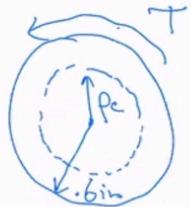
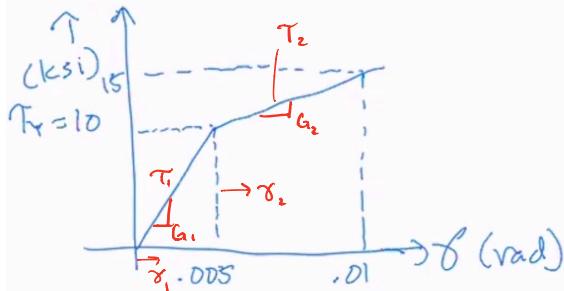


Materials - Lecture #17

TORSION REVIEW

- First problem



Stress-strain Relationship

$$\frac{\tau_1}{\gamma} = \frac{10(10^3)}{0.005} \rightarrow \tau_1 = \frac{2(10^6)}{G_1} \gamma$$

$$\frac{\tau_2 - 10(10^3)}{\gamma - .005} = \frac{15(10^3) - 10(10^3)}{.01 - .005} \Rightarrow \tau_2 = 1(10^6) \gamma + 5(10^3)$$

Find relationship between γ and r_c because $T = 2\pi \int_0^r \tau \rho^2 d\rho$

$$\gamma_{max} = \frac{0.6 \text{ in}}{0.5 \text{ in}} (.005) \quad \text{strain in cross-section is still linear}$$

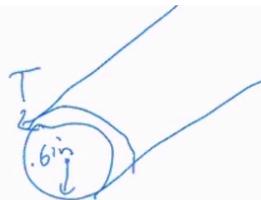
$$\gamma_y \text{ at } r_c = 0.5 \text{ in} \Rightarrow .005 \text{ rad (?)}$$

$$\gamma_{max} = .006 \text{ radians} \\ (\text{at outer surface})$$

$$\gamma = \frac{\rho}{c} \gamma_{max} \rightarrow \gamma = \frac{\rho}{0.6 \text{ in}} (.006) \Rightarrow \gamma = .01 \rho$$

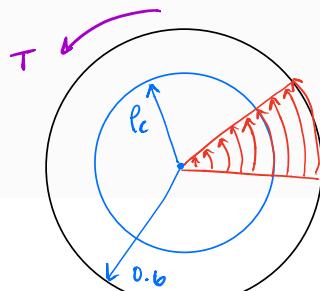
$$\tau_1 = 2(10^6) \gamma = 2(10^6)(.01 \rho) \Rightarrow \tau_1 = 20(10^3) \rho$$

$$\tau_2 = 1(10^6) \gamma + 5(10^3) = 1(10^6)(.01 \rho) + 5(10^3) \Rightarrow \tau_2 = 10(10^3) \rho + 5(10^3)$$



Question?

- What is T to create an elastic core with radius $r_c = 0.5 \text{ in}$



strain is linear and continuous

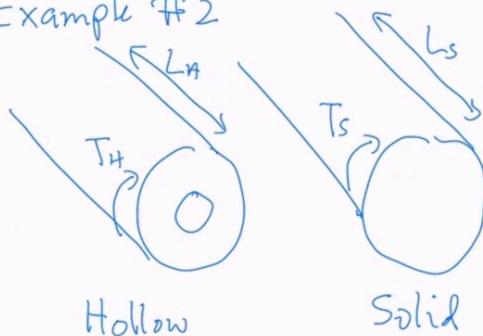
$$T = 2\pi \int_0^c \tau \rho^2 d\rho$$

$$= 2\pi \int_0^{0.5} [20(10^3)\rho] \rho^2 d\rho + 2\pi \int_{0.5}^{0.6} [(10)(10^3)\rho + 5(10^3)] \rho^2 d\rho$$

$$T = 3970 \text{ min}$$

Example #2

Example #2



Hollow

$$\textcircled{1} \quad \phi_s < \phi_H$$

$$\textcircled{2} \quad \phi_s = \phi_H, \text{ both } \neq 0$$

$$\textcircled{3} \quad \phi_s = \phi_H, \text{ both } = 0$$

$$\textcircled{4} \quad \phi_s > \phi_H$$

$$L_s = L_H$$

$$T_s = T_H$$

- Both are same material

- Both have same mass

$$J_H = \frac{\pi}{2} (C_o^4 - C_i^4)$$

$$J_s = \frac{\pi}{2} C^4$$

$$\phi = \frac{TL}{JG} \quad T, L, G \text{ are the same, } J \text{ is different}$$

