

**A Weak Galerkin Finite Element Method for Parallel Solutions of Linear
Elasticity Problems on Unstructured Meshes**

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Dedication

I dedicate this work to my family, for their boundless love.

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I am deeply thankful for my advisor, Professor Chunlei Liang. His broad knowledge inspired me, his restless hardworking encouraged me, his constant advice enlightened me and his generous support helped me through my entire graduate study at the George Washington University. It is my best gratitude to have Dr. Junping Wang, a pioneer of weak Galerkin finite element method, as my co-advisor. His research contribution not only inspired a lot of researchers, but also created a vivifying community. He pointed a promising direction for my entire research life. I would like to thank to all other committee members, Prof. James Lee, Prof. Murli Gupta and Prof. Micheal Plesniak for their insightful advice on this piece of work. Many thanks for my lab officemates and best friends: Junfeng, Bin, Jingjing, Xiaoliang, Mao, Zihua, Zhen and Rongguang. Additional thanks to MAE department for the long-term support.

Abstract

A Weak Galerkin Finite Element Method for Parallel Solutions of Linear Elasticity Problems on Unstructured Meshes

In this paper, we present a novel parallel computing method on solving linear elasticity problems on unstructured meshes efficiently. The numerical method of our parallel computing scheme is based on the weak Galerkin (WG) finite element method which has been recently developed by Dr. Junping Wang and Dr. Xiu Ye. The weak Galerkin finite element method refers to a general finite element method on tackling a variety of partial differential equations. The main feature is that the differential operators are discretized and replaced by weak operators. Through weak functions, a connection between weak operators and unknown variables is reconstructed by solving the elemental matrices for each element. The key idea of utilizing WG method for solving linear elasticity equation is by converting the weak strain and stress tensors to the weak operators from employing the concept of discrete weak gradients.

A long-range goal of this research is to solve large-scale fluid structure interaction problems on parallel computers. For the purpose of solving large-scale fluid structure interaction problems, we will adopt a partitioned approach. In this approach, the governing equations for fluid and the displacement of the structure are calculated by two different solvers. Since an in-house fluid solver for Naiver-Stokes equations has been well-established, now we present an accurate and efficient solid solver which is capable on solving linear elasticity in parallel fashion.

To achieve high efficient computational process for structural dynamics, the non-overlapping domain decomposition is introduced to empower the WG method. We present two different approaches to implement the parallel computing. Initially, we combine the classic continuous Galerkin (CG) finite element with weak Galerkin (WG) finite element method together and result in a hybrid element. The hybrid element

inherits the discontinuous feature from WG method and the computational efficiency from CG method. After the Schur complement method, the accuracy and scalability have been tested. The more advanced approach is to implement the duality concept to split the computational space into primal and dual spaces. The connection of adjacent subdomains is implemented through the balancing domain decomposition with constraints (BDDC) which is originally proposed by Mandel[51]. Locally over each subdomain, matrices are constructed for interior and interface quantities separately. A powerful preconditioner for interface problem is constructed in the parallel fashion through a global primal space and a local dual space. Therefore, such interface related quantities are passed over to their adjacent subdomains through inter-processor communication library (MPI). After the convergence of interface problem, the rest unknown variables are recovered locally.

This dissertation is structured as follow:

In Chapter 1, we introduce the background and objective of this dissertation. We provide the preliminaries which are necessary for the following presentation. We derive the bilinear form of second order elliptical equation and linear elasticity equation.

In Chapter 2, we discuss the weak Galerkin finite element method and the bilinear form of linear elasticity equation. The WG finite element method is based on the variational form of equations. It is compatible with general polygons for finite element computational domain. The stiffness matrix derived from the WG method is symmetric and positive definite. Due to the flexibility of the polynomials basis functions, it's convenient to obtain high order accurate solutions. The convergence rate for the WG method is bounded by the lowest order.

In Chapter 3, we design a novel parallel computing method to solve linear elasticity equation. The key idea of the WG method for solving linear elasticity equation is to replace its gradients by discrete weak strain and stress operators. We develop a novel hybrid element which combines the elements of both weak Galerkin (WG)

finite element method and continuous Galerkin (CG) finite element method. The new hybrid element inherits the discontinuous feature of the WG method. In every hybrid element, we can insert an arbitrary number of CG elements in one single WG element. The hybrid element provides a second order of accuracy for both linear and nonlinear elastic equation. Meanwhile, the superlinear speedup is obtained.

In Chapter 4, we develop a novel parallel computing method to efficiently solve linear elasticity problems on unstructured meshes. To enable parallel computation, the computational grid is divided into arbitrary number of subdomains. The connection of adjacent subdomains is realized through the balancing domain decomposition with constraints (BDDC) which was originally proposed in [51]. Locally over each subdomain, matrices are constructed for interior and interface quantities separately. Consequently, interface related matrices are passed over to their adjacent subdomains through MPI libraries. MPI communications are employed to construct a smaller global preconditioner and distribute most computational effort to local processor. The novel WG-BDDC parallel algorithm achieves outstanding scalability up to 600 processors. Our numerical results also demonstrate that the WG-BDDC method obtained optimal order of accuracy for both 2nd-order and 3rd-order spatial discretization schemes. Moreover, the condition numbers for all test problems are well bounded.

In Chapter 5, We conclude the current stage and explore the future potential work.

Appendix A, we present computational fluid dynamics (CFD) simulations of the blood flow in stenotic peripheral arteries with idealized geometries. These arteries are typically simplified as axisymmetric constriction in straight tubes. The hydrodynamics of blood flow in these arteries are modeled using unsteady incompressible Navier-Stokes equations. The 3D computational domain is represented by unstructured meshes with all hexahedral elements. An efficient pressure-based Finite Volume

Method(FVM)[45] was implemented to solve these equations. Our simulations include computational geometries with a wide range of narrowing degrees of stenoses, from 40% to 80% luminal area reduction. The number of stenoses ranges from 1 to 7. Several different spatial intervals were considered between adjacent stenoses. The upstream flow condition was implemented by using measured peripheral artery flow which at the Reynolds number of 500.

Keyword : weak Galerkin, finite element method, parallel computing, linear elasticity, message passing interface, continuous Galerkin, domain decomposition, balancing domain decomposition by constraints, polygonal meshes

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Chapter 1: Introduction

1.1 Backgrounds

1.1.1 Engineering Background

Peripheral artery disease (PAD) is a major cause of amputation in United States. It is prevalent among smokers, diabetics and patient with dyslipidemia. Meanwhile, the long waiting time and high expense are two major problems for the patients. In the modern clinic room, the doctors are actively looking for a solution which can provide both high fidelity and high resolution images to analyze the PAD. The present technology can only render a two-dimensional, monochrome, static and low-resolution picture. In recent decades, many researchers contribute numerous effort on improving the diagnosis of stenoses and the quality of images[13, 60, 78, 67, 48]. With the fast development on both hardware and computing algorithm, the computational simulations are aiming to provide 3D, dynamic, high resolution and patient-specific results. Comparing to current CT scan, it's significantly faster, cheaper and more accurate. In addition, it's also very important that the simulation results can bring the doctors and patients that the dynamic growth animations of current stenoses and the developing path after the clinic treatment. Fig 1.1 summarizes some factors which contributing to the interests in computational medical simulation technology[3]. Many investigations have been conducted through last decades[27, 6, 59, 62]. However, since this is a fluid-structural interaction (FSI) problem, both high fidelity fluid and solid solvers are needed.

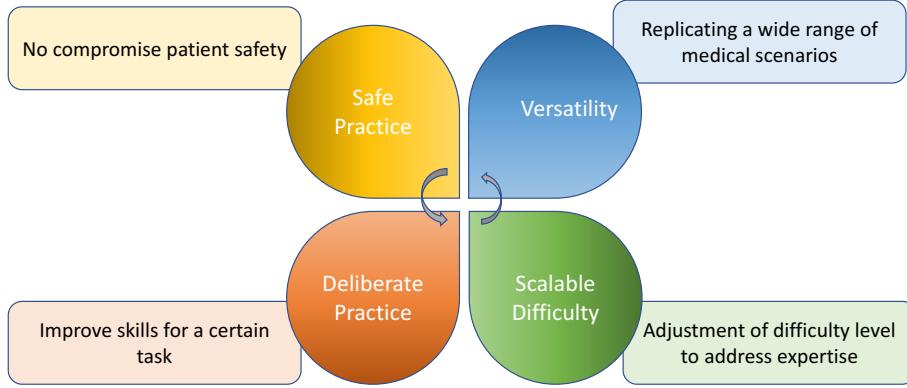


Figure 1.1: Different factors affecting computer-based medical simulation.

Physically, the PAD can be abstracted as a fluid-structural interaction problem and be described by fluid and structure dynamics equations, respectively. The two most popular approaches for solving such a FSI problem are monolithic method[35, 17] and partitioned method[42, 73]. For the monolithic approach, the two sets of equations, fluid and structural, are solved simultaneously. The mutual influence on each other can be considered directly. One significant advantage of this scheme is its simplicity. Only a single global matrix is constructed so that both the fluid and structural parts can be solved under the same spatial discretization and time marching scheme. On the other hand, we lose the flexibility to separately control each partition. The other approach is partitioned method. The two sets of equations are solved separately and pass boundary conditions to each other like a cycle. The solution of fluid equations is calculated while the structural part waits for new input, and vice versa. A coupling algorithm to exchange the interaction solution between two phases as a pair of modules. The Implicit-Explicit (IMEX) Runge-Kutta (RK) time integration approach has been proved to be accurate and efficient on coupling the

high order schemes[81].

A Finite-Volume fluid solver [45, 45, 47] is accomplished and a series of CFD studies has been implemented on the idealized geometries. To implement a more high-fidelity simulation, the tissue of blood vessel wall shall be considered as elastic material. Therefore, an accurate and efficient solver for elasticity equation is needed. The solver should be capable of calculating the material behavior according to the stiffness property, which is the response of the deformable model reacting to the external forces. The simplest model is mass-spring model which is easy to compute but can not accurately determine the details of material behavior. A more precise approach is the finite element method, which is based on the continuum mechanics, has gained popularity. On the contrary to the mass-spring model, the FEM can specify the stiffness of the model by only using a few characteristic parameters, such as Young's modulus, Poisson ratio and the geometries. However, the challenge for 3D FEM simulation is its parallelization in large-scale parallel computational fluid-structure interaction frameworks is to be used in real-time application due to the very high computational expense.

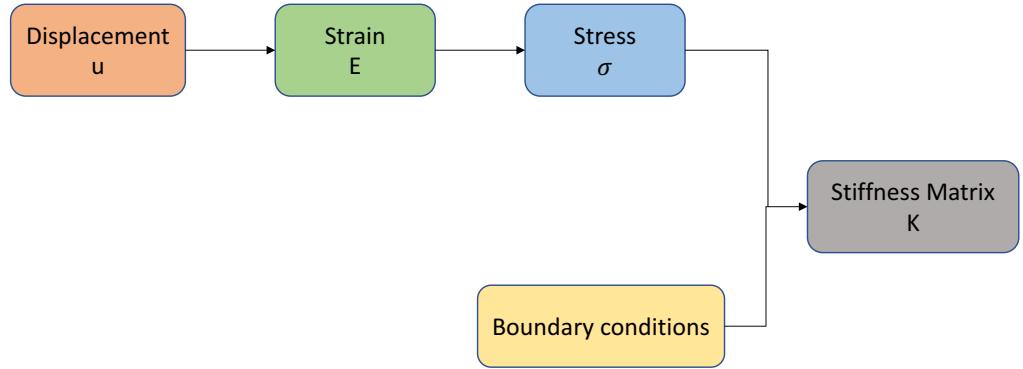


Figure 1.2: Construct stiffness matrix.

Fig 1.2 shows a general process of calculating the stiffness matrix for a deformable material object. The stiffness matrix is derived from the gradient of stress which is calculated by the displacement and material property. The boundary conditions are imposed upon the contact interaction.

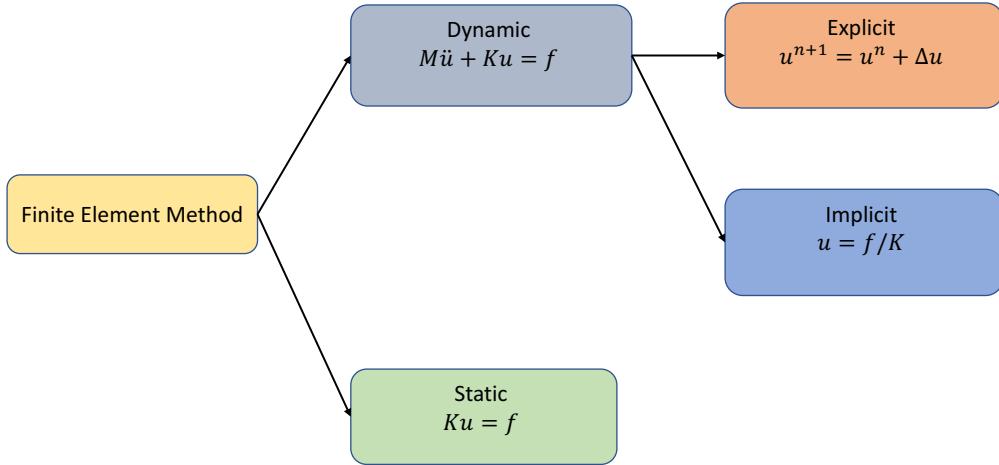


Figure 1.3: Static and dynamic finite element method.

The Fig 1.3 displays both static analysis and dynamic analysis for a set of linear system of elasticity equations. The steady-state and transient finite element problem are studied with static analysis and dynamic analysis respectively. The latter is time dependent. In dynamic analysis, we shall consider the spatial discretization by using both explicit and implicit time integration schemes[4]. The implicit time marching scheme is computationally costly, because the matrix inversion for large linear system is required every time-step, similar to solving the static problem. The benefit is unconditionally stable for large time step. On the other hand, the explicit time marching scheme requires much less computational cost as no linear solver is involved for each time-step. However, the size of each time step has to satisfy the numerical stability criteria.

Commonly, the linear system of elasticity equations derived from the FEM method is positive definite and too large to be direct inverse. The most time consuming part in the FEM analysis in Fig 1.2 is solving the linear system. The complex system

can be written in a matrix form $\mathbf{Ax} = \mathbf{b}$. To solve it, there are two main categories of algorithms, directive method and iterative method. Considering the better performance on memory usage and computing time, the iterative method gains more affirmatives[11] for large scale problems. The Preconditioned Conjugate Gradient (PCG) is the one of the most popular method because of its robustness and fast convergence. However, the PCG requires positive definite matrices. It is still very challenging to implement finite element analysis using implicit scheme to study the material real-time response behavior because of excessive computational cost. Therefore, the domain decomposition method combined with parallel computing scheme is one emerging choice to reduce computational cost.

Parallel computing desires great load balancing in order to achieve competitive concurrent execution in terms of computational time. The challenges to reach that purpose includes learning and understanding programming paradigms, balance the loads to maximize the usage of bandwidth, minimize the overhead on data communication and avoid potential data racing problems. To achieve good level of parallelism, we employ some modern techniques such as MPI, Intel Math-Kernel library and LAPACK-BLAS for optimal scalability by reducing the overhead latency with a large number of active processors and ensure the balance of loads on each processor.

1.1.2 Overview of Related Numerical Methods

The Weak Galerkin Finite Element Method (WG-FEM) is a novel developed, effective numerical method for solving partial differential equations. The WG method is first proposed by Dr. Junping Wang and Xiu Ye in 2011, then applied for solving second order elliptic equations[76]. The WG method introduces a series of weak operators such as weak gradient, weak divergence and weak curl operators for the computation of corresponding strong forms of differential equations. The WG finite element method provides a new perspective to solve numerical problems. The WG method can be applied on variety of partial differential equations include second order

elliptical equation, elasticity equation[74], Stokes equations [77] and Maxwell's equations [55], etc. The WG method inherits the discontinuity from the continuous basis functions per element. The construction of discrete matrices are independent with of external coefficients. The interior unknown variables can be projected to the boundary unknown variables to construct a linear system which only consists of degrees of freedom(DOFs) along the element boundary. In short, even though the total DOFs of WG method is larger than the Discontinuous Galerkin method (DG), the interior unknown variables can be eliminated through Schur complement method. The only remaining DOFs are along the element boundary which are far less DOFs than those in DG finite element method. The elemental stiffness matrix of WG method can be constructed for each element, which is consistent with CG FEM method.

1.2 Objective

The CG FEM analysis of continuum mechanics is widely used to study and predict material deformations. The results are generally accurate and reliable for analyzing the relationship between load and deformation. However, the computational complexity and cost prevents its the application to large-scale FSI problems. Massive parallel computing is required for obtaining large linear system. In this work, we investigate the methods for efficient parallel scheme which enables high-order novel finite element method for solving large scale elasticity equation.

We employ the WG-FEM to convert the bilinear form equations into a positive definite linear system. The WG method can use various types of different elements. The selection of polynomials on interior or boundary region is flexible which enhances the freedom of computational simulation for dealing with complex geometries. Moreover, the discontinuous features in the WG method which empowers the parallel computation capability.

The Balancing Domain Decomposition by Constraints (BDDC) method is an ideal candidate for enabling the massive parallel capacity to WG-FEM. BDDC identifies the

two spaces (primal and dual) and apply the divide and conquer strategy to the original computational domain. By minimizing the global communication and synchronization overhead, WG-BDDC empowers the scalability of elasticity equations as a superlinear speedup curve.

The main objective of this dissertation is to deliver high speed, accuracy and scalability in WG-BDDC based analysis. Fig 1.4 shows the paths to accomplish the objectives in this work. The results of the research significantly improves the computation of the elastic material response, including in clinic judgment and other medical applications.



Figure 1.4: Objectives and methodology.

To validate the WG-BDDC scheme and develop the software, we first design the hybrid WG-CG element. The results are consistent with the benchmark of CG only element so the fidelity is proved. Then we extend the WG element to BDDC method and verify the properties of convergence and the order of accuracy with parallel fashion. Finally, the scalability is discussed under the parallel computing framework.

The rest of this dissertation is organized as :

- Chapter 2 presents the background and basics of WG method.
- Chapter 3 discusses the hybrid WG-CG element the performance on nonlinear elasticity equation.
- Chapter 4 introduces the WG-BDDC method for second order elliptic and elasticity equation.
- Chapter 5 shows the study of effect multiple stenoses on the peripheral artery diseases.
- Chapter 6 concludes this work and outlook the future work.

Chapter 2: Numerical Method

2.1 Mathematical Models and Numerical Method

2.1.1 Linear Elastic Equation and Weak Galerkin Method

Consider an elastic body subject to an exterior force \mathbf{f} , we denote the computational domain as Ω and its continuous boundary as $\Gamma = \partial\Omega$. The governing equation for linear elasticity can be written as

$$\nabla \cdot \sigma(\mathbf{u}) = \mathbf{f}, \quad \text{in } \Omega \quad (2.1)$$

$$\mathbf{u} = \hat{\mathbf{u}}, \quad \text{on } \Gamma \quad (2.2)$$

where $\sigma(\mathbf{u})$ is the symmetric Cauchy stress tensor. For linear, isotropic and homogeneous materials, the stress-strain relation is

$$\sigma(\mathbf{u}) = 2\mu\varepsilon(\mathbf{u}) + \lambda(\nabla \cdot \mathbf{u})\mathbf{I} \quad (2.3)$$

where $\varepsilon(\mathbf{u}) = \frac{1}{2}(\nabla \mathbf{u} + \nabla \mathbf{u}^T)$, μ and λ are Lame indices which can be written as

$$\lambda = \frac{E\mu}{(1+\mu)(1-2\mu)} \quad (2.4)$$

$$\mu = \frac{E}{2(1+\mu)} \quad (2.5)$$

where E is the elasticity modulus and μ is the Poisson's ratio.

The weak function on the domain is $\mathbf{u} = \{\mathbf{u}_0, \mathbf{u}_b\}$, $\mathbf{u}_0 \in L^2(T)$. The first function \mathbf{u}_0 represents the interior domain of the function \mathbf{u} . The second function \mathbf{u}_b represents the value of function \mathbf{u} on the boundary of domain T . The key notion

is that for two function \mathbf{u}_0 and \mathbf{u}_b are independent with each other along . The weak function is defined as

$$V_h = \{\mathbf{v} = \{\mathbf{v}_0, \mathbf{v}_b\} : \mathbf{v}_0 \in P_j(T^0), \mathbf{v}_b \in P_l(e), e \subset \partial T\} \quad (2.6)$$

The key of the weak Galerkin method is to approximate the solution in the weak discrete space $S(T)$. The discrete weak gradient $\nabla_w \mathbf{u} \in [P_r(T)]^d$ for $\mathbf{u} \in V_h$ on each element T :

$$(\nabla_w \mathbf{u}, \mathbf{q})_T = -(\mathbf{u}_0, \nabla \cdot \mathbf{q})_T + \langle \mathbf{u}_b, q \cdot \mathbf{n} \rangle_{\partial T} \quad (2.7)$$

For the discrete weak divergence, $\nabla_w \cdot \mathbf{u} \in [P_r(T)]^d$ is defined

$$(\nabla_w \cdot \mathbf{u}, \mathbf{q})_T = -(\mathbf{u}_0, \nabla \cdot \mathbf{q})_T + \langle \mathbf{u}_b \cdot \mathbf{n}, q \rangle_{\partial T} \quad (2.8)$$

Then we can define the weak strain tensor by using the weak gradient

$$\varepsilon_w(\mathbf{u}) = \frac{1}{2}(\nabla_w \mathbf{u} + \nabla_w \mathbf{u}^T) \quad (2.9)$$

Analogously, we can define the weak stress tensor as

$$\sigma_w(\mathbf{u}) = 2\mu\varepsilon_w(\mathbf{u}) + \lambda(\nabla_w \cdot \mathbf{u})\mathbf{I} \quad (2.10)$$

The bilinear form of governing equation of continuous Galerkin method is following

$$a(\mathbf{u}, \mathbf{v}) = (\mathbf{f}, \mathbf{v}) \quad (2.11)$$

The above approximation function \mathbf{u} and gradient $\nabla \mathbf{u}$ is not well defined for the discontinuous feature of weak Galerkin method, the new form is

$$a(\mathbf{u}_w, \mathbf{v}_w) + s(\mathbf{u}, \mathbf{v}) = (\mathbf{f}, \mathbf{u}) \quad (2.12)$$

the term $s(\mathbf{u}, \mathbf{v})$ is a stabilizer enforcing a weak continuity which measures the discontinuity of the finite element solution. The governing equation in weak form can be introduced by two bilinear equations

$$s(\mathbf{u}, \mathbf{u}) = \sum_{T \in \Omega}^N h_T^{-1} \langle Q_b \mathbf{u}_0 - \mathbf{u}_b, Q_b \mathbf{v}_0 - \mathbf{v}_b \rangle_{\partial T} \quad (2.13)$$

where Q_b is the projection from the interior unknown variables to boundary unknown variables. Commonly it is taken as 1. The discrete bilinear equation has the assembly form

$$a(\mathbf{u}_w, \mathbf{u}_w) = \sum_{T \in \Omega}^N 2(\mu \varepsilon_w(\mathbf{u}), \varepsilon_w(\mathbf{v}))_T + \sum_{T \in \Omega}^N (\lambda \nabla \cdot \mathbf{u}, \nabla_w \cdot \mathbf{v})_T \quad (2.14)$$

2.2 Existing Numerical Methods Review

In this section, we present and analyze several most widely used numerical method to solve finite element problems. The details of each method are presented in the following subsections.

2.2.1 Classic Continuous Galerkin Finite Element Method

Back to 1950s and 1960s, finite element method is arised to solve complex elasticity and structural analysis engineering problems in mechanical and aeronautical field. A. Hrennikoff[34], R. Courant[14] and K.Feng are the earliest pioneers who established this subject. FEM is then proposed as a systematic numerical method to solve variety of partial differential equations. The core characteristic of FEM is that it employs mesh discretization to divide a continuous computational domain so that a big problem is then converted to a set of discrete small problems. That is the source of finite element. Each element represents a small piece of computational sub-domain.

Finite element method is an efficient solution to solve partial differential equations.

The core idea is to convert the original partial differential equation to a equivalent bilinear form weak function. Then we partition the computational domain into polygon meshes. In each mesh element, we construct the finite element space. The bilinear form is discretized into a summation of finite elemental spaces. The solution is approximated based on the calculation of assembled matrix. More details can be found in [82, 12, 36, 64].

