

CHAP 4. 2-d Finite Element Formulation.

4.1 3-d Stress-strain relation.

등방성 재료에서. if $\epsilon^0 = 0$

$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{xy} \\ \sigma_{yz} \\ \sigma_{zx} \end{Bmatrix} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1 & \frac{\nu}{1-\nu} & \frac{\nu}{1-\nu} & 0 & 0 & 0 \\ & 1 & \frac{\nu}{1-\nu} & 0 & 0 & 0 \\ & & 1 & 0 & 0 & 0 \\ & & & \frac{1-2\nu}{2(1-\nu)} & 0 & 0 \\ & & & & \frac{1-2\nu}{2(1-\nu)} & 0 \\ & & & & & \frac{1-2\nu}{2(1-\nu)} \end{bmatrix} \begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{xy} \\ \epsilon_{yz} \\ \epsilon_{zx} \end{Bmatrix}$$

SYM

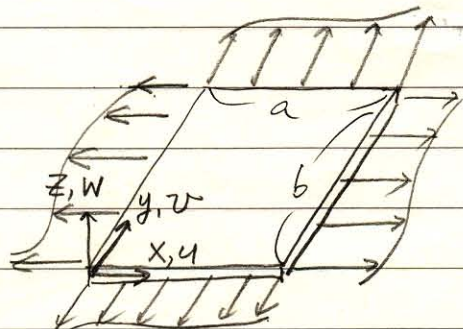
or

$$\begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{xy} \\ \epsilon_{yz} \\ \epsilon_{zx} \end{Bmatrix} = \frac{1}{E} \begin{bmatrix} 1 & -\nu & -\nu & 0 & 0 & 0 \\ & 1 & -\nu & 0 & 0 & 0 \\ & & 1 & 0 & 0 & 0 \\ & & & 2(1+\nu) & 0 & 0 \\ & & & & 2(1+\nu) & 0 \\ & & & & & 2(1+\nu) \end{bmatrix} \begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{xy} \\ \sigma_{yz} \\ \sigma_{zx} \end{Bmatrix}$$

SYM

$$G = \frac{E}{2(1+\nu)}$$

(a) plane stress assumption

두께 $t \ll a, b$

윗면, 아래면에서

$$\sigma_{xz}, \sigma_{yz}, \sigma_{zz} = 0$$

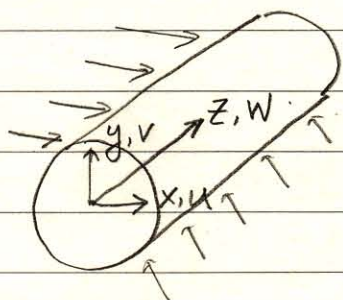
두께 방향으로

$$\sigma_{xz} = \sigma_{yz} = \sigma_{zz} \approx 0$$

Then.

$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{xy} \end{Bmatrix}$$

(b) plane strain assumption

 $x, y \ll z$

$$\frac{\partial u}{\partial z} = 0, \quad \frac{\partial v}{\partial z} = 0, \quad w = 0$$

$$\epsilon_{zz} = \frac{\partial w}{\partial z} = 0$$

$$\epsilon_{xz} = \frac{\partial w}{\partial x} + \frac{\partial u}{\partial z} = 0$$

$$\epsilon_{yz} = \frac{\partial w}{\partial y} + \frac{\partial v}{\partial z} = 0$$

4.2 2-D Finite Element Formulation

principle of virtual work $\delta \Pi$

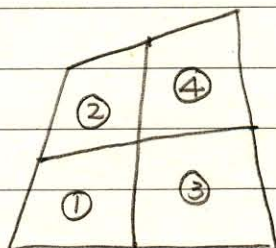
$$\delta \Pi = \int_V \delta \underline{\underline{\epsilon}}^T \underline{\underline{c}} \underline{\underline{\epsilon}} dv - \int_V \delta \underline{\underline{\epsilon}}^T \underline{\underline{c}} \underline{\underline{\epsilon}}^0 dv - \int_V \delta \underline{\underline{u}}^T \underline{\underline{F}}_B dv - \int_{S_\sigma} \delta \underline{\underline{u}}^T \underline{\underline{T}} ds = 0$$

