

BRAND : “ LISENG ”

PRODUCT : INSPECTION CHAMBER

DESCRIPTION : SANITARY INSTALLATION

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LISENG MATERIAL PTE LTD	Contract :		Job Ref.
Blk 549, Hougang Street 51 #02-198, Singapore 530549 HP:9850 1820	Part of Structure :	900x700x2500D 150TK 1H10-150	Calc. Sheet No. 1
	Drawing Ref.	Cal By: Alan Yap	Checked By: Date: 26/8/2019

Member Ref.	Calculations
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Material Design Details:

1. Density of Concrete = 25 kN/m³
2. Steel Reinforcements = 500 N/mm²
3. Load Combination (1.35 DL + 1.5 LL)
4. Concrete Cover = 40 mm

Weight of Precast Inspection Chamber & Weight of Water Within IC

Concrete Density = 25 kN/m ³	Water Density = 10 kN/m ³
Concrete Grade = 35 N/mm ²	
Size : Wi = 900 mm	We = 1200 mm
Bi = 700 mm	Be = 1000 mm
Hi = 2500 mm	He = 2650 mm
T = 150 mm	T = 150 mm
Volume of Walls = 1.43 m ³	
Vol of Base Slab = 0.18 m ³	
Total Vol = 1.61 m ³	
Total Weight of IC = 40.1 kN	Weight of Water within IC = 15.75 kN

Details of Steel Reinforcements

T bars fy = 500 N/mm ²
Dia of Com bars = 10 mm
Spacing = 150 mm
Asc = 549.78 mm ²
Dia of Ten bars = 10 mm
Spacing = 150 mm
Asc = 549.78 mm ²

Soil Properties

Soil density = 21 kN/m ³
Angle of int friction @ = 30 deg
ka = (1-sin@)/(1+sin@) = 0.332
Depth of Soil Layer = 3.65 m

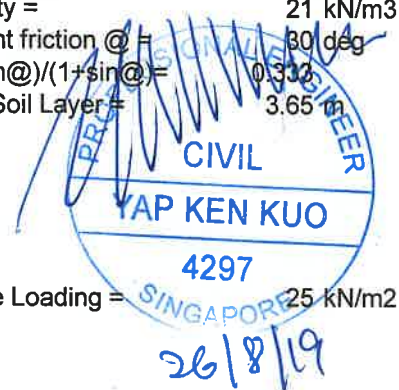
Water Properties

Water Density = 10 kN/m ³

Surcharge Loading = 25 kN/m²

Bearing Pressure on Soil

Pressure of the soil = (Weight of Water + Total Weight of IC) / Area = 47 kN/m²
 < 75 kN/m² }OK



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<p><u>Lateral External Pressure</u></p> <p>Water (p1) = 26.5 kN/m2 (triangle pressure) Surcharge (p2) = 8.33 kN/m2 (rectangular pressure) Soil pressure (p3) = 18.55 kN/m2 (triangle pressure)</p> <p><u>Forces from the elements</u></p> <p>Water (P1) = 35.11 kN Surcharge (P2) = 22.08 kN Soil pressure (P3) = 24.58 kN Total = 81.77 kN</p> <p><u>Max Moments (Analysis as a slab)</u></p> <p>ly/ly = 2.78 < 2 (1 way Slab)</p> <p>For 1m width slab, Ultimate Limit State = 80.08 kN/m2 (Considered Max Pressure)</p> <p>M = 8.10759 kNm</p> <p><u>Bending Reinforcements</u></p> <p>Mu = Klim x fck x bd2 = 0.168 x 35 x 1000 x 110^2 = 71.148 kNm } OK M/bd2fck = 0.01914 So z/d = 0.98 But adopt z/d = 0.95 Therefore, la = 0.95 d = 110 mm So z = 104.5 mm</p> <p>As req = M/0.87fykZ = 178.355 mm2</p> <p>As prov = 549.78 mm2 } OK, As prov > As req } H10-150 } > 0.13bh</p> <p><u>Distribution Bars</u></p> <p>b = 1000 mm d = 150 mm</p> <p>As min = 0.13xbd = 195 mm2</p> <p>As prov = 549.78 mm2 } OK, As prov > As req } H10-150</p> <p><u>Shear Check</u></p> <p>V = Force /2 = 36.80 kN Shear Stress v = 0.33 N/mm2 < 0.8√f_{cu} or 5N/mm2 100As/bd = 0.50 vc = 0.69 N/mm2 Since vc > v } No Shear Reinforcements Required</p>					