The variational formulation which generated by the governing equation determines the characteristic of the finite element method. To obtain the variational form, mathematicians derived several different paths such as Galerkin method, the discontinuous Galerkin method, mixed method, etc. In this chapter, we introduce two most popular method, DG and MFEM. The WG method is inspired from these two method and shares many similarities with them.

2.2.2 Discontinuous Galerkin Finite Element Method

We shall go through a simple example to review the common features of FEM and the new feature of DG-FEM.

We have a 1-dimensional convection equation with domain $[0, \pi]$

$$a \frac{du}{dx} = \cos(x) \quad (2.15)$$

the exact solution is

$$u = \frac{\sin(x)}{a} \quad (2.16)$$

Dirichlet boundary condition is applied on both end.

For the standard continuous Galerkin FEM, we shall first apply the basis function ϕ_i on each element. The solution becomes as

$$u(x) = \sum_{i=1}^N \phi_i(x) U_i \quad (2.17)$$

We integrate the governing equation and obtain the weak form

$$\int_{\Omega} a \frac{du}{dx} \phi_i dx = \int_{\Omega} \cos(x) \phi_i dx \quad (2.18)$$

Then we apply integration by parts

$$-\int_{\Omega} a u \phi_{i,x} dx + [au\phi_i]_{left}^{right} = \int_{\Omega} \cos(x) \phi_i dx \quad (2.19)$$

we can substitute the continuous unknown variable equation by

$$u(x) = \sum_{i=1}^N \phi_i(x) U_i \quad (2.20)$$

So we obtain the new governing equation in matrix form

$$-\sum_{i=1}^N U_i a \int_{\Omega} \phi_i \phi_{j,x} dx + BC = \int_{\Omega} \cos(x) \phi_j dx \quad (2.21)$$

the linear system is simplified as

$$\mathbf{Ax} = \mathbf{b} \quad (2.22)$$

The main distinct feature of DG-FEM is that there is no continuity constraint between element. In another word, the basis functions along the computational domain is discontinuous.

If we apply the original governing equation that

$$a \int_{\Omega} \frac{du}{dx} \phi_i dx = \int_{\Omega} \cos(x) \phi_i dx \quad (2.23)$$

after the discretization, we have the assemble form that

$$\sum_j \int_j a \frac{du}{dx} \phi_i dx = \sum_j \int_j \cos(x) \phi_i dx \quad (2.24)$$

then we apply the integration by parts

$$\sum_j \left\{ - \int_j a u \phi_{i,x} dx + [(au)\phi_i]_{j-\frac{1}{2}}^{j+\frac{1}{2}} \right\} = \sum_j \int_j \cos(x) \phi_i dx \quad (2.25)$$

for every element, we have the elemental form

$$- \int_j a u \phi_{j,x} dx + [(au)\phi_i]_{j-\frac{1}{2}}^{j+\frac{1}{2}} = \int_j \cos(x) \phi_j dx \quad (2.26)$$

We can find that a penalty term is applied on the matrix form. We have several methods to calculate it. Upwinding method is a common useful tool to calculate the u value on the interface.

Comparing to CG, DG method introduces additional DOFs to maintain the continuity. The main advantage of DG method is that we can easily introduce higher order interpolation to obtain higher order accuracy.

There are several good iterative schemes such as blocak Jacobi, Conjugate Gradient method are stable for higher order. Meanwhile, it's easy to parallelize the computational domain with domain decomposition scheme.

2.2.3 Mixed Finite Element Method

The mixed finite element method is a new type of FEM in which an extra independent variables are introduced which discretization of a partial differential equation. In another word, it is characterized by a variational equation. Usually, the extra variables are constrained by an external vector, Lagrange multiplier. The major difference between FEM and mixed FEM is that the extra independent variables. The mixed finite element method has an advantage on computing the elasticity equation

and the stress and strain field.

To illustrate the mixed FEM, a simple elliptic equation is

$$-\nabla \cdot (a \nabla u) = f \quad \text{in} \quad \Omega \quad (2.27)$$

$$u = 0 \quad \text{on} \quad \partial\Omega \quad (2.28)$$

we shall introduce an vector \mathbf{G} that

$$\mathbf{G} = -a \nabla u \quad (2.29)$$

the elliptic problem then can be decomposed into a first order linear system that

$$\mathbf{G} + a \nabla u = 0 \quad \text{in} \quad \Omega \quad (2.30)$$

$$\nabla \cdot \mathbf{G} = f \quad \text{in} \quad \Omega \quad (2.31)$$

$$u = 0 \quad \text{on} \quad \partial\Omega \quad (2.32)$$

we can rewrite the first equation as

$$a^{-1} \mathbf{G} + \nabla u = 0 \quad \text{in} \quad \Omega \quad (2.33)$$

then we apply the integration by parts

$$\int_{\Omega} a^{-1} \mathbf{G} \cdot \mathbf{v} d\Omega - \int_{\Omega} c \nabla \cdot \mathbf{v} d\Omega = 0 \quad \mathbf{v} \in H(\text{div}, \Omega) \quad (2.34)$$

$$\int_{\Omega} \psi \nabla \cdot \mathbf{G} d\Omega = \int_{\Omega} f \psi d\Omega \quad (2.35)$$

where

$$H(\text{div}, \Omega) = \{\mathbf{v} \in L^2(\Omega)^d : \nabla \cdot \mathbf{v} \in L^2(\Omega)\} \quad (2.36)$$

We use the Raviart-Thomas spaces to define a generic element $T \in \Omega$

$$RT_k = (P_k)^d + x P_k \quad (2.37)$$

where k is the order of polynomials degrees.

the variational function \mathbf{G} and u can be approximated by

$$\tilde{\mathbf{G}} = \sum_{i=1}^N g_i \mathbf{v}_i \quad (2.38)$$

$$\mathbf{u} = \sum_{i=1}^N u_i \psi_i \quad (2.39)$$

both \mathbf{v} and ϕ_i are vector basis functions. If we consider the RT_0 space,

$$\mathbf{v} = \begin{pmatrix} ax + b \\ ay + c \end{pmatrix} \quad (2.40)$$

for each element $T_i, (i = 1, 2, 3)$

$$v_i = \begin{pmatrix} a_i x + b_i \\ a_i y + c_i \end{pmatrix} \quad (2.41)$$

the above equation follows the Kronecker property [16]

$$\int_T \mathbf{v}_i \cdot \mathbf{n} dS = \int_T (ax + b)n_x + (ay + c)n_y dS \quad (2.42)$$

$$= a \int_{x_1}^{x_2} (x_1 n_x + y_1 n_y) \frac{1}{n_y} dx + \int_{x_1}^{x_2} (bn_x + cn_y) \frac{1}{n_y} dx \quad (2.43)$$

$$= a(x_1 n_x + y_1 n_y) \frac{x_1 - x_2}{n_y} + (bn_x + cn_y) \frac{x_1 - x_2}{n_y} \quad (2.44)$$

$$= \delta_{ij} \quad (2.45)$$

Then we apply the divergence theorem, we have

$$\int_T \nabla \cdot \mathbf{v} dx dy = \int_T \left(\frac{\partial \mathbf{v}}{\partial x} + \frac{\partial \mathbf{v}}{\partial y} \right) dS \quad (2.46)$$

$$= \int_T 2adS \quad (2.47)$$

$$= 2a|T_i| \quad (2.48)$$

2.3 Weak Galerkin Finite Element Methods

2.3.1 Preliminary

In this section, all computation is in Sobolev space[9, 12]. The Lipschitz boundary is in open area $D \subset R^d$, $d = 2, 3$. The inner product is defined as

$$|v|_{s,D} = \left(\sum_{|\alpha|=s} \int_D |\partial^\alpha v|^2 dD \right)^{1/2} \quad (2.49)$$

where

$$\alpha = (\alpha_1, \dots, \alpha_d) \quad (2.50)$$

$$|\alpha| = \alpha_1 + \dots + \alpha_d \quad (2.51)$$

The definition of divergence is defined by

$$H(\text{div}; D) = \{\mathbf{v} : \mathbf{v} \in [L^2(D)]^d, \nabla \cdot \mathbf{v} \in L^2(D)\} \quad (2.52)$$

the norm is defined as

$$\|\mathbf{v}\|_{H(\text{div}, D)} = (\|\mathbf{v}\|_D^2 + \|\nabla \cdot \mathbf{v}\|_D^2)^{1/2} \quad (2.53)$$

the curl of $H(\text{curl}; D)$ in $L^2(D)$ is defined as

$$H(\text{curl}; D) = \{\mathbf{v} : \mathbf{v} \in [L^2(D)]^d, \nabla \times \mathbf{v} \in L^2(D)\} \quad (2.54)$$

the norm of it is defined as

$$\|\mathbf{v}\|_{H(\text{curl}, D)} = (\|\mathbf{v}\|_D^2 + \|\nabla \times \mathbf{v}\|_D^2)^{1/2} \quad (2.55)$$

2.3.2 Weak operators

Let's assume that $T \subset R^d$ is an arbitrary polygon domain, the boundary is ∂T . The weak function in the domain is $v = \{v_0, v_b\}$, so that $v_0 \in L^2(T)$ and $v_b \in L^2(\partial T)$. The v_0 represents the unknown variable vector belongs to the interior domain, and v_b represents the unknown variable vector along the boundary of T . The keynote is that the v_0 is independent with v_b on ∂T . The weak space for all T is $S(T)$

$$S(T) = \{v = \{v_0, v_b\} : v_0 \in L^2(T), v_b \in L^2(\partial T)\} \quad (2.56)$$

Now we define some commonly used weak operators

The weak gradient operator, for any $v \in S(T)$, the weak gradient of v is $\nabla_w v$

$$\langle \nabla_w v, \mathbf{q} \rangle_T = -(v_0, \nabla \cdot \mathbf{q})_T + \langle v_b, \mathbf{q} \cdot \mathbf{n} \rangle_{\partial T} \quad (2.57)$$

the discrete weak gradient operator is $\nabla_{w,r}v$

$$(\nabla_{w,r}v, \mathbf{q})_T = -(v_0, \nabla \cdot \mathbf{q})_T + \langle v_b, \mathbf{q} \cdot \mathbf{n} \rangle_{\partial T} \quad (2.58)$$

where

$$V(T) = \{\mathbf{v} = \{\mathbf{v}_0, \mathbf{v}_b\} : \mathbf{v}_0 \in [L^2(T)]^d, \mathbf{v}_b \in [L^2(\partial T)]^d\} \quad (2.59)$$

The weak divergence operator, for any $v \in S(T)$, is $\nabla \cdot \mathbf{v}$

$$\langle \nabla_w \cdot \mathbf{v}, \varphi \rangle_T = -(\mathbf{v}_0, \nabla \varphi)_K + \langle \mathbf{v}_b \cdot \mathbf{n}, \varphi \rangle_{\partial T} \quad (2.60)$$

the discrete weak divergence operator is

$$(\nabla_{w,r,T} \cdot \mathbf{v}, \varphi)_T = -(\mathbf{v}_0, \nabla \varphi)_T + \langle \mathbf{v}_b \cdot \mathbf{n}, \varphi \rangle_{\partial T} \quad (2.61)$$

the curl operator , for any $v \in S(T)$, is $\nabla_w \times \mathbf{v}$ which is defined as

$$\langle \nabla_w \times \mathbf{v}, \varphi \rangle_T = (\mathbf{v}_0, \nabla \times \varphi)_T - \langle \mathbf{v}_b \times \mathbf{n}, \varphi \rangle_{\partial T} \quad (2.62)$$

the discrete weak divergence operator is

$$(\nabla_{w,r,T} \times \mathbf{v}, \varphi)_T = (\mathbf{v}_0, \nabla \times \varphi)_T - \langle \mathbf{v}_b \times \mathbf{n}, \varphi \rangle_{\partial K} \quad (2.63)$$

2.3.3 Second order elliptic equation in WG

In this section, we use the weak operators to solve second order elliptic equation.

In domain Ω , we have the equation in form

$$-\nabla \cdot (a \nabla u) = f \quad (2.64)$$

Considering the Dirichlet boundary condition, we have

$$u = -g, \quad \text{on} \quad \partial\Omega \quad (2.65)$$

For Neumann boundary condition, we have

$$(a\nabla u) \cdot \mathbf{n} = -g, \quad \text{on} \quad \partial\Omega \quad (2.66)$$

The primal formulation for Dirichlet boundary condition is

$$(a\nabla u, \nabla v) = (f, v) \quad (2.67)$$

for Neumann boundary condition

$$(a\nabla u, \nabla v) = (f, v) - \langle g, v \rangle_{\partial\Omega} \quad (2.68)$$

The Primal-Mixed formulation is to make $\mathbf{q} = -a\nabla u$, the elliptic equation is

$$a^{-1}\mathbf{q} + \nabla u = 0, \quad (2.69)$$

$$\nabla \cdot \mathbf{q} = f. \quad (2.70)$$

we choose assistant function $\mathbf{p} \in [L^2(\Omega)]^d$, the bilinear form is

$$(a^{-1}\mathbf{q}, \mathbf{p}) + (\nabla u, \mathbf{p}) = 0; \quad (2.71)$$

for any $v \in H_0^1(\Omega)$

$$(\mathbf{q}, \nabla v) = -(f, v) \quad (2.72)$$

For Dirichlet boundary condition, $u \in H^1(\Omega)$, $\mathbf{q} \in [L^2(\Omega)]^d$, so that we enforce the boundary condition $u = -g$ on $\partial\Omega$

$$(a^{-1}, \mathbf{q}, \mathbf{p}) + (\nabla u, \mathbf{p}) \quad (2.73)$$

$$(\mathbf{q}, \nabla v) = -(f, v), \quad (2.74)$$

for Neumann boundary condition

$$(a^{-1}\mathbf{q}, \mathbf{p}) + (\nabla u, \mathbf{p}) = 0, \quad (2.75)$$

$$(\mathbf{q}, \nabla v) = \langle g, v \rangle_{\partial\Omega} - (f, v), \quad (2.76)$$

Let's define the weak function on each element space

$$S(k, T) = \{v = \{v_0, v_b\} : v_0 \in P_k(T), v_b|_e \in P_k\} \quad (2.77)$$

where k is the order of polynomial for each function, and e is the boundary edge of each element.

The weak space S_h is defined as

$$S_h = \{v = \{v_0, v_b\} : v|_T \in S(k, T), v_b|_{\partial T}\} \quad (2.78)$$

for each element T , we use Q_0 to represent the L^2 projection from L^T to $P_k(T)$ and Q_b is the L^2 projection from $L^2(T)$ to $P_k(\partial T)$. The local weak discrete space Q_h is

$$Q_h v = \{Q_0 v_0, Q_b, v_b\} \quad (2.79)$$

in space S_h we have the bilinear form

$$a(u, v) = \sum (a \nabla_w u, \nabla_w v)_T \quad (2.80)$$

To ensure the continuity between each WG element, a stabilizer is introduced. The stabilizer describes the difference of boundary values and the interior value which projected on boundary. Since the interior space and boundary space is independent and the basis functions are different so that the result values are different.

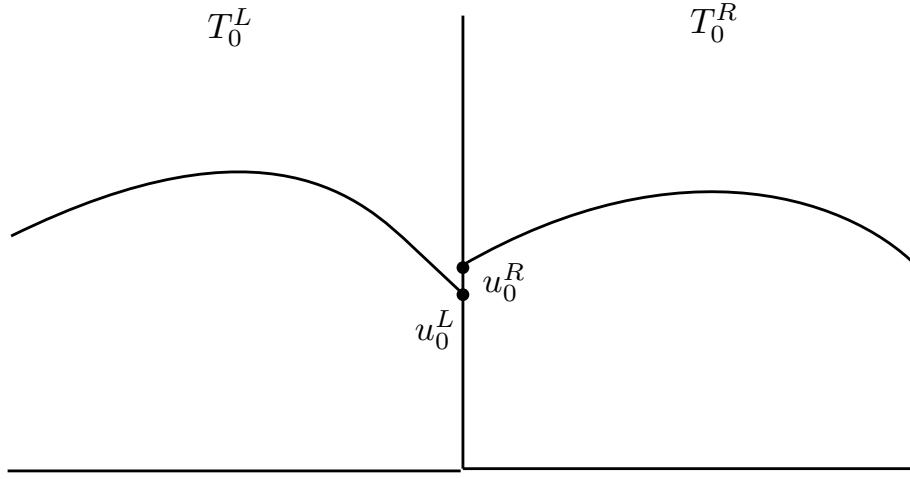


Figure 2.1: 1-D interior basis functions of two adjacent elements.

$$s(u, v) = \rho \sum h_T^{-1} \langle Q_b u_0 - u, Q_b v_0 - v \rangle_{\partial T} \quad (2.81)$$

where ρ is commonly set as 1, and h is the characteristic length of each element. The governing equation is

$$a_s(u, v) = a(u, v) + s(u, v) \quad (2.82)$$

2.3.4 Weak Galerkin triangular meshes

Consider triangular element linear type basis function for both interior and boundary subspaces $P_1(T)/P_1(\partial T)$

$$\phi_k = \{\lambda_k, 0\}, \quad k = 1, 2, 3 \quad (2.83)$$

$$\phi_{3+l} = \{0, \mu_l\}, \quad l = 1, 2, \dots, 2N \quad (2.84)$$

where N is the number of element boundaries.

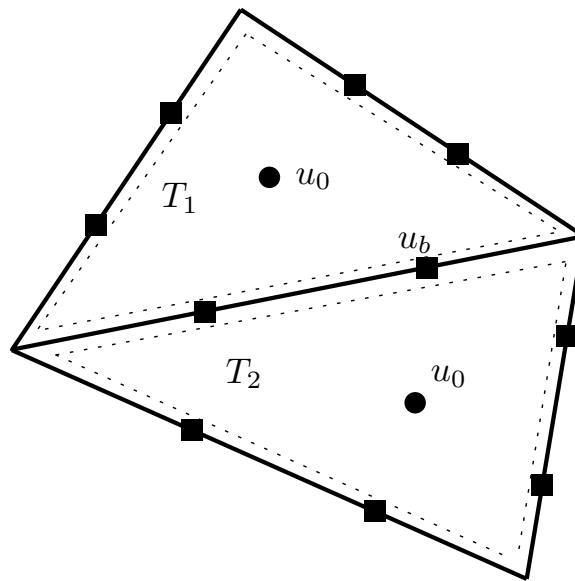


Figure 2.2: Weak Galerkin triangular elements and solution points.

In this section, we present a triangular WG element with linear interior and boundary space. There are three interior basis functions represent the x, y and xy respectively.

2.3.5 Weak Galerkin quadrilateral meshes

Consider triangular element linear type basis function for both interior and boundary subspaces $Q_1(T)/Q_1(\partial T)$

$$\phi_k = \{\lambda_k, 0\}, \quad k = 1, 2, 3 \quad (2.85)$$

$$\phi_{3+l} = \{0, \mu_l\}, \quad l = 1, 2, \dots, 2N \quad (2.86)$$

where N is the number of element boundaries.

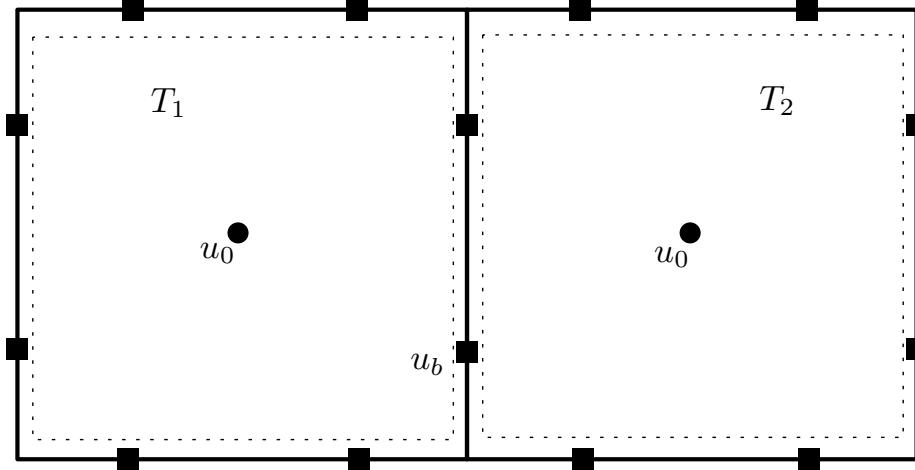


Figure 2.3: Weak Galerkin quadrilateral elements and solution points.

In this section, we present a quadrilateral WG element with linear interior and boundary space. There are three interior basis functions represent the x, y and xy respectively. Meanwhile, there are eight boundary basis functions represents six solution points based on the location of Gaussian points.

Chapter 3: Hybrid Weak Galerkin and Continuous Galerkin Finite Element Method

In this chapter, we present a novel parallel computing method to effectively solve linear and nonlinear elasticity equations. The WG finite element method is newly developed by Junping Wang and Xiu Ye [75]. The core idea of the WG method for solving linear elastic equation is to replace its gradients after the integration by parts by discrete weak strain and stress tensors. We develop a novel hybrid element which combines the elements of both WG and CG finite element. The new hybrid element inherits the discontinuous feature of the WG method. To create a hybrid element, we insert an arbitrary number of CG elements in one single WG element. The hybrid element provides a second order of accuracy for both linear and nonlinear elastic equation. The superlinear speedup is observed.

The objective of this chapter is to explore a new efficient path to the massively parallel computing method with domain decomposition scheme. Specifically, this combination involves a discontinuous weak Galerkin finite element method[75, 44, 76] and implement a hybrid element which enable multiple continuous Galerkin finite elements inserted into the WG element. The WG method is considered as a newly developed robust numerical method and inherits the locking-free feature for the linear elastic equation[74].

Like the standard FEM, the WG method can be used to solve generic partial differential equations. An advanced feature of the WG finite element method is that the entire problem can be decomposed into multiple local problems. In these local problems, the differential operators are approximated and reconstructed by small-size matrices. The WG method is proven robust and has optimal orders of accuracy in spatial discretization for difficult problems on serial computers. However, the performance of the WG method on parallel computers has not yet been examined.

This chapter is organized as following. We introduce the weak Galerkin method and the derivations of the bilinear form of nonlinear elasticity equation. Then we discuss the details of matrix construction and a generic stabilizer. For the numerical method, we present the details of the parallel scheme which including the implicit time marching scheme, namely, the Newmark-Beta method[1]. At the end of this chapter, we show the numerical results of one-dimensional linear and nonlinear elastic equation. The second and third order accuracy achieved with observed superlinear speedup.

3.1 Nonlinear Elasticity Equation

Consider an elastic body subject to an exterior force \mathbf{f} , we denote the computational domain as Ω and its continuous boundary as $\Gamma = \partial\Omega$. The governing equation for elasticity can be written as

$$-\nabla \cdot \boldsymbol{\sigma}(\mathbf{u}) = \mathbf{f}, \quad \text{in } \Omega \quad (3.1)$$

$$\mathbf{u} = \hat{\mathbf{u}}, \quad \text{on } \Gamma \quad (3.2)$$

where $\boldsymbol{\sigma}(\mathbf{u})$ is Cauchy stress. For isotropic and homogeneous materials, the stress-strain relation is

$$\boldsymbol{\sigma}(\mathbf{u}) = 2\mu\varepsilon(\mathbf{u}) + \lambda(\nabla \cdot \mathbf{u})\mathbf{I} \quad (3.3)$$

the μ and λ are Lame indices which can be written as

$$\lambda = \frac{E\nu}{(1+\nu)(1-2\nu)} \quad (3.4)$$

where E is the elasticity modulus and ν is Poisson's ratio.

