Relationship in terms of 'p' and 'q':

$$\begin{bmatrix} p \\ q \end{bmatrix} = \begin{bmatrix} 1/3 & 2/3 \\ 1 & -1 \end{bmatrix} \begin{bmatrix} \sigma_a \\ \sigma_r \end{bmatrix} \quad \text{invert} \rightarrow \begin{bmatrix} \sigma_a \\ \sigma_r \end{bmatrix} = \begin{bmatrix} 1 & 2/3 \\ 1 & -1/3 \end{bmatrix} \begin{bmatrix} p \\ q \end{bmatrix} \quad (2)$$

Strain:

$$\begin{bmatrix} \varepsilon_v \\ \varepsilon_s \end{bmatrix} = \begin{bmatrix} 1 & 2 \\ 2/3 & -2/3 \end{bmatrix} \begin{bmatrix} \varepsilon_a \\ \varepsilon_r \end{bmatrix} \quad (3)$$

Volume - shear coupling

Equation (1) and eq. (2) in eq. (3) to relate $\varepsilon_v, \varepsilon_s$ to p, q :

$$\begin{bmatrix} \varepsilon_v \\ \varepsilon_s \end{bmatrix} = \begin{bmatrix} 3(1-2\nu)/E & 0 \\ 0 & 2(1+\nu)/3E \end{bmatrix} \begin{bmatrix} p \\ q \end{bmatrix}$$

$$\begin{bmatrix} \varepsilon_v \\ \varepsilon_s \end{bmatrix} = \begin{bmatrix} 1/K & 0 \\ 0 & 1/3G \end{bmatrix} \begin{bmatrix} p \\ q \end{bmatrix}$$

- ① Off-diagonal zeros indicate no coupling between volumetric and distortional effects.
- ② Applying pure shear cannot induce volume changes - **not true for soils!**
- ③ Plasticity theory will deal with this.

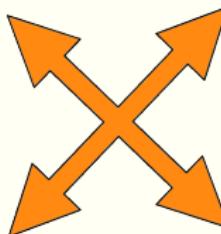
Volume shear coupling

Usually volume change and shear deformation are decoupled (i.e. bulk modulus and shear modulus), but in soils they are coupled:

