

# Chap8. Mixed and Hybrid Formulation.

## Hellinger - Reissner Variational Principle.

$$\left\{ \begin{array}{l} \text{Equilibrium eqn} \\ \text{constraint equation} \end{array} \right\} \quad \text{연립 방정식}$$

식 2개 (set) : assumed  
미지수 2개 (set)  $\left\{ \begin{array}{l} \text{assumed displacement field} \\ \text{assumed strain or stress field} \end{array} \right\}$

Boundary value problem 은 prw (principle of virtual work) 를 이용하여 다음과 같이 표시

$$\delta \Pi = \int_V \delta \underline{\underline{\epsilon}}^T \underline{\underline{\sigma}} \, dv - \int_V \delta \underline{\underline{u}}^T \underline{\underline{F}}_B \, dv - \int_{S_0} \delta \underline{\underline{u}}^T \underline{\underline{T}} \, ds = 0 \quad (1)$$

equilibrium equation  
 $\delta \Pi = \delta U - \delta W = 0 \quad \text{or} \quad \delta U = \delta W$

$$\left\{ \begin{array}{l} \text{virtual strain energy} \\ \text{or internal virtual work} \end{array} \right\} = \text{external virtual work}$$

여기서  $\underline{\underline{\sigma}} = \underline{\underline{C}} (\underline{\underline{\epsilon}}^{\text{total}} - \underline{\underline{\epsilon}}^0) = \underline{\underline{C}} \underline{\underline{\epsilon}}^{\text{mechanical}}$

↑  
constitutive  
matrix

↑  
total  
strain

↑  
initial  
strain

↑  
mechanical  
strain

$$\underline{\underline{\epsilon}} = \underline{\underline{B}} \underline{\underline{g}} = \text{displacement-dependent strain}$$



if 3-d strain state,

$$\underline{\underline{\bar{\epsilon}}} = [\bar{\epsilon}_{xx}, \bar{\epsilon}_{yy}, \bar{\epsilon}_{zz}, \bar{\epsilon}_{xy}, \bar{\epsilon}_{yz}, \bar{\epsilon}_{zx}]^T$$

$$\underline{\underline{B}} = \begin{bmatrix} \frac{\partial}{\partial x} & 0 & 0 \\ 0 & \frac{\partial}{\partial y} & 0 \\ 0 & 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial z} & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial z} & 0 & \frac{\partial}{\partial x} \end{bmatrix} = \text{matrix of differential operator}$$

$$\underline{\underline{d}} = \begin{Bmatrix} u \\ v \\ w \end{Bmatrix}$$

& Boundary Condition  $\begin{cases} u = \bar{u} \\ v = \bar{v} \\ w = \bar{w} \end{cases}$  on  $S_u$

stress-strain relation or H

$$\underline{\underline{C}}(\underline{\underline{\bar{\epsilon}}} - \underline{\underline{\epsilon}}^0) - \underline{\underline{\sigma}} = 0$$

$$\text{or } \underline{\underline{\bar{\epsilon}}} - \underline{\underline{\epsilon}}^0 - \underline{\underline{S}}\underline{\underline{\sigma}} = 0$$

$\underline{\underline{S}}$  = flexibility matrix  
or Compliance matrix

$$= \underline{\underline{C}}^{-1}$$

stress - strain relation 을 이용하여 다음 적분식 구성

$$\boxed{\delta I = \int_V \delta \underline{\underline{\sigma}}^T (\underline{\underline{\bar{\epsilon}}} - \underline{\underline{\epsilon}}^0 - \underline{\underline{S}} \underline{\underline{\sigma}}) dv = 0.} \quad (2)$$

constraint equation or compatibility equation

$\underline{\underline{\sigma}}$  = independent stress vector

$\delta \underline{\underline{\sigma}}$  = virtual independent stress vector

(Note) constraint equation 은 stress - strain relation 을

strain energy 의 integral sense 에서 만족

Therefore,  $\boxed{\delta \Pi_R = \delta \Pi + \delta I = 0}$

$$\delta \Pi_R = 0 \text{ 은 } \left\{ \begin{array}{l} \delta \Pi = 0 \quad (1) \text{식} \\ \delta I = 0 \quad (2) \text{식} \end{array} \right\} \text{ 을 의미한다.}$$

$\delta \Pi_R = 0$  은 2개의 연립방정식

$\rightarrow$  미지수 2개  $\underline{\underline{\sigma}}$ ,  $\underline{\underline{\bar{\epsilon}}}$  (or  $\underline{\underline{u}}$ )