and B.C on S_u

$$\underline{\underline{\epsilon}} = \begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{xy} \end{Bmatrix} = \begin{Bmatrix} \frac{\partial u}{\partial x} \\ \frac{\partial v}{\partial y} \\ \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \end{Bmatrix} \quad \underline{\underline{F}}_B = \begin{Bmatrix} X_B \\ Y_B \end{Bmatrix} \quad \underline{\underline{T}} = \begin{Bmatrix} \overline{T}_x \\ \overline{T}_y \end{Bmatrix}$$

$$\delta \underline{\underline{u}} = \begin{Bmatrix} \delta u \\ \delta v \end{Bmatrix}$$

FE discretization 을 위해서 여러 요소로 나눈다.

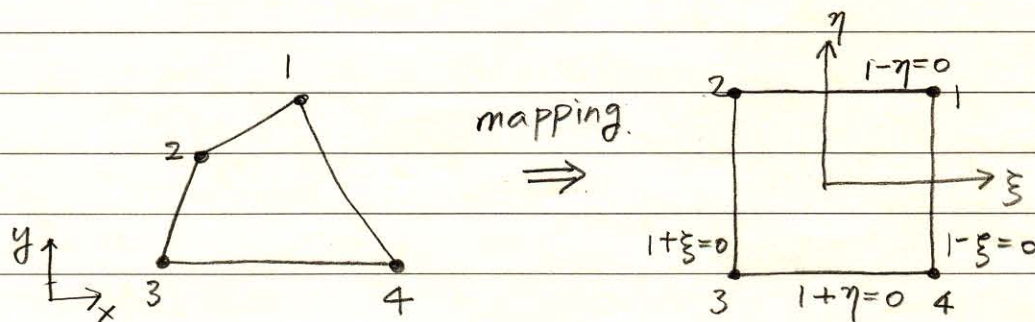


$$\delta \Pi = \sum_{i=1}^n \delta \Pi_i$$

$\delta \Pi_i$ = contribution of i -th element to $\delta \Pi$

n = total number of element

4.2.1. Four-node Quadrilateral element.



$$N_1 = \frac{1}{4} (1+\xi)(1+\eta) \quad : \text{bilinear polynomial}$$

then $N_1 = 1$ at node 1

$N_1 = 0$ at node 2, 3, 4

$$N_2 = \frac{1}{4} (1-\xi)(1+\eta)$$

$$N_3 = \frac{1}{4} (1-\xi)(1-\eta)$$

$$N_4 = \frac{1}{4} (1+\xi)(1-\eta)$$

Then, 4-node 요소에서

$$\left. \begin{aligned} x &= \sum_{i=1}^4 N_i x_i \\ y &= \sum_{i=1}^4 N_i y_i \end{aligned} \right\} \text{exact interpolation}$$

$$\left. \begin{aligned} \text{Assume } u &= \sum_{i=1}^4 N_i u_i \\ v &= \sum_{i=1}^4 N_i v_i \end{aligned} \right\} \begin{aligned} &u \text{ 와 } v \text{ 는 각각} \\ &\Rightarrow \text{isoparametric assumption} \end{aligned}$$

다시 쓰면

$$\begin{Bmatrix} u \\ v \end{Bmatrix} = \underbrace{\begin{bmatrix} N_1 & N_2 & N_3 & N_4 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & N_1 & N_2 & N_3 & N_4 \end{bmatrix}}_{\text{" } \underline{N} \text{ "}} \underbrace{\begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \\ v_1 \\ v_2 \\ v_3 \\ v_4 \end{Bmatrix}}_{\text{" } \underline{q}_i \text{ "}} = \underline{N} \underline{q}_i$$

$$\underline{\varepsilon} = \begin{Bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \varepsilon_{xy} \end{Bmatrix} = \begin{Bmatrix} \frac{\partial u}{\partial x} \\ \frac{\partial v}{\partial y} \\ \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \end{Bmatrix}$$

u, v 는 ξ, η 의 함수 ($N_i(\xi, \eta)$)

