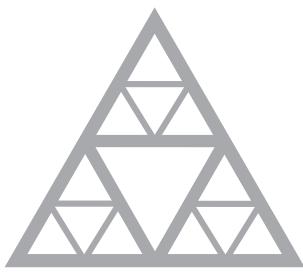


Modeling of bending-torsion couplings in active-bending structures. Application to the design of elastic gridshell.



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2 Ephemeral cathedral : the first GFRP gridshell building

2.1 Introduction

The Ephemeral Cathedral of Créteil, France, is an elastic gridshell structure made of composite materials [dPTBC16]. Built in 2013, this 350 m² religious edifice was initially a temporary church meant to gather the parishioners during the two years renovation (2013 - 2015) of their permanent cathedral (see fig. 2.1). At the time of writing, this building is still in activity and has been erected for almost five years. Although this structure is no more a church it has entered in a reconversion process to become a space for community activities and is now the property of the city of Créteil, France.

2.1.1 Overview

This large-scale prototype represents a first in the building industry which still shows excessive apprehension for the use of non-traditional materials such as composites, especially when it comes to structural applications. This is emphasized by the fact that only pre-norms or professional recommendations exist for composite materials, which is quite insufficient when one has to deal with insurers and legal technical controls. Although this structure is not the first elastic gridshell ever built in *Glass Fiber Reinforced Polymer* (GFRP) composite material, it should be regarded as the first true building using this technology. Indeed, this prototype – which can legally accommodate up to 500 people – complies with all the required performances : structural stiffness, fire safety, waterproofness, lightning, thermal comfort, etc. To our knowledge, this building is still the only one of this kind ever built.

It is worth to mentionne that this project arises thanks to a long-term collaboration between T/E/S/S atelier d'ingénierie¹ and the laboratory Navier² and marks the accomplishment

¹A structural design firm based in Paris, France : <http://tess.fr>

²Architected Materials and Structures (AMS) research team, specializes in the field of mechanics of

of a ten years research project in this field.³ More over, this challenge was both technical and human as the structure was built by the parishioners them selves.

2.1.2 Goals and contributions

This project was at the heart of the motivations of this thesis as it acted as a proof of feasibility and as a validation of the design tools and methods developed until then. The gained experience has highlighted further research directions that are presented in this manuscript.

In this chapter, we present the key aspects of the design and construction of this building. The main contribution of this work is to exhibit a method to design and build gridshells in a shape-driven design process. In such a process, architecture plays its full role as it is less dependent on purely structural considerations. Moreover, a building is much more than a shelter and has to satisfy serious requirements. To fill this gap, a meticulous attention was brought to the development of construction details. These technical developments lead to numerous testings. This chapter focus on this fundamental aspect as it is also a key contribution. Finally, we point out and analyze our experience, and we draw up some recommendations for further developments.

2.1.3 Related work

A thorough review is given in ??.

2.2 Project overview

2.2.1 Context and challenges

Créteil is a city of 90.000 inhabitants in the southeast suburb of Paris. Its urbanization began in the late 50's, impelled by the French architect Charles-Gustave Stoskopf. In 1976 he designed Notre Dame of Crêteil, a modest catholic church made of concrete, which became a cathedral 10 years later (see item 2 in fig. 2.1). Recently, the diocese of Crêteil has undertaken a major architectural redevelopment project of its cathedral, including a timber shell covering the religious area and the creation of a new cultural area. Once transformed, the edifice shall be more visible, more hospitable and livelier for citizens. Inevitably, such a molt takes time and a temporary place of worship was required to ensure liturgical services during the two-years work. In November 2011, T/E/S/S, the structural design office in charge of the cathedral renovation project, made an ambitious proposal to the diocese : based on a previous successful experience – the construction of a composite

materials and structures : <http://navier.enpc.fr/Materiaux-et-Structures?lang=en>

³Note that I developed this project in 2012 while I was a structural engineer at T/E/S/S, using the knowledge I had previously gained on the gridshell project for the Solidays music festival in 2011 while I was a research engineer at Navier. I started this thesis in octobre 2014, 18 months after the opening of the temporary cathedral to pursue my research on this topic started in may 2010.

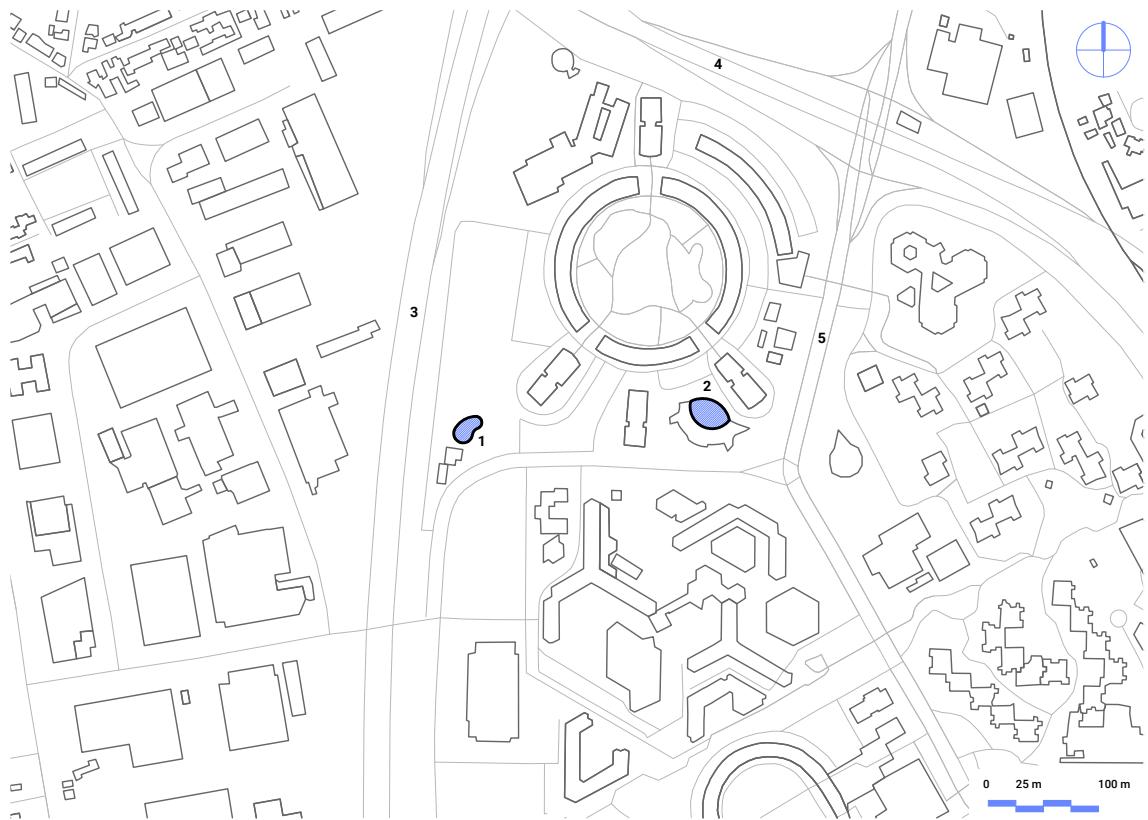


Figure 2.1 – Situation map. The temporary gridshell (1) was built very close to the permanent cathedral (2). Remark that the two buildings cover a quite similar projected area.