$i^{\text{th}}$  element 에서

(a) Assumed displacement ( $\underline{\underline{u}}$ )

$$\underline{\underline{u}} = \underline{\underline{N}} \underline{\underline{q}}_i \quad \rightarrow \quad \underline{\underline{\bar{\epsilon}}} = \underline{\underline{B}} \underline{\underline{q}}_i$$

$$\delta \underline{\underline{u}} = \underline{\underline{N}} \delta \underline{\underline{q}}_i \quad \rightarrow \quad \delta \underline{\underline{\bar{\epsilon}}} = \underline{\underline{B}} \delta \underline{\underline{q}}_i$$



(b) Assumed independent stress

$$\underline{\sigma} = \underline{P} \underline{\beta}_i$$

$$\delta \underline{\sigma} = \underline{P} \delta \underline{\beta}_i$$

여기서  $\underline{P}$  = assumed stress polynomial matrix

$\underline{\beta}_i$  = assumed stress parameter

(Note) assumed independent stress  $\underline{\sigma}$  는

assumed displacement  $\underline{u}$  또는

displacement-dependent strain  $\underline{\epsilon}$  와는 무관한

변수의 독립 변수.

즉,  $\underline{\beta}_i$  는  $\underline{q}_i$  와 무관.

Then, 식 (17) 에서  $i$ th element 에 대해.

$$\begin{aligned} & \int_{V_i} \delta \underline{\epsilon}^T \underline{\sigma} \, dv - \int_{V_i} \delta \underline{u}^T \underline{F}_B \, dv - \int_{S_{\sigma i}} \delta \underline{u}^T \underline{I} \, ds \\ &= \int_{V_i} \delta \underline{q}_i^T \underline{B}^T \underline{P} \underline{\beta}_i \, dv - \int_{V_i} \delta \underline{q}_i^T \underline{N}^T \underline{F}_B \, dv \\ & \quad - \int_{S_{\sigma i}} \delta \underline{q}_i^T \underline{L}^T \underline{I} \, ds \end{aligned}$$

$$\begin{aligned}
&= \delta \underline{\beta}_i^T \left[ \underbrace{\int_{V_i} \underline{B}^T \underline{P} \, dv}_{\underline{G}^T} \right] \underline{\beta}_i \\
&\quad - \delta \underline{\beta}_i^T \left[ \underbrace{\int_{V_i} \underline{N}^T \underline{F}_B \, dv + \int_{S_{oi}} \underline{L}^T \underline{T} \, ds}_{\underline{Q}_i^a} \right] \\
&= \delta \underline{\beta}_i^T \left[ \underline{G}^T \underline{\beta}_i - \underline{Q}_i^a \right] \quad (3)
\end{aligned}$$

여기서  $\underline{L} = \underline{N}$  on  $S_{oi}$

식 (2)는  $i$ th element에서

$$\begin{aligned}
&\int_{V_i} \delta \underline{\epsilon}^T (\underline{\bar{\epsilon}} - \underline{\epsilon}^o - \underline{S} \underline{\sigma}) \, dv \\
&= \int_{V_i} \delta \underline{\beta}_i^T \underline{P}^T \underline{B} \underline{\beta}_i \, dv - \int_{V_i} \delta \underline{\beta}_i^T \underline{P}^T \underline{\epsilon}^o \, dv \\
&\quad - \int_{V_i} \delta \underline{\beta}_i^T \underline{P}^T \underline{S} \underline{P} \underline{\beta}_i \, dv \\
&= \delta \underline{\beta}_i^T \left[ \underbrace{\int_{V_i} \underline{P}^T \underline{B} \, dv}_{\underline{G}} \right] \underline{\beta}_i - \delta \underline{\beta}_i^T \left[ \underbrace{\int_{V_i} \underline{P}^T \underline{\epsilon}^o \, dv}_{\underline{Q}^o} \right] \\
&\quad - \delta \underline{\beta}_i^T \left[ \underbrace{\int_{V_i} \underline{P}^T \underline{S} \underline{P} \, dv}_{\underline{H}} \right] \underline{\beta}_i
\end{aligned}$$

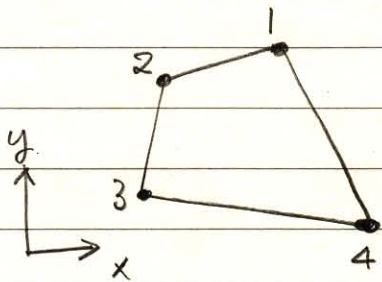


$$= \delta \beta_i^T (\underline{G} \underline{z}_i - \underline{G}_0 - \underline{H} \underline{\beta}_i) \quad (4)$$

