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Stabilized mixed formulation for an implicit MPM for  
viscoplastic fluids by using a variational subgrid-scale  
framework

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# Outline

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The Material  
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# Motivation

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The climatic change is one of the main causes of **water-related extreme events**.

- What is the effect of extreme flow events on existing structures?
- How to properly design new structures?

In recent years, **Material Point Method (MPM)** receive attention in geotechnical engineering.



Bridge failed during Washington **floods** in 2009



Landslide in Wenchuan area of China in 2019

# Goals of the work

**Long term goal:** Simulating the interaction between a flow-like debris and a structure.

A lot of ingredients are needed to achieve it:

- Implementation of different materials.
- Developing a FSI interaction schemes.
- **Simulating granular flows.**
- ...



A damaged bridge in Bad Neuenahr-Ahrweiler, Germany in 2021

## Objectives

- ① Development a stabilized formulation for **incompressible solid mechanics in mixed formulation** for simulating large deformation regimes.
- ② Development of a **Newtonian law** for a linearized displacement-based formulation.

All implementations are performed using **KRATOS Multiphysics**, an open source and high performance simulation software.

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# Why using the Material Point Method (MPM)?

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- ① Classical FEM is not suitable for large deformations.
- ② Discrete approaches are:
  - not suitable for large scale problems,
  - not easy to simulate complex material laws.

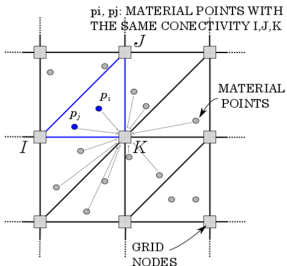


Hokkaido **landslide** after earthquake, Japan in 2018

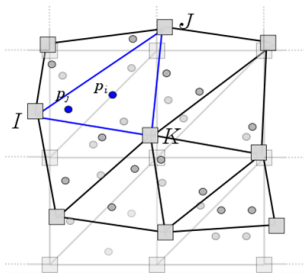
**MPM** is a particle-in-cell (PIC) method which takes advantage of all the potential and the well-established knowledge reached in **FE technology**.

# The Material Point Method

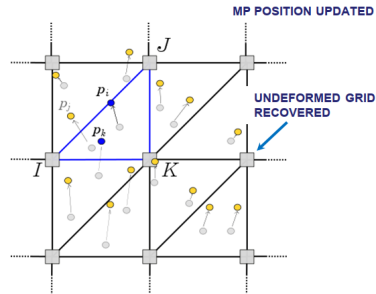
- 1 A background grid (fixed) used for the FEM solution system.
- 2 A collection of Material Points (MP) (Lagrangian).
  - Moving integration points.
  - MPs store historical information.
  - Every step is a FEM-like Lagrangian step.
  - The grid information is reset at the end of the step.



Stage 1. Initialization



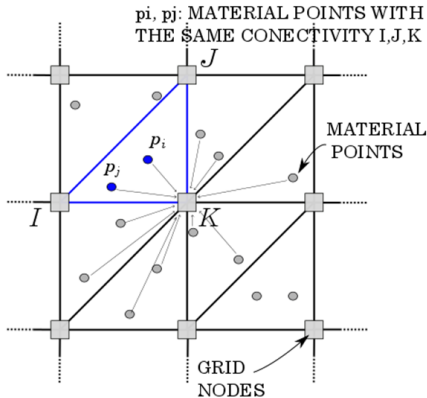
Stage 2. Calculation



Stage 3. Convection

# Stage 1. Initialization phase

## Definition of the initial conditions on the FE grid's nodes.



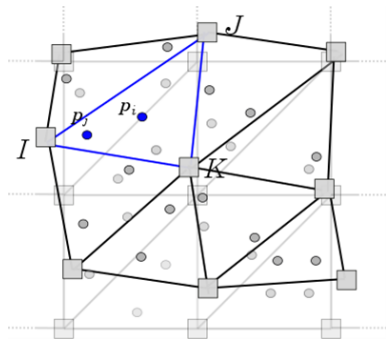
It is composed by:

- 1 Extrapolation on the nodes.
- 2 Prediction of nodal displacement, velocity and acceleration using a Newmark scheme.



## Stage 2. Calculation phase

Update Lagrangian-FEM solution on the nodes of the background grid.