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<p>Deflection Check</p> <p>Span = 900 mm d = 104.5 mm Span / d = 8.61 (< Basic span/d =20) } Deflection OK</p> <p>Lifting Check</p> <table> <tr> <td>Dead Load IC =</td> <td>40.13 kN</td> <td>fcu =</td> <td>35 N/mm²</td> </tr> <tr> <td>Factor of Safety =</td> <td>2.00</td> <td>beta =</td> <td>0.4 deformed bars</td> </tr> <tr> <td>Ultimate Loading =</td> <td>80.25 kN</td> <td>f_{bu} =</td> <td>2.37 N/mm²</td> </tr> <tr> <td></td> <td></td> <td>f_y =</td> <td>500 N/mm²</td> </tr> <tr> <td></td> <td></td> <td>f_s =</td> <td>75.58 N/mm²</td> </tr> </table> <p>No of Lifting Hook = 4 pcs Ult Load per Hook = 20.06 kN</p> <p>Dia of Rebar hook = 13 mm Tensile Strength per hook = 46.46 kN } Tension Capacity OK</p> <p>Bond Length = 103.79 mm Embedded Length = 550 mm } Anchorage OK</p> <p>Provide 4 H 13 Lifting hook with embedded depth of 550mm</p> <p>Design of Base Slab</p> <table> <tr> <td>Dead Load of IC =</td> <td>40.1 kN</td> </tr> <tr> <td>Live Load of IC =</td> <td>25.0 kN</td> </tr> <tr> <td>Water from Below =</td> <td>26.5 kN/m²</td> </tr> <tr> <td>ULS 1 (1.35DL+1.5LL) =</td> <td>76.39 kN/m²</td> </tr> <tr> <td>ULS 2 (1.5WL) =</td> <td>39.75 kN/m²</td> </tr> <tr> <td>Therefore, ULS =</td> <td>76.39 kN/m²</td> </tr> </table> <p>l_y = 1200 mm l_x = 1000 mm l_y/l_x = 1.2 (Table 3.14)</p> <p>As_x = 0.084 As_y = 0.061</p> <p>Ms_x = As_xn_lx² = 6.42 kNm Ms_y = As_yn_ly² = 6.71 kNm</p> <p>Design Bending in the shorter span</p> <p>Ms_x/bd²f_{ck} = 0.015 So z/d = 0.985 But adopt z/d =0.95 Therefore, l_a = 0.95 z = 104.50 mm</p> <p>As req = Ms_x/0.95f_ykz = 129.27 mm²/m As pro = 549.78 mm²/m } OK, As pro > As req } H10-150 } > 0.13bh</p>						Dead Load IC =	40.13 kN	fcu =	35 N/mm ²	Factor of Safety =	2.00	beta =	0.4 deformed bars	Ultimate Loading =	80.25 kN	f _{bu} =	2.37 N/mm ²			f _y =	500 N/mm ²			f _s =	75.58 N/mm ²	Dead Load of IC =	40.1 kN	Live Load of IC =	25.0 kN	Water from Below =	26.5 kN/m ²	ULS 1 (1.35DL+1.5LL) =	76.39 kN/m ²	ULS 2 (1.5WL) =	39.75 kN/m ²	Therefore, ULS =	76.39 kN/m ²
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26/8/19