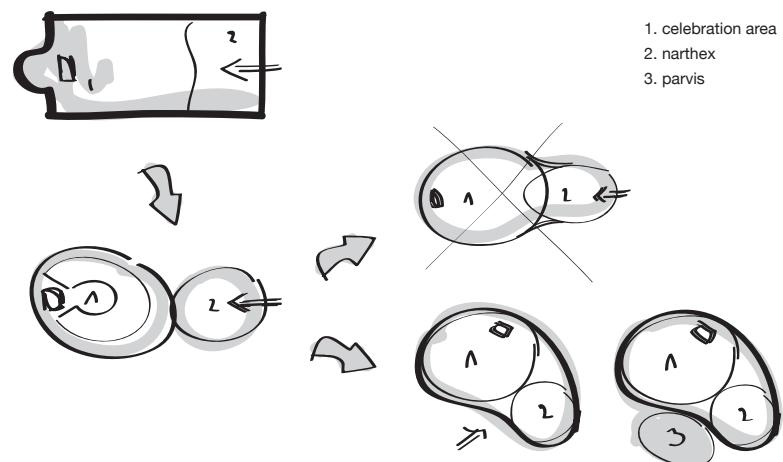


Figure 2.2 – Architectural sketch. Major and minor volumes are agglomerated into one volume. Here, the morphological register allowed by elastic gridshells appears to be relevant.

gridshell for the festival Solidays [BCTdP12] – T/E/S/S suggested that rather installing a basic tent, the parishioners should construct themselves a temporary cathedral.^{4,5}

2.2.2 Architectural considerations on the form

The origin of this building form was driven by two objectives, that is, to provide a variety of appropriate internal spaces within which the community could assemble, and to provide an externally welcoming and visually interesting form. According to the architect Tom Gray, today, the internal organization of a roman catholic church is in large part driven by the post Vatican II vision of a religious celebration being a collective gathering of the community around the Eucharist, center of spiritual life. A circular seating arrangement is often considered the most convivial form to create a sense of belonging while minimizing a sense of hierarchy. However the community is not only using the building for religious celebration but also for encounters on a more informal manner, for example spontaneous gatherings after religious ceremonies. In the early Roman church, such gathering of the community was facilitated by the presence of an anti-space to the main space called a *narthex*, through which one passed on entering the church. It was therefore felt appropriate that the formal freedom which the gridshell system offered would be used to explore forms composed of an agglomeration of major and a minor volumes which contain the two functions : formal and informal gatherings (see item 1 and 2 in [fig. 2.2](#)).

Formal explorations were undertaken using modeling clay. The final form is based loosely on two adjacent semi spherical volumes of different size, which are merged into one complex form. Externally the fear of the design team was that the totally convex blob form could look intimidating. It was therefore decided that the two spherical virtual forms, which would be joined to make the final form, would be arranged not in a symmetrical axial manner, but in an asymmetrical curved composition. The resulting form seen in plan is convex on one side and concave on the other. The concave form in plan allows for double curvature to be introduced into what would be otherwise a simpler blob and gives sensuality and visual interest to the building.

2.2.3 Placing of the building on the site

The temporary cathedral is located on a land owned by the municipality, which is used for sporting and other communal gatherings. The curve in the building defines an external area where the church community could meet in the open air and this is where the entrance to the church is situated. The building was positioned on the site so that the entrance addresses a grass planted area forming a garden forecourt or “parvis” (see item 3 in [fig. 2.2](#)). A service building housing plant, toilets and vestry are housed in a port cabin positioned to the rear of the building (see [fig. 2.9](#)).

⁴See the video of the construction of Solidays’ gridshell here : <https://youtu.be/24LLfcVIZWw>.

⁵See the video of the construction of Creteil’s gridshell here : <https://youtu.be/jLq-UfOdnQQ>.

2.2.4 Entrance

It is formally quite difficult to integrate doors, which must be verticals, into a complex geometry. Either the gridshell could be deformed to accommodate the geometrical requirements of doors, or the doors could be integrated into an independent form. The latter approach was chosen. In looking for forms to house the doors, reference was made to the conical monumental doorways with rings of concentric decoration, which welcome the faithful to romanesque and gothic churches in France. The conical forms were found to be coherent to the overall geometry of the building. The entrance doors were therefore inserted into a conical hooded form made of rolled steel plates and stiffened by concentric steel tubes, which not only make reference to historic precedence but also refer to the gridshell to be discovered inside (see [fig. 2.5b](#)). The cone of the entrance doors was positioned in the concave side of the building giving access directly to the narthex part of the internal volume. To the rear of the church is situated a service door. The steel hood, which houses this door, is curved tightly around the door and takes up an ovoid form.

2.2.5 Daylight

The gridshell is covered in a PVC membrane, which is opaque. How to introduce daylight into the interior was a major subject of reflection. The simplest way found was to use transparent membrane placed occasionally on the membrane. A small amount of light was required in the interior to create a contemplative atmosphere. The lights would in consequence glow and would be seen as luminous insertions in the vault, like stars in the celestial vault or the apse of some Romanesque churches. The stars were patterned on the joints of the PVC membrane. The almond shape came from simplification of the cutting into the panels either side of the joints and to avoid stress concentrations around cuts in the membrane. This shape, known as Mandela, is frequently used in Marian religious imagery. The distribution of the transparent insertions is quite uniform but gets denser above the pinnacle.

2.2.6 Technical description

The gridshell structure is made of long glass fibre tubes ($\varnothing 42$ mm) fastened together with scaffold swivel couplers (see, [fig. 2.6a](#)). The structural members of the grid, all of different lengths, are built from one, two or three composite tubes connected with steel sleeves (see [fig. 2.6b](#)). The length of the tubes is limited to 12 m to enable transportation through standard trucks. The tubes are organized in three layers. During assembly, the first two layers are first placed perpendicular to one another on the ground. They form the *quadrangular primary grid*. The distance between the tubes of these two layers is constant, resulting in a regular grid. This primary grid is elastically deformed to obtain the final shape. The third layer of tubes acts as bracing. It gives the structure a shell-like behavior. The tubes are fixed to the primary grid once the shape has been obtained

The structure is anchored to a concrete strip footing with a special anchorage system, which ensures transfer of loads from the composite structure to the ground (see [fig. 2.6c](#)).



Figure 2.3 – Exterior view. The connections mark the fabric suggesting the interior grid structure. This texture enriches the perception of the building viewed from the outside and creates effects with the light reflections – © L. du Peloux for T/E/S/S.