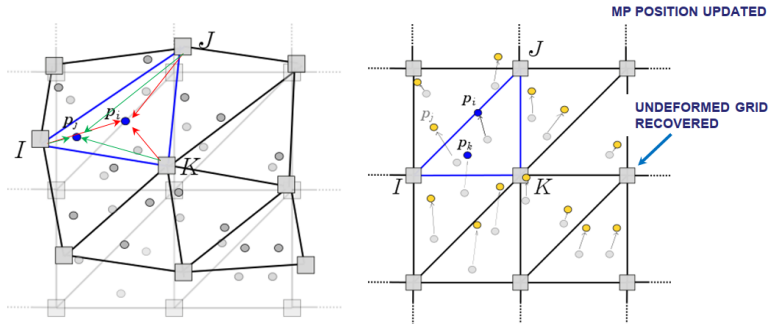
$\Omega$ : Initial configuration.  
 $\varphi(\Omega)$ : Current configuration.

Calculation of the solution of an algebraic system of equations (implicit formulation) from  $t_n$  to  $t_{n+1}$  step. It is obtained following **the FE traditional steps**:

- 1 **Construction** of the elemental system. System is evaluated in the current configuration  $\varphi(\Omega)$  locally.
- 2 **Assembling** of the elemental system.
- 3 **Solving the system** through an iterative scheme.

## Stage 3. Convective phase

Information is interpolated and stored on the particles which are moved on the calculated positions.



- 1 Nodal information at time  $t_{n+1}$  are interpolated back onto the material points.
- 2 MP position is updated.
- 3 The undeformed FE grid is recovered.
- 4 The material points connectivities are updated (identify the element in which each MP falls).

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# Mixed formulation u-p for hyperelastic materials

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A standard Galerkin displacement-based formulation (u-formulation) fails when Poisson's ratio  $\nu \rightarrow 0.5$  ( $\kappa \rightarrow \infty$ ) or when the plastic flow is constrained by the volume conservation condition.

## Updated Lagrangian Formulation

$$\left\{ \begin{array}{ll} \rho \frac{\partial^2 \mathbf{u}}{\partial t^2} - \nabla \cdot \boldsymbol{\sigma} = \rho \mathbf{b} & \text{in } \varphi(\Omega)(t), \\ \frac{p}{\kappa} - \frac{dG}{dJ} = 0 & \text{in } \varphi(\Omega)(t), \\ \mathbf{u} = \bar{\mathbf{u}} & \text{on } \varphi(\partial\Omega_D)(t), \\ \boldsymbol{\sigma} \cdot \mathbf{n} = \bar{\mathbf{t}} & \text{on } \varphi(\partial\Omega_N)(t), \end{array} \right.$$

$$\boldsymbol{\sigma} = \boldsymbol{\sigma}^{\text{dev}} + p\mathbf{I}$$

Cauchy stress tensor

## Hyperelasticity

$$\mathbf{S} = 2 \frac{\partial \Psi(\mathbf{C})}{\partial \mathbf{C}} \rightarrow \boldsymbol{\sigma} = \frac{1}{J} \mathbf{F} \mathbf{S} \mathbf{F}^T.$$

$$\mathbf{S} := 2 \frac{\partial \Psi_{\text{dev}}}{\partial \mathbf{C}} + \frac{d\Psi_{\text{vol}}}{dJ} J \mathbf{C}^{-1}$$

- Deviatoric model: Neo-Hookean  
 $\Psi_{\text{dev}}(\bar{I}_1) = \frac{\mu}{2} (\bar{I}_1 - 3)$

- Volumetric model: Miehe et al.

$$\Psi_{\text{vol}}(J) = \kappa (J - \log(J) - 1) \\ \rightarrow \frac{\partial \Psi_{\text{vol}}(J)}{\partial J} = \kappa \left(1 - \frac{1}{J}\right) = \kappa \frac{dG}{dJ}$$