Change in
confining stress



Volumetric strain
increment



If particulate matter

Change in shear
stress

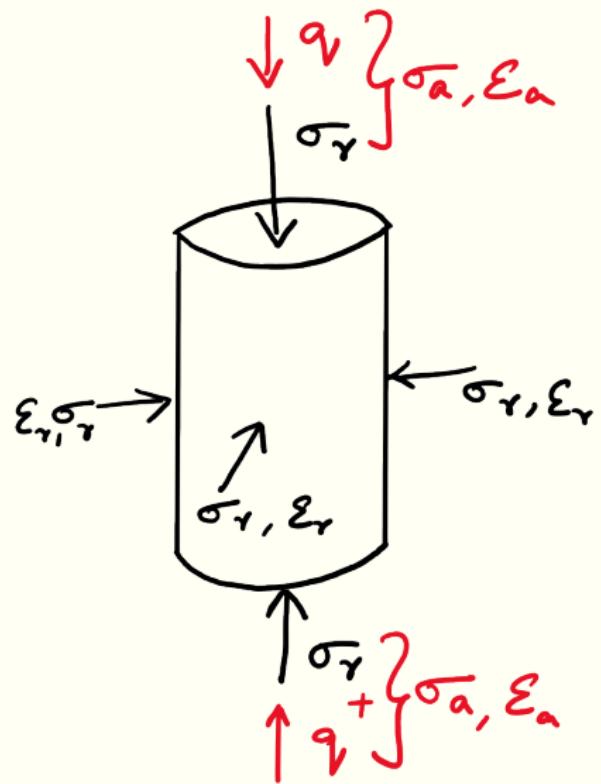


Deviator strain
increment

Overview

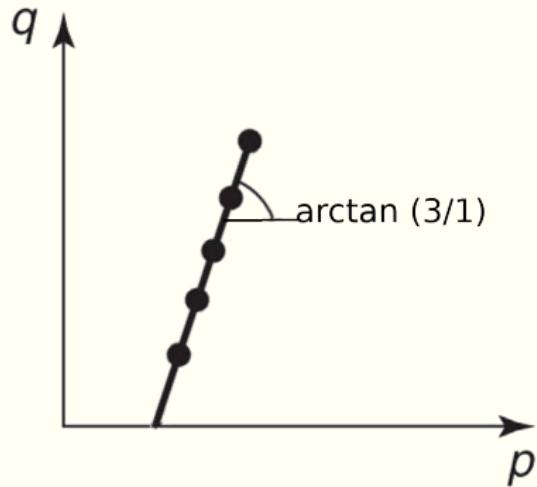
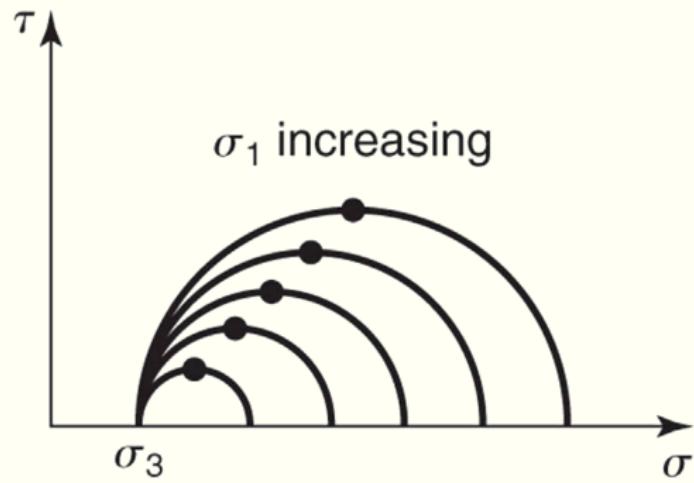
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Triaxial test



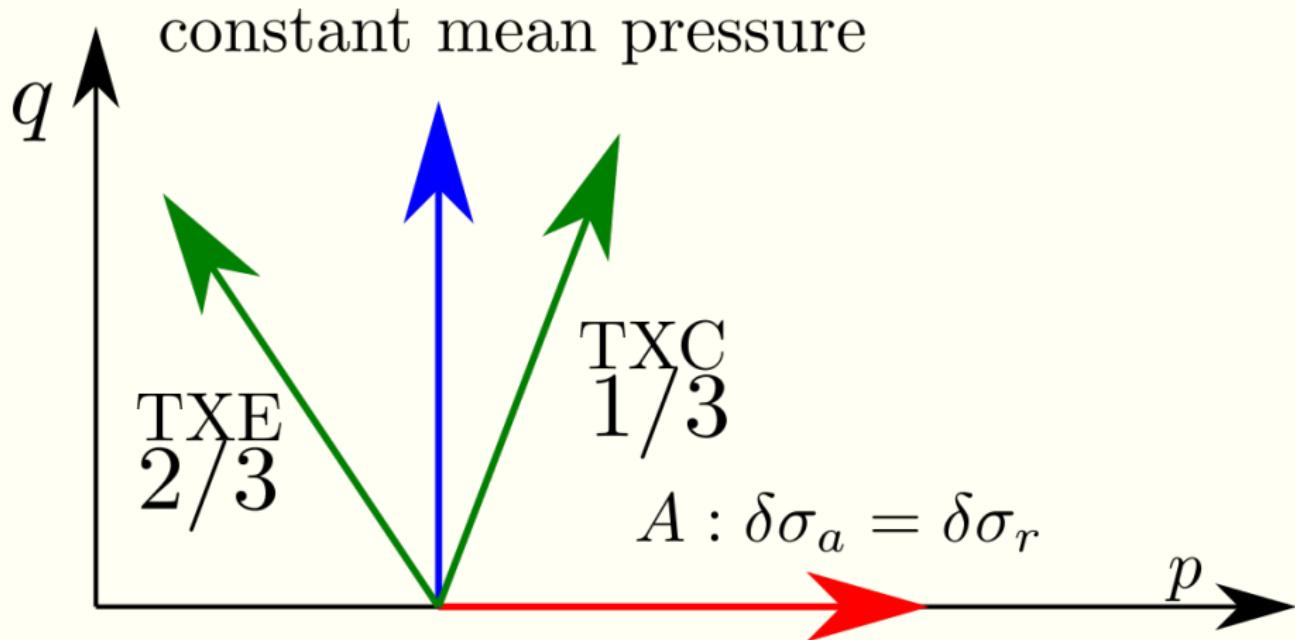
Mohr circle to 3D stress components

Successive Mohr circles and stress path for constant σ_3 and increasing σ_1 :