Figure 2.4 – Interior view. The grid pattern highlights the lightness of the structure and gives its tempo to the internal space. Lines converge to the altar, the heart of the liturgical area where the mass is offered on – © C. Moissinac for T/E/S/S.



(a) Exterior

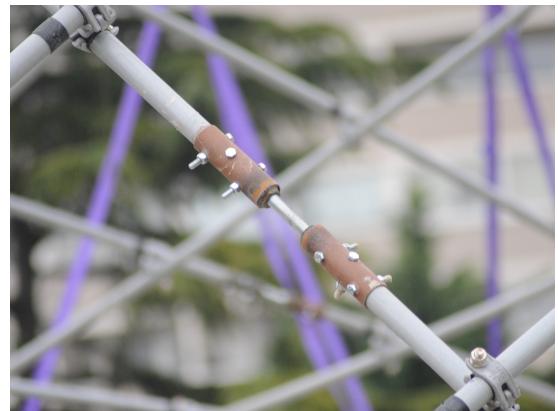


(b) Interior

Figure 2.5 – Entrance. Two steel doors allow the entrance inside the building.



(a) Swivel coupler



(b) Sleeve system



(c) Ground anchorage



(d) Lacing rod

Figure 2.6 – Standard elements that composed the structural system.

A similar system enables fixation of the structure to the doors (see [fig. 2.5b](#)).

A PVC coated fabric (see [fig. 2.3](#)), tailor-made for the purpose, covers the structure. The transparent portion of the structure allows daylight inside the gridshell. The fabric is stretched on the peripheral edge of a dedicated beam with a double-lacing system (halyard and strap, see [fig. 2.6d](#)). At the ground level, the lacing edge of the beam is made of a bent, composite rod nailed to the concrete slab. At the grid-door junction, a steel arch is welded to the doorframe (see [fig. 2.5a](#)).

The PVC fabric is waterproof and, since it is a continuous membrane, has no joints except at the perimeter. At the perimeter, a continuous strip of membrane is prefixed to the internal surface of the membrane and fixed to the ground slab. At the doors, a flexible strip of the membrane is riveted to the doorframe.

Category	Item	Unit	Quantity
Public	seating	p	360
	standing	p	500
Dimensions	length	m	29
	width	m	17
	height	m	7
	contour	lm	75
	area	m ²	350
	volume	m ³	1600
Gridshell	tubes (x176)	lm	1775
	connections		1130
	sleeves		125
	anchorage		127
	<i>ground (single)</i>		77
	<i>ground (double)</i>		16
	<i>door (single)</i>		18
	weight	kg/m ²	5
Fabric	opaque	m ²	530
	transparent	m ²	12
	lacing rod	lm	67
	weight	kg/m ²	1

Figure 2.7 – Key numbers.

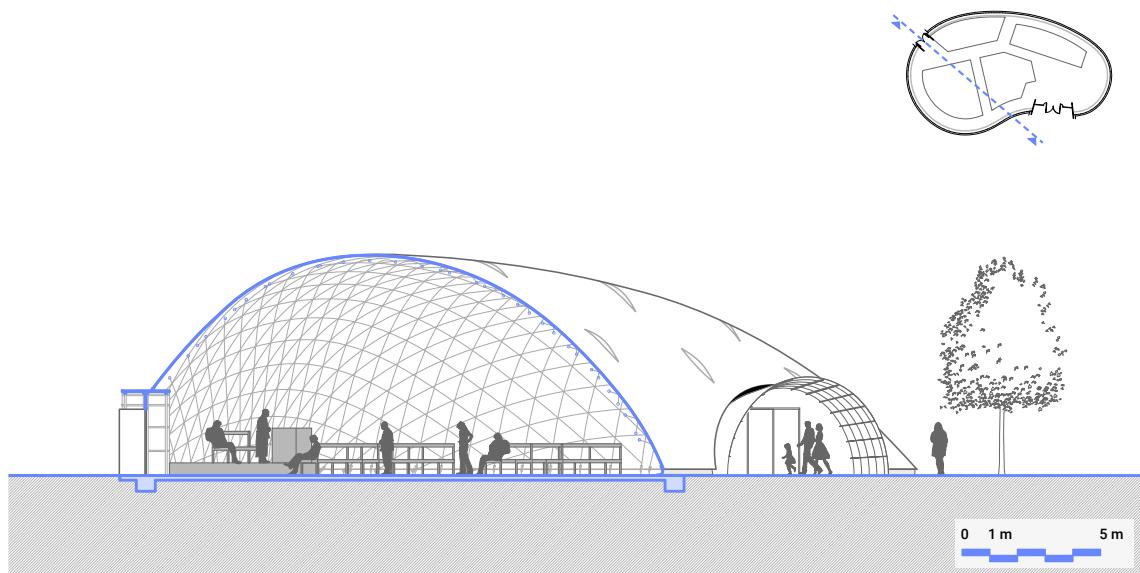


Figure 2.8 – Transversal section of the building. Observe how the grid gets denser at the choir. Two doors give access to the building. The height at the pinnacle is about 7 m.