## Variational form

The weak form problem consists in finding  $\mathbf{U} = [\mathbf{u}, p] : ]0, T[ \rightarrow \mathcal{W}$ , such that the initial conditions are satisfied and for all  $\mathbf{V} = [\mathbf{w}, q] \in \mathcal{W}_0$ ,

$$(\mathcal{D}_t(\mathbf{U}), \mathbf{V}) + B(\mathbf{U}, \mathbf{V}) = \mathcal{F}(\mathbf{V})$$

where:

$$(\mathcal{D}_t(\mathbf{U}), \mathbf{V}) = \left( \rho \frac{\partial^2 \mathbf{u}}{\partial t^2}, \mathbf{w} \right)$$

$$B(\mathbf{U}, \mathbf{V}) = \left( \boldsymbol{\sigma}^{\text{dev}}(\mathbf{u}), \nabla^s \mathbf{w} \right) + (p \mathbf{I}, \nabla^s \mathbf{w}) + \left( \frac{p}{\kappa}, q \right) - \left( \frac{dG}{dJ}, q \right)$$

$$\mathcal{F}(\mathbf{V}) = \langle \rho \mathbf{b}, \mathbf{w} \rangle + \langle \bar{\mathbf{t}}, \mathbf{w} \rangle_{\varphi(\partial\Omega_N)}$$

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# Linearization

- Newton-Raphson's iterative procedure.
- Taylor's series expansion evaluated at the last known equilibrium configuration  $\mathbf{U}^* = [\mathbf{u}^*, p^*]$ .
- $B$  must be allowed to compute a correction  $\delta \mathbf{U} = [\delta \mathbf{u}, \delta p]$ . Note that  $\mathbf{U} = \delta \mathbf{U} + \mathbf{U}^*$ .

$$B(\mathbf{U}, \mathbf{V}) \approx B(\mathbf{U}^*, \mathbf{V}) + B_d(\delta \mathbf{U}, \mathbf{V}) + o(\delta \mathbf{u}) + o(\delta p)$$

Find the correction  $\delta \mathbf{U} = [\delta \mathbf{u}, \delta p] : ]0, T[ \rightarrow \mathcal{W}_0$  such that

$$(\mathcal{D}_t(\delta \mathbf{U}), \mathbf{V}) + B_d(\delta \mathbf{U}, \mathbf{V}) = \mathcal{F}(\mathbf{V}) - (\mathcal{D}_t(\mathbf{U}^*), \mathbf{V}) - B(\mathbf{U}^*, \mathbf{V})$$

$$\mathcal{D}_t(\delta \mathbf{U}) = \left[ \rho \frac{\partial^2 \delta \mathbf{u}}{\partial t^2}, 0 \right]$$

$$\begin{aligned} B_d(\delta \mathbf{U}, \mathbf{V}) = & (\nabla \delta \mathbf{u} \cdot (\boldsymbol{\sigma}(\mathbf{u}^*) + p^* \mathbf{I}), \nabla \mathbf{w}) \\ & + \left( \nabla^s \mathbf{w}, \mathbb{C}^{\text{dev}}(\mathbf{u}^*) + p^* (\mathbf{I} \otimes \mathbf{I} - 2\mathbb{I}) : \nabla^s \delta \mathbf{u} \right) \\ & + (\delta p, \nabla \cdot \mathbf{w}) + \left( \frac{\delta p}{\kappa}, q \right) - (f(J(\mathbf{u}^*)) \nabla \cdot \delta \mathbf{u}, q) \end{aligned}$$

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# Mixed formulation for an incompressible Newtonian fluid material

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It is a displacement based formulation, and it is assumed finite strains.

## Updated Lagrangian Formulation

$$\left\{ \begin{array}{ll} \rho \frac{\partial^2 \mathbf{u}}{\partial t^2} - \nabla \cdot \boldsymbol{\sigma} = \rho \mathbf{b} & \text{in } \varphi(\Omega)(t), \\ 1 - \frac{1}{J} = 0 & \text{in } \varphi(\Omega)(t), \\ \mathbf{u} = \bar{\mathbf{u}} & \text{on } \varphi(\partial\Omega_D)(t), \\ \boldsymbol{\sigma} \cdot \mathbf{n} = \bar{\mathbf{t}} & \text{on } \varphi(\partial\Omega_N)(t), \end{array} \right.$$

$$\boldsymbol{\sigma} = \underbrace{2\mu \nabla^S \mathbf{v}}_{\boldsymbol{\sigma}^{\text{dev}}(\mathbf{u})} + p \mathbf{I}$$

