

# Structural Dynamics and Acoustics

## Case-study 3

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# Key points of the TP

1. Compute the dynamic response of a 3D multi-story building with COMSOL Multiphysics®
2. Export the **Results** into the support notebook `mdof_comsol_todo.ipynb` for further analysis and comparison with a simplified MDOF (see Case-study 2)
  - Compare the Eigen-frequency and Eigen-modes of simplified 1D MDOF and the 3D multi-story building designed in COMSOL Multiphysics®
  - Compare the Participation factors of simplified 1D MDOF and the 3D multi-story building designed in COMSOL Multiphysics®
  - Compute the simplified dynamic response of the MDOF to earthquake loading, using the Participation factors of the 3D multi-story building designed in COMSOL Multiphysics®

# Problem setting: Global Definitions

- ✓ MDOF\_EigenFrequency\_solved.mph
  - ✓ Global Definitions
    - Parameters
    - Default Model Inputs
    - Materials

## Parameters

Settings Properties

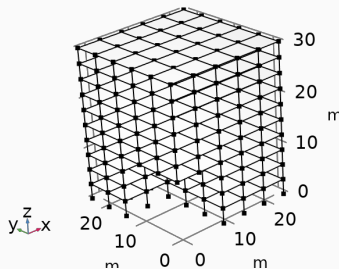
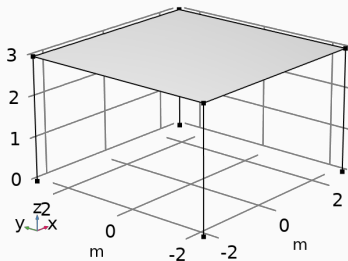
Parameters

Label: Parameters

Parameters

Name	Expression	Value	Description
bb	30 [cm]	0.3 m	Cross sectional base for beam elements
bc	30 [cm]	0.3 m	Cross sectional base for column elements
c1x	-Lx/2	-2.5 m	Origin of master frame in the x direction
c1y	-Ly/2	-2.5 m	Origin of the master frame in the y direction
c1z	0 [m]	0 m	Origin of the master frame in the z direction
E_beam	28.3e9 [Pa]	2.83E10 Pa	Beam/Column concrete Young's modulus
E_floor	25e9 [Pa]	2.5E10 Pa	Floor concrete Young's modulus
ft	hb	0.3 m	Thickness of the floor
hb	30 [cm]	0.3 m	Cross sectional height for beam elements
hc	30 [cm]	0.3 m	Cross sectional height for column elements
Hf	3 [m]	3 m	Floor's height
Lx	5 [m]	5 m	Intercolumn distance in the x direction
Ly	5 [m]	5 m	Intercolumn distance in the y direction
Mf	0	0	Missing frames in the z direction
Mx	0	0	Missing frames on the x direction
My	0	0	Missing frames in the y direction
Nf	3	3	Number of floors
Nx	1	1	Number of frames in the x direction
Ny	1	1	Number of frames in the y direction
rho_beam	500 [kg/m^3]; 500 kg/m^3		Beam/Column concrete unit mass density
rho_floor	2300 [kg/m^3]; 2300 kg/m^3		Floor concrete unit mass density

## Problem setting: component Frame3D



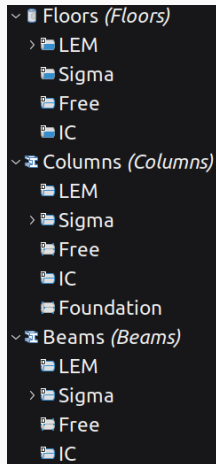
MasterFrame:

- 1D Vertical concrete beams of height  $H_f$  (Columns)
- 1D Horizontal concrete beams of length  $L_x$  and  $L_y$  (Beams)
- 2D Horizontal concrete shells of area  $L_x \times L_y$  (Floors)

MultiStoryBuilding:

- Array of  $N_x \times N_y \times N_f$  MasterFrames
- $M_x \times M_y \times M_f$  missing frames

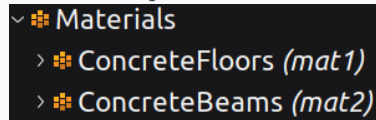
# Problem setting: component Frame3D



Physical entities: Floors, Columns, Beams:

- LEM: Linear Elastic Material
- Sigma: Cross-Section properties
- Free: Free boundaries/edges
- IC: Initial Conditions

Materials (from COMSOL multiphysics®library) are assigned to each component

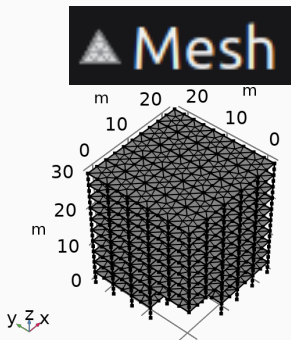


# Problem setting: component Frame3D

Each physical entity is meshed according to Settings:

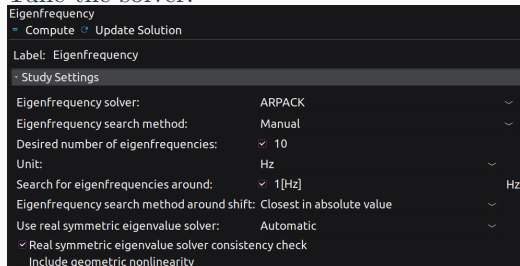
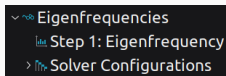
Mesh Settings	
Sequence type:	
Physics-controlled mesh	
Physics-Controlled Mesh	
Element size:	
Fine	
Contributor	Use
Floors (Floors)	<input checked="" type="checkbox"/>
Columns (Columns)	<input checked="" type="checkbox"/>
Beams (Beams)	<input checked="" type="checkbox"/>
Shell-Column Connection (shbc1)	<input checked="" type="checkbox"/>
Shell-Beam Connection (shbc2)	<input checked="" type="checkbox"/>

- Extremely coarse
- ...
- Extremely fine



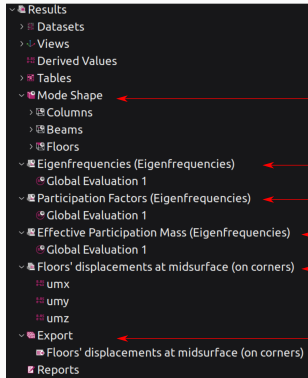
# Problem setting: Study

## Tune the solver:



- Real symmetric eigenvalue solver consistency check
- No Include geometric nonlinearity
- Compute every time the model is modified

# Problem setting: Results



Plot Modal shapes

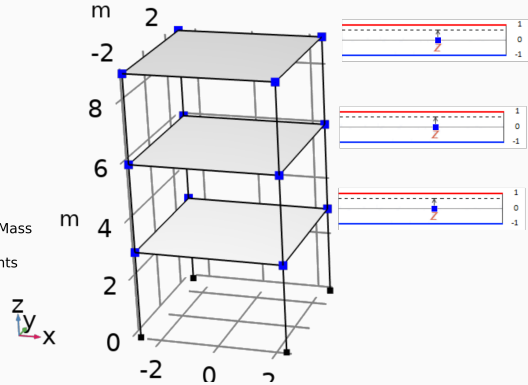
Table for Eigenfrequencies

Table for Participation factors

Table for Effective Participation Mass

Table of midsurface displacements

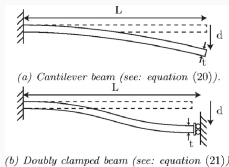
Export tables





# [TASK #1]: COMSOL Multiphysics Modal Shapes and Eigen-frequencies

- In the **Global Definitions**, select  $N_x=N_y=1$  and  $N_f=3$ , with no missing frames ( $M_x=M_y=M_f=0$ )
- Rebuild the **Geometry** by building all the entities (in **Form Union (fin)**)
- **Story Shear Building Model**
  - Constrain the structure in the  $x - z$  plane
    - For each physical entity **Floors**, **Columns**, **Beams**, introduce the adequate kinematic constraints  
(*Hint: Right click on the component  $\rightarrow$  Prescribed...*)



- Constrain **Columns** to pure shear
- (*Hint* Right click on the **Columns** component  $\rightarrow$  ...)
- Compute the **Eigenfrequencies** study for 10 eigen-frequencies (real) around 1 Hz and report the corresponding Modal Shapes in the **jupyter** notebook

`mdof_comsol_todo.ipynb` (snapshots to be placed in the cell *Report the Modal Shapes Eigen-frequencies from COMSOL Multiphysics*)

## [TASK #2]: Compare the Eigen-frequencies

- In the `jupyter` notebook `mdof_comsol_todo.ipynb`, compute the approximate mass and stiffness matrices of each story of the COMSOL Multiphysics) model  
*Nota Bene:* Do not consider the masses of the beams and columns (double clamped beam)
- In the `jupyter` notebook `mdof_comsol_todo.ipynb`, compute the eigen-frequencies and eigen-modes of the MDOF system. What are the natural frequencies? Do they correspond to the COMSOL Multiphysics® ones?
- Plot the eigen-modes of MDOF system and compared them to the COMSOL Multiphysics ones (evaluated on at the midsurface of each floor)  
(*Hint:* Export `umx`, `umy` and `umz` from COMSOL Multiphysics® and import the files in the notebook).

## [TASK #2]: Compare the Eigen-frequencies

- Compute the Participation Factors  $\Gamma_{ij}$  and compare them to those from COMSOL Multiphysics®((from Export Groups Participation Factors (Eigenfrequencies))).

*Nota bene:* Using modal participation factors is a way to characterize how much a certain mode will be excited by a rigid body acceleration  $\mathbf{d}_j$  in a certain direction  $j$ . The *normalized participation factor* with respect to mode  $i$  and excitation direction  $j$ , is defined as:

$$\tilde{\Gamma}_{ij} = \frac{\langle \phi_i, \mathbf{M} \cdot \mathbf{d}_j \rangle}{\langle \phi_i, \mathbf{M} \cdot \phi_j \rangle} = \frac{\Gamma_{ij}}{m_i}$$

with  $\Gamma_{ij} = \langle \phi_i, \mathbf{M} \cdot \mathbf{d}_j \rangle$  the Participation Factors and  $m_i = \langle \phi_i, \mathbf{M} \cdot \phi_j \rangle$  the  $i^{\text{th}}$  modal mass.