여기서  $\underline{H} = \text{symmetric}$  ( $\because \underline{S} = \text{symmetric}$ )

## 8.1 Mixed Formulation I

4-node plane element 에서  $\beta_i$  가  $i$ th element 의 nodal stress vector 라면



$$\text{nodal dof} = 5\text{개} \left\{ \begin{matrix} \sigma_{xx}, \sigma_{yy}, \sigma_{xy} \\ u, v \end{matrix} \right\}$$

element 당 8-DOF 이어서 20-DOF 로 증가

assembly 와 boundary condition 적용.

$$\delta \underline{q}^T [(\underline{G}^*)^T \underline{\beta} - \underline{Q}^*] = 0$$

$$\delta \underline{\beta}^T [\underline{G}^* \underline{q} - \underline{Q}_0^* - \underline{H}^* \underline{\beta}] = 0$$

여기서  $\underline{G}^*$ ,  $\underline{H}^*$ ,  $\underline{Q}_0^*$ ,  $\underline{q}$ ,  $\underline{\beta}$  = global quantity

$\underline{q}$  = global displacement vector

$\underline{\beta}$  = element stress vector

임의의  $\delta \underline{q}$  와  $\delta \underline{\beta}$  에서

$$\underline{G}^{*T} \underline{\beta} - \underline{Q}^* = 0$$

$$\underline{G}^* \underline{q} - \underline{Q}_0^* - \underline{H}^* \underline{\beta} = 0$$



$$\text{or } \begin{bmatrix} \underline{H}^* & -\underline{Q}^* \\ -\underline{Q}^{*T} & \underline{0} \end{bmatrix} \begin{Bmatrix} \underline{\beta} \\ \underline{q} \end{Bmatrix} = \begin{Bmatrix} -\underline{Q}_0^* \\ -\underline{Q}^* \end{Bmatrix} \quad (5)$$

(Note) Mixed Formulation I is not practical

DOF 증가가 너무 크다.



## 8.2 Mixed Formulation II

If, stress가 element 사이에서 연속이라는 조건이

없으면, stress

parameter  $\beta_i$ 는  $i$ th element의 independent parameter 이다.

→  $\beta_i$ 를 global 에서 계산하지 않고, 각 element 에서 계산

→  $\beta_i$ 는 element level 에서 하기

식 (4)에서  $\delta \beta_i$ 는 임의 값이므로

$$\underline{G} \underline{z}_i - \underline{G}_0 - \underline{H} \beta_i = 0$$

$$\rightarrow \underline{\beta}_i = \underline{H}^{-1} \underline{G} \underline{z}_i - \underline{H}^{-1} \underline{G}_0 \quad (6)$$

식 (6)을 식 (3)에 대입

$$\delta \underline{z}_i^T [ \underline{G}^T (\underline{H}^{-1} \underline{G} \underline{z}_i - \underline{H}^{-1} \underline{G}_0) - \underline{Q}_i^a ]$$

$$= \delta \underline{z}_i^T [ \underbrace{\underline{G}^T \underline{H}^{-1} \underline{G}}_{\underline{K}_i} \underline{z}_i - \underbrace{(\underline{G}^T \underline{H}^{-1} \underline{G}_0 + \underline{Q}_i^a)}_{\underline{Q}_i} ]$$

$$= \delta \underline{z}_i^T ( \underline{K}_i \underline{z}_i - \underline{Q}_i )$$

여기서  $\underline{\underline{K}}_i = \underline{\underline{G}}^T \underline{\underline{H}}^T \underline{\underline{G}}$

$$= \left[ \int_{V_i} \underline{\underline{B}}^T \underline{\underline{P}} \, dv \right] \left[ \int_{V_i} \underline{\underline{P}}^T \underline{\underline{S}} \underline{\underline{P}} \, dv \right]^{-1} \left[ \int_{V_i} \underline{\underline{P}}^T \underline{\underline{B}} \, dv \right]$$