Cauchy stress tensor

## Incompressible Newtonian fluid material

$$\boldsymbol{\sigma} = \frac{1}{J} \mathbf{F} \mathbf{S} \mathbf{F}^T$$

$$\mathbf{S} := J \left( \mu \mathbf{C}^{-1} \dot{\mathbf{C}} \mathbf{C}^{-1} \right) + p J \mathbf{C}^{-1}$$

where

$$\dot{\mathbf{C}} = \frac{D\mathbf{C}}{Dt} = 2\mathbf{F}^T \nabla^S \mathbf{v} \mathbf{F}; \quad \mathbf{C} = \mathbf{F}^T \mathbf{F}$$



## Variational form

The weak form problem consists in finding  $\mathbf{U} = [\mathbf{u}, p] : ]0, T[ \rightarrow \mathcal{W}$ , such that the initial conditions are satisfied and for all  $\mathbf{V} = [\mathbf{w}, q] \in \mathcal{W}_0$ ,

$$(\mathcal{D}_t(\mathbf{U}), \mathbf{V}) + B(\mathbf{U}, \mathbf{V}) = \mathcal{F}(\mathbf{V})$$

where:

$$(\mathcal{D}_t(\mathbf{U}), \mathbf{V}) = \left( \rho \frac{\partial^2 \mathbf{u}}{\partial t^2}, \mathbf{w} \right)$$

$$B(\mathbf{U}, \mathbf{V}) = \left( \boldsymbol{\sigma}^{\text{dev}}(\mathbf{u}), \nabla^s \mathbf{w} \right) + (p\mathbf{I}, \nabla^s \mathbf{w}) + \left( 1 - \frac{1}{J}, q \right)$$

$$\mathcal{F}(\mathbf{V}) = \langle \rho \mathbf{b}, \mathbf{w} \rangle + \langle \bar{\mathbf{t}}, \mathbf{w} \rangle_{\varphi(\partial\Omega_N)}$$

## Linearization

- Analogous to the hyperelastic materials.
- **Difficulty:** finding the spatial constitutive tensor  $\mathbb{C}^{\text{dev}}(\mathbf{u}^*)$ .

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# Spatial and temporal discretizations

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## Spatial discretization of the linearized form.

**Galerkin finite element** approximation. It consists in finding  $\delta \mathbf{U}_h \in \mathcal{W}_{h,0}$  such that:

$$\underbrace{(\mathcal{D}_t(\delta \mathbf{U}_h), \mathbf{V}_h)}_{\text{Temporal terms}} + \underbrace{B_d(\delta \mathbf{U}_h, \mathbf{V}_h)}_{\text{Semi-linear form}} = \mathcal{F}(\mathbf{V}_h) - (\mathcal{D}_t(\mathbf{U}_h^*), \mathbf{V}_h) - B(\mathbf{U}_h^*, \mathbf{V}_h)$$

for all  $\mathbf{V}_h \in \mathcal{W}_{h,0}$ . To discretise the continuum body  $\mathcal{B}$  by a set of  $n_p$  material points and  $\Omega_p$  a finite volume of the body to each of those material points.

$$\mathcal{B} \approx \mathcal{B}_h = \bigcup_{p=1}^{n_p} \Omega_p.$$

## Time discretization

**Monolithic** time discretization using a Newmark scheme.

# Stabilization technique: Variational Multi-Scale (VMS) Methods

- **Objective:** to approximate the components of the continuous problem solution that cannot be resolved by the finite element mesh.
- Unknown splitting:  $\delta \mathbf{U} = \underbrace{\delta \mathbf{U}_h}_{\in \mathcal{W}_{h,0}} + \underbrace{\tilde{\mathbf{U}}}_{\in \tilde{\mathcal{W}}}$  and  $\mathcal{W} = \mathcal{W}_{h,0} \oplus \tilde{\mathcal{W}}$ .