The geometric nonlinear strain-displacement relation can be written as following

format

$$\boldsymbol{\varepsilon}(\mathbf{u}) = \frac{1}{2}(\nabla u + \nabla u^T + (\nabla u^T) \cdot \nabla u) \quad (3.5)$$

The weak function on domain is $u = \{u_0, u_b\}$, $u_0 \in L^2(T)$. The first function u_0 represents the interior domain of the function u . The second function u_b represents the boundary value of function u . The key notion is that for two functions u_0 and u_b are independent with each other. The weak function is defined as

$$U_h = \{u = \{u_0, u_b\} : u_0 \in P_j(T^0), u_b \in P_l(e), e \subset \partial T\} \quad (3.6)$$

The key of the weak Galerkin method is to approximate the solution in weak discrete space $S(T)$. The discrete weak gradient $\nabla_w u \in [P_r(T)]^d$ for $u \in V_h$ on each element T :

$$(\nabla_w, q)_T = -(u_0, \nabla \cdot q)_T + \langle u_b, q \cdot \mathbf{n} \rangle_{\partial T} \quad (3.7)$$

for the discrete weak divergence, $\nabla_w \cdot \mathbf{u}$ in $[P_r(T)]^d$ is defined

$$(\nabla_w \cdot u, q)_T = -(u_0, \nabla q)_T + \langle u_b \cdot \mathbf{n}, q \rangle_{\partial T} \quad (3.8)$$

The we may define the weak linear strain tensor by using the weak gradient

$$\boldsymbol{\varepsilon}_w(u) = \frac{1}{2}(\nabla_w u + \nabla_w u^T) \quad (3.9)$$

Analogously, the nonlinear weak strain tensor is defined by

$$\boldsymbol{\varepsilon}_w(u) = \frac{1}{2}(\nabla_w u + \nabla_w u^T + (\nabla_w u^T) \cdot \nabla_w u) \quad (3.10)$$

The weak stress can be defined as

$$\sigma_w(u) = 2\mu\epsilon_w(u) + \lambda(\nabla_w \cdot u)\mathbf{I} \quad (3.11)$$

The weak form of governing equation of continuous Galerkin method as the form as

$$a(\mathbf{u}, \mathbf{v}) = (\mathbf{f}, \mathbf{v}) \quad (3.12)$$

the above approximation function v and the gradient ∇v is not well defined for the discontinuous feature of weak Galerkin method. The new form is

$$a(\mathbf{u}_w, \mathbf{v}_w) + s(\mathbf{u}, \mathbf{v}) = (\mathbf{f}, \mathbf{v}) \quad (3.13)$$

the term $s(\mathbf{u}, \mathbf{v})$ is a stabilizer enforcing a weak continuity which measures the discontinuity of the finite element solution. The governing equation in weak form can be introduced by two bilinear equations

$$s(\mathbf{u}, \mathbf{v}) = \sum_{T \in \Omega}^N h_T^{-1} \langle Q_b u_0 - u_b, Q_b v_0 - b_b \rangle_{\partial T} \quad (3.14)$$

where Q_b the projection parameter defined by the interior and boundary value. h_T is the characteristic length of mesh element. It is a constant number which usually is taken as 1.

The governing equation has the weak form as

$$a(\mathbf{u}_w, \mathbf{v}_w) = \sum_{T \in \Omega}^N 2(\mu\epsilon_w(u), \epsilon_w(v))_T + \lambda(\nabla_w \cdot \mathbf{u}, \nabla_w \cdot \mathbf{v})_T \quad (3.15)$$

3.2 Hybrid WG-CG Element

In this section, we introduce the design of hybrid element, the construction of the basis functions. The one dimensional WG element has the shape function

$$\phi_0^1 = \frac{x - x_{i+1}}{x_i - x_{i+1}}, \quad \phi_b^1 = 1 \quad (3.16)$$

$$\phi_0^2 = \frac{x - x_i}{x_{i+1} - x_i}, \quad \phi_b^2 = 1 \quad (3.17)$$

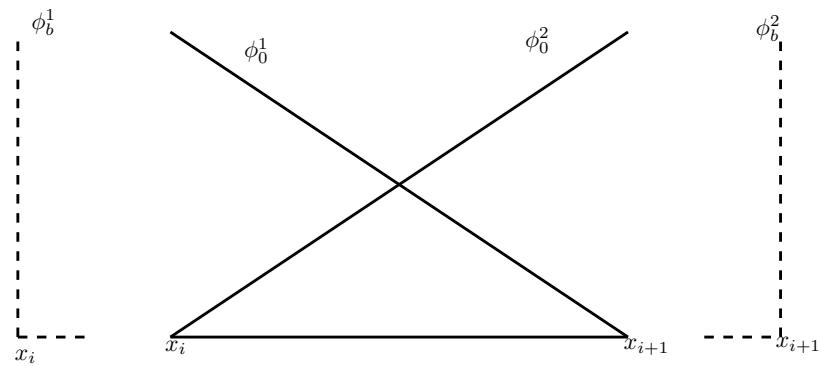


Figure 3.1: Basis function of one dimensional weak Galerkin element

Based on the weak gradient function, the boundary basis functions are independent with the interior basis functions, Therefore, we can insert an arbitrary number of CG elements into the WG element. Each weak Galerkin element is considered as one sub-element that neighbored with adjacent elements. The interior CG elements are found mesh level with are corrected by the coarse mesh. The boundary basis function of WG element is shared by adjacent elements and constitute the coarse mesh. The newly designed basis function of hybrid element can be illustrated as

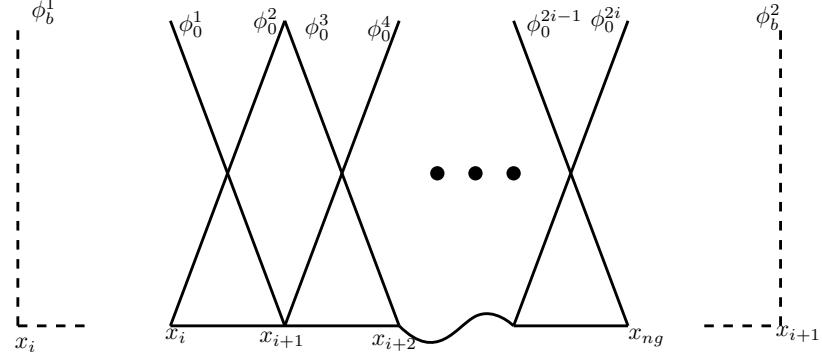


Figure 3.2: Basis function of hybrid WG-CG element, an arbitrary number of CG elements are inserted into WG element

3.3 Generic Stabilizer

Here we present the parallel computing scheme for one dimensional linear and nonlinear elastic equations. The governing equation for dynamic equation is

$$M\ddot{u} + Ku = f \quad (3.18)$$

In the hybrid elements, the information of mass is missing for the corresponding boundary values. The original governing equation in matrix form is

$$\begin{bmatrix} M_{00} & 0 \\ 0 & 0 \end{bmatrix} \begin{bmatrix} \ddot{u}_0 \\ \ddot{u}_b \end{bmatrix} + \begin{bmatrix} K_{00} & K_{0b} \\ K_{b0} & K_{bb} \end{bmatrix} \begin{bmatrix} u_0 \\ u_b \end{bmatrix} = \begin{bmatrix} f_0 \\ 0 \end{bmatrix} \quad (3.19)$$

from above equation we can find that the mass matrix for \ddot{u}_b is not complete. The information of mass is missing on the boundary basis functions. Under this circumstance, the explicit time marching scheme is unable to initiate. To avoid the deficit of time marching scheme, we introduce a newly designed stabilizer, namely, the generic stabilizer.

$$s(\mathbf{u}, \mathbf{v}) = \sum_{T \in \Omega}^N h_T^{-1} \langle Q_b \ddot{u}_0 - \ddot{u}_b, Q_b v_0 - v_b \rangle_{\partial T} \quad (3.20)$$

The original stabilizer requires that the values in the equation must be the same category. We extend the rule to all the unknown variables in the bilinear form. After the injection of the new stabilizer, the above equation becomes

$$\begin{bmatrix} M_{00} & 0 \\ 0 & M_{bb} \end{bmatrix} \begin{bmatrix} \ddot{u}_0 \\ \ddot{u}_b \end{bmatrix} + \begin{bmatrix} K_{00} & K_{0b} \\ K_{b0} & K_{bb} \end{bmatrix} \begin{bmatrix} u_0 \\ u_b \end{bmatrix} = \begin{bmatrix} f_0 \\ 0 \end{bmatrix} \quad (3.21)$$

Consequently, both explicit and implicit time marching scheme can be performed on above matrices.

3.4 Parallel Computing Method

For both linear and nonlinear equations, we implement the implicit time marching scheme, the Newmark-Beta method. It has the following steps

$$\ddot{u}_0^{n+1} = \frac{(u_0^{n+1} - \tilde{u}_0)}{\beta \Delta t^2} \quad (3.22)$$

$$\ddot{u}_b^{n+1} = \frac{(u_b^{n+1} - \tilde{u}_b)}{\beta \Delta t^2} \quad (3.23)$$

The \tilde{u} represents the intermediate step, then we can use it to calculate the value of next step

$$\tilde{u}_0 = u_0^n + \dot{u}_0^n \Delta t + \frac{1}{2} \Delta t^2 \ddot{u}_0^n (1 - 2\beta) \quad (3.24)$$

$$\tilde{u}_b = u_b^n + \dot{u}_b^n \Delta t + \frac{1}{2} \Delta t^2 \ddot{u}_b^n (1 - 2\beta) \quad (3.25)$$

To reduce the local unknown variable u_0 into a smaller size global matrix of unknown variable u_b , we apply the Schur complement method. The correlation between

interior unknown variables and boundary variables is following equation

$$u_0^{n+1} = -G_{00}^{-1}G_{0b}u_b^{n+1} + G_{00}^{-1}g_1 \quad (3.26)$$

Therefore, the original large sparse global matrix is transformed into a small condensed global matrix with the only one set of unknown variables, u_b . The governing equation has the new form

$$u_b^{n+1} = [G_{b0}(-G_{00}^{-1}G_{0b}) + G_{bb}]^{-1}(g_2 - G_{b0}(G_{00}^{-1}g_1)) \quad (3.27)$$

Then we can simplify the above equation into

$$u_b^{n+1} = G' f' \quad (3.28)$$

3.5 The Parallel Computing Work Flow

For one-dimensional elastic equation, the original governing equation can be converted to a sparse tri-diagonal stiffness matrix to a blocked decomposed submatrices

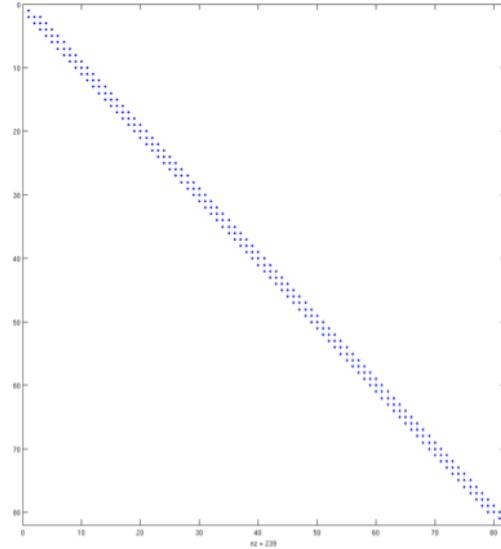


Figure 3.3: Global sparse tri-diagonal stiffness matrix

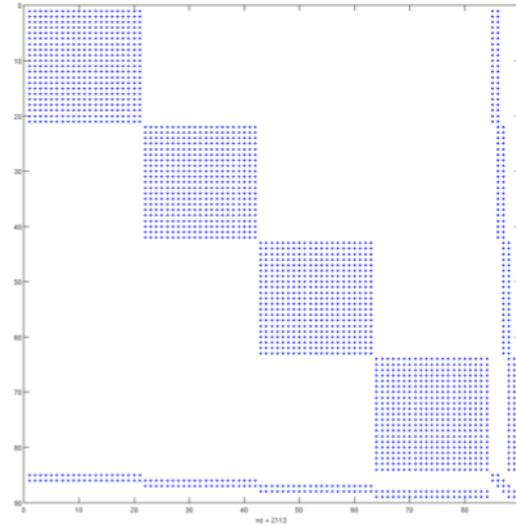


Figure 3.4: Blocked stiffness matrix after domain decomposition

The original large sparse stiffness matrix is compressed in a very small matrix on the right bottom corner. In another word, the main computational load is split into the calculation of square matrices. Each square matrix represents one single hybrid element. The size of hybrid element is determined by the number of CG elements which inserted into one WG element. We can compare the WG element as the cargo ship and CG elements are the container on the board. The entire computational domain is decomposed into multiple subdomains naturally since the discontinuous feature of hybrid WG-CG element.

The parallel computing work flow chart is described as following

1. Each CPU reads in preprocessed file and initiates MPI communication[30].
2. Every hybrid element is dealt by each individual processor. Local stiffness matrices and load vectors are constructed on parallel machine.
3. The global stiffness boundary matrix is assembled additively. The global boundary unknown variables are obtained and passed to each processor through MPI communication.

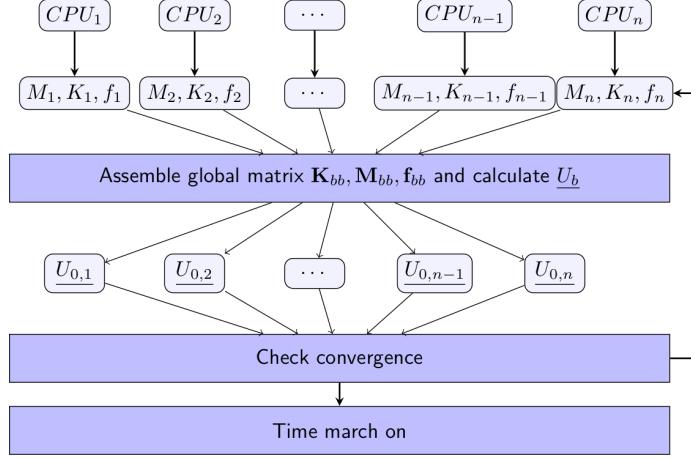


Figure 3.5: Parallel computing workflow chart for WG-CG method

4. Once the global residual is less than the tolerant value, current Newmark-Beta time iteration complete and move to the next time step.

To accelerate the performance, we use the open source library such as LAPACK/BLAS to calculate the computationally expensive operators in terms of the Cholesky transportation, matrix multiplication and vector operations. This implementation largely benefits the programming process and works very well among different high performance computing platforms.

The software has been tested on the George Washington University super computing cluster, ColonialOne, with Xeon E5-2650v2 2.6GHz 8-core processors with 128 GB of RAM each.

3.6 Numerical Results

3.6.1 Geometric linear elastic equation

We design a numerical test to verify the hybrid element with analytical solution $u = \sin(2\pi x)$. In each WG element, we insert 20 CG elements and then observe the accuracy of u_0 , u_b and the effect of stabilizer.

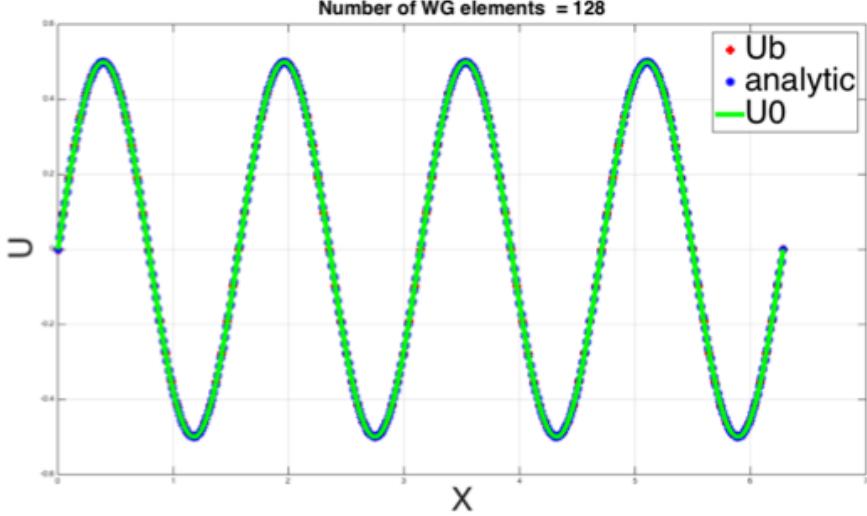


Figure 3.6: Linear elastic equation results for hybrid WG-CG element

Table 3.1: Error and accuracy of hybrid WG-CG element for linear elasticity.

# CG = 20				
#WG	$E(u_0)$	$O(u_0)$	$E(u_b)$	$O(u_b)$
16	$1.7636e - 4$	-	$3.8909e - 4$	-
32	$4.4745e - 5$	1.98	$9.8515e - 5$	1.99
64	$1.1271e - 5$	1.99	$2.4744e - 5$	1.99
128	$2.8286e - 6$	1.99	$6.1941e - 6$	2.00

With the increasing of the number of hybrid WG-CG elements, we observed a second order accuracy of both interior and boundary unknown variables. This test probes that the hybrid element has the excellent compatibility for linear elastic equation.

To explore the capability in dynamic problem, we extend the test case with the same analytic solution. By implementing the generic stabilizer above, both explicit and implicit time marching scheme are ready to test. For explicit time marching scheme, we choose the forward Euler method[38] and define the relative small time step to fit the critical time marching step[15]. For the implicit scheme, we choose the

Newmark-Beta method[31]. For every test case, a constant loading force is applied on the right-hand side as a Neumann boundary condition[61]. Meanwhile, we fix the left-hand side as Dirichlet boundary condition. The results are shown as following charts

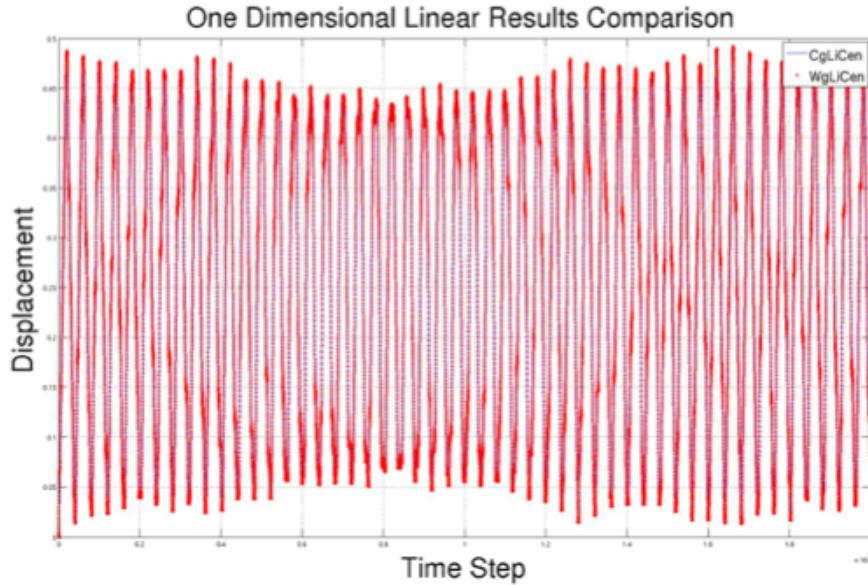


Figure 3.7: The results comparison between CG only and hybrid WG-CG element for explicit scheme

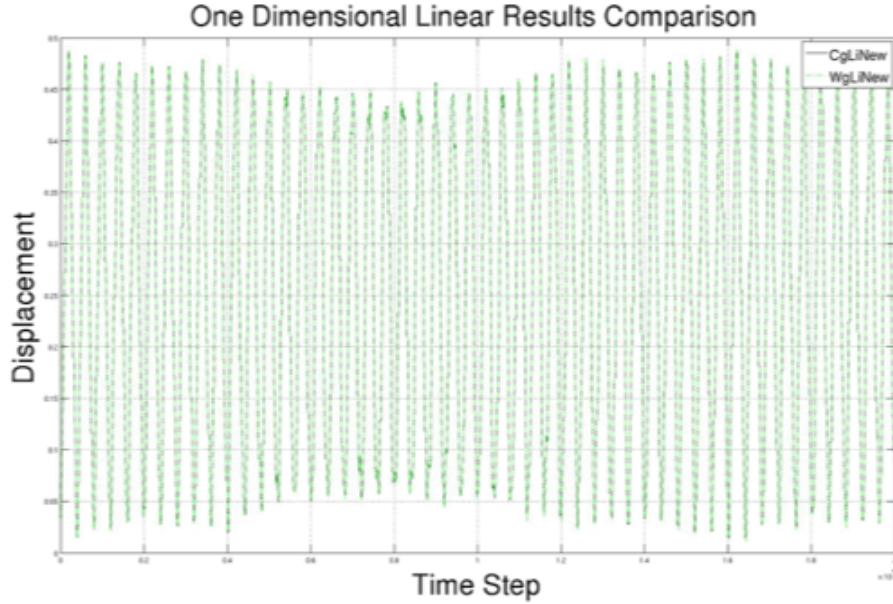


Figure 3.8: The results comparison between CG only and hybrid WG-CG element for implicit scheme

We compare the solutions between our hybrid WG-CG element and classic CG element. In both implicit and explicit time marching scheme, the average difference between two methods is less than $1e^{-5}$. We can safely draw the conclusion that hybrid element achieves an optimal accuracy for not only static case, by also dynamic problems.

3.6.2 Geometric nonlinear elasticity equation

Both the large deflection and body rotation require the nonlinear strain-displacement relation. We test the performance of accuracy and efficiency of hybrid WG-CG element. We continue using the classic CG element solver as the reference and compare our new method to the reliable existing solutions.

By following the linear elastic equation test cases, we load constant force as the boundary condition and compare the solutions between the serial CG solver and parallel implicit hybrid WG-CG solver. We divide the entire computational domain into 20 subdomains. In another word, each subdomain corresponds to an individual

hybrid WG-CG element. In each hybrid element, we insert 20 standard CG elements. Each processor handles one hybrid element and the communicates with each other through MPI library.

The CG solver choose 400 standard linear CG elements to maintain the same resolution. We track the deflection on the right tip of the domain and plot the two sets of solutions in the same figure to compare the difference as following

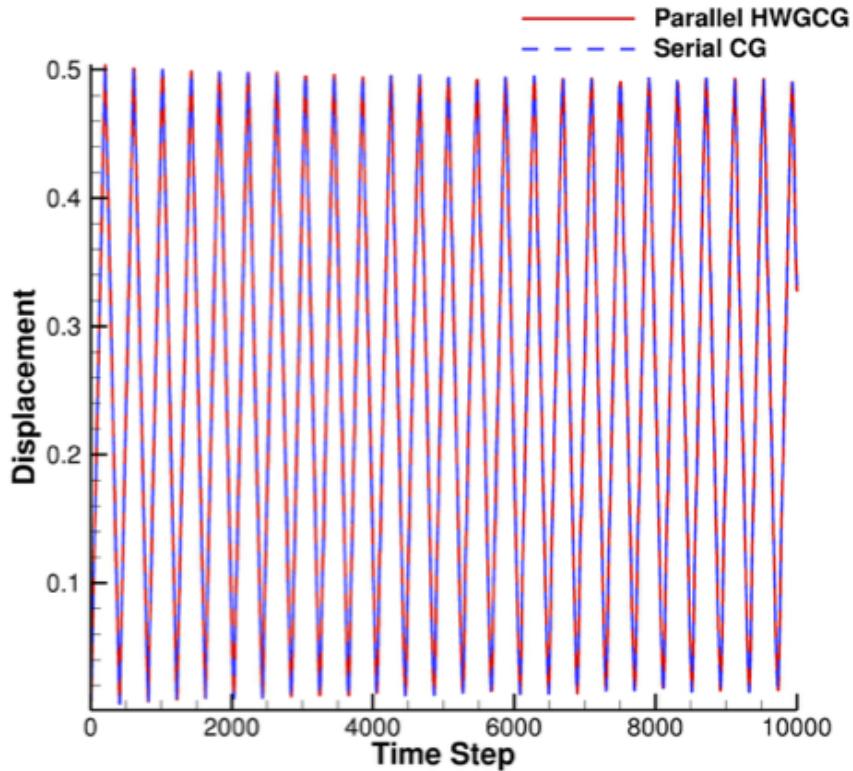


Figure 3.9: The results comparison between CG only and hybrid WG-CG element by using parallel implicit scheme with constant boundary condition

An optimal convergence of the two independent solutions shows an excellent accuracy of the parallel hybrid solver. The average difference is less than $1e^{-4}$, which is same as the previous linear test.