$\underline{\varepsilon}$ 을 구하려면 $\frac{\partial}{\partial x}, \frac{\partial}{\partial y}$ 와 $\frac{\partial}{\partial \xi}, \frac{\partial}{\partial \eta}$ 의 관계식 필요

chain rule 예시

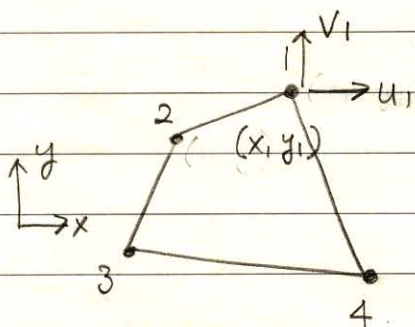
$$\frac{\partial}{\partial \xi} = \frac{\partial}{\partial x} \frac{\partial x}{\partial \xi} + \frac{\partial}{\partial y} \frac{\partial y}{\partial \xi}$$

$$\frac{\partial}{\partial \eta} = \frac{\partial}{\partial x} \frac{\partial x}{\partial \eta} + \frac{\partial}{\partial y} \frac{\partial y}{\partial \eta}$$

$$\text{or } \begin{Bmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{Bmatrix} = \underbrace{\begin{bmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \xi} \\ \frac{\partial x}{\partial \eta} & \frac{\partial y}{\partial \eta} \end{bmatrix}}_{\text{" } \underline{J} = \text{Jacobian} \text{ "}} \begin{Bmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{Bmatrix} = \underline{J} \begin{Bmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{Bmatrix}$$

$$\text{or } \begin{Bmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{Bmatrix} = \underline{J}^{-1} \begin{Bmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{Bmatrix} = \frac{1}{|J|} \begin{bmatrix} \frac{\partial y}{\partial \xi} & -\frac{\partial y}{\partial \eta} \\ -\frac{\partial x}{\partial \xi} & \frac{\partial x}{\partial \eta} \end{bmatrix} \begin{Bmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{Bmatrix}$$

$|J|$ = determinant of \underline{J}



$$\text{Since } x = \sum N_i x_i$$

$$y = \sum N_i y_i$$

$$\frac{\partial x}{\partial \xi} = \sum \frac{\partial N_i}{\partial \xi} x_i$$

$$\frac{\partial x}{\partial \eta} = \sum \frac{\partial N_i}{\partial \eta} x_i$$

$$\frac{\partial y}{\partial \xi} = \sum \frac{\partial N_i}{\partial \xi} y_i$$

$$\frac{\partial y}{\partial \eta} = \sum \frac{\partial N_i}{\partial \eta} y_i$$

$$\text{Since } \frac{\partial}{\partial x} = \frac{1}{|J|} \begin{bmatrix} \frac{\partial y}{\partial \xi} & -\frac{\partial y}{\partial \eta} \\ -\frac{\partial x}{\partial \xi} & \frac{\partial x}{\partial \eta} \end{bmatrix} \begin{Bmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{Bmatrix}$$

$$\frac{\partial}{\partial y} = \frac{1}{|J|} \begin{bmatrix} -\frac{\partial x}{\partial \xi} & \frac{\partial x}{\partial \eta} \\ \frac{\partial y}{\partial \xi} & -\frac{\partial y}{\partial \eta} \end{bmatrix} \begin{Bmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{Bmatrix}$$

$$\underline{J} = \begin{Bmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \xi} \\ \frac{\partial x}{\partial \eta} & \frac{\partial y}{\partial \eta} \end{Bmatrix}$$