$$\begin{aligned}
 & \underbrace{(\mathcal{D}_t(\delta \mathbf{U}_h), \mathbf{V}_h) + B_d(\delta \mathbf{U}_h, \mathbf{V}_h)}_{\text{Galerkin terms}} + \underbrace{\langle \cancel{\mathcal{D}_t(\tilde{\mathbf{U}})}, \mathbf{V}_h \rangle + \sum_K \langle \tilde{\mathbf{U}}, \overbrace{\mathcal{L}_d^*(\mathbf{u}_h; \mathbf{V}_h)}^{\text{adjoint operator of } \mathcal{L}_d} \rangle_K}_{\text{Stabilization terms}} \\
 & = \underbrace{\mathcal{F}(\mathbf{V}_h) - B(\mathbf{U}_h^*, \mathbf{V}_h) - (\mathcal{D}_t(\mathbf{U}_h^*), \mathbf{V}_h)}_{\text{Galerkin terms}}
 \end{aligned}$$

$$\cancel{\frac{\partial \tilde{\mathbf{U}}}{\partial t}} + \tau^{-1} \tilde{\mathbf{U}} = \tilde{P} \left[ \underbrace{\tilde{\mathfrak{F}} - \mathcal{L}(\mathbf{U}_h^*) - \mathcal{D}_t(\mathbf{U}_h^*) - \mathcal{D}_t(\delta \mathbf{U}_h) - \mathcal{L}_d(\delta \mathbf{U}_h)}_{\text{Residual from the linearization}} \right]$$

Sub-grid scale

# Main operators of the problem

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1. For computing the residual and obtaining  $\tilde{\mathbf{U}}$ :

$$\mathcal{L}(\mathbf{U}^*) := \begin{pmatrix} -\nabla \cdot (\boldsymbol{\sigma}^{\text{dev}}(\mathbf{u}^*) + p^* \mathbf{I}) \\ \frac{p^*}{\kappa} - \frac{dG}{dJ} \end{pmatrix}, \quad \mathcal{D}_t(\mathbf{U}^*) := \begin{pmatrix} \rho \frac{\partial^2 \mathbf{u}^*}{\partial t^2} \\ 0 \end{pmatrix}, \quad \mathcal{D}_t(\delta \mathbf{U}) := \begin{pmatrix} \rho \frac{\partial^2 \delta \mathbf{u}}{\partial t^2} \\ 0 \end{pmatrix},$$

$$\mathfrak{F} = [\rho \mathbf{b}, 0],$$

$$\mathcal{L}_d(\delta \mathbf{U}) := \begin{pmatrix} -\nabla \cdot (\nabla \delta \mathbf{u} \cdot (\cancel{\boldsymbol{\sigma}^{\text{dev}}(\mathbf{u}^*)} + p^* \mathbf{I})) - \nabla \cdot (\cancel{\boldsymbol{\sigma}^{\text{dev}}(\mathbf{u}^*)} + p^* (\mathbf{I} \otimes \mathbf{I} - 2\mathbb{I}) : \nabla^s \delta \mathbf{u}) - \nabla \delta p \\ \frac{\delta p}{\kappa} + f(J(\mathbf{u}^*)) \nabla \cdot \delta \mathbf{u} \end{pmatrix}$$

2. The adjoint operator is:

$$\mathcal{L}_d^*(\mathbf{V}) :=$$

$$\begin{pmatrix} -\nabla \cdot (\nabla \mathbf{w} \cdot (\cancel{\boldsymbol{\sigma}^{\text{dev}}(\mathbf{u}^*)} + p^* \mathbf{I})) - \nabla \cdot (\nabla^s \mathbf{w} : (\cancel{\boldsymbol{\sigma}^{\text{dev}}(\mathbf{u}^*)} + p^* (\mathbf{I} \otimes \mathbf{I} - 2\mathbb{I})) - f(J(\mathbf{u}^*)) \nabla q) \\ \nabla \cdot \mathbf{w} + \frac{q}{\kappa} \end{pmatrix}$$

# Final expression for linear elements

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$$\begin{aligned} & \underbrace{(\mathcal{D}_t(\delta \mathbf{U}_h), \mathbf{V}_h) + B_d(\delta \mathbf{U}_h, \mathbf{V}_h)}_{\text{Galerkin terms}} + \underbrace{S_1(\mathbf{U}_h^*; \delta \mathbf{U}_h, \mathbf{V}_h) + S_2(\mathbf{U}_h^*; \delta \mathbf{U}_h, \mathbf{V}_h)}_{\text{Stabilization terms}} \\ &= \underbrace{\mathcal{F}(\mathbf{V}_h) - B(\mathbf{U}_h^*, \mathbf{V}_h) - (\mathcal{D}_t(\mathbf{U}_h^*), \mathbf{V}_h)}_{\text{Galerkin terms}} + \underbrace{R_1(\mathbf{U}_h^*, \mathbf{V}_h) + R_2(\mathbf{U}_h^*, \mathbf{V}_h)}_{\text{Stabilization terms}} \end{aligned}$$