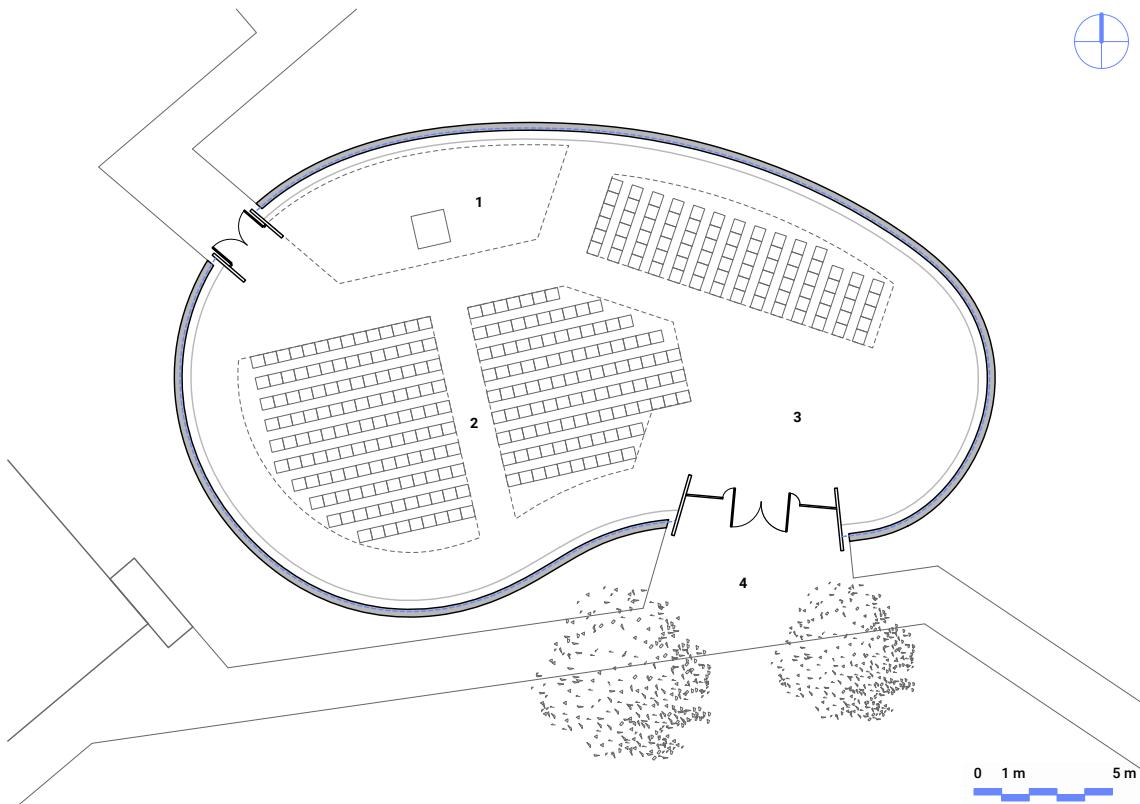


Figure 2.9 – Top view of the building. The interior space is composed of a choir (1), a place of assembly (2) and a narthex (3). The main entrance overlook the parvis (4). The shell spans about 29 m in the longitudinal direction and about 17 m in the transversal direction. The covered area is about 350 m². The space can accommodate 360 seating people or 500 standing people.

2.3 Construction process

2.3.1 Assembly of the grid

The first two directions of tubes were assembled perpendicularly on the ground with the swivel couplers (see [fig. 2.6a](#)) to form the *primary grid*. The resulting grid covered about 600 m² (see [fig. 2.10a](#)). At each intersection, the tubes were fastened together with a coupler, installed manually by the volunteers. They were asked not to tighten the bolts but just to engage the collars in order to prevent potential damages from the collar over the tube. Once the assembly of the primary grid was completed, the swivel couplers were tightened with a torque wrench to the optimal torque specified by the laboratory Navier. The whole stage took two full days. Note that because the anchorages stucked out from the slab, it was decided not to assemble the grid on the concrete slab to ensure that the grid would be able to slide freely on the ground and not get clung in the anchorages during the erection stage.

2.3.2 Deformation of the grid

The next stage consisted in lifting the grid simultaneously with two mobile cranes (35t). Once lifted up, the grid took nearly its final form (see [fig. 2.10b](#)). The structure was slowly moved above the slab until tube endings faced at best their respective anchor points. Then, tube after tube, the workers pinned the grid to the ground anchorages (see [fig. 2.10c](#)). This stage is tricky, especially at the beginning because only few tubes are connected to the ground. If the grid moves it can easily break these few tubes. The action of pinning a tube is done with a single bolt. The end of each composite tube is equipped with a rotating steel clevis. Similarly, each ground anchorage is composed of a steel plate fixed to the concrete slab and a rotating clevis. To pin a tube to an anchorage, their clevis are aligned one to each other and a pin is positioned in their central hole (see [fig. 2.6c](#)). When all the tubes were pinned to their anchorage, the grid was stable and secured and the cranes were removed (see [fig. 2.10d](#)). The stage lasted one full day.

2.3.3 Bracing of the grid

Once the primary grid was deformed into the final shape, it was braced by a third direction of tubes called the *triangulation*. The triangulation tubes split the quadrangular mesh of the primary grid into triangles (see [fig. 2.10e](#)). This work was tedious as it required working at height in aerial buckets. Tubes were hand-conveyed in the structure and attached to the tubes of the second layer with an additional swivel coupler. Each node of the structure would then be composed of two connections (see [fig. 2.14](#)). Once triangulated the structure behaves like a shell and its stiffness increases largely.

2.3. Construction process



(a) Flat grid



(b) Erection



(c) The grid is anchored



(d) Deformed grid



(e) Grid is braced



(f) Membrane

Figure 2.10 – Construction process of the gridshell.

2.3.4 Covering of the gridshell

Finally, the structure is covered with a PVC coated fabric. The membrane comes rolled up. The roll is positioned at one side. Then it is progressively unrolled toward the other side (see [fig. 2.10f](#)). This step requires professional rope workers. Once the membrane is in place, it is hand tensioned with a system of halyard and strap (see [fig. 2.6d](#)). All included, this stage lasted no more than a single day for a team of six workers.

This step appears as the moment of truth : if the membrane perfectly fits the gridshell, making no crease, that means the structural analysis were successfully conducted with the required accuracy (see ??).

2.4 Structural design

2.4.1 Overall design process

The goal of the design process is to identify a gridshell structure that works and respects as faithfully as possible the architectural project – a shape and a program. It represents “the path from shape to structure”. Its progress, sequential and iterative, revolves around three major stages: shape, mesh and structure. It is not trivial to go through this complex process. Indeed, for each step, the method, the tool, the criteria, that offers both a sufficient explorative richness to find out enough candidate solutions, and the means to evaluate and compare the suitability of those solutions, have to be found.

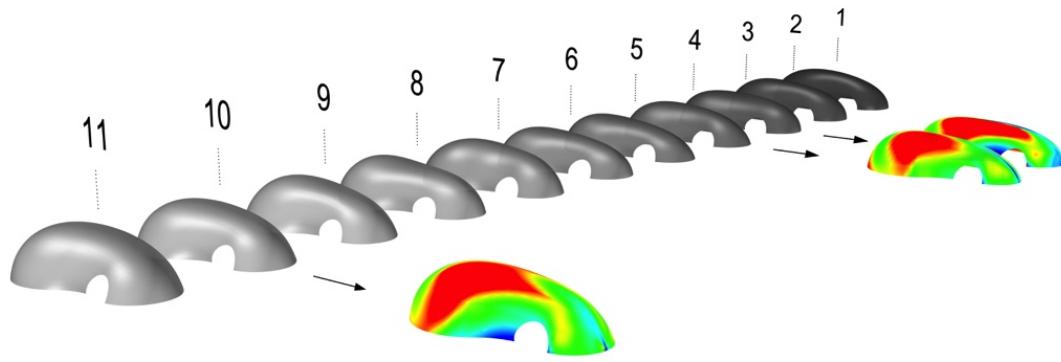


Figure 2.11 – Evaluating the mechanical potential of shapes regarding principal curvatures

2.4.2 3D modelling of the intended shape

The first step of the process consists in building a precise geometric model from the sketch of the architect and to evaluate its mechanical potential (Figure 2.11). At this stage, the goal is to estimate the probability a given shape would lead to the generation of a structurally feasible gridshell.