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		Drawing Ref.		Cal By: Alan Yap	Checked By:
				Date: 26/8/2019	
Member Ref.	Calculations				
	<p><u>Design Bending in the longer span</u></p> <p> $M_{sx}/bd^2f_{cu} = 0.016$ So $z/d = 0.983$ $d = 94.50$ mm Therefore, $z/d = 0.95$ $z = 89.78$ mm </p> <p> $A_{s\ req} = M_{sy}/0.95f_{yk}z$ = 157.36 mm²/m $A_{s\ pro} = 549.78$ mm²/m } OK, $A_{s\ pro} > A_{s\ req}$ } H10-150 } $> 0.13bh$ </p> <p><u>Shear Check</u></p> <p> $V = \text{Force} / 2 = 45.83$ kN Shear Stress $v = 0.42$ N/mm² $< 0.8\sqrt{f_{cu}}$ or 5N/mm² $100A_s/bd = 0.58$ $v_c = 0.76$ N/mm² Since $v_c > v$ } No Shear Reinforcements Required </p> <p><u>Deflection Check</u></p> <p> Span = 900 mm $d = 94.50$ mm Span / d = 9.52 ($< \text{Basic span}/d = 20$) } Deflection OK </p>				



26/8/19

LISENG MATERIAL PTE LTD

Contract :

Job Ref.

Blk 549, Hougang Street 51 #02-198, Singapore 530549
HP: 9850 1820

Part of Structure : 900x700x2500D 150TK 1H10-150

Calc. Sheet No. 5

Drawing Ref.

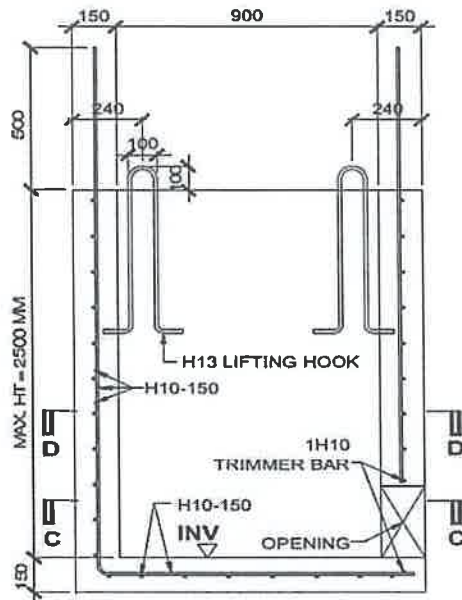
Cal By: Alan Yap

Check By:

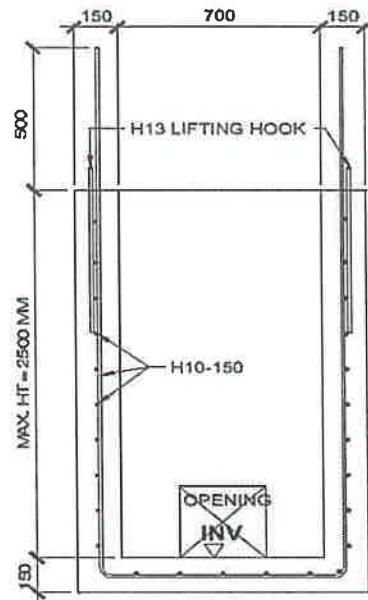
Date: 26/8/19

GENERAL NOTE

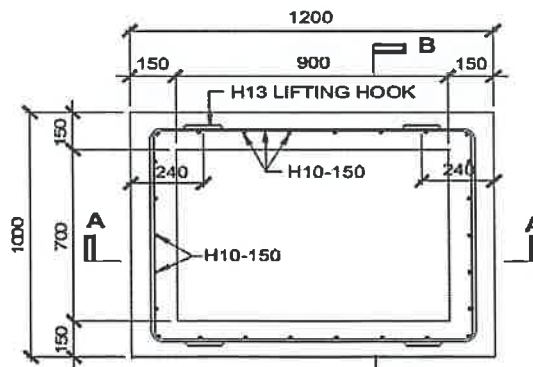
1. Grade of concrete = 35 N/mm²
2. Grade of steel
Mild Steel = 300 N/mm²
High Yield Steel = 500 N/mm²
Mesh Steel = 500 N/mm²
3. Concrete cover = 40 mm



