

Summer 2024

MATH 4650-E01

CSCI 4650-E01

MATH 5660-E01 (additional exercises will be assigned by email for 5660 students)

Homework 7

Due on Monday July 22nd 2024

This is your homework 7. It is inspired from Reality Check #2 in Sauer. First there is an introduction and then there are questions.

To get you started, I copied all the questions in this notebook:

https://colab.research.google.com/drive/16mdKti0JnNOemxNOFGB1l_JzvDf10MeW

The Euler-Bernoulli beam is a fundamental model for a material bending under stress. Discretization converts the differential equation model into a system of linear equations. The smaller the discretization size, the larger is the resulting system of equations. This example will provide us an interesting case study of the roles of system size and ill-conditioning in scientific computation.

The vertical displacement of the beam is represented by a function $y(x)$, where $0 \leq x \leq L$ along the beam of length L . We will use MKS units in the calculation: meters, kilograms, seconds. The displacement $y(x)$ satisfies the Euler-Bernoulli equation

$$E I y'''' = f(x) \quad (2.27)$$

where E , the Young's modulus of the material, and I , the area moment of inertia, are constant along the beam. The right-hand-side $f(x)$ is the applied load, including the weight of the beam, in force per unit length.

Techniques for discretizing derivatives are found in Chapter 5, where it will be shown that a reasonable approximation for the fourth derivative is

$$y'''' = \frac{1}{h^4} (y(x-2h) - 4y(x-h) + 6y(x) - 4y(x+h) + y(x+2h)) \quad (2.28)$$

for a small increment h . The discretization error of this approximation is proportional to h^2 (see Exercise 5.1.21). Our strategy will be to consider the beam as the union of many segments of length h , and to apply the discretized version of the differential equation on each segment.

For a positive integer n , set $h = L/n$. Consider the evenly spaced grid $0 = x_0 < x_1 < \dots < x_n = L$, where $h = x_i - x_{i-1}$ for $i = 1, \dots, n$. Replacing the differential equation (2.27) with the difference approximation (2.28) to get the system of linear equations for the displacements $y_i = y(x_i)$ yields

$$y_{i-2} - 4y_{i-1} + 6y_i - 4y_{i+1} + y_{i+2} = \frac{h^4}{E I} f(x_i). \quad (2.29)$$

We will develop n equations in the n unknowns $y_1 < \dots < y_n$. The coefficient matrix, or structure matrix, will have coefficients from the left-hand side of this equation. However, notice that we must alter the equations near the ends of the beam to take the boundary conditions into account.

A diving board is a beam with one end clamped at the support, and the opposite end free. This is called the ****clamped-free**** beam or sometimes the ****cantilever**** beam. The boundary conditions for the clamped (left) end and free (right) end are

$$y(0) = y'(0) = y''(L) = y'''(L) = 0.$$

In particular, $y(0) = 0$. Note that finding y_1 , however, presents us with a problem, since applying the approximation (2.29) to the differential equation (2.27) at x_1 results in

$$y_{-1} - 4y_0 + 6y_1 - 4y_2 + y_3 = \frac{h^4}{EI} f(x_1), \quad (2.30)$$

and y_{-1} is not defined. Instead, we must use an alternate derivative approximation at the point x_1 near the clamped end. Exercise 5.1.22(a) derives the approximation

$$y''''(x_1) = \frac{1}{h^4} \left(16y(x_1) - 9y(x_1 + h) + \frac{8}{3}y(x_1 + 2h) - \frac{1}{4}y(x_1 + 3h) \right) \quad (2.31)$$

which is valid when $y(0) = y'(0) = 0$.

Calling the approximation "valid," for now, means that the discretization error of the approximation is proportional to h^2 , the same as for equation (2.28). In theory, this means that the error in approximating the derivative in this way will decrease toward zero in the limit of small h . This concept will be the focal point of the discussion of numerical differentiation in Chapter 5. The result for us is that we can use approximation (2.31) to take the endpoint condition into account for $i = 1$, yielding

$$16y_1 - 9y_2 + \frac{8}{3}y_3 - \frac{1}{4}y_4 = \frac{h^4}{EI} f(x_1).$$

The free right end of the beam requires a little more work because we must compute y_i all the way to the end of the beam. Again, we need alternative derivative approximations at the last two points x_{n-1} and x_n . Exercise 5.1.22 gives the approximations

$$y''''(x_{n-1}) = \frac{1}{17h^4} (-28y_n + 72y_{n-1} - 60y_{n-2} + 16y_{n-3}) \quad (2.32)$$

$$y''''(x_n) = \frac{1}{17h^4} (72y_n - 156y_{n-1} + 96y_{n-2} - 12y_{n-3}) \quad (2.33)$$

which are valid under the assumption $y''(x_n) = y'''(x_n) = 0$. Now we can write down the system of n equations in n unknowns for the diving board. This matrix equation summarizes our approximate versions of the original differential equation (2.27) at each point x_1, \dots, x_n , accurate within terms of order h^2 :