Stresses in the grid are mainly due to the bending of the profiles. They derive directly from their geometric curvature. Thus, the principal curvatures of the surface – because they give a qualitative measurement of the local curvature of any curve drawn on a surface – are relevant indicators to evaluate the stress rate of a grid laying on it. Particularly, one should ensure the following condition is satisfied everywhere, where r is the pipe's outer radius, R_{min} is the minimum principal radius, E is the flexural modulus, $\sigma_{k,flex}$ the characteristic flexural strength and γ_{lt} the long-term partial coefficient of material resistance :

$$E = \frac{r}{R_{min}} < \frac{\sigma_{k,flex}}{\gamma_{lt}} \quad (2.1)$$

Ideally, the shape is controlled by few key parameters. Thus, it can be adapted and

optimized through an iterative process, towards this criterion.

2.4.3 Mesh from the compass method

During the second step, the candidate surface is meshed and the mechanical potential of the resulting grid is evaluated. At this stage, we try to estimate the probability a given mesh could lead to the generation of a viable gridshell structure. Simultaneously, meshes are compared according to their architectural relevance.

This time, the geometric curvature of the polylines drawn on the surface is the criterion to characterize the mechanical potential of the grid. In particular, one should ensure the following condition is satisfied everywhere, where R_{spline} is the spline's local curvature radius :

$$E = \frac{r}{R_{spline}} < \frac{\sigma_{k,flex}}{\gamma_{lt}} \quad (2.2)$$

The mesh is obtained by the compass method, described in [OSHH74], which develops a regularly spaced grid on a surface from two secant directrix. For a given shape there are an infinite number of meshes. The aim is to identify at least one grid, suitable towards architectural and structural criteria. The laboratory tried various numerical methods to generate such grids [BBC09]. Here, a specific software [dPBCT11], developed for rhino & grasshopper, allows generating this kind of mesh on any NURBS surface.

2.4.4 Formfinding and bending prestress

From now on, the initial form has been optimized and promising meshes for the materialization of the future gridshell have been identified. However, they do not take account of any true mechanical reality, because only geometric rules have led to their generation. The formfinding step consists precisely in finding the geometry of the grid at mechanical equilibrium, and the corresponding permanent bending stresses. The calculations, performed numerically thanks to a dynamic relaxation algorithm with kinetic damping [Bar75], is composed of the following steps:

1. The grid is bent by a set of applied displacements from rest to compass position
2. The grid is then relaxed through its mechanical equilibrium
3. Bending stresses of the triangulation are calculated relative to the relaxed geometry
4. Geometry and bending stresses of the triangulation are reinjected in the model

Two analysis models are built during this process to study the structure with or without bracing tubes. Note that the algorithm has been improved to take account for the eccentricity due to connections [DBC06].

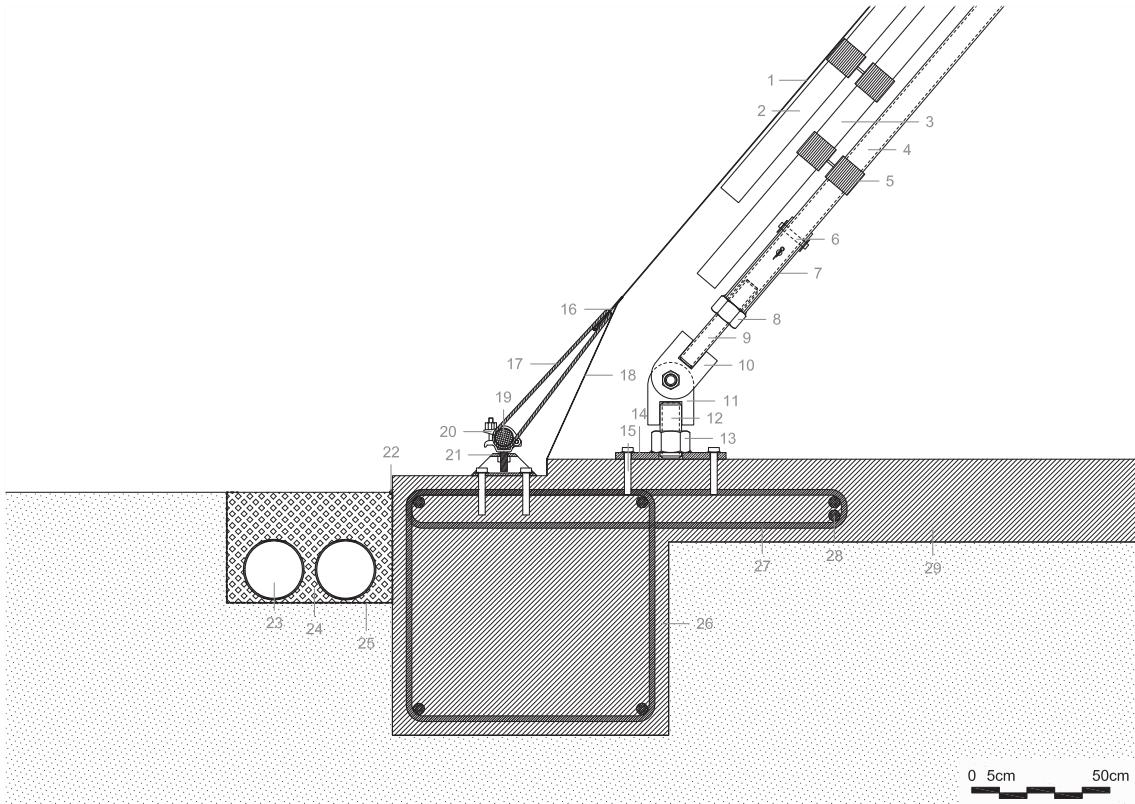


Figure 2.12 – Section on the footer

2.4.5 As-built geometry

Indeed, the eccentricity remains small compared with the span of the shell, but it is not negligible compared with the mesh size ($e = 68 \text{ mm}$; $l = 17 \text{ m}$; $w = 1 \text{ m}$). Thus, the pipe lengths and anchorage positions could not be determined with sufficient accuracy without taking into account the thickness of the structural grid due to this eccentricity. The employed method, purely geometrical, assessed that the neutral fibre of the shell is equidistant from the first two layers of pipes. The form-finding was performed only with those two layers. Connection axes had to be parallel to the local normal of the shell surface. This assumption was not exact, but, in this case, gave sufficient accuracy. The red pipe was offset by $e/2$, the green by $e/2$ and the blue by $3e/2$ along the normal.

2.4.6 Structural analysis

A full structural analysis is performed on the gridshell, using the two mechanical models created previously during the formfinding stage. The non-braced model is used to check the grid's behavior during the construction stages. In particular, it must be verified that the primary grid - the one with no triangulation tubes - has no risk of buckling, both for obvious safety reasons and to ensure the accuracy of the final geometry. Indeed, the more the form is likely to buckle, the more it can be triangulated in a buckled geometry different to the targeted geometry. The model with the triangulated grid is used to confirm the gridshell complies with all the structural requirements during its lifetime. Its behavior

under standard loadings is evaluated.