$$S_1(\mathbf{U}_h^*; \delta \mathbf{U}_h, \mathbf{V}_h) =$$

$$\sum_K \tau_1 \left\langle \tilde{P} \left[ -\rho \frac{\partial^2 \delta \mathbf{u}_h}{\partial t^2} + (\nabla \delta \mathbf{u}_h \cdot \nabla p_h^*) + (\nabla \cdot p_h^* (\mathbf{I} \otimes \mathbf{I} - 2\mathbb{I}) : \nabla^s \delta \mathbf{u}_h) + \nabla \delta p_h \right], \right. \\ \left. - (\nabla \mathbf{w}_h \cdot \nabla p_h^*) - (\nabla^s \mathbf{w}_h : \nabla \cdot p_h^* (\mathbf{I} \otimes \mathbf{I} - 2\mathbb{I})) - f(J(\mathbf{u}_h^*)) \nabla q_h \right\rangle_K$$

$$S_2(\mathbf{U}_h^*; \delta \mathbf{U}_h, \mathbf{V}_h) = \sum_K \tau_2 \left\langle \tilde{P} \left[ -\frac{\delta p_h}{\kappa} + f(J(\mathbf{u}_h^*)) \nabla \cdot \delta \mathbf{u}_h \right], \nabla \cdot \mathbf{w}_h + \frac{q_h}{\kappa} \right\rangle_K$$

- $\tilde{P}$  is the  $L^2$  – projection onto the space of sub-grid scales,
- $\tau$  is a matrix computed within each element

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# Cook's membrane. Main features.

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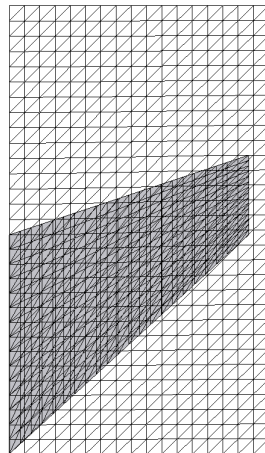
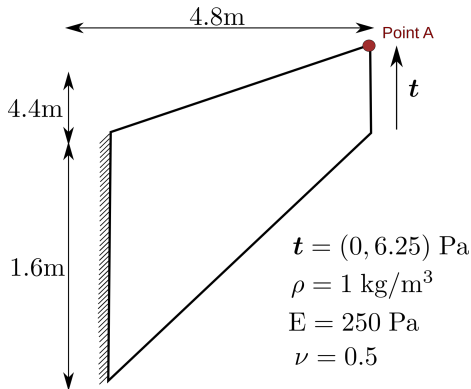
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**Temporal discretization:**  $\delta t = 0.0025 \text{ s}$

**Spatial discretization:**  $h = 0.6, 0.3, 0.15, 0.75, 0.325 \text{ m}$  with 16 material points per element.



# Cook's membrane: static and dynamic cases.

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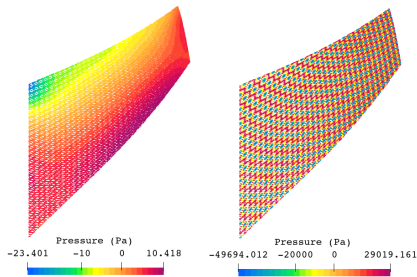
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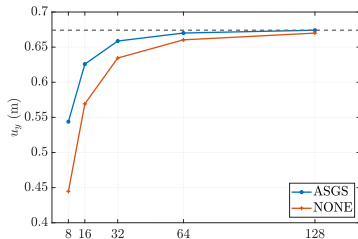
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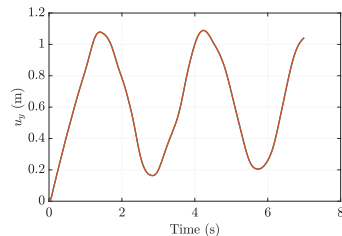
Pressure using stabilization VS without stabilization.



Number of elements in the minor side

Mesh convergence. Static case.