$$\begin{bmatrix} 16 & -9 & \frac{8}{3} & -\frac{1}{4} & & & & \\ -4 & 6 & -4 & 1 & & & & \\ 1 & -4 & 6 & -4 & 1 & & & \\ & 1 & -4 & 6 & -4 & 1 & & \\ & & \ddots & \ddots & \ddots & \ddots & \ddots & \\ & & & 1 & -4 & 6 & -4 & 1 \\ & & & & 1 & -4 & 6 & -4 \\ & & & & & \frac{16}{17} & -\frac{60}{17} & \frac{72}{17} & -\frac{28}{17} \\ & & & & & -\frac{12}{17} & \frac{96}{17} & -\frac{156}{17} & \frac{72}{17} \end{bmatrix} \begin{bmatrix} y_1 \\ y_2 \\ y_3 \\ y_4 \\ \vdots \\ y_{n-3} \\ y_{n-2} \\ y_{n-1} \\ y_n \end{bmatrix} = \begin{bmatrix} f(x_1) \\ f(x_2) \\ f(x_3) \\ f(x_4) \\ \vdots \\ f(x_{n-3}) \\ f(x_{n-2}) \\ f(x_{n-1}) \\ f(x_n) \end{bmatrix} \quad (2.34)$$

The structure matrix A in (2.34) is a banded matrix, meaning that all entries sufficiently far from the main diagonal are zero. Specifically, the matrix entries $a_{ij} = 0$, except for $|i - j| \leq 3$. The bandwidth of this banded matrix is 7, since $i - j$ takes on 7 values for nonzero a_{ij} .

Finally, we are ready to model the clamped-free beam. Let us consider a solid wood diving board composed of Douglas fir. Assume that the diving board is $L = 2$ meters long, 30 cm wide, and 3 cm thick. The density of Douglas fir is approximately 480 kg/m³. One Newton of force is 1 kg-m/sec², and the Young's modulus of this wood is approximately $E = 1.3 \times 10^{10}$ Pascals, or Newton/m². The area moment of inertia I around the center of mass of a beam is $w d^3/12$, where w is the width and d the thickness of the beam.

You will begin by calculating the displacement of the beam with no payload, so that $f(x)$ represents only the weight of the beam itself, in units of force per meter. Therefore $f(x)$ is the mass per meter 480wd times the downward acceleration of gravity $-g = -9.81\text{m/sec}^2$, or the constant $f(x) = f = -480\text{wd}g$. The reader should check that the units match on both sides of Equation (2.27). There is a closed-form solution of Equation (2.27) in the case f is constant, so that the result of your computation can be checked for accuracy.

Following the check of your code for the unloaded beam, you will model two further cases. In the first, a sinusoidal load (or "pile") will be added to the beam. In this case, there is again a known closed-form solution, but the derivative approximations are not exact, so you will be able to monitor the error of your modeling as a function of the grid size h , and see the effect of conditioning problems for large n . Later, you will put a diver on the beam.

Exercises:

1. **Exercise EX.5.1.21. Handwritten or typed.** Prove the second-order formula for the third derivative

$$f'''(x) = \frac{f(x-2h) - 4f(x-h) + 6f(x) - 4f(x+h) + f(x+2h)}{h^4} + \mathcal{O}(h^2).$$

2. **Colab.** Write a python program to define the structure matrix A in Equation (2.34). Then, using the `np.linalg.solve` command or code of your own design, solve the system for the displacements y_i using $n = 10$ grid steps. Plot the solution from Step 1 against the correct solution

$$y(x) = \frac{f}{24EI} x^2(x^2 - 4Lx + 6L^2),$$

where $f = f(x)$ is the constant defined above. Check the error at the end of the beam, $x = L$ meters. In this simple case the derivative approximations are exact, so your error should be near machine roundoff.

3. **Colab.** Rerun the calculation in Step 1 for $n = 10 \cdot 2^k$, where $k = 1, \dots, 8$. Make a table of the errors at $x = L$ for each n . Add a column to your table with the condition number of A as a function of n .

Handwritten or typed. For which n is the error smallest? Why does the error begin to increase with n after a certain point? (Hint: look at the column with condition number values.)

4. **Handwritten or typed.** Add a sinusoidal pile to the beam. This means adding a function of form $s(x) = -pg \sin(\pi/Lx)$ to the force term $f(x)$. Prove that the solution

$$y(x) = \frac{f}{24EI} x^2(x^2 - 4Lx + 6L^2) - \frac{pgL}{EI\pi} \left(\frac{L^3}{\pi^3} \sin\left(\frac{\pi}{L}x\right) - \frac{x^3}{6} + \frac{L}{2}x^2 - \frac{L^2}{\pi^2}x \right)$$

satisfies the Euler–Bernoulli beam equation and the clamped-free boundary conditions.

5. **Colab.** Rerun the calculation as in Step 3 for the sinusoidal load. (Be sure to include the weight of the beam itself.) Set $p = 100 \text{ kg/m}$ and plot your computed solutions against the correct solution. Plot the error versus h on a log-log graph.

Handwritten or typed. For which n is the error smallest? Why does the error begin to increase with n after a certain point? (Hint: look at the column with condition number values.) Is the error at $x = L$ proportional to h^2 as claimed above? Does the condition number come into play?