let's say $\frac{TL}{G} = K$ then $\phi = \frac{K}{J}$

$$\phi_H = \frac{K}{(\frac{\pi}{2})(C_o^4 - C_i^4)} \quad \text{vs.} \quad \phi_s = \frac{K}{(\frac{\pi}{2})C^4}$$

$$\frac{1}{C_o^4 - C_i^4} \quad \text{vs.} \quad \frac{1}{C^4}$$

$$M = M_H = M_s \quad \therefore \ell = \ell_H = \ell_s$$

what we did

$$m_s = \ell \pi r_s^2 L \quad m_H = \pi \varphi (r_{oH}^2 - r_{iH}^2) L \quad \Rightarrow m_s = m_H$$

$$\ell \pi r_s^2 L = \pi \varphi (r_{oH}^2 - r_{iH}^2) L \quad \Rightarrow r_s^2 = r_{oH}^2 - r_{iH}^2$$

$$J_s = \frac{\pi}{2} (r_{oH}^4 - 2r_{oH}^2 r_{iH}^2 + r_{iH}^4)$$

substitute this into J !
(for J_s)

$$\textcircled{1} \quad J_s = \frac{\pi}{2} (r_{oH}^4 - r_{iH}^4)$$

$$\cancel{\frac{\pi}{2}} (r_{oH}^4 - 2r_{oH}^2 r_{iH}^2 + r_{iH}^4) \quad \text{vs.} \quad \cancel{\frac{\pi}{2}} (r_{oH}^4 - r_{iH}^4)$$

$$J_s$$

$$J_H$$

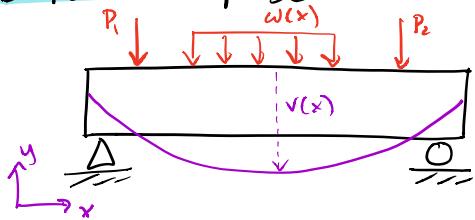
$$-2r_{oH}^2 r_{iH}^2 + r_{iH}^4 \quad \text{vs.} \quad -r_{iH}^4$$

assume that $r_{oH} > r_{iH}$

$$\text{then } J_H > J_s \quad \therefore \phi_s > \phi_H$$

Ch. 12 Hibbeler

Deflection of Beams



— = Hypothetical deflected shape $v(x)$

Are these beams going to deflect so much that they become structurally unstable?

- beams are designed to deflect downward
- need to place limits on how much deflection beams can undergo

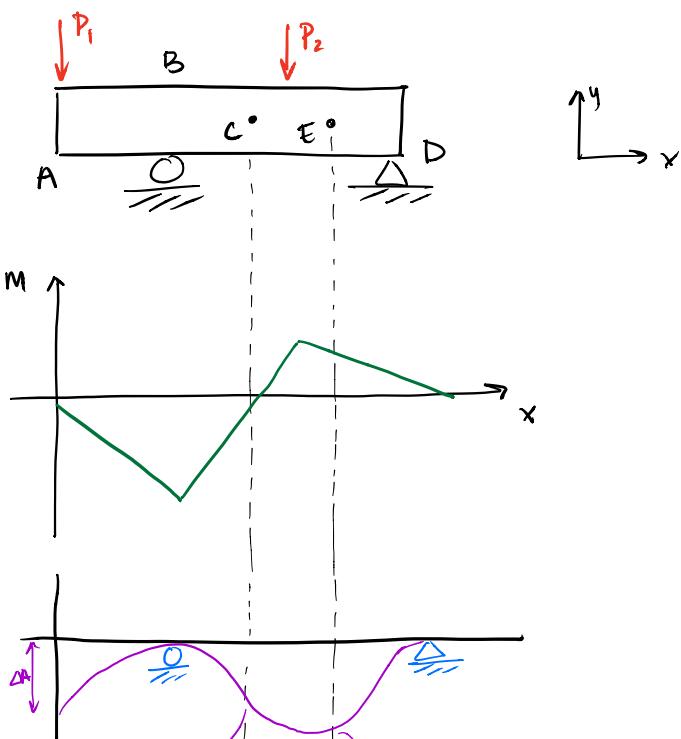
→ Goal: mathematically characterize deflection $v(x)$ as function of internal shear or moment