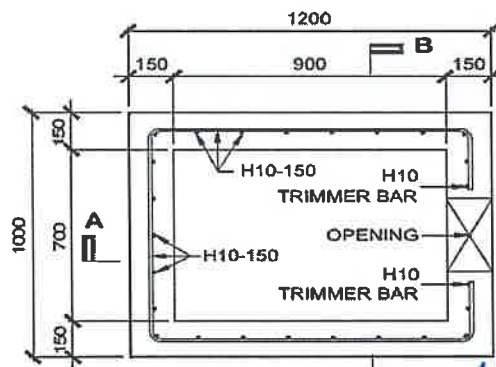
SECTION A-A



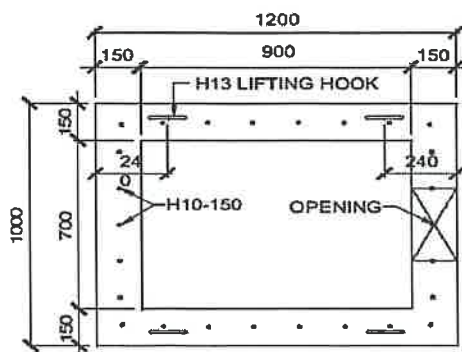
SECTION B-B



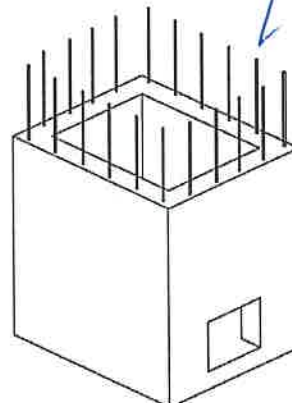
SECTION D-D



SECTION C-C



**PLAN OF PRECAST IC
900x700x150T**

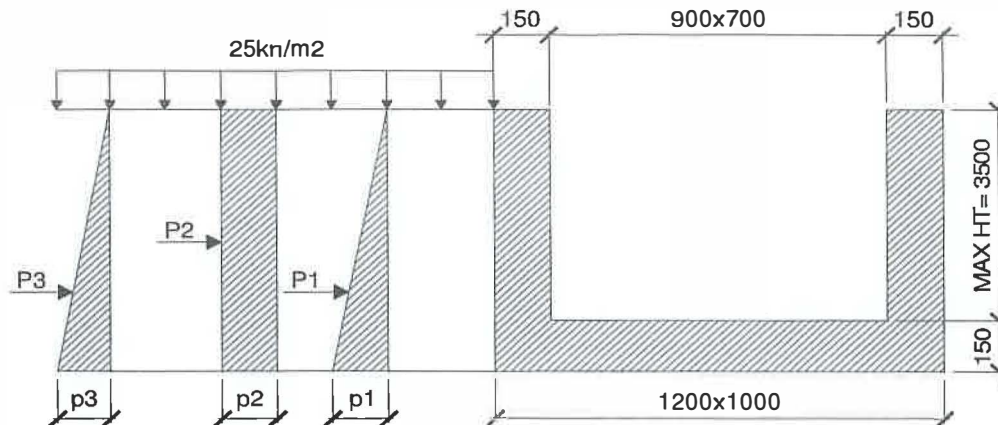


26/8/19

PROJECT	MEMBER	SHEET	1
PROPOSED PROJECT IN SINGAPORE	PRE-CAST INSPECTION CHAMBER 900 IW x 700 IB x 3500 IH x 150 T W/ 2H10-150	PREP	Achilles
		DATE	18/11/2020
		CHECK	

Eurocode Design Details

1. Density of Concrete: $\gamma = 25 \text{ kN/m}^3$
2. Steel Reinforcement: $f_y = 500 \text{ N/mm}^2$
3. Load combination (1.35 DL + 1.5 LL)

Pre-cast Inspection Chamber

Concrete density	γ	=	25 kN/m ³
Concrete grade	f_{cu}	=	35 N/mm ²
Size : Internal	WI	=	900 mm
	BI	=	700 mm
	HI	=	3500 mm
	T	=	150 mm
Volume	Vol	=	2.18 m ³
Weight	Wt	=	54.4 kN