= element stiffness matrix

(Note) displacement formulation 이니

$$\underline{\underline{K}}_i = \int_{V_i} \underline{\underline{B}}^T \underline{\underline{C}} \underline{\underline{B}} \, dv$$

$$\underline{\underline{Q}}_i = \underline{\underline{G}}^T \underline{\underline{H}}^T \underline{\underline{G}}_0 + \underline{\underline{Q}}_i^a$$

$$= \left[ \int_{V_i} \underline{\underline{B}}^T \underline{\underline{P}} \, dv \right] \left[ \int_{V_i} \underline{\underline{P}}^T \underline{\underline{S}} \underline{\underline{P}} \, dv \right]^{-1} \left[ \int_{V_i} \underline{\underline{P}}^T \underline{\underline{\varepsilon}}^0 \, dv \right]$$

$$+ \left[ \int_{V_i} \underline{\underline{N}}^T \underline{\underline{F}}_B \, dv + \int_{S_{\partial i}} \underline{\underline{L}}^T \underline{\underline{T}} \, ds \right]$$

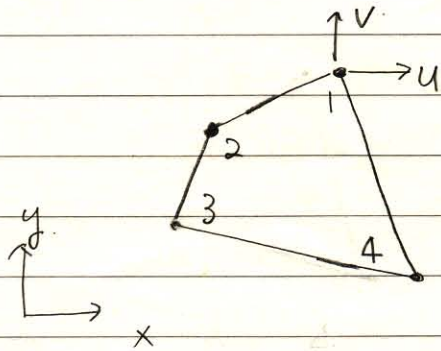
= element nodal load vector

(Note) displacement formulation 이니

$$\underline{\underline{Q}}_i = \int_{V_i} \underline{\underline{B}}^T \underline{\underline{C}} \underline{\underline{\varepsilon}}^0 \, dv + \int_{V_i} \underline{\underline{N}}^T \underline{\underline{F}}_B \, dv + \int_{S_{\partial i}} \underline{\underline{L}}^T \underline{\underline{T}} \, ds$$



(example) Four-node plane element 이거



4-node

8-dof

$$u = N_1 u_1 + N_2 u_2 + N_3 u_3 + N_4 u_4$$

$$v = N_1 v_1 + N_2 v_2 + N_3 v_3 + N_4 v_4$$

assumed stress 를 다음 같이 가정

$$\sigma_{xx} = \beta_1 + \beta_2 x + \beta_3 y$$

$$\sigma_{yy} = \beta_4 + \beta_5 x + \beta_6 y$$

$$\sigma_{xy} = \beta_7 + \beta_8 x + \beta_9 y$$

assumed displacement 이거  
제한한 stress 와 동일한  
polynomial

$$\underline{\sigma} = \begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} = \underbrace{\begin{bmatrix} 1 & x & y & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x & y & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & x & y \end{bmatrix}}_{\underset{||}{\underline{P}}} \underbrace{\begin{Bmatrix} \beta_1 \\ \beta_2 \\ \vdots \\ \beta_9 \end{Bmatrix}}_{\underset{||}{\underline{\beta}}}$$

$$= \underline{P} \underline{\beta}$$



Assembly of boundary condition 적용

$$\sum \epsilon^T (K \underline{u} - \underline{F}) = 0$$

$$\text{or } K \underline{u} = \underline{F}$$



## 8.3 Hybrid stress formulation

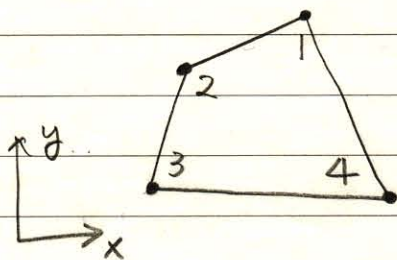
(or Assumed stress hybrid formulation)

각 element의 assumed stress를 선택할 때

stress equilibrium equation을 만족시키도록 선택한다.

(example) 4-node plane element

body force가 존재하지 않는 2-d prob.



set assumed stress

$$\sigma_{xx} = \beta_1 + \beta_4 x + \beta_5 y$$

$$\sigma_{yy} = \beta_2 + \beta_6 y + \beta_7 x$$

$$\sigma_{xy} = \beta_3 - \beta_4 y - \beta_6 x$$

Then, stress equilibrium equation

$$\left. \begin{aligned} \frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} &= \beta_4 - \beta_4 = 0 \\ \frac{\partial \sigma_{xy}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} &= -\beta_6 + \beta_6 = 0 \end{aligned} \right\} \text{만족}$$

$$9\beta - 2\beta = 7\beta \text{ 사용}$$

$$\underline{\sigma} = \begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} = \begin{bmatrix} 1 & 0 & 0 & x & y & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & y & x \\ 0 & 0 & 1 & -y & 0 & -x & 0 \end{bmatrix} \begin{Bmatrix} \beta_1 \\ \beta_2 \\ \vdots \\ \beta_7 \end{Bmatrix}$$

$$= \underline{P} \underline{\beta}$$



#### 8.4 Assumed strain Mixed Formulation.

Hellinger - Reissner principle or assumed stress 대신

assumed strain 사용.