Stress paths p-q

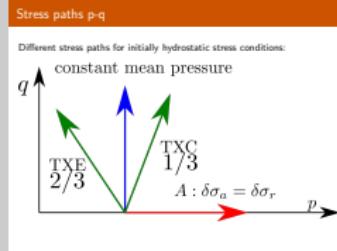
Different stress paths for initially hydrostatic stress conditions:



CE394M: Stresses - paths & invariants

└ Stress paths and Pi-plane

└ Stress paths p-q



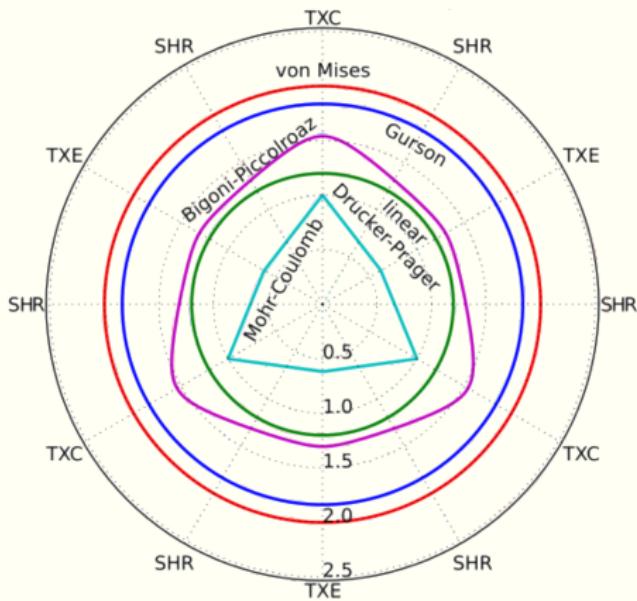
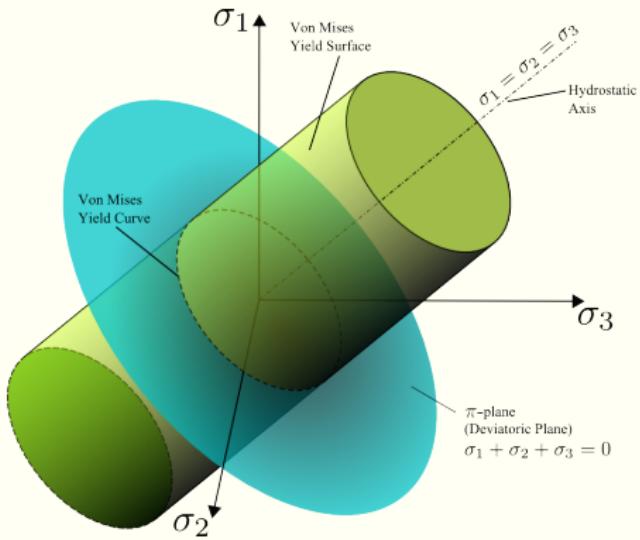
A mean total stress $p = \frac{\sigma_a + 2\sigma_r}{3}$ and deviatoric stress $q = \sigma_a - \sigma_r$. In triaxial compression the cell pressure is held constant, $\delta\sigma_r = 0$ and axial load is increased.