To verify the the robustness of the new solver, a periodic boundary condition is enforced to substitute the constant variable where $f = \sin(2\pi t)$. The comparing

results is as following chart

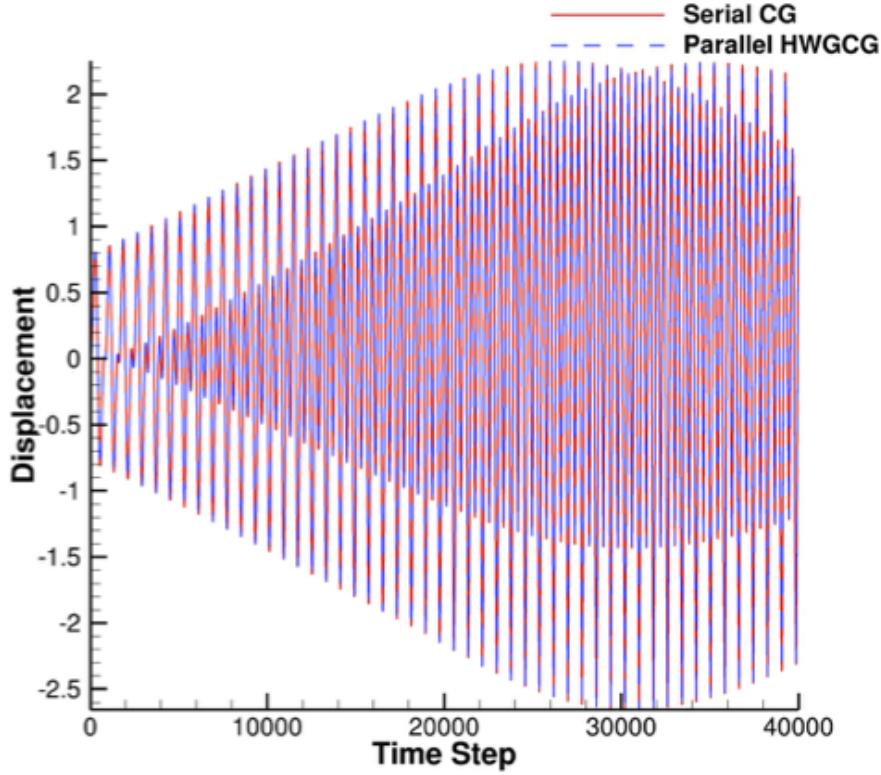


Figure 3.10: The results comparison between CG only and hybrid WG-CG element by using parallel implicit scheme with periodic boundary condition

Analogously, an optimal convergence is also observed from this test case. We have a strong confidence on the accuracy of our nonlinear parallel computing solver.

After the verification of accuracy and precision, we want to test the performance and parallel computing scalability on high performance clusters. We increase the size of the problem to a higher level, 20 times larger than the original test case. Meanwhile, we gradually increase the number of processor applied for the problem from 10 to 60. The time using graph is plotted as following

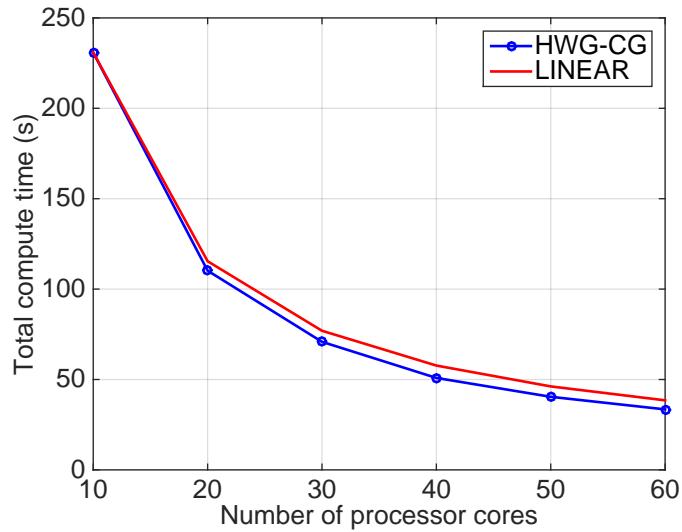


Figure 3.11: Time decreasing .vs. number of processors increasing

then we also plot the speedup curvature is proportional to the increasing number of processors

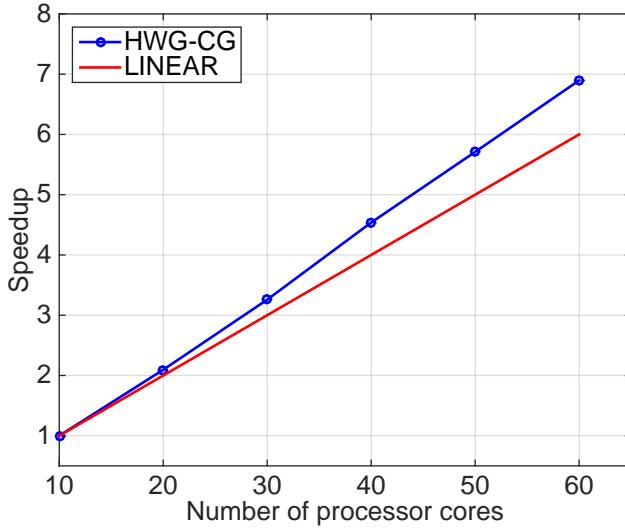


Figure 3.12: Speedup .vs. number of processors increasing

A superlinear speedup is observed from the above test cases. The reason is that the computational effort to solve local matrices decreases nonlienarly when the local problem is divided into a series of small subdomains. The computational expense for converting stiffness matrix decreased cubics to the size of matrix. Meanwhile, the communication overhead growing relatively slow. The time complexity is $O(n^3)$ reducing ans $O(n)$ increasing. Overall, the trade-off benefit for domain decomposition is larger than MPI communication which leads to the superlinear results.

We also consider the linear elasticity Equation in the square domain $\Omega = (0, 1)^2$. For each subdomain, it is partitioned into uniform triangular and quadrilateral mesh

wish mesh size h . The right-hand side function f is chosen. The exact solution is given by

$$u = \begin{pmatrix} \sin(2\pi x)\sin(2\pi y) \\ 1 \end{pmatrix} \quad (3.29)$$

the solution is shown as below

Table 3.2: Numerical results for triangular element.

# CG = 20				
#WG	$E(u_0)$	$O(u_0)$	$E(u_b)$	$O(u_b)$
4	$9.484e - 2$	-	$8.484e - 2$	-
16	$2.678e - 3$	1.9	$2.178e - 3$	1.9
64	$5.570e - 4$	2.0	$5.470e - 4$	1.9
256	$1.437e - 4$	2.0	$1.367e - 4$	2.0

Table 3.3: Numerical results for quadrilateral element.

# CG = 20				
#WG	$E(u_0)$	$O(u_0)$	$E(u_b)$	$O(u_b)$
4	$8.368e - 2$	-	$9.103e - 2$	-
16	$1.944e - 3$	1.9	$2.058e - 3$	1.9
64	$5.337e - 4$	2.0	$5.280e - 4$	2.0
256	$1.086e - 4$	2.0	$1.451e - 4$	2.0

In summary, we present a newly developed hybrid element combine the weak Galerkin finite element and continuous Galerkin finite element. It inherits the discontinuity from WG method and the convenience on implementation from CG method. The implementation of hybrid element for parallel computing is compatible for MPI library platform. The second order accuracy and superlinear speedup are observed

for the hybrid element.

Both geometric linear and nonlinear test cases are studied. The results of new hybrid element provides and optimal convergence which is accordance with WG and CG elements. The hybrid elements proved a strong robustness for Dirichlet and Neumann boundary condition.

We present the unstructured mesh results of two-dimensional solution for linear elasticity equation. We will extend the parallel computing to two dimensional with more advanced technicals. More details of parallel computing for larger problem will be discussed in the following chapter.

Chapter 4: Weak Galerkin Parallel Solutions of Linear Elasticity on Unstructured Meshes

This chapter focuses on solving linear elasticity problem on parallel computer by combining a novel finite element method with an efficient parallel computing scheme. Specifically, this combination involves a discontinuous weak Galerkin finite element method [56, 44, 76, 54] and a non-overlapping domain decomposition scheme, namely, the balancing domain decomposition with constraints (BDDC) [18, 70, 69]. The WG method is considered as a newly developed robust numerical method and inherits the locking-free feature for the linear elastic equation [74].

Like the standard Finite Element method (FEM), the WG method can be used to solve generic partial differential equations. An advanced feature of the WG finite element method is that the entire problem is decomposed into multiple elemental problems. Such elemental problems are often derived from weak formulations of the corresponding differential operators after integration by parts. In these elemental problems, the differential operators are approximated and reconstructed by smaller-size matrices. The WG method has been proven robust and possessing optimal orders of accuracy in spatial discretization on serial computers [57, 58]. Wang et al [74] recently extended the WG method to solve linear elasticity problems and also successfully demonstrated its locking-free property. However, the performance of the WG method on parallel computers has not yet been examined.

4.1 Domain Decomposition Scheme

The basic idea of domain decomposition is to split the computational mesh of an entire domain into many smaller meshes for a set of non-overlapping subdomains. Each subdomain contains its own set of grid elements. For finite element methods, after domain decomposition, a remaining challenging task is to connect these subdomains' interfaces by satisfying continuity constraints to correctly represent the

solution of the original set of equations over the complete domain. In this work, the BDDC method is used to serve this purpose. The original balancing domain decomposition (BDD) method [51] has only considered two level meshes. It used a multiplicative coarse domain to correct the local fine mesh subdomain. However, the significant difference between BDDC and BDD is that the method in this paper applies the coarse problem in an additive routine rather than multiplicative manner. In this case, a more flexible of constraints will reduce the complexity and improve the efficiency. In our BDDC method, we assemble the preconditioner matrix additively in contrast to the multiplicative coarse grid correction used in the BDD method. In the BDDC method, the flexibility of choosing constraining points leads to reduced complexity of implementation and improved efficiency of computations in comparison to the standard BDD method. The details of the choice of constraints for BDDC will be discussed in this section.

4.1.1 FETI-DP Method

Finite Element Tearing and Interconnect (FETI) , first proposed by Farhat[26, 25, 39, 24, 43, 40], is an iterative method for solving large finite element problems generated by linear equations. Originally, FETI is proposed to solve the discontinuity when apply domain decomposition on second order elliptic partial differential equations, particularly linear elasticity equations. FETI method partitioned the entire computational domain into two level meshes, the coarse grid and fine grid. Lagrangian multiplier is employed to conquer the discontinuity between each subdomain. For the fine mesh, since local matrix is not positive definite, the pseudo-inverse is applied to convert local matrix.

Then FETI-DP (Dual Primal) is introduced for two-dimensional problems by Farhat and Rixen[23]. In this new method, the unknown variables along the interface is partitioned into primal and dual spaces. The continuity of the primal unknown variables along the interface, the vertices of each subdomain, is maintained by as-

semblage. For the rest dual spaces, the constrains are still controlled by Lagrange multipliers. However, this constrain is only at the convergence of the method.

In this section, we begin with an example of a two-dimensional, two partitioned subdomain case. We enforce the Dirichlet boundary condition on the boundary. The original geometry and computational domain is

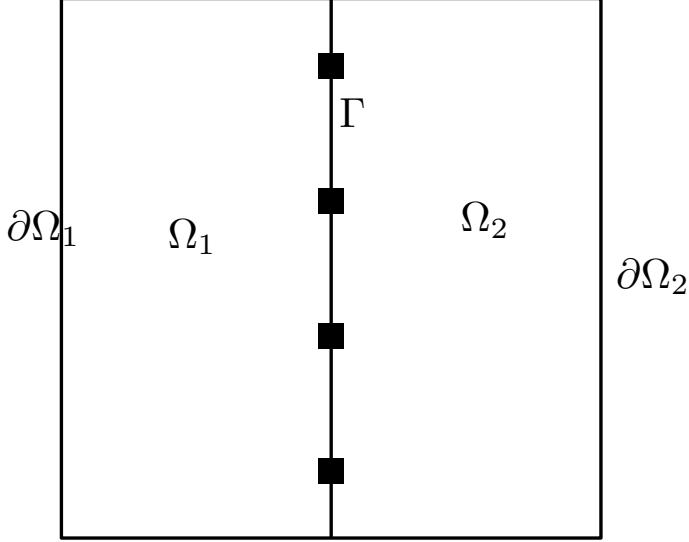


Figure 4.1: Computational domain partitioned into two nonoverlapping subdomains.

the original governing equation in matrix form is $\mathbf{A}\mathbf{u} = \mathbf{f}$. We begin to compute the stiffness matrix for each subdomain that

$$\begin{pmatrix} A_{II}^{(j)} & A_{I\Gamma}^{(j)} \\ A_{\Gamma I}^{(j)} & A_{\Gamma\Gamma}^{(j)} \end{pmatrix} \begin{pmatrix} u_I^{(j)} \\ u_\Gamma^{(j)} \end{pmatrix} = \begin{pmatrix} f_I^{(j)} \\ f_\Gamma^{(j)} \end{pmatrix} \quad (4.1)$$

where $j = 1, 2$.

The boundary condition along $\partial\Omega$ is Dirichlet condition, a homogeneous Neumann condition is applied on Γ .

The global problem after assemblage is that

$$\begin{pmatrix} A_{II}^{(1)} & 0 & A_{I\Gamma}^{(1)} \\ 0 & A_{II}^{(2)} & A_{I\Gamma}^{(2)} \\ A_{\Gamma I}^{(1)} & A_{\Gamma I}^{(2)} & A_{\Gamma\Gamma} \end{pmatrix} \begin{pmatrix} u_I^{(1)} \\ u_I^{(2)} \\ u_\Gamma \end{pmatrix} = \begin{pmatrix} f_I^{(1)} \\ f_I^{(2)} \\ f_\Gamma \end{pmatrix} \quad (4.2)$$

where $A_\Gamma = A_{\Gamma\Gamma}^{(1)} + A_{\Gamma\Gamma}^{(2)}$ and $f_\Gamma = f_\Gamma^{(1)} + f_\Gamma^{(2)}$. The unknown variables are decomposed into two computational subdomains. The DOFs are represented by Ω_1, Ω_2 and Γ respectively.

Since the interior matrix are square and can be inversed. Then we can eliminate the interior unknown variables by Schur complement operators. The interface unknown DOFs shall have the new form as

$$S^{(j)} := A_{\Gamma\Gamma}^{(j)} - A_{\Gamma I}^{(j)} A_{II}^{(j)-1} A_{I\Gamma}^{(j)} \quad (4.3)$$

$$g_\Gamma^{(j)} := f_\Gamma^{(j)} - A_{\Gamma I}^{(j)} A_{II}^{(j)-1} f_I^{(j)} \quad (4.4)$$

based on the given matrix $Au = f$, we can reduce the equation matrices into interface DOF only system

$$(S^{(1)} + S^{(2)})u_\Gamma = g_\Gamma^{(1)} + g_\Gamma^2 \quad (4.5)$$

Now we can find that the original large matrix A is reduced to a small global matrix $S^{(j)}$ which times unknown variable vector along the interface. The interior unknown variables can be recovered locally based the solution of interface.

For FETI method, we introduce the Lagrange multiplier to enforce the continuity

along the interface. The governing equation in matrix form is

$$\begin{pmatrix} A_{II}^{(j)} & A_{I\Gamma}^{(j)} \\ A_{\Gamma I}^{(j)} & A_{\Gamma\Gamma}^{(j)} \end{pmatrix} \begin{pmatrix} u_I^{(j)} \\ u_\Gamma^{(j)} \end{pmatrix} = \begin{pmatrix} f_I^{(j)} \\ f_\Gamma^{(j)} + \lambda_\Gamma^{(j)} \end{pmatrix} \quad (4.6)$$

where $j = 1, 2$ and $\lambda_\Gamma = \lambda_\Gamma^{(1)} = -\lambda_\Gamma^{(2)}$. λ_Γ is an unknown flux vector, namely, Lagrange multiplier. We can solve this linear system following previous fashion that

$$g_\Gamma^{(j)} = f_\Gamma^{(j)} - A_{\Gamma I}^{(j)} A_{II}^{(j)-1} f_I^{(j)} \quad (4.7)$$

$$u_\Gamma^{(j)} = S^{(j)-1} (g_\Gamma^{(j)} + \lambda_\Gamma^{(j)}) \quad (4.8)$$

to maintain the continuity along the interface, the condition is set as

$$u_\Gamma^{(1)} = u_\Gamma^{(2)} \quad (4.9)$$

the value of DOFs for the same position has to be same. To resolve the rigid body motion problem, we obtain

$$F\lambda_\Gamma = d_\Gamma \quad (4.10)$$

$$F = S^{(1)-1} + S^{(2)-1} \quad (4.11)$$

An improved version of FETI is FETI-DP. We can partitioned the subdomains with floating constraints.

From the above figure, the interface edge is continued to split into $(u_1, \dots, u_m, \dots, u+$

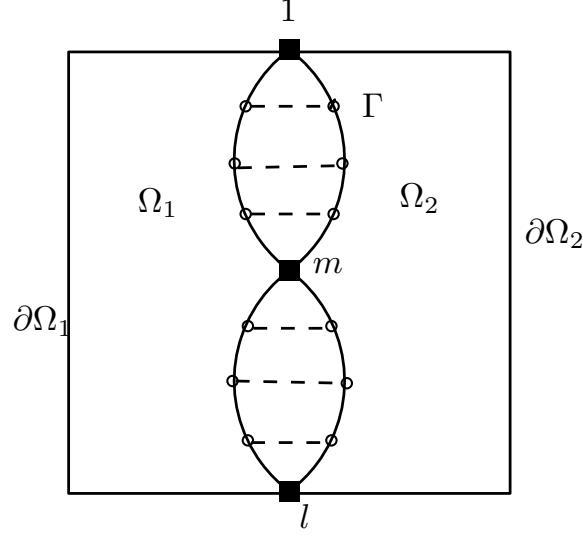


Figure 4.2: Computational domain partitioned into two nonoverlapping subdomains with floating constraint.

l) then the linear system can be written as

$$\begin{pmatrix} A_{II}^{(j)} & A_{1I}^{(j)} & \cdots & A_{mI}^{(j)} & \cdots & A_{lI}^{(j)} \\ A_{1I}^{(j)} & A_{11}^{(j)} & \cdots & A_{1m}^{(j)} & \cdots & A_{1l}^{(j)} \\ \vdots & \vdots & \ddots & \vdots & \ddots & \vdots \\ A_{mI}^{(j)} & A_{m1}^{(j)} & \cdots & A_{mm}^{(j)} & \cdots & A_{ml}^{(j)} \\ \vdots & \vdots & \ddots & \vdots & \ddots & \vdots \\ A_{lI}^{(j)} & A_{l1}^{(j)} & \cdots & A_{lm}^{(j)} & \cdots & A_{ll}^{(j)} \end{pmatrix} \begin{pmatrix} u_I^{(j)} \\ u_1^{(j)} \\ \vdots \\ u_m^{(j)} \\ \vdots \\ u_l^{(j)} \end{pmatrix} = \begin{pmatrix} f_I^{(j)} \\ f_1^{(j)} \\ \vdots \\ f_m^{(j)} \\ \vdots \\ f_l^{(j)} \end{pmatrix} \quad (4.12)$$

The variable along the interface of two nonoverlapping subdomains can be elaborate as

$$\begin{pmatrix} u_1^{(j)} \\ \vdots \\ u_m^{(j)} \\ \vdots \\ u_l^{(j)} \end{pmatrix} = T_E \begin{pmatrix} \hat{u}_1^{(j)} \\ \vdots \\ \hat{u}_m^{(j)} \\ \vdots \\ \hat{u}_l^{(j)} \end{pmatrix} = \begin{pmatrix} 1 & \cdots & 1 & 0 & \cdots \\ \vdots & \ddots & \vdots & 0 & \cdots \\ -1 & \cdots & 1 & \cdots & -1 \\ \cdots & 0 & \vdots & \ddots & \vdots \\ \cdots & 0 & 1 & \cdots & 1 \end{pmatrix} \begin{pmatrix} \hat{u}_1^{(j)} \\ \vdots \\ \hat{u}_m^{(j)} \\ \vdots \\ \hat{u}_l^{(j)} \end{pmatrix} \quad (4.13)$$

After the multiplication between vector and matrix we have

$$T_E \begin{pmatrix} \hat{u}_1^{(j)} \\ \vdots \\ \hat{u}_m^{(j)} \\ \vdots \\ \hat{u}_l^{(j)} \end{pmatrix} = \begin{pmatrix} 1 \\ \vdots \\ 1 \\ \vdots \\ 1 \end{pmatrix} \hat{u}_m^{(j)} + \begin{pmatrix} \hat{u}_1^{(j)} \\ \vdots \\ -\hat{u}_1^{(j)} - \dots - \hat{u}_{m-1}^{(j)} \hat{u}_{m+1}^{(j)} - \hat{u}_l^{(j)} \\ \vdots \\ \hat{u}_l^{(j)} \end{pmatrix} \quad (4.14)$$

the T_E is a square matrix and the columns representing the new space of interface unknown variables. The original interface is divided into two parts. One part is that the value of unknown variables has a value on its own subdomain. The other part is that the function has zero interface averages. The the problem can be rewrite as

$$T^T \begin{pmatrix} A_{II}^{(j)} & A_{1I}^{(j)} & \dots & A_{mI}^{(j)} & \dots & A_{lI}^{(j)} \\ A_{1I}^{(j)} & A_{11}^{(j)} & \dots & A_{1m}^{(j)} & \dots & A_{1l}^{(j)} \\ \vdots & \vdots & \ddots & \vdots & \ddots & \vdots \\ A_{mI}^{(j)} & A_{m1}^{(j)} & \dots & A_{mm}^{(j)} & \dots & A_{ml}^{(j)} \\ \vdots & \vdots & \ddots & \vdots & \ddots & \vdots \\ A_{lI}^{(j)} & A_{l1}^{(j)} & \dots & A_{lm}^{(j)} & \dots & A_{ll}^{(j)} \end{pmatrix} T \begin{pmatrix} u_I^{(j)} \\ \hat{u}_1^{(j)} \\ \vdots \\ \hat{u}_m^{(j)} \\ \vdots \\ \hat{u}_l^{(j)} \end{pmatrix} = T^T \begin{pmatrix} f_I^{(j)} \\ f_1^{(j)} \\ \vdots \\ f_m^{(j)} \\ \vdots \\ f_l^{(j)} \end{pmatrix} \quad (4.15)$$

same as previous equation, T is a diagonal block matrix

$$T = \begin{pmatrix} I & 0 \\ 0 & T_E \end{pmatrix} \quad (4.16)$$

We find out that the change is an individual interface edge local procedure. We can assume that the unknown variables on each subdomain have been changed when using primal edges or faces. In last figure, we can see that the primal space is the only connection along the interface. The other DOFs are in dual space including interior

unknowns and the rest interface nodes.

$$\begin{pmatrix} A_{II}^{(1)} & A_{\Delta I}^{(1)} & 0 & 0 & A_{\Pi I}^{(1)} & 0 \\ A_{\Delta I}^{(1)} & A_{\Delta \Delta}^{(1)} & 0 & 0 & A_{\Pi \Delta}^{(1)} & B_{\Delta}^{(1)} \\ 0 & 0 & A_{II}^{(2)} & A_{\Delta I}^{(2)} & A_{\Pi I}^{(2)} & 0 \\ 0 & 0 & A_{\Delta I}^{(2)} & A_{\Delta \Delta}^{(2)} & A_{\Pi \Delta}^{(2)} & B_{\Delta}^{(2)} \\ A_{\Pi I}^{(1)} & A_{\Pi \Delta}^{(1)} & A_{\Pi I}^{(2)} & A_{\Pi \Delta}^{(2)} & A_{\Pi \Pi}^{(1)} + A_{\Pi \Pi}^{(2)} & 0 \\ 0 & B_{\Delta}^{(1)} & 0 & B_{\Delta}^{(2)} & 0 & 0 \end{pmatrix} \begin{pmatrix} u_I^{(1)} \\ u_{\Delta}^{(1)} \\ u_I^{(2)} \\ u_{\Delta}^{(2)} \\ u_{\Pi} \\ \lambda \end{pmatrix} = \begin{pmatrix} f_I^{(1)} \\ f_{\Delta}^{(1)} \\ f_I^{(2)} \\ f_{\Delta}^{(2)} \\ f_{\Pi}^{(1)} + f_{\Pi}^{(2)} \\ 0 \end{pmatrix} \quad (4.17)$$

We can further eliminate the local variables $u_I^{(1)}, u_{\Delta}^{(1)}, u_I^{(2)}$ and $u_{\Delta}^{(2)}$, so that we can obtain

$$\begin{pmatrix} S_{\Pi \Pi} & \tilde{B}_{\Lambda \Pi}^T \\ \tilde{B}_{\Lambda \Pi} & \tilde{B}_{\Lambda \Lambda} \end{pmatrix} \begin{pmatrix} u_{\Pi} \\ \lambda \end{pmatrix} = \begin{pmatrix} g_{\Pi} \\ d_{\Lambda} \end{pmatrix} \quad (4.18)$$

the details of above equation is

$$S_{\Pi \Pi} = \sum_{j=1}^2 R_{\Pi}^{(j)} \left(A_{\Pi \Pi}^{(j)} - \begin{bmatrix} A_{\Pi I}^{(j)} & A_{\Pi \Delta}^{(j)} \end{bmatrix} \begin{bmatrix} A_{II}^{(j)} & A_{I \Delta}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta \Delta}^{(j)} \end{bmatrix}^{-1} \begin{bmatrix} A_{\Pi I}^{(j)} \\ A_{\Pi \Delta}^{(j)} \end{bmatrix} \right) R_{\Pi}^{(j)}, \quad (4.19)$$

$$\tilde{B}_{\Lambda \Pi} = - \sum_{j=1}^2 \begin{bmatrix} 0 & B_{\Pi}^{(j)} \end{bmatrix} \begin{bmatrix} A_{II}^{(j)} & A_{\Delta I}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta \Delta}^{(j)} \end{bmatrix}^{-1} \begin{bmatrix} A_{\Pi I}^{(j)} \\ A_{\Pi \Delta}^{(j)} \end{bmatrix} R_{\Pi}^{(j)}, \quad (4.20)$$

$$\tilde{B}_{\Lambda \Lambda} = - \sum_{j=1}^2 \begin{bmatrix} 0 & B_{\Delta}^{(j)} \end{bmatrix} \begin{bmatrix} A_{II}^{(j)} & A_{\Delta I}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta \Delta}^{(j)} \end{bmatrix}^{-1} \begin{bmatrix} 0 \\ B_{\Delta}^{(j)} \end{bmatrix}, \quad (4.21)$$

$$g_{\Pi} = \sum_{j=1}^2 R_{\Pi}^{(j)} \left(f_{\Pi}^{(j)} - \begin{bmatrix} A_{\Pi I}^{(j)} & A_{\Pi \Delta}^{(j)} \end{bmatrix} \begin{bmatrix} A_{II}^{(j)} & A_{\Pi \Delta}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta \Delta}^{(j)} \end{bmatrix}^{-1} \begin{bmatrix} f_I^{(j)} \\ f_{\Delta}^{(j)} \end{bmatrix} \right), \quad (4.22)$$

$$d_\Lambda = - \sum_{j=1}^2 \begin{bmatrix} 0 & B_\Delta^{(j)} \end{bmatrix} \begin{bmatrix} A_{II}^{(j)} & A_{\Delta I}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta\Delta}^{(j)} \end{bmatrix}^{-1} \begin{bmatrix} f_I^{(j)} \\ f_\Delta^{(j)} \end{bmatrix} \quad (4.23)$$

where the $R_\Pi^{(j)}$ is a projection matrix that maps the local subdomain element to global elements with $\{0, 1\}$ values. In this example, both projection values are equal to 1.