H-R 그래프

$$\delta \Pi = \int_V \underline{\delta \bar{\epsilon}}^T \underline{\sigma} dv - \int_V \underline{\delta u}^T \underline{F_B} dv - \int_{S_F} \underline{\delta u}^T \underline{T} ds = 0 \quad (1)$$

$$\delta I = \int_V \delta \underline{\underline{\sigma}}^T (\underline{\underline{\epsilon}} - \underline{\underline{\epsilon}}^0 - \underline{\underline{\zeta}} \underline{\underline{\sigma}}) dv = 0 \quad (2)$$

여기서  $\underline{\underline{\epsilon}} = \underline{\underline{B}} \underline{\underline{q}}$  = displacement-dependent strain

$\underline{\hat{\sigma}} = \underline{P} \underline{\beta}$  = independent assumed stress

$$Q = Q_E \text{ or } Q_F$$

$$\delta \Pi = \int_V \underline{\delta \tilde{\epsilon}}^T \underline{C} \underline{\epsilon} dv - \int_V \underline{\delta u}^T \underline{F_B} dv - \int_{S_0} \underline{\delta u}^T \underline{T} ds = 0 \quad (3)$$

$$\delta I = \int_V \delta \underline{\underline{\varepsilon}}^T \underline{\underline{C}} (\underline{\underline{\varepsilon}} - \underline{\underline{\varepsilon}}^0 - \underline{\underline{\varepsilon}}) dV = 0 \quad (4)$$

여기서  $\underline{\underline{\varepsilon}} = \underline{B} \underline{u}$ ,  $\delta \underline{\underline{\varepsilon}} = \underline{B} \delta \underline{u}$

$$\underline{\underline{\varepsilon}} = \underline{\underline{P}} \underline{\underline{\alpha}}, \quad \delta \underline{\underline{\varepsilon}} = \underline{\underline{P}} \delta \underline{\underline{\alpha}}$$

$P$  = assumed strain polynomial matrix

$\alpha$  = assumed strain parameter







식 (7)을 식 (5)에 대입

$$\delta \underline{q}^T \left[ \underline{G}^T (\underline{H}^T \underline{G} \underline{q} - \underline{H}^T \underline{q}_0) - \underline{Q}^a \right]$$

$$= \delta \underline{q}^T \left[ \underbrace{\underline{G}^T \underline{H}^T \underline{G} \underline{q}}_{\text{"K"}} - \underbrace{(\underline{G}^T \underline{H}^T \underline{q}_0 + \underline{Q}^a)}_{\text{"Q"}} \right]$$

$$= \delta \underline{q}^T (\underline{K} \underline{q} - \underline{Q})$$

여기서  $\underline{K} = \underline{G}^T \underline{H}^T \underline{G}$

$$= \left[ \int_V \underline{B}^T \underline{C} \underline{P} dv \right] \left[ \int_V \underline{P}^T \underline{C} \underline{P} dv \right]^{-1} \left[ \int_V \underline{P}^T \underline{C} \underline{B} dv \right]$$

= element stiffness matrix with assumed strain

$$\underline{Q} = \underline{G}^T \underline{H}^T \underline{q}_0 + \underline{Q}^a$$

$$= \left[ \int_V \underline{B}^T \underline{C} \underline{P} dv \right] \left[ \int_V \underline{P}^T \underline{C} \underline{P} dv \right]^{-1} \left[ \int_V \underline{P}^T \underline{C} \underline{\varepsilon}^0 dv \right]$$

$$+ \left[ \int_V \underline{N}^T \underline{F}_B dv + \int_{S_{\sigma_i}} \underline{L}^T \underline{\bar{T}} ds \right]$$

= element nodal load vector with assumed strain

Assumed stress formulation	Assumed strain formulation
$\underline{\sigma} = \underline{P} \underline{\epsilon}$	$\underline{\epsilon} = \underline{P} \underline{\alpha}$
Kinematic mode 을 조정하기 힘들다.	Kinematic mode 조정 가능
stress equilibrium 을 만족시킬 수 있다.	stress equilibrium 을 만족시키기 힘들다.