$$\delta p = \frac{\delta q}{3}$$

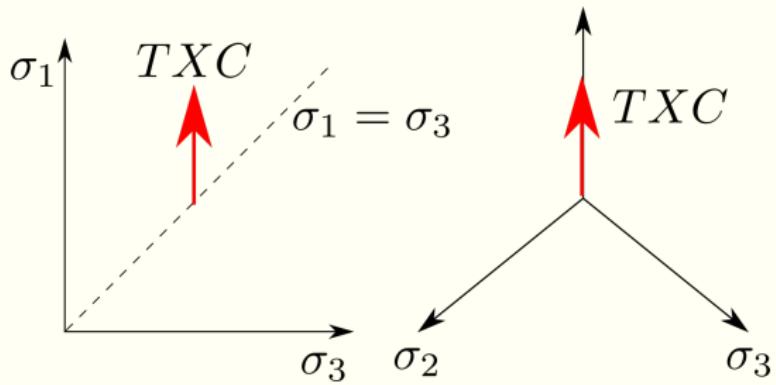
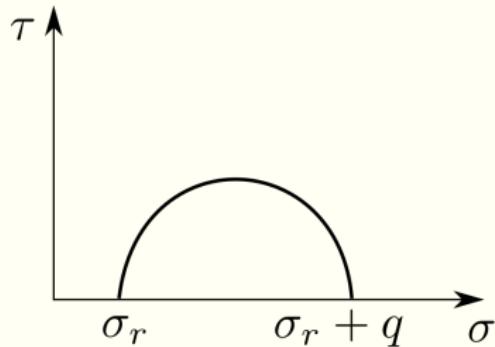
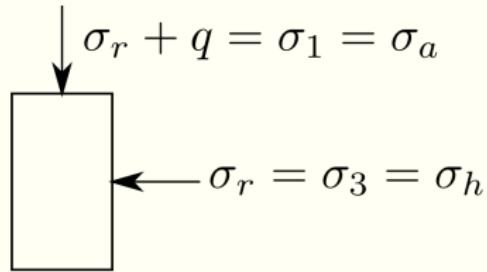
In triaxial extension, to keep the axial stress constant while changing the cell pressure requires that the ram force (or deviatoric stress) and cell pressure must be changed simultaneously. For $\delta\sigma_a = 0, \delta\sigma_r = -\delta q$. The differential form is:

$$\delta p = \frac{2\delta\sigma_r}{3} = \frac{-2\delta q}{3}$$

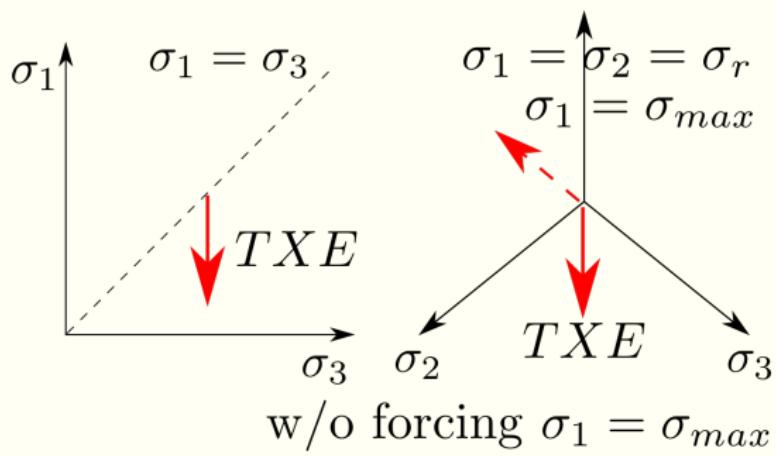
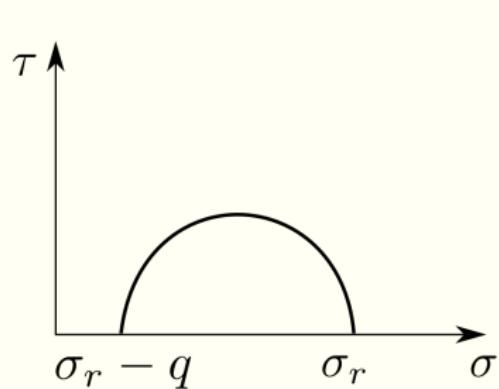
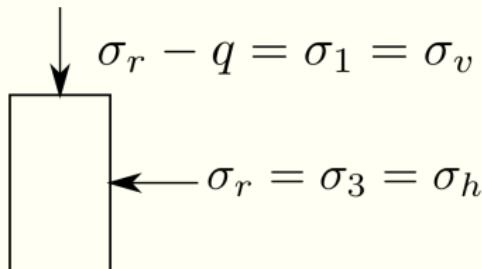
Pi-plane