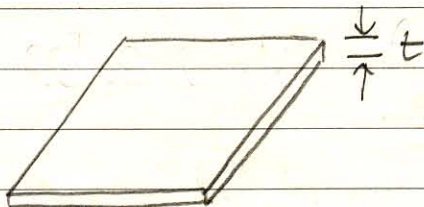
$$= \frac{1}{|J|} \underbrace{\begin{bmatrix} \frac{\partial y}{\partial \xi} & -\frac{\partial y}{\partial \zeta} & 0 & 0 \\ 0 & 0 & -\frac{\partial x}{\partial \xi} & \frac{\partial x}{\partial \zeta} \\ -\frac{\partial x}{\partial \eta} & \frac{\partial x}{\partial \zeta} & \frac{\partial y}{\partial \xi} & -\frac{\partial y}{\partial \zeta} \end{bmatrix}}_{\text{"D"}} \begin{Bmatrix} \frac{\partial u}{\partial \xi} \\ \frac{\partial u}{\partial \eta} \\ \frac{\partial v}{\partial \xi} \\ \frac{\partial v}{\partial \eta} \end{Bmatrix}$$

$$= \underbrace{\begin{bmatrix} \frac{\partial N_1}{\partial \xi} & \frac{\partial N_2}{\partial \xi} & \frac{\partial N_3}{\partial \xi} & \frac{\partial N_4}{\partial \xi} & 0 & 0 & 0 & 0 \\ \frac{\partial N_1}{\partial \eta} & \frac{\partial N_2}{\partial \eta} & \frac{\partial N_3}{\partial \eta} & \frac{\partial N_4}{\partial \eta} & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & \frac{\partial N_1}{\partial \xi} & \frac{\partial N_2}{\partial \xi} & \frac{\partial N_3}{\partial \xi} & \frac{\partial N_4}{\partial \xi} \\ 0 & 0 & 0 & 0 & \frac{\partial N_1}{\partial \eta} & \frac{\partial N_2}{\partial \eta} & \frac{\partial N_3}{\partial \eta} & \frac{\partial N_4}{\partial \eta} \end{bmatrix}}_{\text{"B"}} \underbrace{\begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \\ v_1 \\ v_2 \\ v_3 \\ v_4 \end{Bmatrix}}_{\text{"d_i"}}$$

$$\underline{\underline{\epsilon}} = \underline{\underline{B}} \underline{\underline{d}}$$

$$\delta \underline{\underline{\epsilon}} = \underline{\underline{B}} \delta \underline{\underline{d}}$$

얇은 평판에서



유한 요소 지배방정식을 적용하면

$$\int_{V_i} \delta \underline{\underline{\epsilon}}^T \underline{\underline{C}} \underline{\underline{\epsilon}} dV = \int_{A_i} \delta \underline{\underline{q}}_i^T \underline{\underline{B}}^T \underline{\underline{C}} \underline{\underline{B}} \underline{\underline{q}}_i t dA$$

$$= \delta \underline{\underline{q}}_i^T \left[\int_{A_i} \underline{\underline{B}}^T \underline{\underline{C}} \underline{\underline{B}} t dA \right] \underline{\underline{q}}_i$$

" $\underline{\underline{K}}^i$ = element stiffness matrix

$$= \delta \underline{\underline{q}}_i^T \underline{\underline{K}}^i \underline{\underline{q}}_i$$

$$\int_{V_i} \delta \underline{\underline{\epsilon}}^T \underline{\underline{C}} \underline{\underline{\epsilon}}^0 dV = \int_{A_i} \delta \underline{\underline{q}}_i^T \underline{\underline{B}}^T \underline{\underline{C}} \underline{\underline{\epsilon}}^0 t dA$$

$$= \delta \underline{\underline{q}}_i^T \int_{A_i} \underline{\underline{B}}^T \underline{\underline{C}} \underline{\underline{\epsilon}}^0 t dA$$

" initial strain에 의한 element load vector

initial strain

$$\int_{V_i} \delta \underline{\underline{u}}^T \underline{\underline{F}}_B dV = \int_{A_i} \delta \underline{\underline{q}}_i^T \underline{\underline{N}}^T \underline{\underline{F}}_B t dA$$