External	W	=	1200 mm
	B	=	1000 mm
	H	=	3650 mm
	T	=	150 mm

STEEL REINFORCEMENT

H bar	f_y	=	500 N/mm ²
Compression steel :	\emptyset	=	10 mm
	Asc	=	78.54 mm ²
Tension steel :	\emptyset	=	10 mm
	Ast	=	78.54 mm ²

SOIL PROPERTIES

Soil density	γ_s	=	21 kN/m ³
Angle of int friction	ϕ	=	30 °
$k_a = (1 - \sin \phi) / (1 + \sin \phi)$		=	0.333
Depth of soil layer		=	3.65 m

WATER PROPERTIES

Water density	γ_w	=	10 kN/m ³
Water column		=	1 m

SURCHARGE

Live load	q	=	25 kN/m ²
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PROJECT	MEMBER	SHEET	2
PROPOSED PROJECT IN SINGAPORE	PRE-CAST INSPECTION CHAMBER 900 IW x 700 IB x 3500 IH x 150 T W/ 2H10-150	PREP	Achilles
		DATE	28/11/2020
		CHECK	

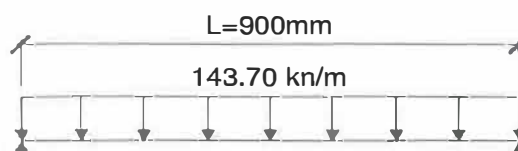
LATERAL PRESSURE (EXTERNAL)

1 WATER	$\gamma \cdot H$	=	36.50 kN/m ² (p1)	H = 3650 mm	(triangle pressure)
2 Surcharge	$k_a \cdot q$	=	8.33 kN/m ² (p2)		(rectangle pressure)
3 Soil	$k_a \cdot \gamma \cdot H$	=	25.55 kN/m ² (p3)		(triangle pressure)

Total
Lateral
press.
(kN/m)

P1	66.61
P2	30.42
P3	46.63

Total **143.7 kN/m**

**MOMENT CALCULATION****Method 1 :**

h	=	1050	K1	=	2.235	K6	=	7.235
l	=	850	K2	=	3.235	K7	=	9.471
hs	=	150	K3	=	4.235	K8	=	11.71
hw	=	150	K4	=	13.94			
k	=	1.2	K5	=	5.471			

SPAN (mm)	q (kN/m ²)	Moment	Ma (kN-m)	Mc (kN-m)
1050	143.7	Ma=Mc= -qh ² k/12K1	-7.29	-7.29
850	143.7	Ma=Mc= -ql ² k/12K1	-4.78	-4.78
Total moment			-12.1	-12.1

Method 2 :

Larger span	=	900 mm
Pressure	=	143.7 kN/m
Moment	=	14.5 kN-m
Factor of safety	=	1.50
Adopt moment	=	14.5 kN-m
Ultimate design moment	=	21.8 kN-m

DESIGN DATA

Thickness	=	150 mm
Concrete cover	=	40 mm
Size of tensile bar	=	10 mm
d	=	105 mm
b	=	1000 mm
0.95d	=	99.8 mm

k	=	0.057
z	=	97.93 mm
Adopt z	=	97.93 mm
Required As	=	512.2 mm ² /m
Actual rebar As	=	1047 mm ² /m
Provide 2 layers	=	(H 10 @ 150 c/c)

Provide	Bar Size =	10 mm
	Spacing =	150 mm
	Nos. of bar =	6.7 pcs
	use 2 layers =	13



28 NOV 2020

PROJECT	MEMBER	SHEET
PROPOSED PROJECT IN SINGAPORE	PRE-CAST INSPECTION CHAMBER 900 IW x 700 IB x 3500 IH x 150 T W/ 2H10-150	3
		PREP Achilles
		DATE 28/11/2020
		CHECK

SHEAR CHECK

F	=	143.7 kN/m ²	
Size of wall : b	=	1000 mm	
L	=	900 mm	
Factor of safety	=	1.50	
Ult F	=	193.9 kN	
V	=	96.97 kN	
v	=	0.924 N/mm ²	(not > 5N/mm ² or 0.8 √ fcu)
100As/bd	=	0.997	(not > 3)
400/d	=	3.81	(not < 1)
vc	=	0.882 N/mm ²	

