3.3 Steel column design

Required steel area calculation

$$A_{req} = \frac{N_{ED}}{\frac{f_y}{\gamma_M}}$$

where:

N_{ED} is the load action on the column

 γ_{M} is partial safety factor for steel

f_y is steel yield strength (for S275 and S355 steel grade, refer EN 10025-2 (Table 7))

Steel section - universal steel column sheetError! Reference source not found.

Depth, width, flange thickness, web thickness, root radius and radius of gyration on both axis are provided from the steel section sheet.

Section class

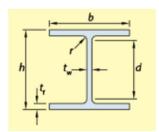
Outstand flange

$$\frac{c}{t} = \frac{B - t_w - 2 x r}{2t}$$

Internal compression part

$$\frac{c}{t} = \frac{D - 2t_f - 2xr}{t}$$

	Flange outstand limiting value, c/t _f	Web in bending limiting value, d/t _w			
Class 1	9ε	72 ε			
Class 2	10 ε	83 ε			
Class 3	14 ε	124 ε			
Class 4	requirements,	it does not meet Class 3 ments, the section is classified as Class 4			



where:

$$\varepsilon = (235/f_y)^{0.5}$$

where:

f_v is steel yield strength

Compression resistance

$$\frac{N_{ED}}{N_{c,Rd}} \le 1.0$$

where:

N_{Ed} is the design value of the axial compression load

N_{c,Rd} is the design resistance to axial compression, given by the minimum of

i) Plastic resistance

$$N_{c,Rd} = \frac{Af_y}{\gamma_M}$$
 (for class 1,2 or 3)

$$N_{c,Rd} = \frac{A_{eff} f_y}{\gamma_M}$$
 (for class 4)

where:

A and A_{eff} are area and effective area of steel

 f_y is steel yield strength (for S275 and S355 steel grade, refer EN 10025-2 (Table 7))

 γ_M is partial safety factor for steel

ii) Buckling resistance

 $N_{b,Rd}$, in general the flexural buckling resistance, which govern the design of a member in pure compression.

$$N_{b,Rd} = \chi \frac{Af_y}{\gamma_M}$$
 (for class 1,2 or 3)

$$N_{b,Rd} = \chi \frac{A_{eff} f_y}{\gamma_M}$$
 (for class 4)

where:

 χ is a reduction factor for the relevant buckling mode

A and Aeff are area and effective area of steel.

 f_{y} is steel yield strength (for S275 and S355 steel grade, refer

EN 10025-2 (Table 7))

γ_M is partial safety factor for steel

Reduction factors (χ)

Buckling lengths – Assuming that the design forces were obtained by a second order structural analysis, the buckling lengths are considered (conservatively) equal to the real lengths (mid-distance between floors), given by:

$$\chi = \frac{1}{\Phi + \sqrt{\Phi^2 - (\lambda^*)^2}}$$

where:

$$\Phi = 0.5 \left[1 + \alpha (\lambda^* - 0.2) + (\lambda^*)^2 \right]$$

where:

 λ^* is slenderness coefficient - which ever maximum is to be used for the design from λ_x^* and λ_y^* .

$$\lambda_x^* = \frac{\lambda_x}{\lambda_1}$$
 $\lambda_y^* = \frac{\lambda_y}{\lambda_1}$

where:

$$\lambda_x = L_{Ex}/i_x$$
 $\lambda_y = L_{Ey}/i_y$

$$\lambda_1 = \pi \sqrt{\frac{E}{f_y}}$$

where:

E is elastic modulus of steel

 f_y is steel yield strength (for S275 and S355 steel grade, refer EN 10025-2 (Table 7))

i_x and i_y are radius of gyration in x and y axis

 L_{Ex} and L_{Ey} are the effective length of the column in the x-z and y-z axis; $L_E = L * K$

K = 1.0, pinned at both end

K = 0.7, fixed at one end and pinned at the other end

K = 0.5, fixed at both end

K = 2.0, fixed at one end and free at the other end (cantilever)

 α is buckling factor in flexural buckling mode about x and y axis

Selection of buckling curve for a cross-section (refer Table EN 1993-1-1: 2005 (E))

If h/b > 1.2

 $t_{\rm f} \leq$ 40 mm, a and b are values of bucking curve about axis x-x and y-y.

 $40\text{mm} \le t_f \le 100$, b and c are values of bucking curve about axis x-x and y-y.

If $h/b \le 1.2$

 $t_{\rm f} \leq 100$ mm, b and c are values of bucking curve about axis x-x and y-y.

 $t_{\rm f} > 100$, d is the value of both bucking curve about axis x-x and y-y.

	f _y (N/mm²) nominal thickness of element, t (mm)					
S 275	275	265	255	245		
S 355	355	345	335	325		

EN 10025-2 (Table 7)

X

Table 6.1: Imperfection factors for buckling curves

Buckling curve	a_0	a	Ъ	c	d
Imperfection factor α	0,13	0,21	0,34	0,49	0,76