2.5 Designing with GFRP materials

This section focuses on the GFRP tubes employed for the structure. We present how we managed to deal with this composite material in the eyes of the existing regulatory framework although there is no applicable norms for composite materials (see §2.5.2). Beyond the administrative strategy, we present how their flexural strength was evaluated (see §2.5.3) and how the corresponding partial safety factors were determined (see §2.5.4).

2.5.1 Properties of the tubes

The technical properties of the tube employed for this project are given in [table 2.1](#). Although these data were provided by the manufacturer at the time of the project, a test campaign was done to verify the flexural resistance of the tubes taking into account the influence of the swivel couplers clamped on the tubes (see §2.5.3).

2.5.2 Codes for composite materials

Beyond the technical difficulties related to both design and structural analysis of the shell, the regulatory framework was a vital issue for the success of the project. As it was the first time a structure of this kind was going to host a large number of people for over two years, the question of its reliability over time was a major issue. In order to be built, the gridshell had to comply with existing standards, which do not take into account such an innovative edifice, all in composite material. The strategy adopted to bypass this obstacle is presented herafter.

First level : administrative classification of the building

The first level, administrative, consisted of obtaining from the French authorities an appropriate classification for the building, taking into consideration the project's real-time requirement : a light-weight structure with a short lifespan. As expected, the structure was classified as a "building open to the public" (EPR in French) from the category "big tops and tents" (CTS in french) [[Sit](#)]. In this classification, construction procedures and regulations are adapted to the short lifespan of buildings.

Second level : compliance with existing standards

The second level, normative, consisted of ensuring that most of the existing regulatory framework justified the compliance of a structure that would not, at first sight, be considered by standards that do not include composite materials.

As far as possible, the design was made in compliance with the Eurocode, where the structural design is done according to the limit states under normalized loadings (self-weight, snow, wind, etc.). Although, the Eurocodes do not directly take into account composite materials, they propose some probabilistic methods to introduce new materials

Item	Standard	Polyester Mat-Roving-Mat
External diameter		41.7 mm
Internal diameter		34.7 mm
Wall thickness		3.5 mm
Section area		$4.20 \times 10^{-2} \text{ m}^2$
Section moment of inertia		$7.7259 \times 10^{-4} \text{ m}^4$
Torsion constant		$15.4518 \times 10^{-4} \text{ m}^4$
Shipping length		12.0 m
Glass content by weight	ISO 1172	60 %
Specific weight	ASTM D792	1.75 kg/m ³
Linear weight		0.735 kg/lm
Coefficient of thermal expansion	ASTM D696	$11 \times 10^{-6} \text{ K}^{-1}$
Tensile strength	ASTM D638	400 MPa
Tensile modulus	ASTM D638	26 GPa
Flexural strength	ASTM D790	400 MPa
Flexural modulus	full bending	25 GPa
Compressive strength	ASTM D695	220 MPa
Compressive modulus	ASTM D695	20 GPa

Table 2.1 – Technical properties of the tube.

Item	Standard	Précontraint 702 Opaque Alu
Yarn		1100 dtex PES HT
Weight	EN ISO 2286-2	830 g/m ²
Width		267 cm
Standard jumbo roll		50 lm or 300 lm
Finish		2-face acrylic varnish
Tensile strength (warp/weft)	EN ISO 1421	280/280 daN/5 cm
Tear strength (warp/weft)	DIN 53.363	30/28 daN
Elongation under load (warp/weft)	NF EN 15619	< 1 % / < 1 %
Adhesion	EN ISO 2411	10 daN/5 cm
Solar transmission	NFP 38511	13.5 %
Flame retardancy	NFP 92-507	M2
	DIN 4102-1	B1
Euroclass	EN 13501-1	B-s2, d0
Cold resistance	ISO 4675	-30 °C
Heat resistance	DIN 4102-1	70 °C

Table 2.2 – Technical properties of the membrane.

(EN1990, Annexe D). The mechanical properties of the GFRP pipe were determined as far as possible by tests in conformance with these methods. Alternatively, values were taken according from the Eurocomp [Cla03].⁶ In some cases, such as for the sleeve, the construction design also benefited from this approach.

2.5.3 Flexural strength of the tubes

The characteristic flexural strength ($\sigma_{k,flex}$) of the GFRP tube was used to verify if the structure complied with the Eurocode. This parameter had a critical impact on the structure's reliability because in this particular application stresses in the tubes are mainly due to the bending. Thus, it was important to confirm the manufacturer's permitted value through testing. Three-point flexural tests were carried out with and without connections (see fig. 2.13) to determine the characteristic strength according to the Eurocode protocol (Annex D) :

$$\sigma_{k,flex} = \bar{\sigma}(1 - k_n \sigma_x) \quad (2.3)$$

For five tests, the factor $k_{n,5\%}$ is 1.80 assuming a normal distribution. It has been proved in [Tay15] that the connections caused more scattering in the results. Finally, the manufacturer allowed value of 400 MPa was confirmed and retained for further calculations.

Connection	σ_1	σ_2	σ_3	σ_4	σ_5	σ_k
Without	456	441	445	460	477	430
With (20 Nm)	444	478	434	479	427	408

Table 2.3 – Three-point flexural tests of the GFRP tubes (MPa).

2.5.4 Partial safety factors

The partial coefficients of material resistance (see table 2.4) used in the project were calculated according to the Eurocomp. The short-term coefficient proposed in Eurocomp ($\gamma_{st} = 1.3$) was increased to consider the critical stage of erection, where the deformations could not be controlled accurately. When dealing with long-term effects in permanently loaded, pultruded composite materials subjected to creeping and relaxation designers should be careful [KW12, Ban06]. In this project, this was reflected in the high partial coefficient for long-term effects.

⁶The Eurocomp is a kind of pre-standard intended for the structural design of buildings and civil engineering works using GFRP composites, consistent with the Eurocode approach. It is considered as the reference design code for GFRP materials.