- It is crucial to use a stabilization method to obtain a **pressure distribution without jumps** of pressure values between elements.
- Dynamic case has been impossible to run without stabilization.



Evolution of the displacement in time. Dynamic case.

# Bending beam problem. Main features

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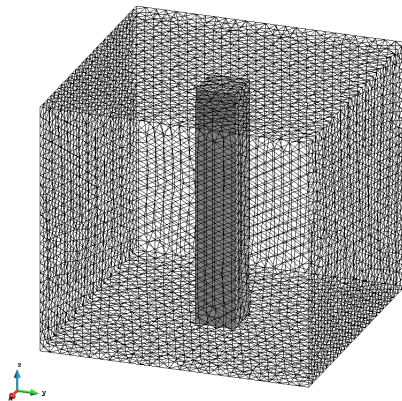
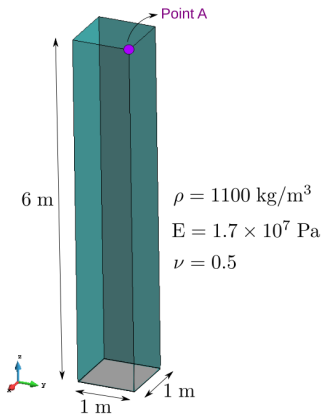
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$$\mathbf{v}^0(x, y, z) = \frac{5}{3}(z, 0, 0)^T \text{ m/s}$$

**Temporal discretization:**  $\delta t = 0.01 \text{ s}$ ,  $T_{\text{fin}} = 3 \text{ s}$ ,

**Spatial discretization:** mesh size  $h = 0.2 \text{ m}$ , and 16 material points per element in the body.

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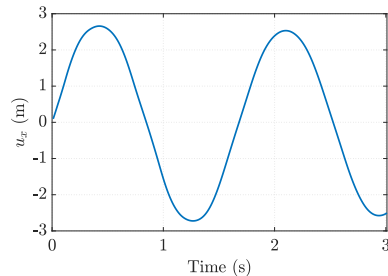
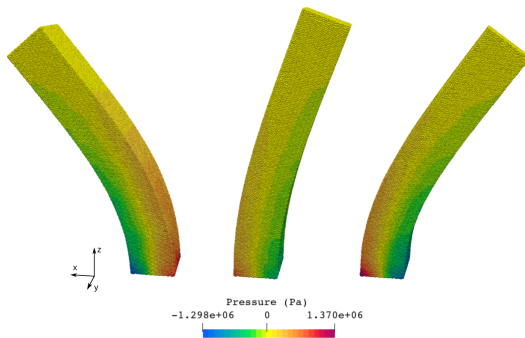
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Bending column at  $t = 0.5$  s,  $t = 1$  s and  $t = 1.25$  s. Evolution of x-displacement in time of the point A.

Deformation is well reproduced. Pressure maximum and minimum peaks are located on the bottom of the beam as it is expected.

# Twisting column problem. Main features

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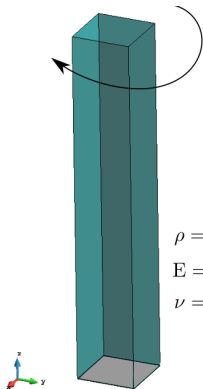
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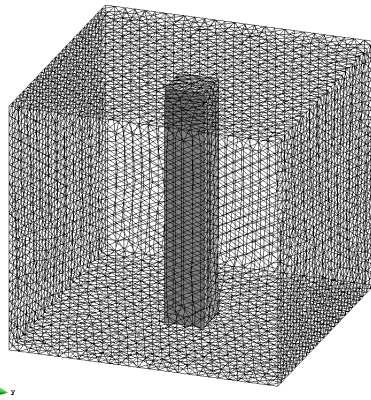
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$$\begin{aligned}\rho &= 1100 \text{ kg/m}^3 \\ E &= 1.7 \times 10^7 \text{ Pa} \\ \nu &= 0.5\end{aligned}$$