- positive internal moments
- concave up



- negative internal moments
- concave down

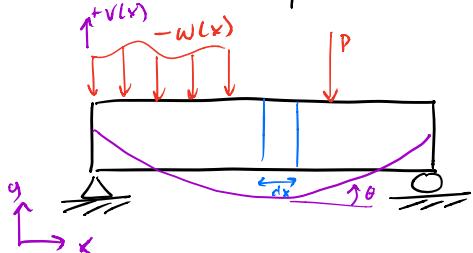


inflection pt
 where curve
 changes
 concavity
 moment = 0

slope of "elastic curve" = 0

elastic curve = deflection θ .

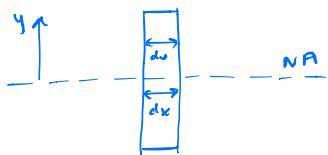
→ Develop relationship between internal moment in beam and radius of curvature ρ of elastic curve



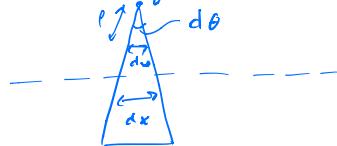
θ = slope of elastic curve

$v(x)$ = deflection at beam (positive up)

Before Deformation



after deformation



We want to find strain in ds : $\epsilon = \frac{ds' - ds}{ds}$

$$ds = dx = \rho d\theta$$

$$ds' = (\rho - y)d\theta$$

$$\epsilon = \frac{ds' - ds}{ds} = \frac{(\rho - y)d\theta - \rho d\theta}{\rho d\theta}$$

$$\hookrightarrow \frac{1}{\rho} = -\frac{\epsilon}{y}$$

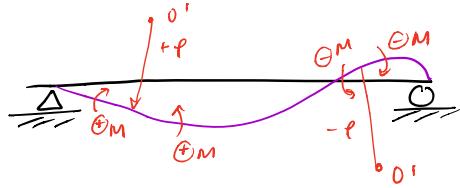
assume satisfied Hooke's Law, so it's a linear elastic material

$$\epsilon = \frac{\sigma}{E} \quad \text{flexure formula } \sigma = -\frac{My}{I}$$

$$\epsilon = -\frac{My}{EI} \quad \text{then} \quad \frac{1}{\rho} = \frac{M}{EI} \quad EI = \text{"flexural rigidity"} > 0$$

... this tells us that sign of ρ (radius of curvature) depends on sign of moment M

graphically, this means



... turn this into an equation of elastic curve deflection: $v(x)$

$$\frac{1}{\rho} = \frac{M}{EI} = \frac{\frac{d^2v}{dx^2}}{\left[1 + \left(\frac{dv}{dx}\right)^2\right]^{\frac{3}{2}}} \quad \leftarrow \text{from calculus}$$

Define $\frac{dv}{dx} = \theta$ = slope of elastic curve

→ assume $\frac{dv}{dx}$ is small → then if square small number,
 $\frac{d^2v}{dx^2} \approx 0$

so then $\frac{d^2v}{dx^2} = \frac{M}{EI}$

take derivative of both sides

$$\frac{d}{dx} \left(EI \frac{d^2v}{dx^2} \right) = \frac{dM}{dx} = v(x) \quad \leftarrow \text{shear}$$

$$\frac{d^2}{dx^2} \left(EI \frac{d^2v}{dx^2} \right) = \frac{d^2M}{dx^2} = \frac{d^2w}{dx^2} = w(x) \quad \leftarrow \text{distributed load}$$

assume EI (flexural rigidity) is constant then

$EI \frac{d^4v}{dx^4} = w(x)$
$EI \frac{d^3v}{dx^3} = v(x)$
$EI \frac{d^2v}{dx^2} = M(x)$

three differential equations

this implies that need to integrate $w(x)$ four times to get $v(x)$ and then need four constants of integration

need to integrate $M(x)$ twice to get the deflection $v(x)$

need two constants of integration

get this from the boundary conditions of the problem

so we have three different ways to calculate the beam deflection (3 different differential equations)