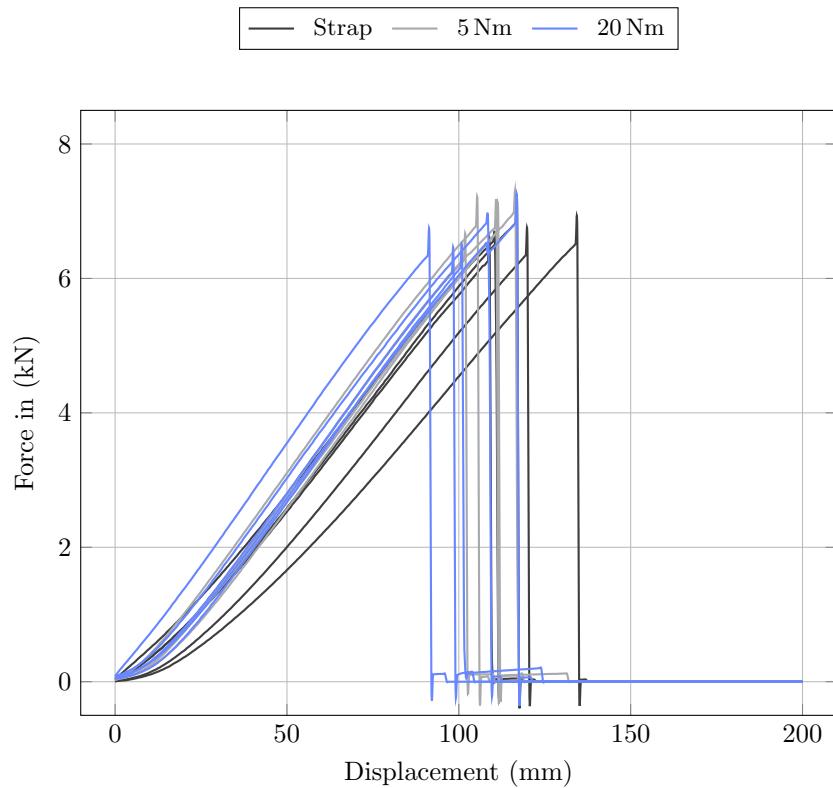


Figure 2.13 – Flexural tests of the GFRP tubes. Results from [Tay15].

Time scale	γ	σ_d
Short-term	2.0	200
Long-term	3.0	133

Table 2.4 – Short-term and long-term values for material resistance. γ is the partial coefficient for safety factor. σ_d is the flexural design strength.

2.6 Construction details

In this project, one can identify 4 major structural details : the swivel coupler for connecting composite tubes to assemble the grid (see [fig. 2.6a](#)) ; the steel sleeve for connecting several composite tubes to obtain long members from initially short pieces of tubes (see [fig. 2.6b](#)) ; the ground anchorages for fixing the structure to the concrete slab (see [fig. 2.6c](#)) and the lacing edge beam of the fabric (see [2.6d](#)). The challenging issue of connecting the steel and composite parts was solved similarly sleeve and anchorage details.

2.6.1 The swivel coupler

Tubes are connected together with scaffold swivel couplers (see [fig. 2.6a](#)). Each connection is composed of two collars (\varnothing 42 mm and 38 mm wide) linked by a steel axis (see [fig. 2.14](#)). Thus the collars can freely rotate around the axis of the connection. This degree of freedom is responsible for the lack of in-plane shear stiffness of the primary grid and this is precisely this mechanism that allows the flat grid to deform into a free form surface. Each collar is itself composed of two hemicylindrical parts so that it can be opened to easily engage a tube. A M12 nut and a swivel T-bolt allow to lock the tube in the collar using friction. Collars are positioned over a 1.5 mm thick epdm ribbon wrapped around the tubes (see [fig. 2.14](#)). Once clamped in the connection, the tubes are spaced by a 68 mm distance from axis to axis. Although the mechanical consequences of this eccentricity could be neglected to a first-order approximation, this is not the case for the geometric consequences it induces as explained in §[2.4.5](#).

Interface layer

This coupler is made to assemble two scaffold steel tubes together. Workers should tightly the collars of the coupler to ensure that the steel tubes won't slide in their collar. Here, it is clearly impossible to do that. Indeed the GFRP tube is too thin (only 3.5 mm thick) and tightening the collars to the maximum would damage it or even make it collapse. However, preventing the connections to slide along the tubes is critical to maintain the in-plane shear degree of freedom of the grid. If connections would slide, the grid would probably not deploy in space as intended. The grid kinematic would be blocked at some points, developing high stresses that would lead to breakages. To maintain a sufficient level of sliding resistance while preserving the material integrity it was decided to introduce an interface layer to :

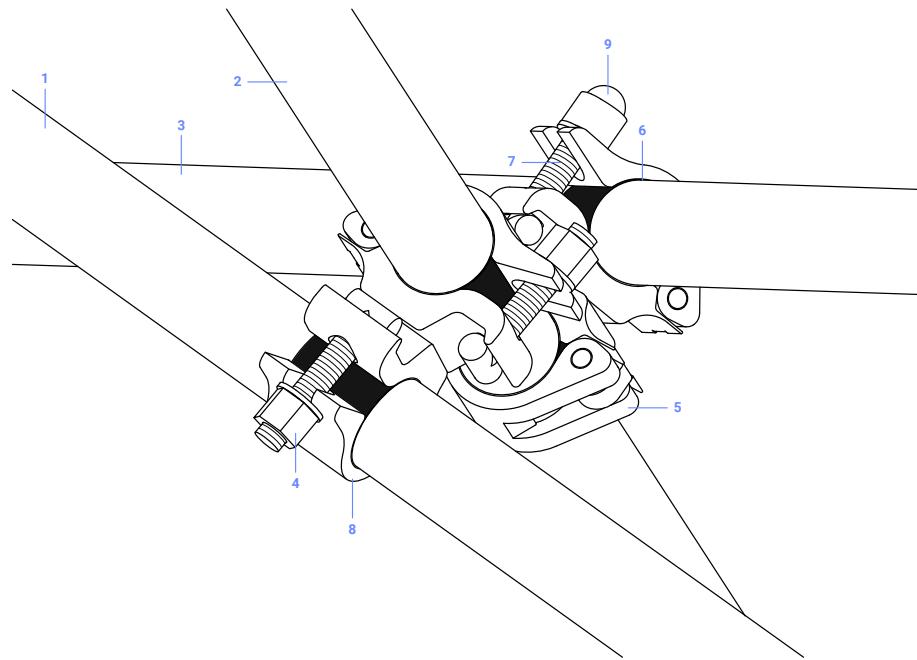


Figure 2.14 – Technical drawing of the swivel coupler. GFRP tubes (1, 2, 3). Swivel couplers (5, 8). EPDM layer (6). M12 swivel T-bolt (7). M12 nut (4) and plastic cap (9).

- increase the poor friction coefficient between the steel collar and the GFRP tube ;
- improve the distribution of stresses generated by the transverse compression of the collar over the tube, thus allowing a stronger clamping of the collar.

Several materials were tested in different thickness. Some of the results are presented in [fig. 2.15](#). A 1.5 mm thick EPDM ribbon was found to be suitable for the design requirements of the project. This layer improves significantly the sliding resistance from about 300 N to about 1200 N. Once the interface layer had been chosen, further tests were conducted to determine the appropriate clamping for the connections. The aim was to maximize the clamping to get the best sliding resistance (see [fig. 2.16](#)) while preserving the integrity of the tubes. It was found that a tightening torque between 15 Nm to 20 Nm was the optimal solution [[Tay15](#)].

Finally, the ribbons were ordered with a double sided tape (DST) face to facilitate their placement on the tubes. The influence of the temperature regarding the presence of the adhesive has been investigated. The results show that after 40 °C the scotch creeps and a loss of resistance occurs (see [fig. 2.17](#)). A recent thermal study of the structure has demonstrated that with no cooling system the temperature inside the building could rise up to 70 °C (see §[2.7.1](#) and [figures 2.23a](#) and [2.23b](#)).