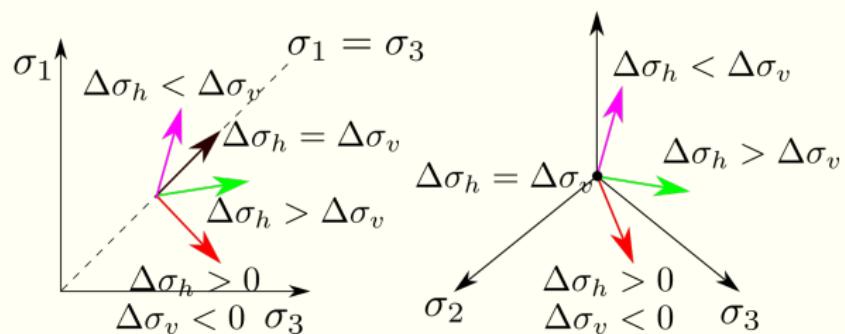
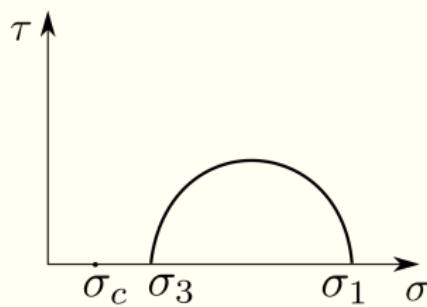
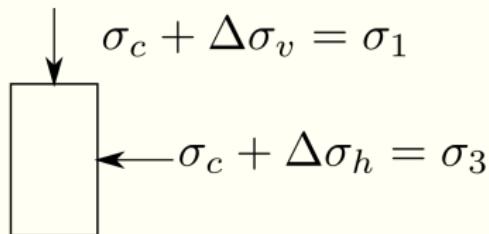
π plane: Triaxial compression



π plane: Triaxial extension



π plane: Random stress paths



Overview

1 Stresses / strains in typical geotechnical lab tests

2 Stress paths and Pi-plane

3 Stress invariants

4 References

Stress and strain invariants in 3D

6 stresses and strains

- Mean pressure: $p' = (\sigma'_{11} + \sigma'_{22} + \sigma'_{33})/3$
- Deviator stress: $q = \sqrt{3/2} \sqrt{s_{11}^2 + s_{22}^2 + s_{33}^2 + 2\tau_{12}^2 + 2\tau_{23}^2 + 2\tau_{31}^2}$
- where $s_{11} = \sigma'_{11} - p'$, $s_{22} = \sigma'_{22} - p'$, $s_{33} = \sigma'_{33} - p'$
- Volumetric strain: $\varepsilon_v = \varepsilon_{11} + \varepsilon_{22} + \varepsilon_{33}$
- Deviatoric strain:
$$\varepsilon_s = \sqrt{2/3} \sqrt{e_{11}^2 + e_{22}^2 + e_{33}^2 + \gamma_{12}^2/2 + \gamma_{23}^2/2 + \gamma_{31}^2/2}$$
- where $e_{11} = \varepsilon_{11} - \varepsilon_v/3$, $e_{22} = \varepsilon_{22} - \varepsilon_v/3$, $e_{33} = \varepsilon_{33} - \varepsilon_v/3$

Stress and strain invariants in 3D

3 Principal stresses and strains

- Mean pressure: $p' = (\sigma'_I + \sigma'_{II} + \sigma'_{III})/3$
- Deviator stress: $q = \sqrt{1/2} \sqrt{(\sigma'_I - \sigma'_{II})^2 + (\sigma'_{II} - \sigma'_{III})^2 + (\sigma'_{III} - \sigma'_I)^2}$
- Volumetric strain: $\varepsilon_v = \varepsilon_I + \varepsilon_{II} + \varepsilon_{III}$
- Deviatoric strain: $\varepsilon_s = \frac{\sqrt{2}}{3} \sqrt{(\varepsilon'_I - \varepsilon'_{II})^2 + (\varepsilon'_{II} - \varepsilon'_{III})^2 + (\varepsilon'_{III} - \varepsilon'_I)^2}$

In triaxial condition (principal stresses/strains)

$$p' = (\sigma_I + 2\sigma_{III})/3 \quad q = \sigma_I - \sigma_{III} \quad \varepsilon_v = \varepsilon_I + 2\varepsilon_{III} \quad \varepsilon_s = 2(\varepsilon_I - \varepsilon_{III})/3$$