Hence, no shear reinforcement is required

DEFLECTION CHECK

Span	=	900 mm	
d	=	105 mm	
Span/d	=	8.571 mm	(< Basic span/d= 20)

Deflection check satisfactory

LIFTING CHECK

Dead load	=	54.4 kN	fcu =	10 N/mm ²
Factor of safety	=	2	β =	0.4 deformed
Ultimate load	=	108.8 kN	fbu =	1.265 N/mm ²
			fy =	500 N/mm ²
Nos. of lifting hook	=	4 pcs		
Ultimate load per hook	=	27.19 kN		

Try	Size of bar	=	13 mm
	Tensile strength of bar	=	57.74 kN
	Bond length	=	526.3 mm
	Embedded length	=	550 mm

Provide 4 H13 lifting hook with min. embedded length of 550mm.

BASE SLAB**DL**

IC	=	54.4 kN	b	=	1000 mm
WATER (Full)	=	22.05 kN	d	=	105 mm

LL

Live load	=	30 kN	0.95d	=	99.75 mm
			k	=	0.086
			z	=	93.71 mm
			adopt z	=	93.71 mm

Gk	=	76.43 kN	required As	=	733.6 mm ² /m
Qk	=	30 kN			

Ultimate load (1.35DL+1.5LL) F = 148.2 kN

For span	L	=	900 mm
Ultimate moment M= (FL / 4)		=	33.34 kN-m

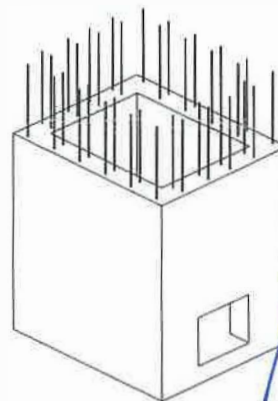
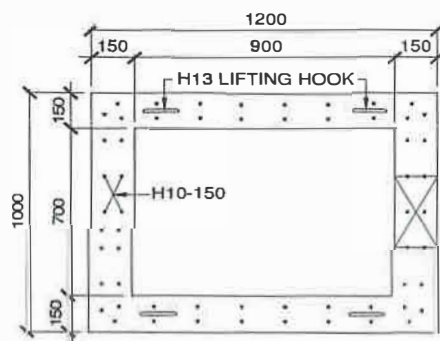
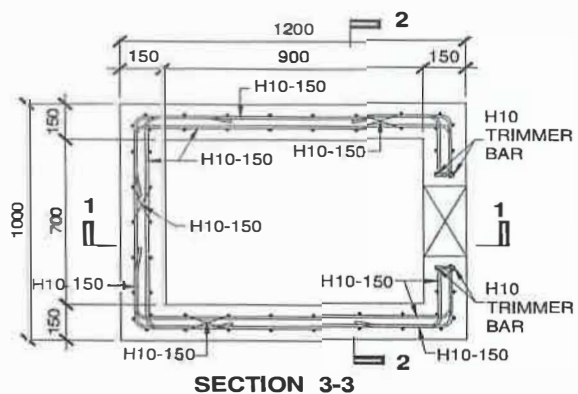
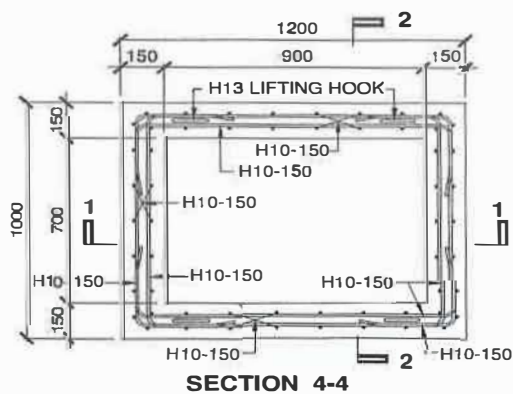
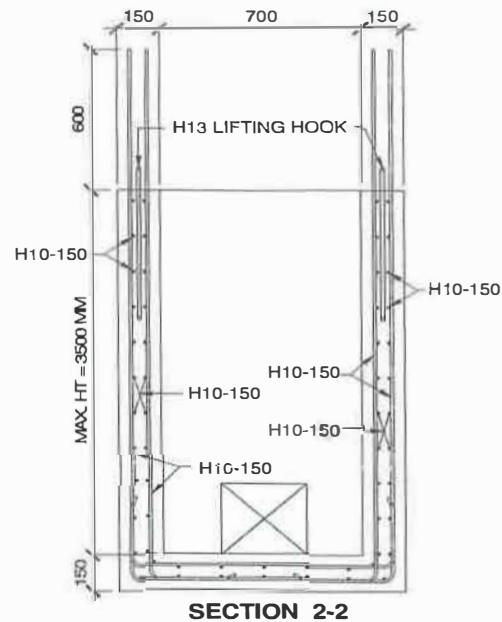
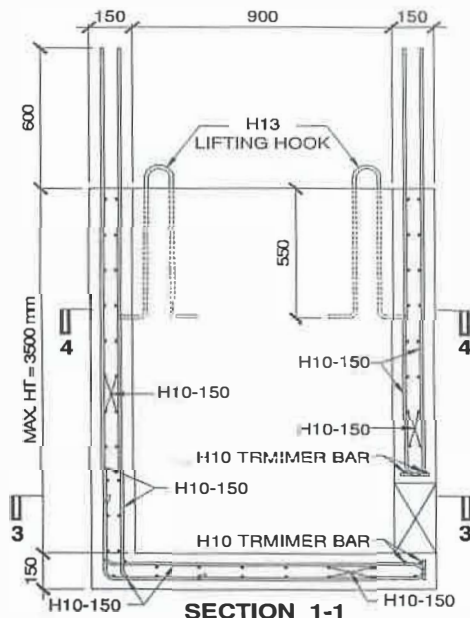
Actual Rebar As	=	1047 mm ² /m	>	733.6 mm ² /m
Provide 2 layers	=	(H 10 @ 150 c/c bothways)		