For the linear system, the Lagrange multiplier λ is

$$\left(\tilde{B}_{\Lambda\Lambda} - \tilde{B}_{\Lambda\Pi} S_{\Pi\Pi}^{-1} \tilde{B}_{\Pi\Pi}^T \right) \lambda = d_\Lambda - \tilde{B}_{\Lambda\Pi} S_{\Pi\Pi}^{-1} g_\Pi \quad (4.24)$$

A Dirichlet preconditioner is applied in FETI-DP method for solving above equation.

Based on above linear system, we can find out that the Lagrange multiplier is an external unknown vector and requires lots of computational effort to assemble the matrix. When we solve such a linear system, it's common to obtain the result along the dual space that $u_\Delta^{(1)} \neq u_\Delta^{(2)}$. To maintain the continuity, we shall restore the computing a weighted average of the vectors. Therefore, an assembled residual of resulting vector needs to be computed. The residue is mapped into the appropriate space of enforced vector on the right-hand side of the equation. Then we can use the residue vector to correct the solution and gradually obtain the final results. An improved algorithm, BDDC method, will be discussed in the following section.

4.1.2 Block Cholesky elimination

First, we consider how to represent the inverse of a matrix M , which is symmetric and positive definite block matrix as

$$M = \begin{bmatrix} A & B^T \\ B & C \end{bmatrix} \quad (4.25)$$

where M is the global stiffness matrix, A is the square matrix represents the interior values of each subdomain, C is the values on the interface and B represents the connectivity between A and C .

We employ the block Cholesky elimination and obtain the following equation that

$$\begin{bmatrix} A & B^T \\ B & C \end{bmatrix} = \begin{bmatrix} I_A & 0 \\ BA^{-1} & I_C \end{bmatrix} \begin{bmatrix} A & 0 \\ 0 & C - BA^{-1}B^T \end{bmatrix} \begin{bmatrix} I_A & A^{-1}B^T \\ 0 & I_C \end{bmatrix} \quad (4.26)$$

the S can be seen as a smaller global matrix containing the information among A , B and C . A Schur complement matrix S , represents a smaller global stiffness matrix partially assembled by each individual subdomains. However, after the decomposition, the size of S is still too large to be directly inverted. Consequently, we introduce the primal and dual spaces to calculate a preconditioner in parallel fashion to distribute the computational load and accelerate the overall speed. We redefine the Schur complement matrix S and compute the preconditioner S_{BDDC} by a parallel fashion. The number of primal variables, the constraints through the interface, is much less than the size of Schur complement matrix.

Both I_A and I_C are identity matrices. The Schur complement is represented as

$$S = C - BA^{-1}B^T \quad (4.27)$$

The inverse of original matrix M is

$$M^{-1} = \begin{bmatrix} A & B^T \\ B & C \end{bmatrix}^{-1} = \begin{bmatrix} I_A & -A^{-1}B^T \\ 0 & I_C \end{bmatrix} \begin{bmatrix} A^{-1} & 0 \\ 0 & S^{-1} \end{bmatrix} \begin{bmatrix} I_A & 0 \\ -BA^{-1} & I_C \end{bmatrix} \quad (4.28)$$

$$= \begin{bmatrix} A^{-1} & 0 \\ 0 & 0 \end{bmatrix} + \Phi S^{-1} \Phi^T \quad (4.29)$$

$$\text{where } \Phi = \begin{bmatrix} -A^{-1}B^T \\ I_C \end{bmatrix}$$

The original inverse global matrix is partitioned into one matrix containing A^{-1} and the other matrix associated with the S^{-1} . The interface matrix S takes from adjacent local subdomains, corresponding to the degrees of freedom on the solid squares shown in Figure 4.5.

After decompose the original computational domain into subdomains, we demonstrate the BDDC method to solve the matrix of the governing equation

$$M\mathbf{u} = \mathbf{f} \quad (4.30)$$

4.1.3 Balancing Domain Decomposition by Constraints

The balancing domain decomposition method was introduced by Mandel [52]. The original idea of BDD method is applying a coarse correction to guarantee the convergence of residuals. The BDDC is a domain decomposition method for solving large symmetric, positive definite equations of linear systems. The main function is to solve problems arises from the finite element method, including WG method. It is inspired by FETI-DP method of Farhat et al [25, 23] which has been extended multi-dimension varying problems. Comparing to BDD method, the substructure spaces and the coarse spaces are connected by the corner cell as constraints only. The main difference is that the BDDC method applies the coarse problem in an additive routine, which makes it possible to use a different bilinear form on the coarse problem. In this way, the BDDC method is considered as a simpler primal alternative to FETI-DP domain decomposition method [43]. In this paper, we only consider the corner connections of subdomain as the only constraints. The substructure spaces, coarse space, and the substructure bilinear forms are same as Mandels paper. Comparing with FETI-DP, BDDC method adds coarse degrees of freedom involving averages

over edges and faces of elements. This improvement causes an obvious simplification through domain decomposition and matrix calculation.

After the Schur complement, the preconditioner for interface is

$$\hat{S}u_\Gamma = \sum_{j=1}^N R_\Gamma^{(j)} g_\Gamma \quad (4.31)$$

where

$$\tilde{S} = \tilde{R}_\Gamma^T \tilde{S}_\Gamma \tilde{R}_\Gamma \quad (4.32)$$

In the BDDC algorithm, a two-level Neumann-Neumann type preconditioner for solving this interface problem is as above equation. In the BDDC preconditioner, the coarse grid is assembled from coarse basis functions. We can apply the minimum energy method on the subdomains to obtain primal constraints. The primal constraints maintain the continuity along the edge interface between two subdomains, as in FETI-DP algorithm.

Dohrmann's BDDC preconditioner [18, 19, 52] has the form

$$M_{BDDC}^{-1} = R_{D,\Gamma}^T T R_{D,\Gamma} \quad (4.33)$$

for the coarse-level T is defined by

$$T = \Psi(\Psi^T S \Psi)^{-1} \Psi^T \quad (4.34)$$

the coarse level basis function vector is defined by

$$\Psi = \begin{pmatrix} \Psi^{(1)} \\ \vdots \\ \Psi^{(N)} \end{pmatrix} \quad (4.35)$$

Then the Schur complement coarse-level matrix can be written as

$$\begin{pmatrix} S^{(j)} & C^{(j)T} \\ C^{(j)} & 0 \end{pmatrix} \begin{pmatrix} \Psi^{(j)} \\ V^{(j)} \end{pmatrix} = \begin{pmatrix} 0 \\ R_{\Pi}^{(j)} \end{pmatrix} \quad (4.36)$$

where $C^{(j)}$ is the primal constraints of each subdomain and $V^{(j)}$ is Lagrange multiplier vector. If we assume the variable is changing, then the Lagrange multiplier vector is no longer needed to enforce the primal continuity constraints and the new BDDC preconditioner can be designed as

$$M_{BDDC}^{-1} = R_{D,\Gamma}^T \{R_{\Gamma\Delta}^T S_{\Delta}^{-1} R_{\Gamma\Delta} + \Psi(\Psi^T S \Psi)^{-1} \Psi^T\} R_{D,\Gamma} \quad (4.37)$$

The primal space DOFs are used to enforce the continuity by restricting the operators to the dual interface space Δ . The governing matrix equation can be designed as

$$\begin{pmatrix} A_{II}^{(j)} & A_{\Delta I}^{(j)} & A_{\Pi I}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta\Delta}^{(j)} & A_{\Pi\Delta}^{(j)} \\ A_{\Pi I}^{(j)} & A_{\Pi\Delta}^{(j)} & A_{\Pi\Pi}^{(j)} \end{pmatrix} \begin{pmatrix} u_I^{(j)} \\ \Psi_{\Delta}^{(j)} \\ R_{\Pi}^{(j)} \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ S_{\Pi\Pi}^{(j)} R_{\Pi}^{(j)} \end{pmatrix} \quad (4.38)$$

where

$$\Psi^{(j)} = \begin{pmatrix} \Psi_{\Delta}^{(j)} \\ R_{\Pi}^{(j)} \end{pmatrix} \quad (4.39)$$

$$\Psi^{(j)} = \left(- \begin{pmatrix} 0 & I_{\Delta}^{(j)} \end{pmatrix} \begin{pmatrix} A_{II}^{(j)} & A_{I\Delta}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta\Delta}^{(j)} \end{pmatrix}^{-1} \begin{pmatrix} A_{I\Pi}^{(j)} \\ A_{\Pi\Delta}^{(j)} \end{pmatrix} R_{\Pi}^{(j)} \right) R_{\Pi}^{(j)} \quad (4.40)$$

and

$$S_{\Pi\Pi}^{(j)} = A_{\Pi\Pi}^{(j)} - \begin{pmatrix} A_{\Pi I}^{(j)} & A_{\Pi\Delta}^{(j)} \end{pmatrix} \begin{pmatrix} A_{II}^{(j)} & A_{I\Delta}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta\Delta}^{(j)} \end{pmatrix}^{-1} \begin{pmatrix} A_{I\Pi}^{(j)} \\ A_{\Delta\Pi}^{(j)} \end{pmatrix} \quad (4.41)$$

the global Schur complement operator S_{III} is assembled by every subdomain that

$$\Psi^T S \Psi = \sum_{j=1}^N \Psi^{(j)T} S^{(j)} \Psi^{(j)} \quad (4.42)$$

this equals to

$$\sum_{j=1}^N R_{\Pi}^{(j)T} S_{\text{III}}^{(j)} R_{\Pi}^{(j)} = S_{\text{III}} \quad (4.43)$$

where Ψ represents the interface vectors with distributed coarse-level variables. Φ represents the DOFs vectors on coarse-level.

If we have the assumption that

$$\delta_j^\dagger(x) = 1, \quad x \in \Gamma \quad (4.44)$$

$$R_{D,\Gamma}^T \Psi = \tilde{R}_{D,\Gamma}^T \Phi \quad (4.45)$$

therefore

$$M_{BDDC}^{-1} = R_{D,\Gamma}^T R_{\Gamma\Delta}^T S_{\Delta}^{-1} R_{\Gamma\Delta} R_{D,\Gamma} + R_{D,\Gamma}^T \Psi \left(\Psi^T S \Psi \right)^{-1} \Psi^T R_{D,\Gamma} \quad (4.46)$$

after substitute Ψ by Φ , we can derive

$$M_{BDDC}^{-1} = \tilde{R}_{D,\Gamma}^T R_{\Gamma\Delta}^T S_{\Delta}^{-1} R_{\Gamma\Delta} \tilde{R}_{\Delta,\Gamma} + \tilde{R}_{D,\Gamma}^T \Phi S_{\text{III}}^{-1} \Phi^T \tilde{R}_{D,\Gamma} \quad (4.47)$$

we can simplify the equation by

$$M_{BDDC}^{-1} = \tilde{R}_{D,\Gamma}^T \tilde{S}_{\Gamma}^{-1} \tilde{R}_{D,\Gamma} \quad (4.48)$$

the preconditioned BDDC operator is designed by

$$\tilde{R}_{D,\Gamma}^T \tilde{S}_\Gamma^{-1} \tilde{R}_{D,\Gamma} \tilde{R}_\Gamma^T \tilde{S}_\Gamma \tilde{R}_\Gamma \quad (4.49)$$

4.2 WG-BDDC Method

In this section, we discuss the details of design the parallel computing scheme by combining WG method with BDDC method.

The preconditioned conjugate gradient method is adopted as the linear solver for BDDC method. The construction of preconditioner is crucial in the problem. The BDDC preconditioner combines the solution of the local problem on each subdomain with the solution of a global coarse problem and the coarse degrees of freedoms as unknowns.

The preconditioned conjugate gradient method is adopted as the linear solver for BDDC method. The construction of preconditioner is crucial in the problem. The BDDC preconditioner combines the solution of the local problem on each subdomain with the solution of a global coarse problem and the coarse degrees of freedoms as unknowns.

In FETI method, local matrices after domain decomposition are singular and the pseudo-inverses must be computed. On the contrary, the WG-BDDC has the advantage to bypass this difficulty.

BDDC shall be processed by the following steps:

1. Schur Complement [20] of problems on each subdomain will eliminate all the interior unknowns and only retain the unknowns on the interface of . Denote the interface by .
2. Reduce the unknowns on the interface to construct the preconditioner.
3. Solve the linear system by using preconditioned conjugate gradient solver.

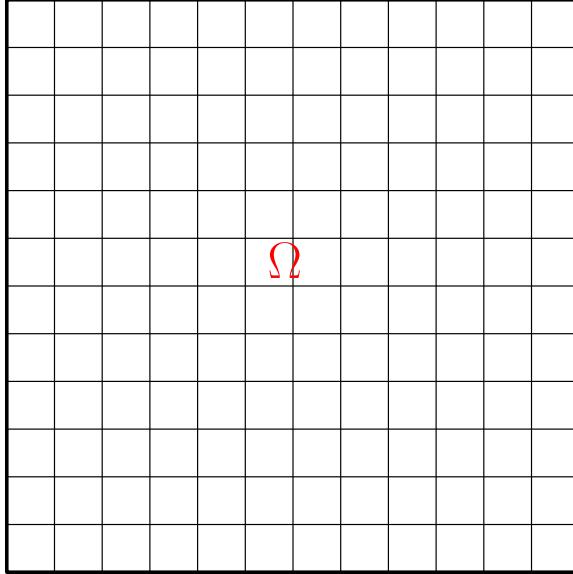


Figure 4.3: The total computational domain.

In the second step, the solid dot represents the unknown variables along the interface. They are shared by adjacent subdomains and should be calculated in the global matrix through Schur complement method. Even though the number of DOF in global matrix is drastically decreased, the communication overhead and scale of global matrix are still not satisfied the standard for high performance computing. In this way, we shall continuously split the interface into two spaces.

In the third graph, we split the interface into primal and dual spaces. The circle represents the unknown variables belongs to dual space. They are calculated only in local matrices. We bridged the information from dual space to primal space through preconditioner. The remain dots are unknown variables in the primal spaces. They are the only information shall be communicated and calculated through MPI functions. Now the global matrix has been decreased to an optimal level which benefit us significantly in speedup test.

One significant feature of this method is that when the number of subdomains increasing, the condition number of this method is bounded under the circumstance that an appropriate choice of the coarse DOFs and with regular subdomain shapes.

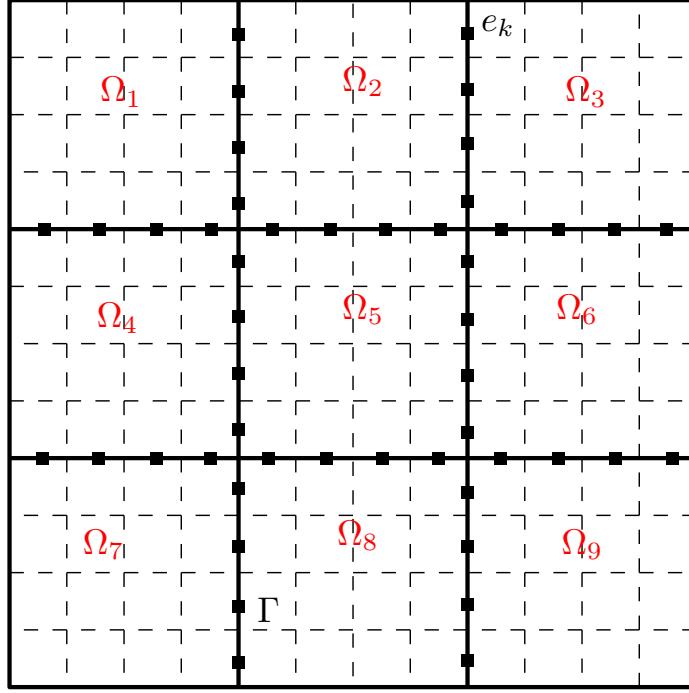


Figure 4.4: After the Schur complement method the computational domain becomes interior and interface.

The condition number grows only very slowly with the number of elements in each subdomain.

The number of iterations is also bounded in the same fashion. Meanwhile, the method scales well with the number of subdomains and size of the problem.

4.3 Schur Complement Method for subdomain Ω_j

Denote the weak Galerkin solution on each subdomain Ω_j . For the consistency with Equation (1), we will use u_h to represent the weak Galerkin solution on each Ω_j .

To define the Schur complement system, the degrees of freedom u_h on each subdomain Ω_j are partitioned into interior u_I and interface u_Γ parts. Then, we can rewrite the unknown variable function as

$$u_h = [u_{I0}, u_{Ib}, u_{\Gamma b}] \quad (4.50)$$

and denote $u_I = [u_{I0}, u_{Ib}]$, meanwhile, the $u_\Gamma = u_{\Gamma b}$. Consequently, the local Schur

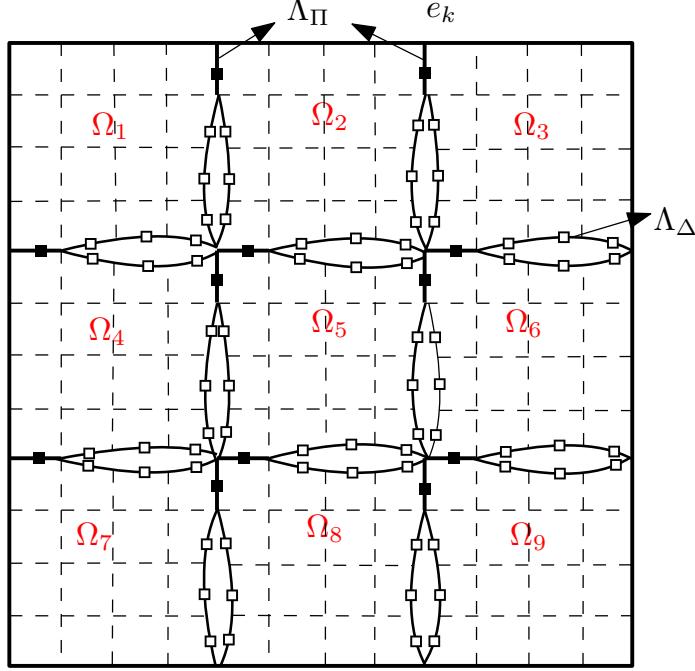


Figure 4.5: BDDC computational domain with only one cell boundary.

complements can be applied to each subdomain in the following form

$$\begin{pmatrix} A_{II} & A_{\Gamma I}^T \\ A_{\Gamma I} & A_{\Gamma\Gamma} \end{pmatrix} \times \begin{pmatrix} u_I \\ u_\Gamma \end{pmatrix} \quad (4.51)$$

To define the Schur complement system, the DOFs on each subdomain are partitioned by interior and interface categories. Now the unknown function becomes $u_h = [u_{I0}, u_{Ib}, u_{\Gamma b}]$, $v_h = [v_{I0}, v_{Ib}, v_{\Gamma b}]$ and denote the interior unknown variable $u_i = [u_{I0}, u_{Ib}]$ on the interface we have $u_\Gamma = u_{\Gamma b}$. For the assistant function the same rule applied to the assistant function $v_I = [v_{I0}, v_{Ib}]$, $v_\Gamma = v_{\Gamma b}$. The matrix form includes the assistant function should be following

$$\begin{pmatrix} A_{II}^u & (A_{\Gamma I}^u)^T & 0 & 0 \\ A_{\Gamma I}^u & A_{\Gamma\Gamma}^u & 0 & A_{\Gamma\Gamma}^{uv} \\ 0 & 0 & A_{II}^v & (A_{\Gamma I}^v)^T \\ 0 & A_{\Gamma\Gamma}^{uv} & A_{\Gamma I}^v & A_{\Gamma\Gamma}^v \end{pmatrix} \begin{pmatrix} u_I \\ u_\Gamma \\ v_I \\ v_\Gamma \end{pmatrix} \quad (4.52)$$

then we apply Schur complement method to eliminate the interior unknowns which will give the following equations

The interface stiffness matrix has the form as following

$$S_{\Gamma\Gamma}^j = A_{\Gamma\Gamma}^{(j)} - \left[A_{\Gamma I}^{(j)} \right] \times \left[A_{II}^{(j)} \right]^{-1} \times \left[A_{\Gamma I}^{(j)} \right]^T \quad (4.53)$$

The loading force along the interface has the form

$$f_{\Gamma}^{(j)} = b_{\Gamma}^{(j)} - \left[A_{\Gamma I}^{(j)} \right] \times \left[A_{II}^{(j)} \right]^{(-1)} \times b_I^{(j)} \quad (4.54)$$

Then denote the assembled matrix form

$$S_{\Gamma\Gamma} = \sum_{j=1}^N R_{\Gamma}^{(j)T} S_{\Gamma}^{(j)} R_{\Gamma}^{(j)} \quad (4.55)$$

where R_{Γ}^j is the mapping vector to convert unknown variables between Γ global interface to Γ_i interfaces on subdomains Ω_j

Therefore, the global interface problem is constructed as

$$S_{\Gamma\Gamma} \begin{pmatrix} u_{\Gamma} \\ v_{\Gamma} \end{pmatrix} = \begin{pmatrix} f_{\Gamma}^u \\ f_{\Gamma}^v \end{pmatrix} \quad (4.56)$$

4.4 BDDC Preconditioner

Now, we eliminate most of the continuity across the interfaced, refers to Fig 5, and construct the BDDC preconditioner for the inverse of matrix S_{Γ} .

In our BDDC formulation, the primal constraints are introduced over edges/faces. To define the BDDC preconditioner for the Schur complement problem, the interface space $\Lambda_{\Gamma}^{(j)}$ s is a partitioned into two spaces dual, $\Lambda_{\Delta}^{(j)}$ and primal, $\Lambda_{\Pi}^{(j)}$. The dual space, $\Lambda_{\Delta}^{(j)}$, corresponds to the subset of function in $\Lambda_{\Gamma}^{(j)}$.

We define the partially assembled space as:

$$\hat{\Lambda}_\Gamma = \hat{\Lambda}_\Pi \oplus \left(\sum_{i=1}^N \Lambda_\Delta^{(j)} \right) \quad (4.57)$$

where $\hat{\Lambda}_\Pi$ is the assembled global primal space, single valued on Γ , which is formed by assembling the local primal space, $\Gamma_\Pi^{(j)}$. The BDDC preconditioner has been viewed as solving a finite element problem on partially assembled finite element space, $\hat{\Lambda}_\Gamma$, to precondition the Schur complement problem whose solution lies in the fully assembled space $\hat{\Lambda}_\Gamma$.