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└ Stress invariants

└ Stress and strain invariants in 3D

3 Principal stresses and strains

- Mean pressure: $p' = (\sigma'_I + \sigma'_H + \sigma'_{III})/3$

- Deviator stress: $q = \sqrt{1/2} \sqrt{(\sigma'_I - \sigma'_{II})^2 + (\sigma'_H - \sigma'_{III})^2 + (\sigma'_{III} - \sigma'_I)^2}$

- Volumetric strain: $\varepsilon_v = \varepsilon_I + \varepsilon_H + \varepsilon_{III}$

- Deviatoric strain: $\varepsilon_d = \frac{\sqrt{2}}{3} \sqrt{(\varepsilon'_I - \varepsilon'_H)^2 + (\varepsilon'_H - \varepsilon'_{III})^2 + (\varepsilon'_{III} - \varepsilon'_I)^2}$

In triaxial condition (principal stresses/strains)

$$\rho' = (\sigma_I + 2\sigma_{HH})/3 \quad q = \sigma_I - \sigma_{HH} \quad \varepsilon_v = \varepsilon_I + 2\varepsilon_{HH} \quad \varepsilon_d = 2(\varepsilon_I - \varepsilon_{HH})/3$$

The magnitudes of the components of the stress vector (i.e. $\sigma_{xx}, \sigma_{yy}, \sigma_{zz}, \tau_{xy}, \tau_{xz}, \tau_{yz}$) depend on the chosen direction of the coordinate axes. The principal stresses (σ_I, σ_H , and σ_{III}) however, always act on the same planes and have the same magnitude, no matter which direction is chosen for the coordinate axes. They are therefore invariant to the choice of axes. Consequently, the state of stress can be fully defined by either specifying the six component values for a fixed direction of the coordinate axis, or by specifying the magnitude of the principal stresses and the directions of the three planes on which these principal stresses act. In either case six independent pieces of information are required.

The missing stress invariant

- **General stresses:** σ_{ij} ($i, j = 1, 2, 3$) $\sigma_{11}, \sigma_{22}, \sigma_{33}, \sigma_{12}, \sigma_{13}, \sigma_{23}$

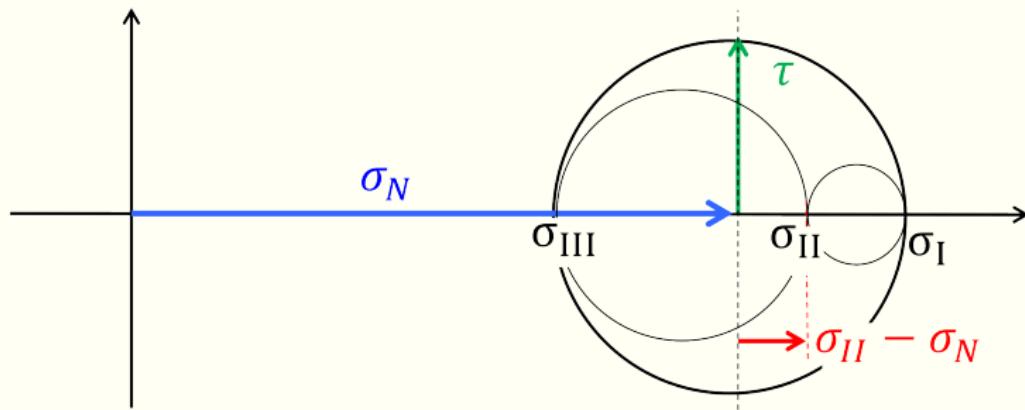


- **Principal stresses:** $\sigma_I, \sigma_{II}, \sigma_{III}$ + 3 angles



Two stress invariant (mean pressure p or s , deviator stress q or t). One more stress invariant is needed to go back to 3 principal stresses: **Lode angle!**