PROJECT	PROPOSED PROJECT IN SINGAPORE	MEMBER	PRE-CAST INSPECTION CHAMBER 900 IW x 700 IB x 3500 IH x 150 T W/ 2H10-150	SHEET	4
				PREP	Achilles
				DATE	28/11/2020
				CHECK	

GENERAL NOTE

1. Grade of concrete = 35 N/mm²
2. Grade of steel High Yield Steel = 500 N/mm²
3. Concrete cover = 40 mm



28 NOV 2020

LISENG MATERIAL PTE LTD

CO REG: 201632829M

Blk 549 Hougang ST 51 #02-198 Singapore 530549

Tel: 6362 1811 Fax: 6362 0895 H/P: 9850 1820 Email: sales@liseng.com.sg

PROJECT REFERENCE (HDB)

DATE: 05-04-2017

PC INSPECTION CHAMBER

COMPANY NAME

PROJECT

Chang Hua Constn PL	Kallang Whampoa RC21
Chiu Teng Constn PL	Bidadari C4
Chuan Lim Constn PL	Tech Whye Lane
Eng Chin Hang P&S PL	Clementi N4C10@Commwealth DR
Hi-tek Constn P L	Bt Batok N4C5 & 7
LC & T Builder (1971) PL	Sengkang N3C28A
Nanjing Dadi Constn (Group) Co Ltd	Sembawang N1C12
Progressive Builders PL	Dawson C5
Qingjian International (SP) Group	Bt Batok N4C18-C21
Rich Construction PL	Tampines N4C8
Sunhuan Constn PL	Clementi N4C10
Straits Constn PL	Bt Merah C50
Straits Constn PL	16 Yio Chu Kang EC
Tiong Seng Contractor PL	Woodlands N7C29
Teambuild E&C PL	Yishun N4C22@Yishun Ave 1
Teambuild E&C PL	Bt Batok N4C16&17@Bt Batok West Ave 3