The key component of BDDC preconditioner [22]:

- An averaging operator which restricts functions from Λ_Γ to $\hat{\Lambda}_\Gamma$
- A positive scaling factor $\delta_i^\dagger(e_k)$ is defined for each interface e_k of the subdomain Ω_j such that $\delta_i^\dagger(e_k) + \delta_j^\dagger(e_k) = 1$ where $e_k = \partial\Omega_i \cap \partial\Omega_j$
- Define $D_\Gamma^{(i)}$ as the diagonal matrix formed by setting the diagonal entries corresponding to each nodal degree of freedom on e_k to $\delta_i^\dagger(e_k)$
- Define $R_{D,\Gamma} : \hat{\Lambda}_\Gamma \rightarrow \Lambda_\Gamma$ as the product of $R_{D,\Gamma} := D_\Gamma R_\Gamma$

The BDDC preconditioner has the following form that

$$M_{\Gamma_{BDDC}}^{-1} = R_{D,\Gamma}^T \tilde{S}_{\Gamma\Gamma}^{-1} R_{D,\Gamma} \quad (4.58)$$

We interpret the above equation by using the unknown variable function $u_\Gamma =$

$[u_r, u_c]^T$ and the matrix can be written as

$$M = \begin{pmatrix} S_{rr}^{(1)} & 0 & 0 & \cdots & 0 & S_{rc}^{(1)} \\ 0 & S_{rr}^{(2)} & 0 & \cdots & 0 & S_{rc}^{(2)} \\ 0 & 0 & S_{rr}^{(3)} & \cdots & 0 & S_{rc}^{(3)} \\ \vdots & \vdots & \vdots & \ddots & \vdots & \vdots \\ 0 & 0 & 0 & \cdots & S_{rr}^{(N)} & S_{rc}^{(N)} \\ S_{cr}^{(1)} & S_{cr}^{(2)} & S_{cr}^{(3)} & \cdots & S_{cr}^{(N)} & S_{cc} \end{pmatrix} \quad (4.59)$$

the subscript c represents the unknown variables of constraints. The r represents the rest of unknown variables in computational subdomains.

The Lanczos matrix is applied to estimate the upper and lower eigenvalue bounds.

The matrix is in a tridiagonal form and generated from the PCG iterations.

The BDDC method can be written as the form with preconditioner as

$$M_{\Gamma BDDC}^{-1} S_{\Gamma\Gamma} u_{\Gamma} = M_{\Gamma BDDC}^{-1} f_{\Gamma} \quad (4.60)$$

4.5 WG-BDDC Method Implementation

In the Equation (28), we can expand the constraints matrix as

$$S_{cc} = \sum_{i=1}^N A_{\Pi\Pi}^{(j)} \quad (4.61)$$

meanwhile, the rest unknown variables matrix can be written in

$$S_{rr}^{(i)} = \begin{bmatrix} A_{II}^{(j)} & A_{I\Delta}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta\Delta}^{(j)} \end{bmatrix} \quad (4.62)$$

The implementation of the WG-BDDC algorithm is presented as following:

$$\hat{R}_{D,\Gamma}^T \left\{ R_{\Gamma,\Delta}^T \left(\sum_{j=1}^N \begin{bmatrix} 0 & R_{\Delta}^{(j)T} \end{bmatrix} \begin{bmatrix} A_{II}^{(j)} & A_{I\Delta}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta\Delta}^{(j)} \end{bmatrix}^{-1} \begin{bmatrix} 0 \\ R_{\Delta}^{(j)} \end{bmatrix} \right) R_{\Gamma\Delta} + \Phi S_{\Pi}^{-1} \Phi^T \right\} \hat{R}_{D,\Gamma} \quad (4.63)$$

with

$$\Phi = R_{\Gamma\Pi}^T - R_{\Gamma\Delta}^T \sum_{j=1}^N \begin{bmatrix} 0 & R_{\Delta}^{(j)T} \end{bmatrix} \begin{bmatrix} A_{II}^{(j)} & A_{I\Delta}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta\Delta}^{(j)} \end{bmatrix}^{-1} \begin{bmatrix} A_{\Pi I}^{(j)T} \\ A_{\Pi\Delta}^{(j)T} \end{bmatrix} R_{\Pi}^{(j)} \quad (4.64)$$

and

$$S_{\Pi} = \sum_{j=1}^N R_{\Pi}^{(j)T} \left\{ A_{\Pi\Pi}^{(j)} - \begin{bmatrix} A_{\Pi I}^{(j)} & A_{\Pi\Delta}^{(j)} \end{bmatrix} \begin{bmatrix} A_{II}^{(j)} & A_{I\Delta}^{(j)} \\ A_{\Delta I}^{(j)} & A_{\Delta\Delta}^{(j)} \end{bmatrix}^{-1} \begin{bmatrix} A_{\Pi I}^{(j)T} & A_{\Pi\Delta}^{(j)T} \end{bmatrix} \right\} R_{\Pi}^{(j)} \quad (4.65)$$

here the S_{Π} is the global coarse system matrix.

The preconditioned conjugate gradient is applied to solving above linear system.

Theoretically, the condition number should be bounded as

$$\kappa(M_{\Gamma_{BDDC}}^{-1} \hat{S}_{\Gamma\Gamma}) \leq C(1 + \log(\frac{kH}{h}))^2 \quad (4.66)$$

for a second order elliptic problem. The constant C is independent of solution order, p , element size h , and the subdomain size H . Thus, the condition number and hence number of iteration required to converge are independent of the number of subdomains and only weakly dependent on the solution order and the size of subdomains.

4.6 Preconditioned Conjugate Gradient Method

The conjugate gradient (CG) method is a well-known iterative method for solving large-scale symmetric and positive definite linear systems. The method is straightforward to implement and has the capability to handle complex domains and boundary conditions.

The preconditioned conjugate gradient method has been reported by Bramble and Pasciak [8] to iteratively solving the symmetric saddle point problems. It inherits all great features of CG method and extends it to a higher level. This method is applied to a sparse system which is too large to handle by a direct method such as the Cholesky decomposition.

The details of PCG method is discussed by the following chart step by step. In terms of preconditioned, the major effort is to assemble the global preconditioner matrix. Then CG similar method is applied to solve the small global matrix. Thus, we can obtain the global corner solution with minimum overhead. Since both global and local matrices are sparse, the open source library LAPACK/BLAS [2] benefits the matrices calculation substantially.

The algorithm of PCG method is following:

Algorithm 1: Preconditioned Conjugate Gradient Algorithm

Input : $r_0 := b - Ax_0$

$$z_0 := M^{-1}r_0$$

$$p_0 := z_0$$

$$k := 0$$

1 Repeat ;

2

$$\alpha_k := \frac{r_k^T z_k}{p_k^T A p_k}$$

$$x_{k+1} := x_k + \alpha_k p_k$$

$$r_{k+1} := r_k - \alpha_k A p_k$$

if r_k is sufficiently small **then**

3 | return x_{k+1} ;

4 **else**

5

$$z_{k+1} := M^{-1}r_{k+1}$$

$$\beta_k := \frac{z_{k+1}^T r_{k+1}}{z_k^T r_k}$$

$$p_{k+1} := z_{k+1} + \beta_k p_k$$

$$k := k + 1$$

6 **end**

The condition number is calculated from Lanczos matrix. The global preconditioner matrix M is transferred into a tridiagonal matrix T_{mm} . When the m is equal to the dimension of M , T_{mm} is similar to M . Then we calculate the eigenvalues of

T_{mm} and obtain the condition number from calculating the ratio of the maximum and minimum eigenvalue.

4.7 Parallel Computing Scheme

Message Passing Interface (MPI) [30] is portable and widely used as the communicator. We applied MPI to exchange the information between each non-overlapping subdomain. MPI provides a standard set of Fortran subprogram definitions. Intel MKL supports the modern MPI version which allows us to migrate the software on a variety of platforms. Besides, the MPI subprograms introduce the minimum overhead in both coding and testing stages.

The workflow of parallel computing scheme is following:

1. MPI communicator initiates work and distribute the parameters and mesh data to all the processors.
2. In every processor, the connectivity is analyzed and the local elemental matrices are constructed.
3. Through MPI subprograms, the local matrices are communicated and global preconditioner is constructed on every processor.
4. The global problem, whose size is significantly small, is calculated on every processor with PCG linear solver.
5. The global solution is reduced to each processor for the local solution recovery.

The software has been tested on the George Washington University cluster, ColonialOne, with Xeon E5-2650v2 2.6 GHz 8-core processors with 128 GB of RAM each.

4.8 Numerical Results

4.8.1 Poisson Equation

The WG element can choose different order of basis function in weak gradient equation. Hence, we test the combination order of interior, boundary and weak

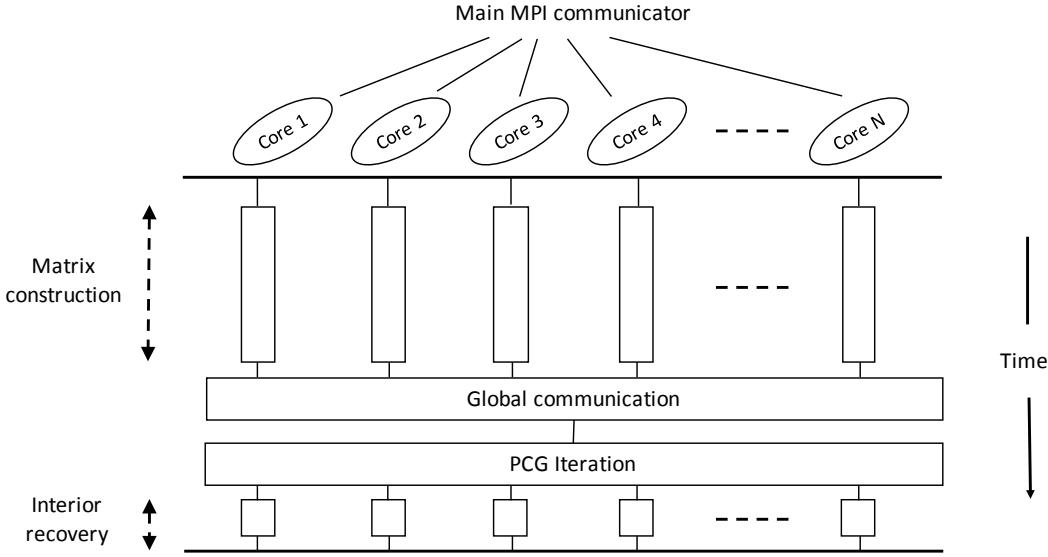


Figure 4.6: Parallel computing work flow.

gradient shape functions.

The WG element can choose different order of basis function in weak gradient equation among gradient, interior and boundary basis functions. Hence, we test the different combination orders of interior, boundary and weak gradient shape functions.

The Poisson equation $-\nabla \cdot (\nabla \mathbf{u}) = \mathbf{f}$ is considered in the test. Let $\Omega = (0, 1) \times (0, 1)$, $a = I$, and f are chosen such that the exact solution is $u = \sin(\pi x)\sin(\pi y)$

We choose different weak Galerkin elements for validating our WG-BDDC numerical scheme. The unit square is firstly decomposed into $N \times N$ subdomains as the coarse mesh with length $H = 1/N$. In every subdomain, all elements are further triangulated into $2n \times n$ triangles, and the finite mesh has the size $h = 1/(N \times n)$. The preconditioned system is solved by the PCG solver. In every iteration, the L^2 -norm of the residual is reduced by a factor of 10^{-6} .

#sub	$k = 1$						$k = 2$					
	Cond.	Iter.	L^2 -error	O	Cond.	Iter.	L^2 -error	O	Cond.	Iter.	L^2 -error	O
4×4	2.217	5	1.6013e-3	-	3.528	8	7.1456e-5	-				
8×8	2.390	9	3.9939e-4	2.0	3.803	10	8.9214e-6	3.0				
16×16	2.335	8	9.9789e-5	2.0	3.768	10	1.1150e-6	3.0				
32×32	2.325	8	2.4944e-5	2.0	3.758	10	1.3938e-7	3.0				

Table 4.1: The accuracy and convergence properties of the $P_k P_{k-1} P_{k-1}^2$ element on different numbers of subdomains where each subdomain has 128 triangular elements.

H/h	$k = 1$						$k = 2$					
	Cond.	Iter.	L^2 -error	O	Cond.	Iter.	L^2 -error	O	Cond.	Iter.	L^2 -error	O
4	1.722	7	1.6013e-3	-	2.900	10	7.1456e-5	-				
8	2.390	9	3.9939e-4	2.0	3.803	10	8.9214e-6	3.0				
16	3.245	10	9.9789e-5	2.0	4.957	12	1.1150e-6	3.0				
32	4.239	11	2.4944e-5	2.0	6.218	13	1.3938e-7	3.0				

Table 4.2: The accuracy and convergence $P_k P_{k-1} P_{k-1}^2$ scheme on different number of fine element in each subdomain. The entire computational domain is partitioned into 64 non-overlapping subdomains.

The first test is implemented for weak Galerkin element with $u_0 \in P_k, u_b \in P_{k-1}$, and $\nabla_w u \in P_{k-1}$. The Tables 4.1 and 4.2 show the condition number of Lanczos matrix and the iteration number in the PCG solver. From the Table 4.1 and Table 4.2, we can see that the condition number is independent of the number of subdomain, while it depends on the resolution of mesh, H/h , as $(1 + \log(\frac{H}{h}))^2$. In the mean time,

the communication overhead between each adjacent subdomain does not introduce any error to the results. With the increasing of number of subdomains, we obtain stable second and third order accuracy results. The iteration number indicates that the increasing of parallel computing expense is much lower than the gain from domain decomposition. It comes from that the size of global matrix grows slower than the DOFs in primal space.

#sub	$k = 1$			$k = 2$				
	Cond.	Iter.	L^2 -error	Cond.	Iter.	L^2 -error		
4×4	2.451	7	1.0109e-3	-	3.805	8	6.6333e-5	-
8×8	2.648	9	2.5117e-4	2.0	4.003	12	8.2709e-6	3.0
16×16	2.629	9	6.2696e-5	2.0	3.943	12	1.0334e-6	3.0
32×32	2.617	9	1.5668e-5	2.0	3.917	12	1.2917e-7	3.0

Table 4.3: The accuracy and convergence properties of the $P_k P_k P_{k-1}^2$ element on different numbers of subdomains where each subdomain has 128 triangular elements.

H/h	$k = 1$ and #sub=64			$k = 2$ and #sub=64				
	Cond.	Iter.	L^2 -error	O1	Cond.	Iter.	L^2 -error	O
4	1.968	8	1.0109e-3	-	3.926	11	6.6333e-5	-
8	2.648	9	2.5117e-4	2.0	4.003	12	8.2709e-6	3.0
16	3.529	10	6.2696e-5	2.0	5.084	13	1.0334e-6	3.0
32	4.619	12	1.5668e-5	2.0	6.329	13	1.2918e-7	3.0

Table 4.4: The accuracy and convergence $P_k P_k P_{k-1}^2$ scheme on different number of fine element in each subdomain. The entire computational domain is partitioned into 64 non-overlapping subdomains.

The second test is the weak Galerkin element with order $u_0 \in P_k$, $u_b \in P_k$ and $\nabla_w u \in P_{k-1}$. In Table 4.3 and Table 4.4 the condition number has the identical pattern of the theoretical convergence rate. Comparing with the first example, we can find the convergence rates and the orders of accuracy have optimal agreement to the degree of polynomial in u_0 . We also measure the scalability of this solver can be applied on the number of processors are used is up to 1,024.

#sub	$k = 1$				$k = 2$			
	Cond.	Iter.	L^2 -error	O	Cond.	Iter.	L^2 -error	O
4 × 4	3.671	8	2.1451e-4	-	4.620	8	6.7627e-6	-
8 × 8	3.965	10	5.2129e-5	2.0	4.987	12	7.8998e-7	3.0
16 × 16	3.934	10	1.2937e-5	2.0	4.921	12	9.6925e-8	3.0
32 × 32	3.922	10	3.2281e-6	2.0	4.901	12	1.2058e-8	3.0

Table 4.5: The accuracy and convergence properties of the $P_k P_k P_k^2$ element on different numbers of subdomains where each subdomain has 128 triangular elements.

H/h	$k = 1$				$k = 2$			
	Cond.	Iter.	L^2 -error	O	Cond.	Iter.	L^2 -error	O
4	3.024	10	2.1451e-4	-	3.859	11	6.7628e-6	-
8	3.965	10	5.2129e-5	2.0	4.987	12	7.8998e-7	3.0
16	5.153	12	1.2937e-5	2.0	6.235	13	9.6931e-8	3.0
32	6.472	14	3.2281e-6	2.0	7.673	15	1.2060e-8	3.0

Table 4.6: The accuracy and convergence $P_k P_k P_k^2$ scheme on different number of fine element in each subdomain. The entire computational domain is partitioned into 64 non-overlapping subdomains.

In Tables 4.5 and 4.6, we implement the weak Galerkin element $u_0 \in P_k$, $u_b \in P_k$ and $\nabla_w u \in P_k$. The $P_k P_k P_k^2$ element deliveries the most accurate result among all three types of element. The reason is that all three shape functions of interior variable, boundary variable and weak gradient have the same order.

4.8.2 Linear Elastic Equation

We consider the linear elastic equation (1) in the square domain $\Omega = (0, 1)^2$ which is decomposed into uniform square subdomains with size H . For each subdomain, it is partitioned into uniform quadrilateral mesh with size h . The exact solution is given by

$$u = \begin{pmatrix} \sin(2\pi x)\sin(2\pi y) \\ 1 \end{pmatrix} \quad (4.67)$$

Table 4.7: Case 1: Performance with $P_1P_1P_1^2$, $\lambda = 1, \mu = 0.5$

#sub	$H/h = 8$				H/h	$k = 1$ and #sub=64			
	Cond.	Iter.	L_{Max} -error	O		Cond.	Iter.	L^2 -error	O
2×2	2.281	8	8.484e-2	-	4	2.212	11	2.993e-1	-
4×4	3.922	12	2.1787e-2	1.96	8	3.069	12	8.567e-2	1.9
8×8	4.895	17	5.4706e-3	1.99	16	4.143	13	2.217e-2	2.0
16×16	5.238	17	1.3675e-3	2.00	32	5.437	15	5.575e-3	2.0

We test the performance of WG-BDDC on cluster and present the speedup figure which indicates the superlinear acceleration and good scalability.

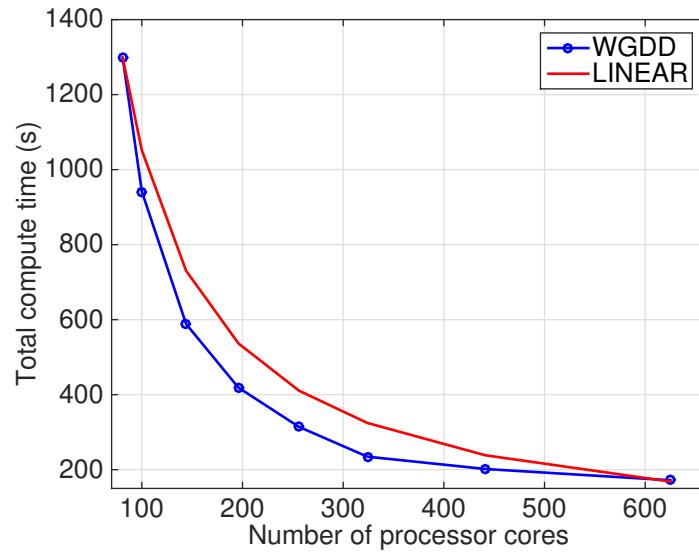


Figure 4.7: The running time .vs. number of processors.

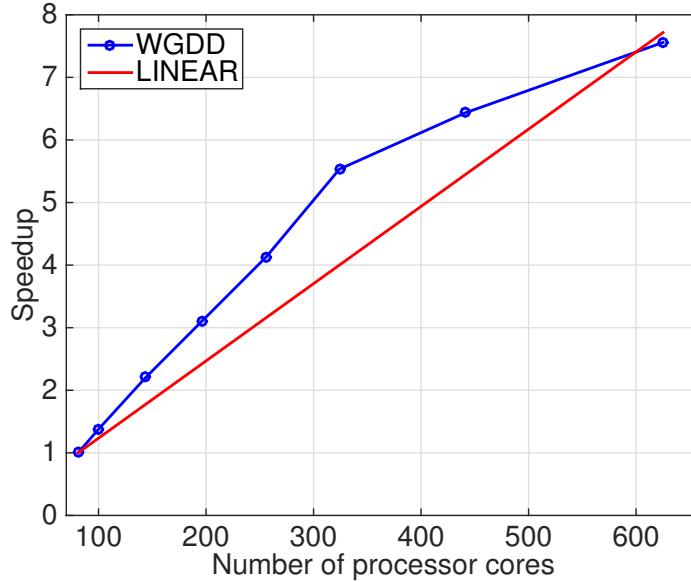


Figure 4.8: The speedup .vs. number of processors.

The blue dot line called WGDD represents WG-BDDC method. we can see from the above figures that the superlinear speedup is captured within the increasing of the number of subdomains. The main concern for BDDC method is that the balance between global matrix, the preconditioner, and the local matrices. It is important to estimate the size of preconditioner and choose proper number of processors.

we report a novel parallel computing method. This method integrated the newly developed weak Galerkin finite element method with balancing domain decomposition with constraints. The MPI library is set up for information communication between each processor. The optimal convergence rate and promising scalability of this WG-BDDC method are observed.

To this method, it is convenient to implement high order element. We have multiple choices on interior, boundary and gradient basis functions which is an obvious flexibility to numerical calculation. We can design the element type based on our need. From the optimal convergence rate, we can conclude that this method is robust and highly compatible to different element orders.

For the results, all test cases have well bounded condition numbers for the global

matrices which fits our initial assumption properly. Meanwhile, the superlinear feature from the speedup graph is also in favor of high performance computing. This is the first attempt to introduce weak Galerkin to engineering purpose implementation. The optimal performance on parallel computing indicates a promising future of this method.

Appendix A: Numerical Simulations of Stenotic Flow

Peripheral arterial occlusive diseases, especially in femoral arteries are commonly seen by clinical doctors in the U.S.[50, 7, 33]. These patients may have critical stenoses or multiple sequential moderate stenoses. The current clinical practice usually recommends simple balloon angioplasty[21] to treat critical stenotic lesion, i.e. $> 60\%$ luminal area reduction. However, it is a difficult process for the doctors to make a decision on whether or not to treat multiple sequential moderate stenoses using the stent or balloon angioplasty. This is partially because of the lack of methods to measure how these multiple stenoses affect their downstream blood flow. The scientific study of fluid dynamics associated with multiple stenoses is far less than that of a single stenosis. To assist on making clinical decision for treating multiple stenoses, a quantitative approach, for instance, the computational fluid dynamics (CFD) [28] method is very much needed. In this study, we present a high-fidelity CFD simulations of the blood flow in the stenotic arteries using idealized geometries. We solve unsteady incompressible Navier-Stokes equations[68] using unstructured meshes with all hexahedral elements. The upstream flow condition is prescribed which gives a Reynolds number of 500. Our simulations results reveal several new discoveries of fluid dynamics in these multiple stenoses considering a range of different geometric parameters.

A.1 Background

Peripheral arterial occlusive disease are the major cause of amputation in the U.S. as they are prevalent among smokers, diabetics, hypertensives, and patients with dyslipidemia. Nowadays, since the disease can be visualized as areas of stenosis or occlusion on a diagnostic arteriogram, simple balloon angioplasty with or without stenting is usually recommended for treating critical stenotic lesions, i.e. $> 60\%$ (by area) luminal reduction. However, the clinical decision is rather difficult on whether or not

to treat subcritical stenotic lesions, i.e., < 50% luminal reduction, in particular, multiple sequential moderate stenoses. The risks of routine balloon angioplasty/stenting include intimal injury-induced acute arterial thrombosis or restenosis[32] from neointimal hyperplasia[41], etc. These risks led to the difficulty of telling the benefits of treating mildly stenotic lesions using routine clinical treatment. Meanwhile, there is no existing method to measure the effects of multiple sequential moderate stenoses on blood flow in the part of the artery that is downstream of the stenotic lesions. The closest measurement that correlated with change in blood flow is the change in arterial pressures. If the arterial pressure drop across the stenoses is greater than the clinical threshold (20-30mmHg) [53], the lesions are considered physiologically significant enough to favor treatment. However, it's clinically very difficult to quantify. Consequently, CFD simulation is a helpful supplement for the stenosis study.