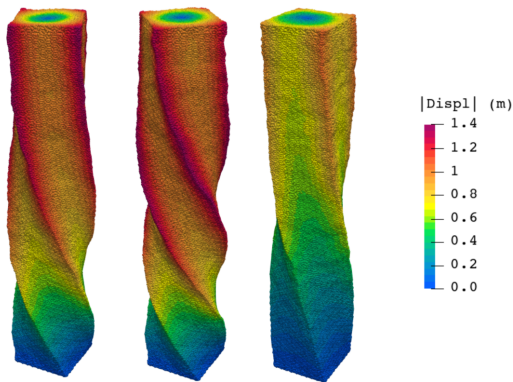


$$\mathbf{v}^0(x, y, z) = 105 \sin\left(\frac{\pi z}{12}\right) (y, -x, 0)^T \text{ m/s}$$

**Temporal discretization:**  $\delta t = 0.001 \text{ s}$ ,  $T_{\text{fin}} = 0.5 \text{ s}$ ,

**Spatial discretization:** mesh size  $h = 0.2 \text{ m}$ , and 16 material points per element in the body.

# Twisting column problem



- Displacement field is well captured.
- Simulation is done without remeshing techniques, ALE or level set.

Twisting column at  $t = 0.05$  s,  $t = 0.1$  s and  $t = 0.2$  s.

## Conclusion

ASGS stabilization method is **more robust than other methods**, such as Polynomial Pressure Method (PPP). Moreover, the later two examples were no able to be computed with PPP.

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# Dam break problem. Main features

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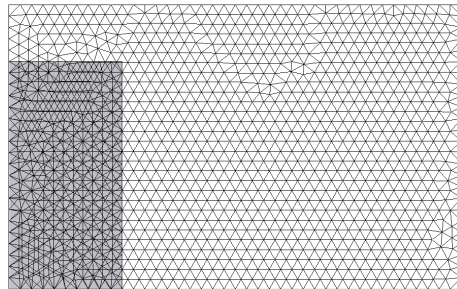
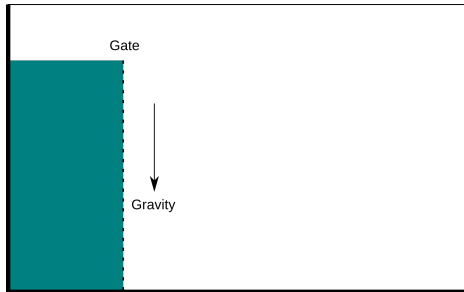
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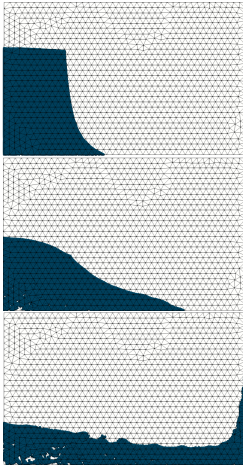
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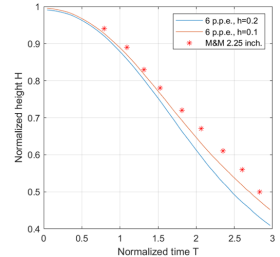
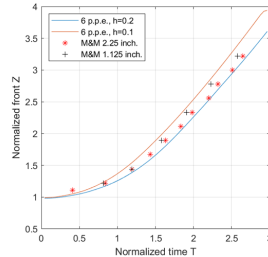
**Spatial discretization:** Mesh size  $h = 0.1$  m and  $h = 0.2$  m, and 6 material points per element.

**Temporal discretization:**  $\delta t = 0.001$  s,  $T_{\text{fin}} = 0.9$  s.

# Dam break problem. Results



Dam break test at  $t=0.25, 0.5$  and  $0.85$  s.



Left: distance of the surge front from axis of plane of symmetry;  
Right: height of the residual column.

- It is one of the ultimate performed tests in free-surface fluid dynamics.
- **Volumetric locking** has been efficiently avoided.
- The results are in agreement with **experimental evidence**.



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- VMS-type stabilization have been developed and implemented for a solid mechanics framework and the Material Point Method.
- **ASGS stabilization** is more robust for simulating incompressible hyperelastic materials in comparison with others stabilization techniques such as the Polynomial Pressure Projection.
- This stabilization is able to compute very challenging **large deformation regimes** using the Material Point Method.
- The development of a **Newtonian law** for an Updated Lagrangian linearized **displacement-based formulation**. It is checked for the Material Point Method comparing with an experimental test.

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Thank you for your attention!!

Laura Moreno Martínez