

Discrete Damage

Andres, Seb, ...

a, \dots

Abstract

Keywords:

1. Variational formulation

A bar of length L and section S is stretched along the x axis. The material of the bar has nominal Young's modulus Y . We note $u(x)$ the horizontal displacement. The bar is clamped at its left end, and a displacement $\Delta \geq 0$ is imposed of the right end

$$u(0) = 0, \quad u(L) = \Delta \quad (1)$$

The total energy of the bar is

$$\mathcal{E}(\epsilon(x), \alpha(x)) = \int_0^L \frac{1}{2} Y S a(\alpha(x)) \epsilon^2(x) dx + \int_0^L W S w(\alpha(x)) dx \quad (2)$$

where the first integral is the strain energy and W is the dissipated energy (per unit volume) due to damage and ϵ is the longitudinal strain. The functions $a(\alpha)$ and $w(\alpha)$ are non-dimensionalized functions, to be discussed later on. As the boundary conditions are written with $u(x)$, we need to include the constraint

$$\epsilon = u'(x) \quad (3)$$

that is, work with the Lagrangian

$$\mathcal{L}(\epsilon, \alpha, u) = \mathcal{E}(\epsilon, \alpha) - \int_0^L \sigma(x) S (\epsilon - u') dx \quad (4)$$

where the continuous lagrange multiplier $\sigma(x)$ is identified with the axial stress in the bar.

2. Non-dimensionalisation

In this statics problem, we can freely (and with no loss of generality) choose a unit length and a unit force. We choose L as unit length, and YS as unit force. We introduce the 'hat' adim variables

$$\hat{x} = x/L, \quad \hat{u} = u/L, \quad \hat{\Delta} = \Delta/L, \quad \hat{W} = W/Y, \quad \hat{\sigma} = \sigma/Y, \quad \hat{\mathcal{E}} = \frac{\mathcal{E}}{YSL}, \quad \hat{\mathcal{L}} = \frac{\mathcal{L}}{YSL} \quad (5)$$

and simplify (4) to

$$\hat{\mathcal{E}}(\epsilon, \alpha) = \int_0^1 \frac{1}{2} a(\alpha) \epsilon^2(\hat{x}) d\hat{x} + \int_0^1 \hat{W} w(\alpha) d\hat{x} \quad (6a)$$

$$\hat{\mathcal{L}}(\epsilon, \alpha, \hat{u}) = \hat{\mathcal{E}}(\epsilon, \alpha) - \int_0^1 \hat{\sigma}(\hat{x}) (\epsilon - u'(\hat{x})) d\hat{x} \quad (6b)$$

And from now on, we drop the 'hats' while keeping in mind that we deal with adim variables.

3. First variation

We seek extremal configurations of $\mathcal{E}(\epsilon, \alpha)$, under the constraints (3) and (1). Applying the lagrange multiplier rule, we work with \mathcal{L} and look for the conditions under which the first variation of \mathcal{L} vanishes, for the two variables ϵ and \bar{u} . The minimisation with regard to the third variable α , involving irreversibility conditions, will be considered in a second step. We introduce the variations $\bar{\epsilon}(x)$, $\bar{u}(x)$. The boundary conditions (1) imply that

$$\bar{u}(0) = 0 \text{ and } \bar{u}(1) = 0 \quad (7)$$

$$\left. \frac{\mathcal{L}(\epsilon + \eta \bar{\epsilon}, \alpha, u + \eta \bar{u}) - \mathcal{L}(\epsilon, \alpha, u)}{\eta} \right|_{\eta \rightarrow 0} = \int_0^1 a(\alpha) \epsilon \bar{\epsilon} dx - \int_0^1 \sigma(x) (\bar{\epsilon} - \bar{u}') dx = 0 \quad \forall \bar{\epsilon}, \bar{u} \quad (8a)$$

$$= \int_0^1 (a(\alpha) \epsilon - \sigma(x)) \bar{\epsilon} - \sigma'(x) \bar{u} dx = 0 \quad \forall \bar{\epsilon}, \bar{u} \quad (8b)$$