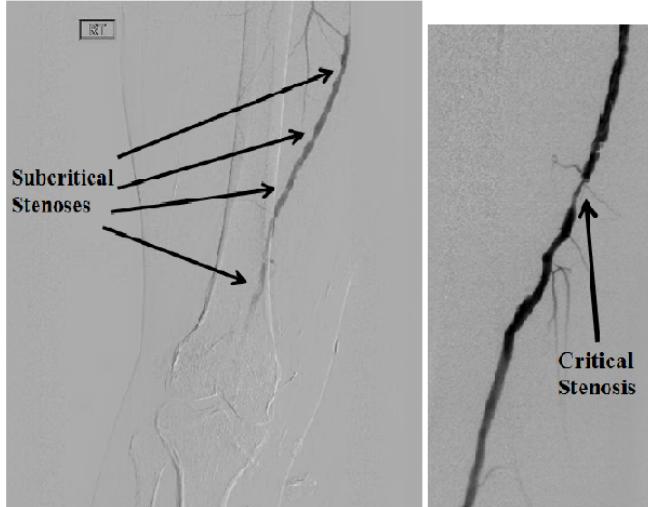


Figure A.1: Contrast of subcritical and critical stenoses.

A good amount of investigation have been addressed on pressure drop across single stenosis from both experimental and analytical perspective[63, 71, 80, 79, 66, 72]. At the beginning, investigators derived empirical function from conservation equations with the experimental coefficients for both steady and pulsatile flow condition. The empirical function is a simplified estimation for pressure drop across single stenosis.

A series computational simulations and experiments rendered the details of flow field across constriction. Flanigan et al[29] conducted a series of experiments and proposed non-linear relationship between number of applied stenoses and pressure drop across the stenoses. Bertolotti et al[5], using finite element method, simulated the pressure drop and velocity field through two adjacent stenoses at very low Reynolds number.

We have implemented an efficient in-house CFD package which has the capability to simulate more complex 3D geometries. This paper reports a range of parametric studies including varying the number of stenoses, the narrowing degrees, the shapes, and streamwise spacing. The object is providing to doctors an accessible effective approach which can predict the pressure and the velocity field of stenotic arteries. These stenotic arteries consist of patient-specific geometries which are challenging to define computationally. These arteries are typically simplified as axisymmetric constriction in straight tubes. The 3D computational domain is represented by unstructured meshes with all hexahedral cells. An efficient pressure-based Finite Volume Method(FVM)[46] was implemented to solve these equations. Our simulations include computational geometries with a wide range of narrowing degrees of stenoses, from 40% to 80% luminal area reduction. The number of stenoses ranges from 1 to 7. Several different spatial intervals were considered between adjacent stenoses.

A.2 Numerical Method

The unsteady incompressible Navier-Stokes equations, describing the conservation of mass and momentum in the computational region, are discretized using a second-order central differencing scheme in space and a Crank-Nicolson[10] method in time. The pressure and velocity are stored in cell centers using the collocated method. Pressure-velocity coupling is dealt by a Rhee-Chow interpolation method[65] and a PISO algorithm for pressure correction[37].

Mass conservation and Navier-Stokes equations as following:

$$\frac{\partial u_i}{\partial x_i} = 0, \quad (\text{A.1})$$

$$\frac{\partial u_i}{\partial t} + \frac{\partial u_i \partial u_j}{\partial x_j} = -\frac{1}{\rho} \frac{\partial P}{\partial x_i} - \frac{\partial \tau_{ij}}{\partial x_j} + \nu \frac{\partial^2 u_i}{\partial x_j \partial x_i}, \quad (\text{A.2})$$

where the index $i = 1, 2, 3$ represents three directions in the Cartesian coordinate system, P is the pressure, ρ is density, and ν is kinematic viscosity.

A.3 Geometry and Condition

A.3.1 Geometry

The geometry of stenoses along peripheral artery is complex and irregular. A symmetrical constriction in a straight cylindrical tube is a proper idealization method to simplify the complicated problem[49]. The shape of the stenoses are optimized by using third order degree polynomial. In the following figure shows three sequential 50% degree stenoses along the straight vessel.

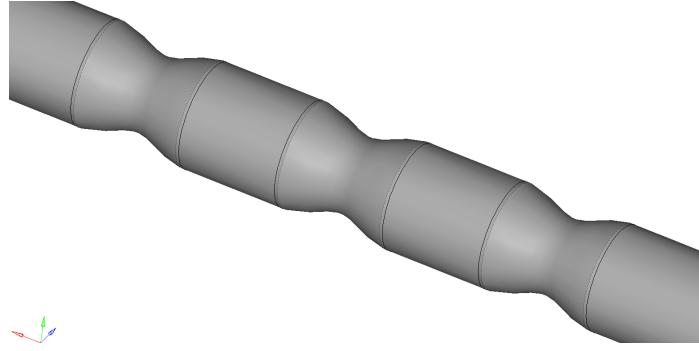


Figure A.2: Geometry of sequential stenoses.

A.3.2 Mesh

We implement unstructured hexahedral meshes for idealized stenoses geometry. Internal mesh is total unstructured as the following figure. Moreover, we impose flour boundary layers along the surrounding wall for more accurate prediction of boundary

flow.

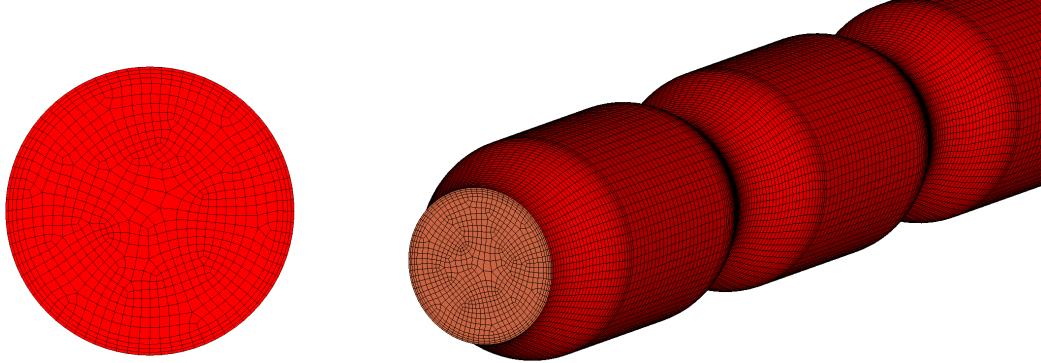


Figure A.3: Cross-sectional view of hexahedral meshes.

To better analyze the flow cross the stenotic area, we refine the longitudinal mesh across the stenosis. We increase the number of layers along x-axis direction for a better mesh adaptation of stenosis curve.

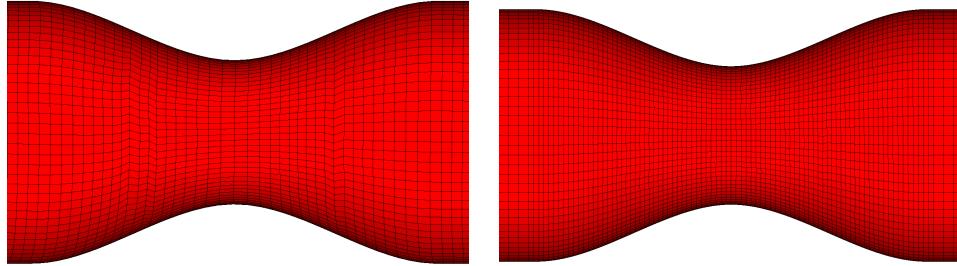


Figure A.4: Mesh refinement for stenotic area.

A.3.3 Dimensionless wall distance

The transition flow happens when the flow passing critical (i.e. $> 70\%$) stenosis. The dimensionless wall distance is defined as:

$$y^+ = \frac{u_* y}{\nu} \quad (\text{A.3})$$

where u_* is the friction velocity at nearest wall, y is the distance to the nearest wall, ν is the local kinematic viscosity of fluid. We control the $y^+ \leq 1$ after refine the mesh with boundary layers. This condition insure that viscosity plays an important

role rather than advection.

A.3.4 Conditions

Table A.1: Simulation condition parameters.

Variable	Value
Stream wise length	30D
Reference mesh points	720,000
Reynolds number for inlet	500
Maximum of CFL number	0.79

We implement parabolic velocity profile as inlet boundary condition, Neumann condition as the outlet boundary condition. We impose no-slip boundary condition on rigid and non-porous walls. The fluid is incompressible Newtonian with same mean Reynolds number as blood in peripheral arteries $Re_{mean} = 500$. The entrance length of 6 diameters is sufficient for flow development.

A.4 Verification

A.4.1 Straight vessel test

First of all, we implement a simulation on a straight tube without any constriction. For the straight tube, we use the following analytical function to calculate the pressure drop

$$\Delta P = \frac{128\mu l Q}{\pi D^4} \quad (\text{A.4})$$

We compare the pressure drop between simulation results and analytical solution. The error is less than 0.25%.

For the following cases, we use the value of pressure drop through the straight tube as a reference bar. All the parameters including absolute and stream-wise pressure drop are plotted over the reference value. The coordinate information are plotted over the diameter. Then all the parameters on the following figures are dimensionless.

Table A.2: Straight vessel test results.

Cases	Value
Simulation result	408.6
Analytical solution	409.9
Error	< 0.25%

A.4.2 Resolution independence test

In this section, we present the results of same geometry with two different mesh resolutions. The test cases are straight tubes with 10 diameters long.

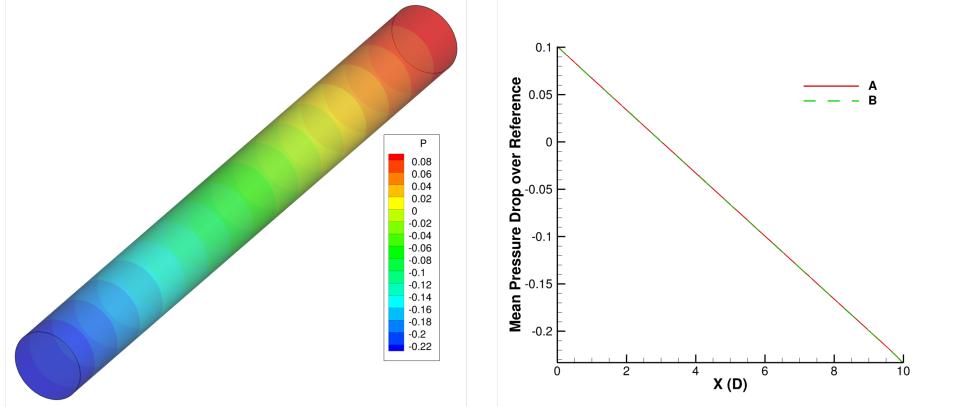


Figure A.5: Resolution independence test plot and results comparison.

The reference mesh grids number of case A is 240000. The resolution of case A is as same as the rest simulations of this paper. We double the number of mesh grid along the longitudinal direction. The finer mesh case B has 480000 grid points. Then we simulate both cases with our solver and compare the results with analytical solution.

The error of both cases are very low and neglectable. The resolution of case A is same as all the simulations in this paper. From this comparison we can conclude that our resolution is sufficient for pipe-flow study. It is a good compromise on accuracy and efficiency of simulations.

Table A.3: Resolution independence test results.

Cases	Value	Error
A	136.946	< 0.57%
B	137.211	< 0.76%
Analytical	136.169	0

A.5 Results

To analyze the hemodynamic effect caused by variant stenoses, we introduced two key parameters which describe the pressure field through stenotic artery. The first one is the stream-wise pressure drop which indicates the pressure difference from the inlet to the outflow area through the whole arterial domain. It describes the hemodynamic effect of stenoses among the entire flow domain. The doctors currently are using it as a prime clinical indicator to evaluate blood supplement to downstream bodies. Doctors are using this parameter to make the treatment decision. The other one is the absolute pressure drop which means the pressure difference between the maximum pressure value (at the inflow area) and minimum pressure value (at the last post-stenotic area). This parameter demonstrates the stenotic lesion induces a low pressure field concentrated at the post-stenotic area. The certain low pressure flow area is a potential serious damage source to the blood vessels which may leads to restenosis after the treatment.

A.5.1 Verification Empirical Solution

In Young and Tsai[80] study, the major factors controlling the pressure drop, Δp , across a single stenosis can be estimated from the following empirical equation:

$$\Delta p = \frac{K_v \mu}{D} U + \frac{K_t}{2} \left(\frac{A_0}{A_1} - 1 \right)^2 \rho |U| U \quad (\text{A.5})$$

where A_0 = area of the unobstructed tube, A_1 = minimum cross-sectional area of the stenosis, D = unobstructed tube, K_v and K_t = experimentally determined coefficients, L = length over which the pressure drop is measured, U = instantaneous velocity in the unobstructed tube, ρ = blood flow density, and μ = blood flow viscosity.

Young et al[79] pointed out that K_v and K_t are dependent on stenosis geometry and narrowing degree. K_v can be approximated from steady-flow tests with streamlined plug as single stenosis. The values of K_v in our simulations range from 630 to 2300 of the stenosis narrowing degree from 40% to 80% area reduction. Another empirical coefficient, K_u , gives the best fit of data is 1.2.

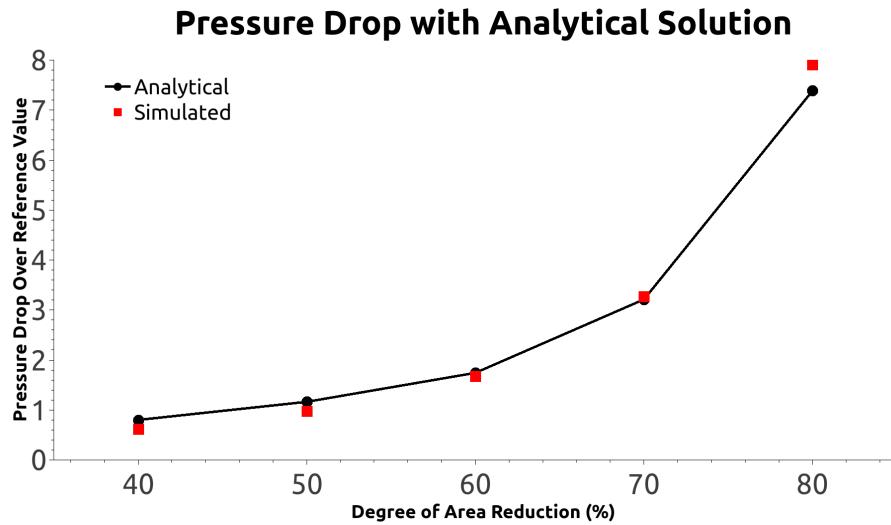


Figure A.6: Pressure drop after single stenosis results from analytic equation and simulation.

In an effort to obtain the theoretic solution, we calculated the pressure drop introduced by single subcritical and critical stenosis with using equation (4). The parameters including diameters, Reynolds number, viscosity, density and reduction area degrees are all identical for both analytical calculations and computational simulations. We plot the pressure drop introduced from single stenosis ranges from 40% to 80% stenotic narrowing degrees. The absolute pressure drop marker from simulations are very close to the theoretic calculations. The average error between two sets of

values are less than 20%.

A.5.2 Single stenosis evaluation

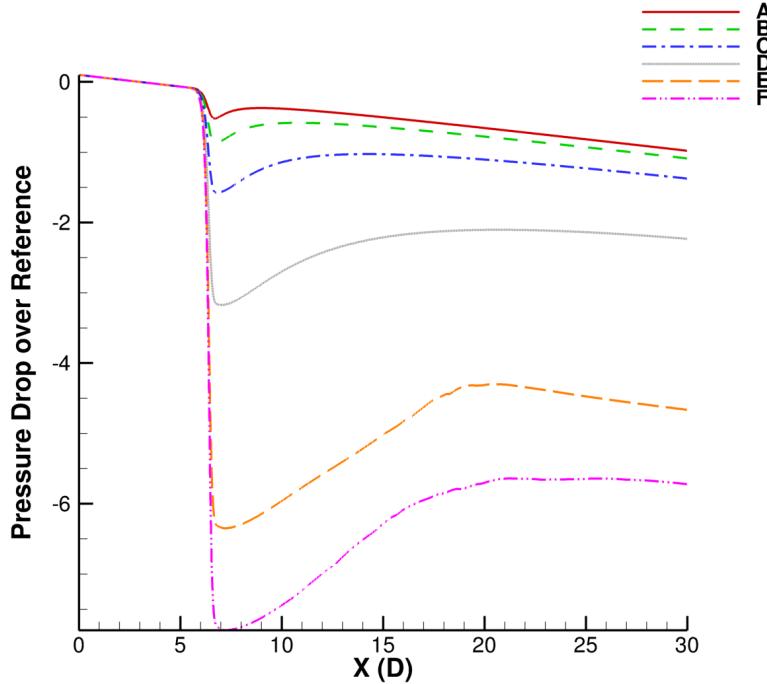


Figure A.7: Mean pressure drop of single stenosis for different blocking ratio

Table A.4: Mean pressure drop of single stenosis for different blocking ratio

Cases	Degree	Streamwise PD	Absolute PD
A	40%	1.080	0.622
B	50%	1.089	0.975
C	60%	1.476	2.027
D	70%	2.437	3.897
E	78%	4.766	7.543
F	80%	5.823	9.184

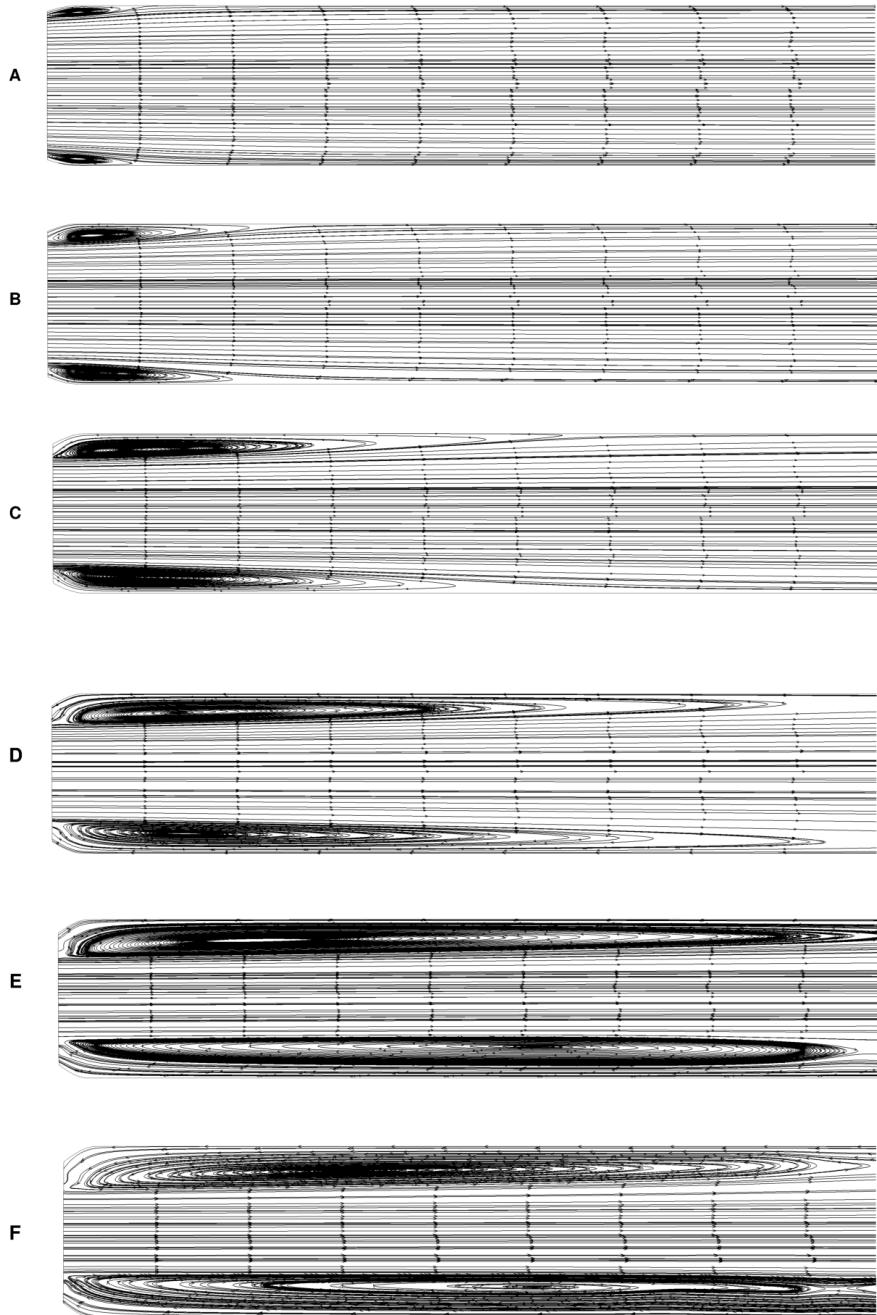


Figure A.8: Single stenosis post-stenotic streamlines

In Figure A.8, we conduct a series simulations that range from 40% to 80% narrowing degree and plot the results on Figure A.7 and Figure A.8. Figure A.7 demonstrates

the effects of single stenosis on the pressure drop from subcritical to critical regime. From the clinical perspective, when the area reduction ratio of stenosis is or over 60%, it can be treated as a critical one. However, according to the above chart, the centreline pressure along streamwise direction decreases slowly in the unobstructed tube due to the wall shear stress. After the entrance tract, the pressure decreases significantly when the flow approaching to the stenotic region then slowly recovers past the narrowest cross-sectional area. In the subcritical stenosis regime, the flow is mainly laminar, pressure fluctuation with decreasing and increasing is smooth and the scale is relative small. When the degree of stenosis is more than 60%, post-stenotic flow turns into transition, that produces large recirculation in the post-stenotic area. The more severe narrowing degree leads to larger pressure drop and longer recovery length. The above chart describes the detail of pressure distribution along streamwise direction and corresponds to the plot from empirical equation.

Figure A.8 presents the time-averaging both pressure field and velocity streamlines in the post-stenotic region. With the increasing of narrowing degree, the recirculation area increases and the pressure difference along the x direction becomes larger. From the case C to case D, the recirculation area grows rapidly. Moreover, the plots of case E and case F show that their flow fields are unsteady. As a result, when the narrowing degree is higher than 70%, the constriction leads the original steady flow to unsteady stage.