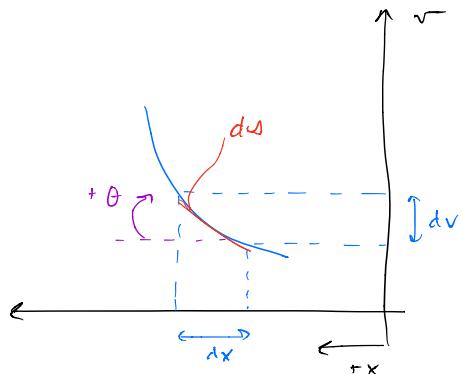
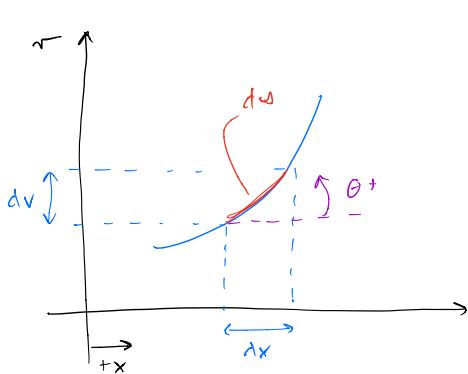
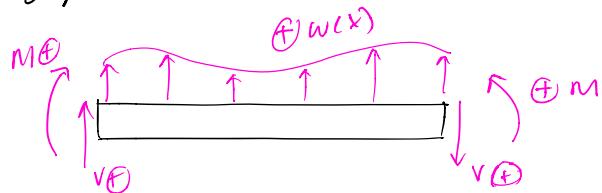
↳ need to pick most convenient way for problem that doing

↳ generally involves solving

$$EI \frac{d^2v}{dx^2} = M(x) \quad \text{because only need 2 boundary conditions to integrate only twice}$$

↓
then need to calculate internal moment diagram,
i.e. $M(x)$

Sign Convention



for both cases

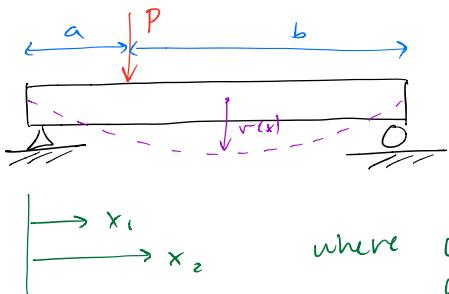
$$\begin{aligned} ds &= \sqrt{(dx)^2 + (dv)^2} \\ &= \sqrt{1 + (\dot{\theta})^2} dx \end{aligned}$$

$$ds = dx$$

→ implies that points on elastic curve displaced vertically and not horizontally

→ then elastic curve represents deflection of neutral axis

Continuity Conditions



where $0 \leq x_1 \leq a$
 $0 \leq x_2 \leq b$

2 differential equations to solve

$$EI \frac{d^2v_1}{dx^2} = M_1(x) \text{ and}$$

$$EI \frac{d^2v_2}{dx^2} = M_2(x)$$

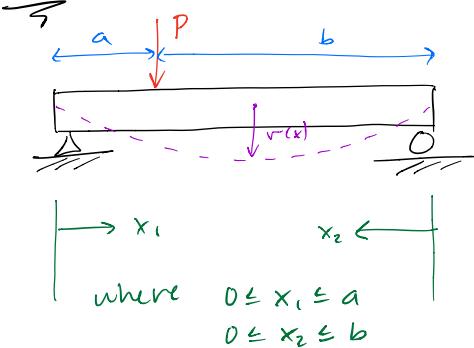
↑ we'll connect
these DE's using
continuity equations

remember
that $\theta = \text{slope}$

Continuity conditions:

$$\begin{aligned} \textcircled{1} \quad v_1(a) &= v_2(a) \\ \textcircled{2} \quad \theta_1(a) &= \theta_2(a) \end{aligned}$$

BUT!



where $0 \leq x_1 \leq a$
 $0 \leq x_2 \leq b$

Continuity Conditions:

$$\begin{aligned} \textcircled{1} \quad v_1(a) &= v_2(b) \\ \textcircled{2} \quad \theta_1(a) &= -\theta_2(b) \end{aligned}$$

slopes go in
different
directions bc
 θ_1 is \oplus ccw
and
 θ_2 is \oplus cw