The boundary term involved in the integration by part on $\bar{u}'(x)$ identically vanishes because of (7). Condition (8b) consequently yield

$$\sigma'(x) = 0 \quad (9a)$$

$$\sigma = a(\alpha(x)) \epsilon(x) \quad (9b)$$

We find that the axial stress in the beam σ is uniform. As the damage field $\alpha(x)$ might not be uniform, the longitudinal strain $\epsilon(x)$ still generically depend on x . We now seek to minimise \mathcal{E} using the equilibrium conditions we have just found. First we discard the variable $u(x)$ and replace the imposed displacement condition (1) with

$$u(1) - u(0) = \int_0^1 u'(x) dx = \int_0^1 \epsilon(x) dx = \Delta \quad (10)$$

Consequently we now work with the Lagrangian

$$\mathcal{L}(\epsilon(x), \alpha(x)) = \int_0^1 \frac{1}{2} a(\alpha) \epsilon^2 dx + \int_0^1 W w(\alpha) dx - \sigma \int_0^1 \epsilon(x) dx \quad (11)$$

where the lagrange multiplier associated the the displacement condition (10) is directly identified with σ , the axial stress in the bar which is also the applied external tension. Extremizing with regard to $\epsilon(x)$ leads to (9b) which we use to rewrite (10) as

$$\sigma = \frac{\Delta}{\int_0^1 a^{-1}(\alpha) dx} \quad (12)$$

which enable us to rewrite the strain energy as

$$\int_0^1 \frac{1}{2} a(\alpha) \epsilon^2 dx = \int_0^1 \frac{1}{2} \frac{\sigma^2}{a(\alpha)} dx = \frac{1}{2} \frac{\Delta^2}{\int_0^1 a^{-1}(\alpha) dx} \quad (13)$$

We finally obtain an energy which only depends on $\alpha(x)$

$$\mathcal{E}(\alpha(x)) = \frac{1}{2} \frac{\Delta^2}{\int_0^1 a^{-1}(\alpha) dx} + \int_0^1 W w(\alpha) dx \quad (14)$$

During the loading process, $\Delta = \Delta(t)$, the field $\alpha(x, t)$ cannot decrease. A necessary condition is that, at all time

$$\forall x : \quad \dot{\alpha}(x, t) \geq 0 \text{ and } \mu(x) \geq 0 \text{ and } \mu(x) \dot{\alpha}(x, t) = 0 \quad (15a)$$

$$\text{with } \left. \frac{\mathcal{E}(\alpha + \eta \bar{\alpha}) - \mathcal{E}(\alpha)}{\eta} \right|_{\eta \rightarrow 0} = \int_0^1 \mu(x) \bar{\alpha} dx \quad \forall \bar{\alpha} \quad (15b)$$

$$\text{hence } \mu(x) = W w'(\alpha) + \frac{1}{2} \frac{a'(\alpha)}{a^2} \frac{\Delta^2}{\left(\int_0^1 a^{-1}(\alpha) dx \right)^2} \quad (15c)$$

And a sufficient condition is

$$\forall \bar{\alpha} \geq 0 : \int_0^1 W w''(\alpha) \bar{\alpha}^2 dx + \frac{1}{2} \frac{\Delta^2}{\left(\int_0^1 a^{-1}(\alpha) dx\right)^2} \int_0^1 \left(\frac{a''(\alpha)}{a^2} - 2 \frac{a'(\alpha)^2}{a^3} \right) \bar{\alpha}^2 dx + \Delta^2 \frac{\left(\int_0^1 \frac{a'(\alpha)}{a^2} \bar{\alpha} dx\right)^2}{\left(\int_0^1 a^{-1}(\alpha) dx\right)^3} > 0 \quad (16)$$

or, setting $s(\alpha) = 1/a(\alpha)$

$$\forall \bar{\alpha} \geq 0 : \int_0^1 W w''(\alpha) \bar{\alpha}^2 dx - \frac{1}{2} \frac{\Delta^2}{\left(\int_0^1 s(\alpha) dx\right)^2} \int_0^1 s''(\alpha) \bar{\alpha}^2 dx + \Delta^2 \frac{\left(\int_0^1 s'(\alpha) \bar{\alpha} dx\right)^2}{\left(\int_0^1 s(\alpha) dx\right)^3} > 0 \quad (17)$$