The Lode parameter



$$\text{Maximum shear stress: } \tau = \frac{\sigma_I - \sigma_{III}}{2}$$

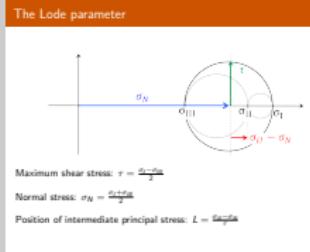
$$\text{Normal stress: } \sigma_N = \frac{\sigma_I + \sigma_{III}}{2}$$

$$\text{Position of intermediate principal stress: } L = \frac{\sigma_{II} - \sigma_N}{\tau}$$

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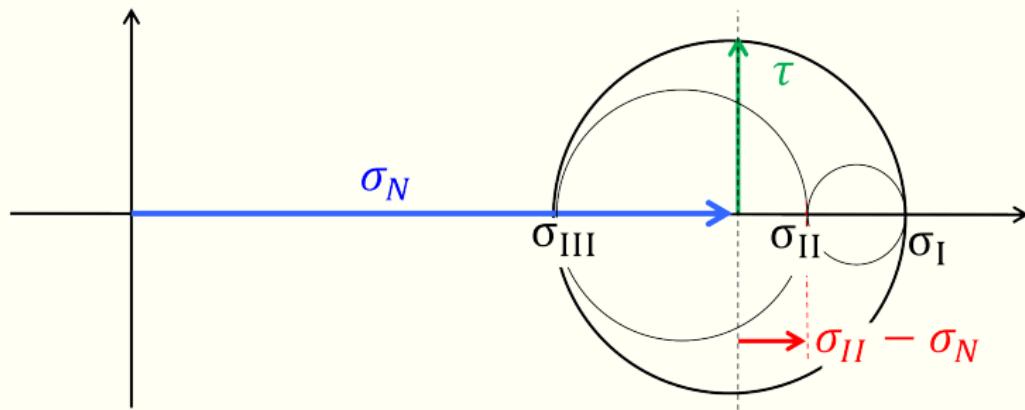
└ Stress invariants

└ The Lode parameter



If one is interested in the overall magnitude of the stress, all three principal stresses would be needed, but not the directions of the planes on which they act. In geotechnical engineering it is often convenient to work with alternative invariant quantities which are combinations of the principal effective stresses.

The Lode parameter



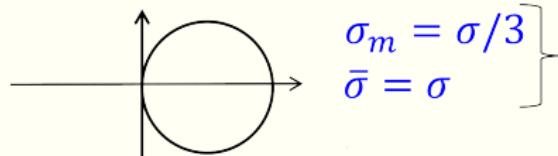
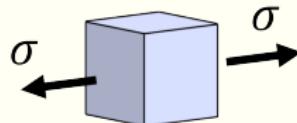
$$\text{Maximum shear stress: } \tau = \frac{\sigma_I - \sigma_{III}}{2}$$

$$\text{Normal stress: } \sigma_N = \frac{\sigma_I + \sigma_{III}}{2}$$

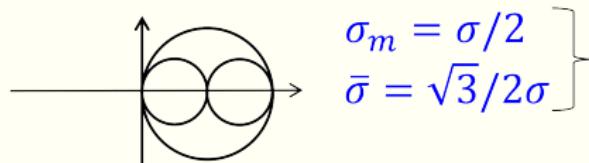
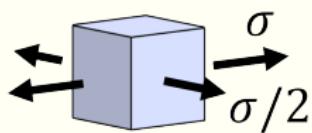
$$\text{Position of intermediate principal stress: } L = \frac{\sigma_{II} - \sigma_N}{\tau}$$

Which states have the same Lode parameter?

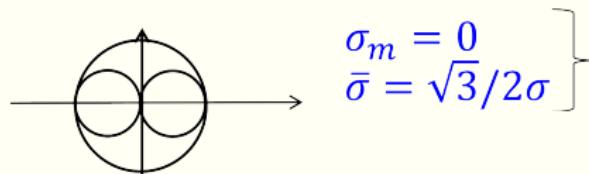
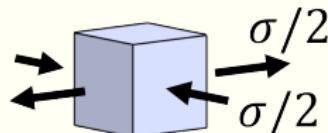
- Uniaxial tension



- Plane strain tension



- Pure shear

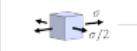


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└ Stress invariants

└ Which states have the same Lode parameter?

Which states have the same Lode parameter?