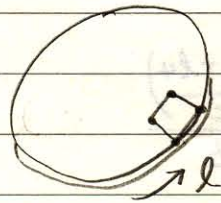
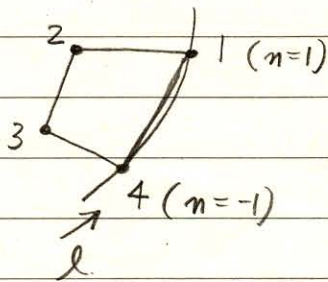
$$= \delta \underline{\underline{q}}_i^T \int \underline{\underline{N}}^T \underline{\underline{F}}_B t dA$$

body force에 의한 element load vector

Applied traction term

2-d, 얇은 평판에서

$$\int_{S_i} \underline{\delta u}^T \underline{\bar{I}} ds = \int_{l_i} \underline{\delta u}^T \underline{\bar{I}} t dl$$

 l = 경계선을 따르는 좌표계곡면 (1-4면)을 직선면으로 가정 \rightarrow geometric approximation

면 1-4를 따라서 변위는 linear

$$u = N_1 u_1 + N_4 u_4 = \frac{1}{2}(1+n)u_1 + \frac{1}{2}(1-n)u_4$$

$$v = N_1 v_1 + N_4 v_4 = \frac{1}{2}(1+n)v_1 + \frac{1}{2}(1-n)v_4$$

$$\begin{Bmatrix} u \\ v \end{Bmatrix} = \underbrace{\begin{bmatrix} \frac{1}{2}(1+n) & \frac{1}{2}(1-n) & 0 & 0 \\ 0 & 0 & \frac{1}{2}(1+n) & \frac{1}{2}(1-n) \end{bmatrix}}_{\text{shape function on 1-4면}} \begin{Bmatrix} u_1 \\ u_4 \\ v_1 \\ v_4 \end{Bmatrix}$$

$\underline{\underline{L}}$ = shape function on 1-4면

$$\begin{Bmatrix} \bar{T}_x \\ \bar{T}_y \end{Bmatrix} = \underline{L} \begin{Bmatrix} \bar{T}_{x1} \\ \bar{T}_{x4} \\ \bar{T}_{y1} \\ \bar{T}_{y4} \end{Bmatrix}$$

$$\int_{l_i} \underline{\delta u}^T \underline{\bar{T}} + dl = \int_{l_i} \underline{L \delta u \quad \delta v} \begin{Bmatrix} \bar{T}_x \\ \bar{T}_y \end{Bmatrix} + dl$$

$$= \underline{L \delta u_1 \quad \delta u_4 \quad \delta v_1 \quad \delta v_4} \left(\int_{-1}^1 \underline{L}^T \underline{L} + \frac{(l_1 - l_4)}{2} dn \right) \begin{Bmatrix} \bar{T}_{x1} \\ \bar{T}_{x4} \\ \bar{T}_{y1} \\ \bar{T}_{y4} \end{Bmatrix}$$

$$= \underline{L \delta u_1 \quad \delta u_4 \quad \delta v_1 \quad \delta v_4} \begin{Bmatrix} Q_{x1}^* \\ Q_{x4}^* \\ Q_{y1}^* \\ Q_{y4}^* \end{Bmatrix}$$

$$= \underline{L \delta u_1 \quad \delta u_2 \quad \delta u_3 \quad \delta u_4 \quad \delta v_1 \quad \delta v_2 \quad \delta v_3 \quad \delta v_4} \begin{Bmatrix} Q_{x1}^* \\ 0 \\ 0 \\ Q_{x4}^* \\ Q_{y1}^* \\ 0 \\ 0 \\ Q_{y4}^* \end{Bmatrix} = \underline{\delta q_i}^T \underline{Q_i}^*$$

↑
traction에 의한
nodal load
vector.

$$\delta \Pi_i = \underline{\delta q_i}^T \underline{K^i} \underline{q_i} - \underline{\delta q_i}^T \underline{Q_i} = \underline{\delta q_i}^T (\underline{K^i} \underline{q_i} - \underline{Q_i}) = 0$$

$$\underline{Q_i} = \text{nodal load vector} \left\{ \begin{array}{l} \text{initial strain} \\ \text{body force} \\ \text{traction} \end{array} \right\}$$