4. Discretisation

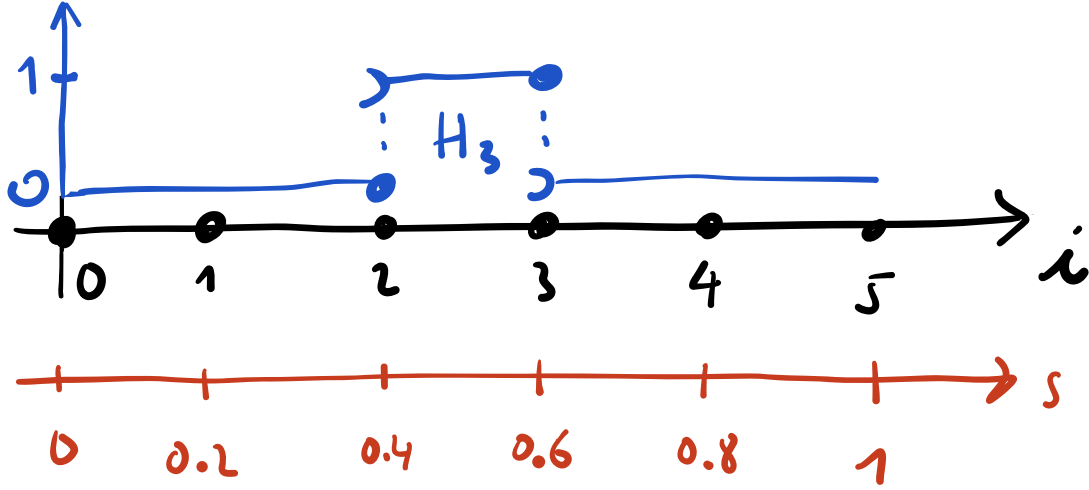


Figure 1: —.

We introduce N segments of equal size $h = 1/N$, with $s_i = ih$ and $i \in (1; N)$, and use the following base functions, see Figure 1

$$H_i(x) = 1 \text{ if } x_{i-1} \leq x \leq x_i \quad (18a)$$

$$H_i(x) = 0 \text{ otherwise} \quad (18b)$$

The variation $\bar{\alpha}(x)$ and $\alpha(x)$ are represented as

$$\bar{\alpha}(s) = \sum_{i=1}^N \bar{\alpha}_i H_i(s) \quad (19a)$$

$$\alpha(s) = \sum_{i=1}^N \alpha_i H_i(s) \quad (19b)$$

We note $S = \int_0^1 s(\alpha) dx$. The second variation in (17) can then be written in matrix form as

$$\text{for } i \neq j : H_{ij} = \Delta^2 h^2 \frac{s'(\alpha_i) s'(\alpha_j)}{S^3} \quad (20a)$$

$$H_{ii} = h W w''(\alpha_i) - \frac{\Delta^2}{2} h \frac{s''(\alpha_i)}{S^2} + \Delta^2 h^2 \frac{s'(\alpha_i)^2}{S^3} \quad (20b)$$

And the condition (17) can be written as

$$\forall \bar{\alpha}_i > 0, \forall \bar{\alpha}_j > 0 : \sum_{i,j} H_{ij} \bar{\alpha}_i \bar{\alpha}_j > 0 \quad (21)$$

this condition is fulfilled if (theorem NB, mais andres dit que c'est Frobenius-Peron...)

$$\forall i, j : H_{ij} > 0 \quad (22)$$

en fait c'est faux, c'est plutot

$$\forall i : 0 < H_{ii} \quad (23a)$$

$$\forall i \neq j : -\sqrt{H_{ii}H_{jj}} < H_{ij} \quad (23b)$$