- Uniaxial tension:
 $\sigma_{eq} = \sigma/3$
 $\beta = \sigma$
- Plane shear tension:
 $\sigma_{eq} = \sqrt{3}/2\sigma$
 $\beta = \sqrt{3}/2\sigma$
- Pure shear:
 $\sigma_{eq} = 0$
 $\beta = \sqrt{3}/2\sigma$

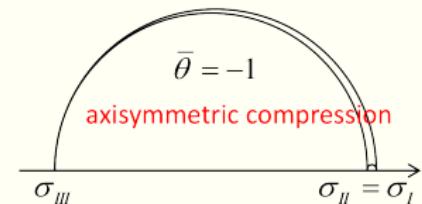
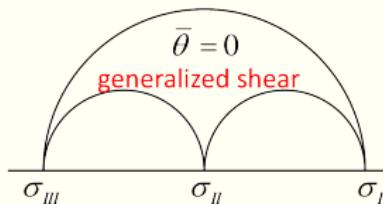
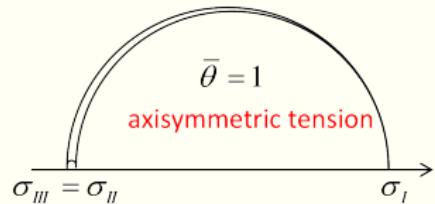
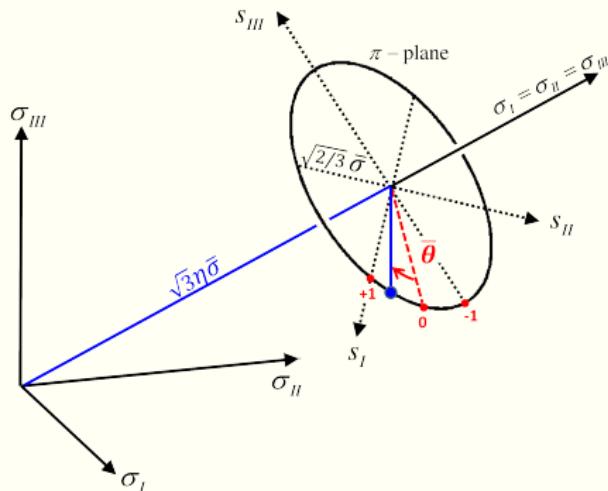
Smallest angle between the line of pure shear and the projection of the stress tensor on the deviatoric plane

Lode angle parameter

Lode angle parameter
(Lode, 1926)

$$L = \frac{2\sigma_{II} - \sigma_I - \sigma_{III}}{\sigma_I - \sigma_{III}}$$

Lode angle $\bar{\theta} \approx -L$



Overview

- 1 Stresses / strains in typical geotechnical lab tests
- 2 Stress paths and Pi-plane
- 3 Stress invariants
- 4 References

Stress invariant

Principal stresses (3 components) \Leftrightarrow Invariants (3 components)

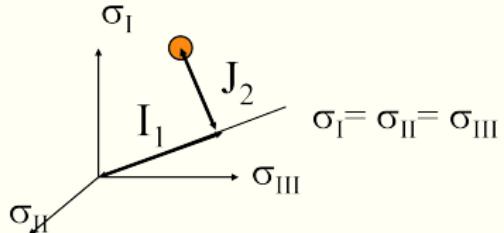
- $I_1 = p' = \sigma'_{ii}/3$

- $s_{ij} = \sigma'_{ij} - \delta_{ij}p'$

- $J_2 = \frac{1}{2}s_{ij}s_{ij}$

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- $q = \sqrt{3/2}(s_{ij}s_{ij})^{0.5} = \sqrt{3}J_2$



Lode angle:

- $\sin 3\theta = \frac{-3\sqrt{3}}{2} \frac{J_3}{J_2^{3/2}}$.

- $\theta = \tan^{-1} \left[\frac{1}{\sqrt{3}} \left(2 \frac{(\sigma_2 - \sigma_3)}{(\sigma_1 - \sigma_3)} - 1 \right) \right]$

- $TXC = \pi/6 \quad TXE = -\pi/6$

- $SHR = 0$, when

$$\sigma_2 = (\sigma_1 + \sigma_3)/2 \text{ (note } SHR \neq PS\text{)}$$