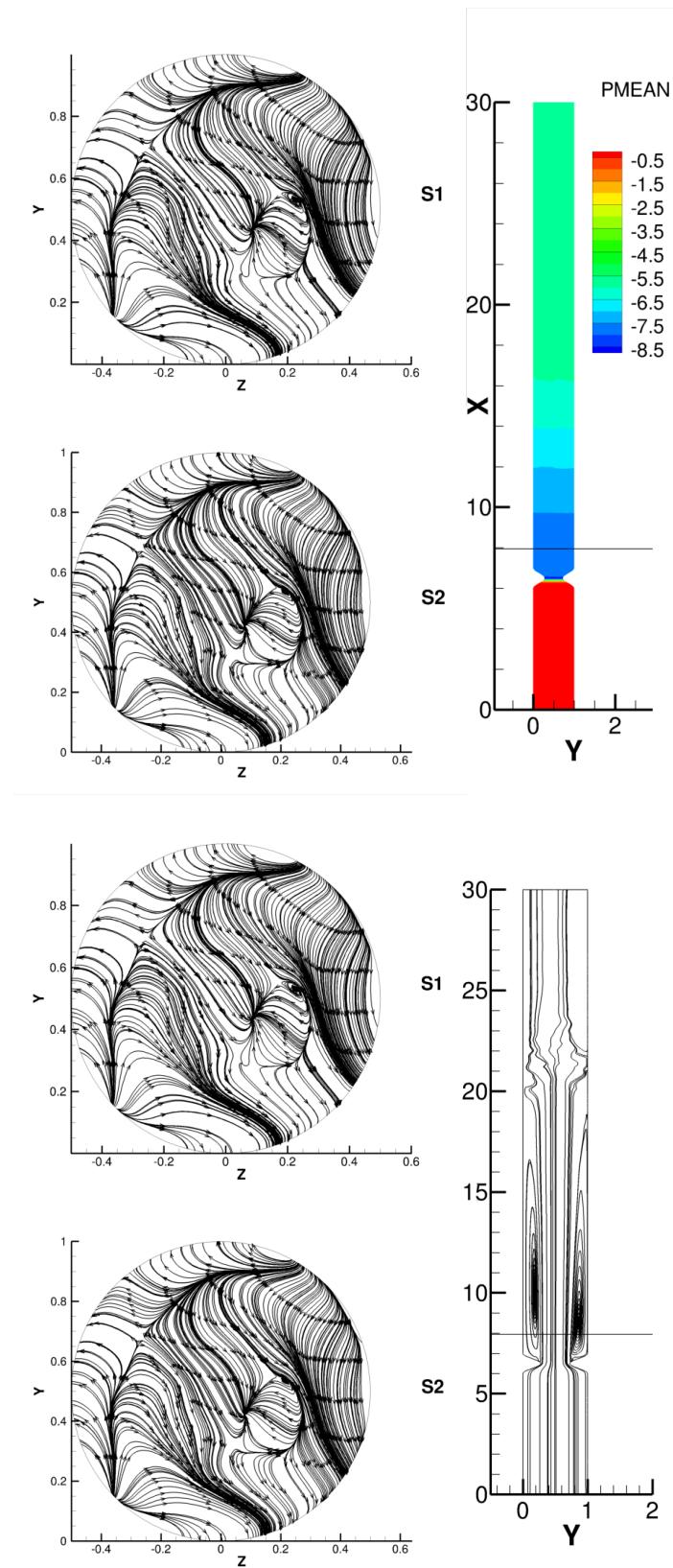


Figure A.9: Stream line comparison in post-stenotic area

To verify the flow pass critical stenosis turns into highly unsteady flow. We present the streamline of both the transitional and time-averaging streamline plots of 80% single stenosis in Figure A.9. In the downstream region, the recirculation exists as the length of 8-10 diameters along streamwise direction. After the recirculation region, the time-averaging streamlines are smoothly going to the outlet while the transitional streamlines are curvature and circuitous. This comparison indicates that the flow passing critical stenosis is unsteady. Meanwhile, the recirculation region streamwise length corresponds to the pressure recovery length.

The clinical criterion between subcritical and critical stenosis is varying from 60% to 75% narrowing degree. This criterion is a practical guidance for doctors making decisions on occlusive disease treatment. From the hemodynamic perspective, the non-laminar flow may cause endothelial dysfunction and increased thrombogenicity. For the healthy femoral arteries, the Reynolds number of blood flow is around 500. The stenosis accelerates flow speed and increases the Reynolds number. For flow in pipe of diameter D , experimental observations show that the laminar flow occurs when $Re_D < 2300$. The following Reynolds number equation can predict the flow pattern in stenotic area:

$$Re = \frac{\mathbf{Q}D_H}{\nu A} \quad (\text{A.6})$$

where \mathbf{Q} is the volumetric flow rate, A is the pipe cross-sectional area, ν is kinematic viscosity and D_H is the diameter. If the flow pass 50% diameter reduction (correspondingly 75% area reduction) stenosis, the Reynolds number increases from 500 to 1000.

According to our single stenosis simulation results in Figure A.8 and Figure A.9, when the narrowing degree is over 70%, the post-stenotic flow unsteadiness increases significantly. Also, the recirculation area grows rapidly with the narrowing degree

increasing.

In between case D and case F, we insert one 78% stenosis as case E. Although the difference of area narrowing degree between case E and case F is only 2%, the streamwise PD and absolute PD increase by 22.18% and 21.76%. When the narrowing degree is over the critical threshold, even slight magnitude change causes significant effect on flow field. The growth of pressure drop is not linear to the narrowing changing. For general stenotic cases, the definition of critical stenosis depends on variant flow condition. We present the CFD simulation which has the capability to predict the flow field and find out specific critical stenosis threshold.

A.5.3 Pressure drop between multiple sub-critical stenoses and single critical stneosis

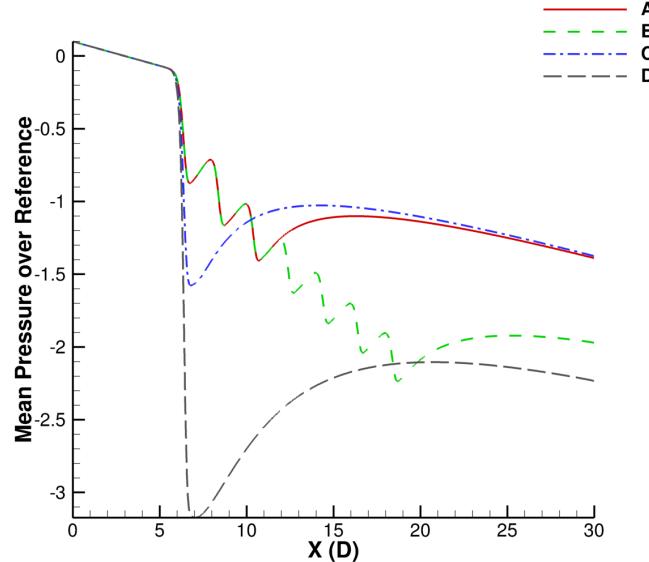


Figure A.10: Pressure drop among multiple subcritical stenoses

In Figure A.10 and Table A.5, we can find that the streamwise pressure drop of three and seven subcritical (50%) stenoses is higher than the pressure drop introduced by single critical (60%) stenosis (Figure 8). Therefore, the assumption of multiple sequential subcritical stenosis produces more pressure drop than single critical stenosis

Table A.5: Pressure drop among multiple subcritical stenoses

]

Cases	Degree	Streamwise PD	Absolute PD
A	Three 50%	1.471	—
B	Seven 50%	2.387	—
C	Single 60%	1.476	2.027
D	Single 70%	2.437	3.897

is proved by quantitative plot. We find that the pressure drop of both three 50% stenoses and one single 60% stenosis is closer while the flow propagating towards to the outlet, that because of the length of control vessel plays a non-neglectable role when we deciding the pressure drop difference. The pressure plot clearly illustrates pressure recovery region then we can put the check point out of that section. We also noticed that the absolute pressure drop of one single 60% stenosis is higher than that of any 50% stenoses. The more severe absolute pressure drop of critical stenosis introduces more unpleasant hemodynamic effect on vessels such as the larger wall shear stress applied on post-stenotic tube walls.

A.5.4 Multiple critical stenoses analysis

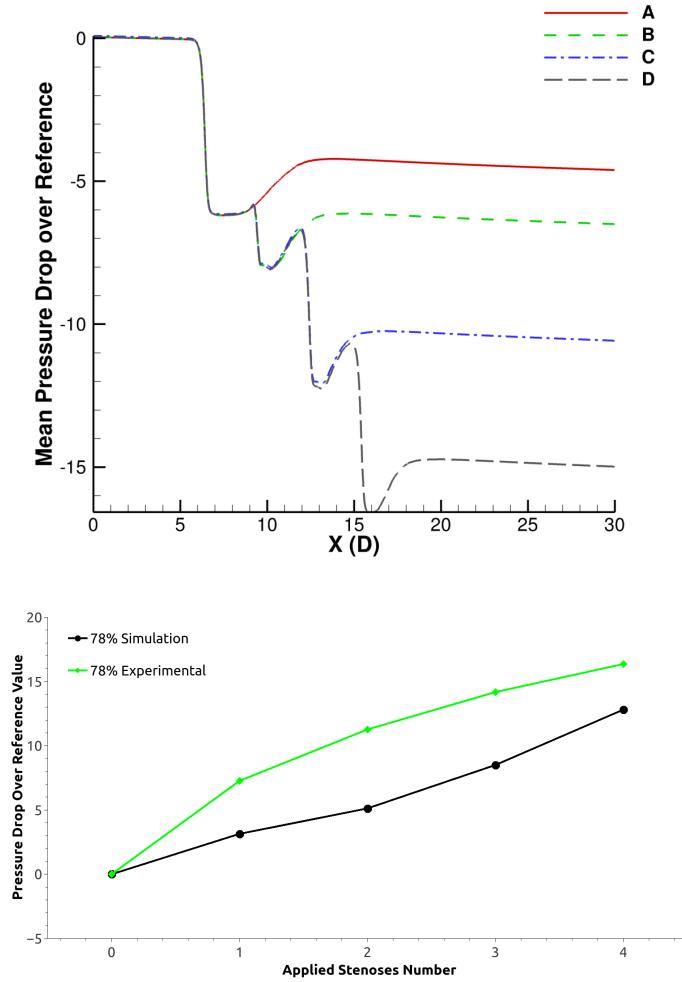


Figure A.11: Pressure drop among multiple critical stenoses

In Figure A.11, for very critical stenoses, such as 78% area deduction, the blood through stenotic area is into the transition stage. The error between simulation and experiments is less than 20%. We have legit results in simulation. Furthermore, we can find the pressure drop is linearly proportional to the number of critical stenoses. The scale of pressure drop is close to 15 times of the regular inlet pressure which means a significant pressure and recirculation damage is caused upon the post-stenotic region. Now we also proved the dangerousness of multiple critical stenoses along the artery.

A.5.5 Multiple subcritical stenoses pressure drop

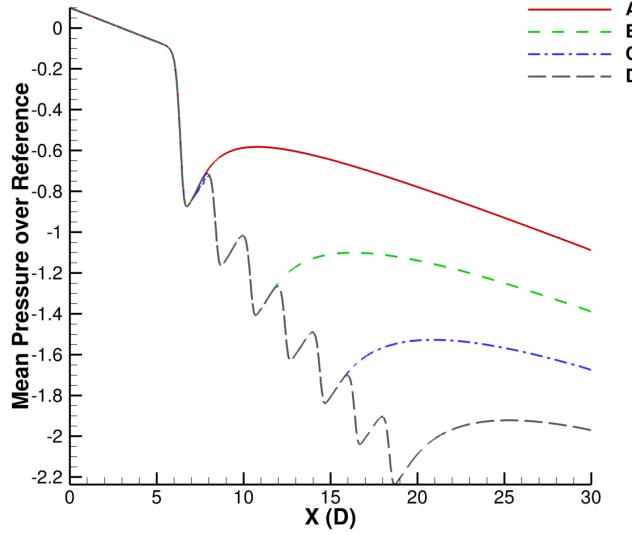


Figure A.12: Pressure drop among multiple subcritical stenoses

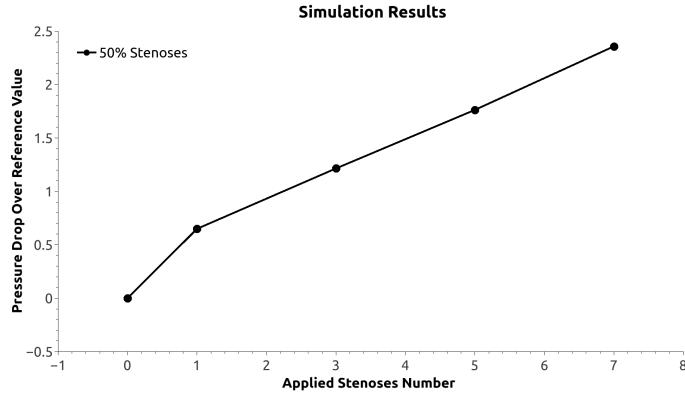


Figure A.13: Pressure drop values of multiple subcritical stenoses

The effect of adding subcritical stenoses onto the same vessel is not well discussed yet. We simulate the 50% degree subcritical stenosis and then sequentially add same stenosis to the downstream region. The maximum number is seven. We plot the pressure drop of one, three, five and seven stenoses along the centerline. The Figure A.12 shows the pressure drop increasing with more stenoses applied on the same straight tube. The following added stenoses do not have the effect on the upstream

stenoses flow field. The effect on pressure drop and pressure recovery area are similar for each constriction. For the right figure, we deduct the pressure drop of non-obstructed tube ,which only generated from wall shear stress, from the total streamwise pressure drop. We plot the net pressure drop from non-obstruction to seven subcritical stenoses correspondingly. The plot on right figure demonstrates the linear increasing of net pressure drop versus the increasing number of applied stenoses. The linear net pressure drop increasing is an important feature of subcritical stenoses.

A.5.6 Stenosis stretching on streamwise direction

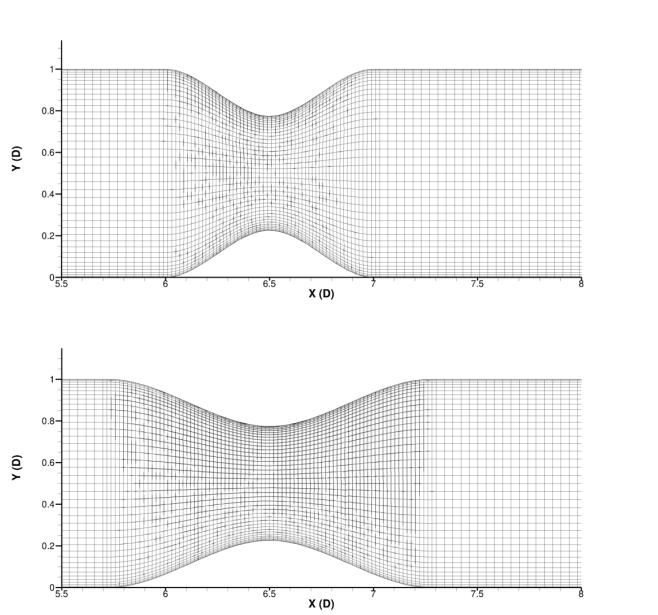


Figure A.14: Mesh along original and deformed stenotic region.

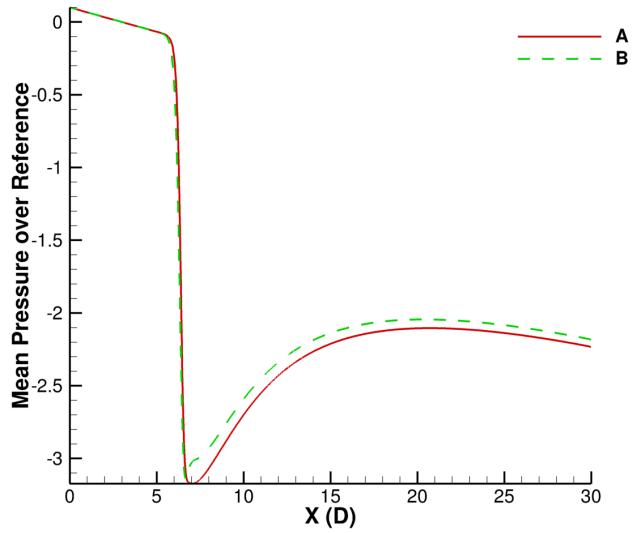


Figure A.15: Pressure drop comparison between original and deformed stenosis.

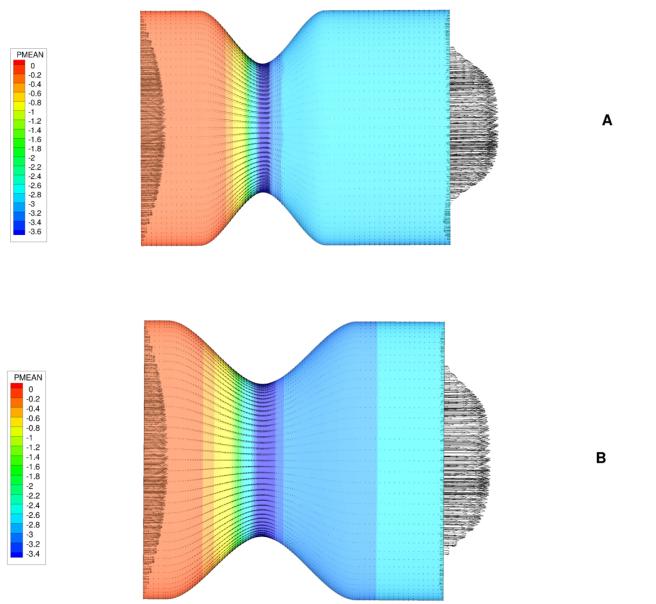


Figure A.16: Streamlines along original and deformed stenosis.

We deform the shape of 70% degree stenosis by stretching it on streamwise direction, see Figure A.14. We elongate the stenotic region and maintain the narrowest cross-sectional area size. The above chart demonstrates the meshes comparison of two shapes of stenosis and the centerline pressure drop along streamwise direction.

Table A.6: Pressure drop comparison between original and deformed stenosis.

Cases	Degree	Streamwise PD	Absolute PD
A	Original 70%	2.437	3.897
B	Stretched 70%	2.284	3.742

The streamwise length of the stenosis is increased by 54% while the corresponding pressure drop is only decreased by 1.8%. With the stretching of stenosis, the smoother narrowing curvature leads to the slightly more pressure recovery which is identified on the right chart ranges from 7D to 15D along the x axis, in Figure A.15 and Table A.6. The streamwise pressure drop is decreased due to the smoother stenotic curve while the absolute pressure drop remains the same level since the narrowing degrees are still the same for both cases. We can conclude that the longitudinal shape of single stenosis plays a minor role in the hemodynamic effect on flow pass stenosis. Moreover, the narrowing degree is the main factor which dominates the scale of total streamwise pressure drop across the constriction.

A.5.7 Pressure drop and wall shear stress

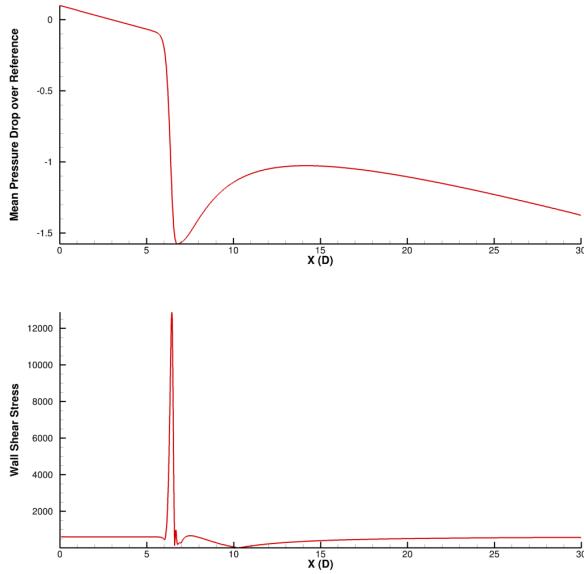


Figure A.17: Pressure drop and wall shear stress across single critical stenosis

In Figure A.17, we plot the pressure and wall shear stress along streamwise direction of single 60% stenosis case. The highest wall shear stress corresponds to the lowest pressure which happened at slightly pass the center of stenosis. For the wall shear stress chart, after the peak point, the plot tend close to the x-axis twice. The recirculation region is between the two lowest points. After the narrowest cross-sectional area, the pressure recovers smoothly while the shear stress fluctuates due to the post-stenotic recirculation.

A.5.8 Interval spacing between subcritical stenoses

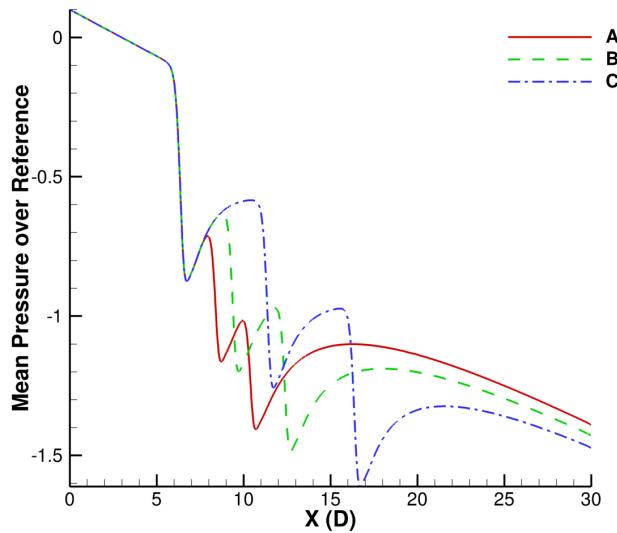


Figure A.18: Pressure drop of different interval spacing.

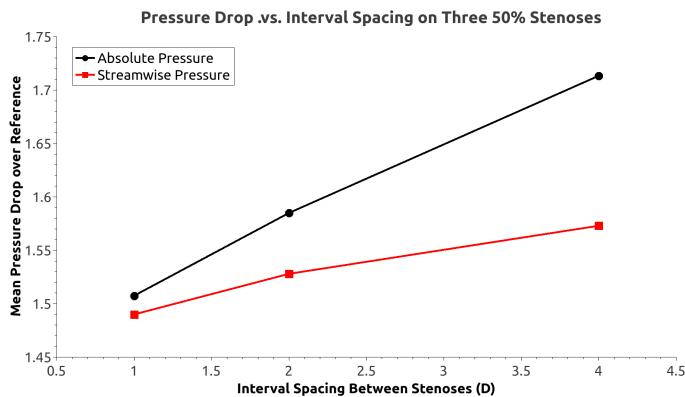


Figure A.19: Pressure drop values of different interval spacing.

The patient specific stenoses structure is highly complicated with lots of uncertain variables. The interval spacing between adjacent stenoses is an important parameter. To our best knowledge, few studies have been conducted on this perspective. Among previous the models that we created, I set same spacing between adjacent sub-critical stenoses which is one diameter for the sake of consistency. However, from the Figure A.1 we find that along the patients' peripheral artery, the spacing is significantly larger

Table A.7: Pressure drop of different interval spacing.

Cases	Degree	Streamwise PD	Absolute PD
A	One time spacing	1.49	1.51
B	Two times spacing	1.53	1.58
C	Four times spacing	1.57	1.71

than one diameter. To reveal how the pressure drop changing with interval spacing increasing, we operate a series of simulations which ranges the interval spacing from 1D to 4D in Figure A.18 and Table A.7. The comparison included interval spacing of one, two and four times of diameter for sub-critical stenoses. Both streamwise pressure drop and absolute pressure drop developed with spacing increasing. We can also see from the chart that the incline slope is not significant. Although the spacing increased from 1 to 4, the pressure drop growth is 16% and 14% referring to streamwise pressure drop and absolute pressure drop respectively.

A.6 Summary and discussion

According to the data and figures, we proved our following predictions: The post-stenotic flow becomes highly unsteady when the narrowing degree is more than 60%. More than 3 50% stenoses cause more streamwise pressure drop than one single 60% stenosis. Pressure drop introduced by subcritical stenoses grows almost linear as the increasing number of applied stenoses. Cross-sectional area reduction is the main factor which contributes to the total. The streamwise length of stenosis has little effect on the flow field. The interval spacing between each stenoses has small contribution to the total pressure drop.

With the our promising results and effective solver, we have built up a solid foundation for multiple stenoses research. The next step will be patient-specific geometry investigation. The high fidelity results are good supplement for doctors diagnosis leading to best optimized treatment strategy.

Appendix B: Conclusions and Future Work

B.1 Conclusion

In this paper, we have successfully designed a novel parallel computing method for solving linear elasticity problems. This method integrates the newly developed weak Galerkin (WG) finite element method with the balancing domain decomposition with constraints (BDDC). The WG-BDDC method is implemented by using Fortran and MPI libraries. The WG-BDDC method is demonstrated to have optimal order of accuracy and convergence properties for both 2nd- and 3rd- order numerical discretization. This method is also proven to have outstanding scalability with superlinear speedup when the number of processors increases by testing over 600 processors. The condition numbers of the Lanczos matrix form the global primal problem for all test cases presented in this paper are well bounded demonstrating a fast and robust convergence.

In hemodynamic perspective, we designed a study on peripheral arterial disease. Based on the collected data, we proved our following predictions: the post-stenotic flow becomes highly unsteady when the narrow degree is over 60% area deduction. More than 3 successive 50% stenoses cause more streamwise pressure drop than one single 60% stenosis. Moreover, pressure drop introduced by subcritical stenoses grows linearly as the increasing number of applied stenoses. In conclusion, the cross-sectional area reduction is the main factor which contributes to the total.

B.2 Future Work

The Future Work includes three major steps: 1) exploit and apply WG-BDDC on nonlinear elasticity, 2) extend the current WG-BDDC to tetrahedron and hexahedron elements, 3) incorporate the IMEX coupling scheme to implement an efficient fluid-structural interaction solver.

The present WG-BDDC method is built on triangular and quadrilateral 2 dimen-

sional elements for solving linear elasticity problems. To solve more complicated and closer to real-world problems, the nonlinear elasticity equation and three dimensional geometry model are urgent. The ultimate goal is simulating complex real-world engineering problems with our efficient high-order accuracy structural and fluid solver. The IMEX coupling scheme will combine the two independent solver together and deliver reliable results.

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