Benefits and drawbacks

This connection has the advantage to be available almost every where, to be really cheap and indestructible compare to the GFRP members. However, there are some drawbacks as

it is not tailor-made for this application :

- The weight of this part is 1.16 kg, which is very heavy compare to the lightness of the system. In this project, the weight of the swivel couplers represents one third of the overall weight of the structure. This could easily be reduced with a dedicated design.
- The actual design is not adapted to resist sliding. This is critical as explained previously. This problem occurred locally during the lifting of the grid and it was a pain to finish the deployment of the structure (see ??).
- Although the clamping of the collars enhance the resistance to sliding of the couplers, they also activate the ability to transfer some torsion to the tubes. Unfortunately, the tubes are very weak regarding this type of sollicitation (see [table 2.1](#)). A better design would propose a kinematics that allow the rotation of each collar around the axis of the connection and the axis of the tube.
- As the number of connections is quite huge, the clamping process should be at the heart of the design. The later has to guarantee that the workers will not damage unintentionally the structural members. If clamping would be found to be the way to go – which indeed might be an relevant option – the structural elements will have to be stronger to resist both clamping and torsion.

Furthermore, other design criterion should be taken into account such as the fact that the connection must not damage the membrane. This was resolved in the project thanks to plastic caps (see [fig. 2.18](#)). It is worth to mentionne that the problematic of the connection should be treated in symbiosis with the difficult question of the bracing and the covering as it interacts with all the key parts of the system : grid, bracing and membrane. Some propositions to this complex problem were designed and tested during this thesis and are presented in ?????. A noticeable design attempt was proposed in the roof of Chiddingstone's orangery ??.

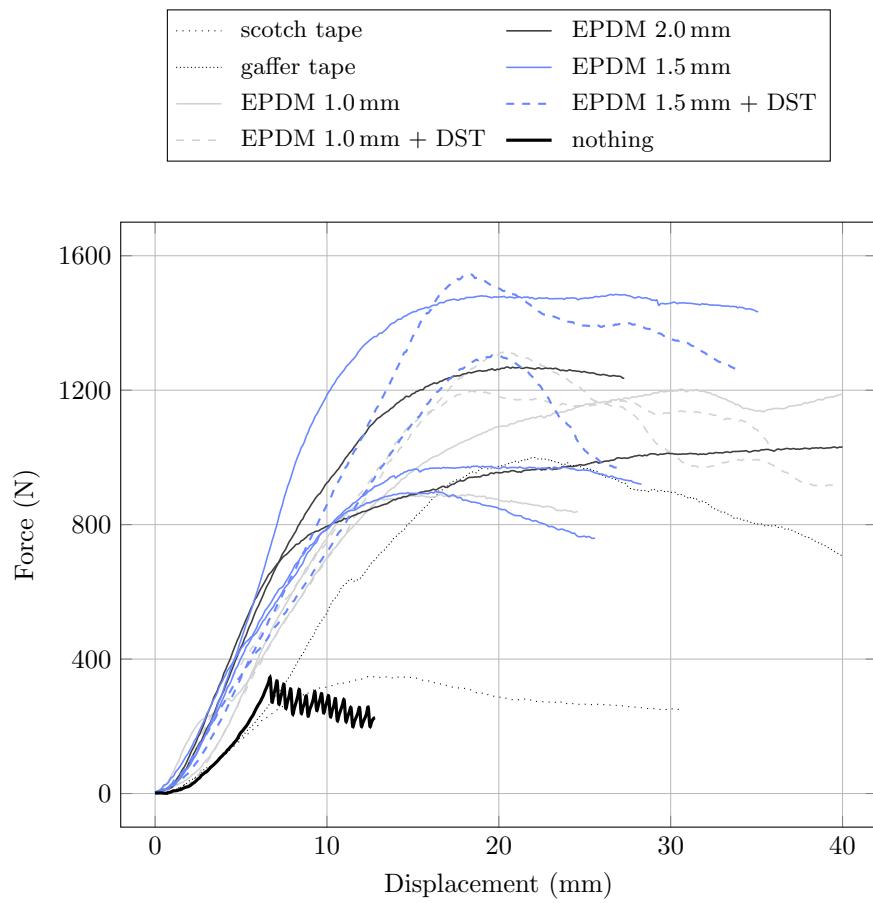


Figure 2.15 – Influence of the interface layer on the sliding resistance (5 Nm).
Results from [Tay15].

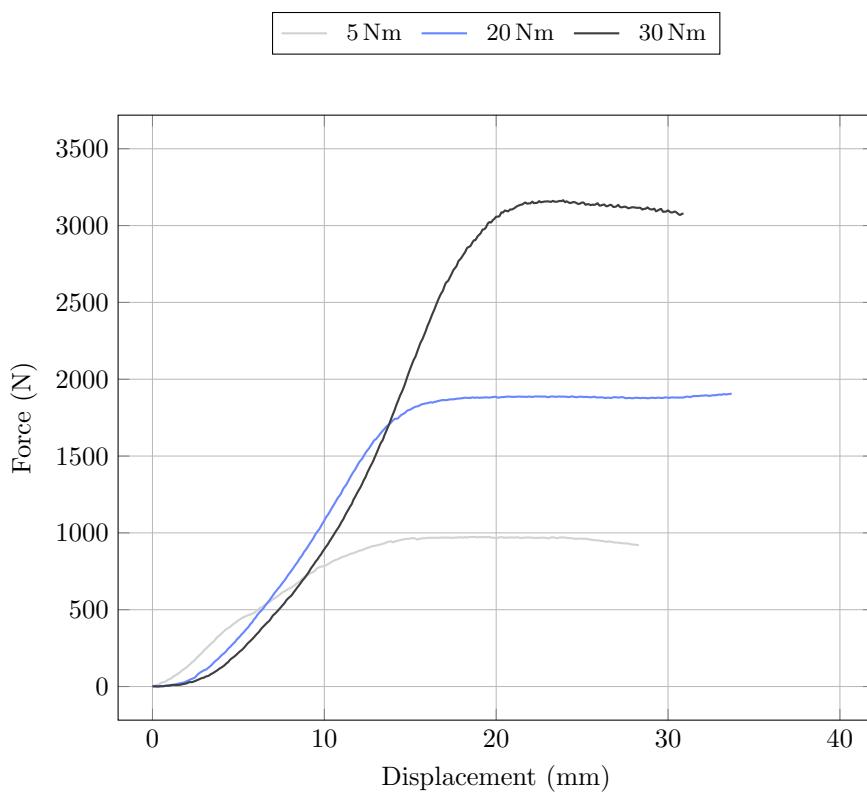


Figure 2.16 – Influence of the tightening on the sliding resistance for a 1.5 mm EPDM layer.
Results from [Tay15].

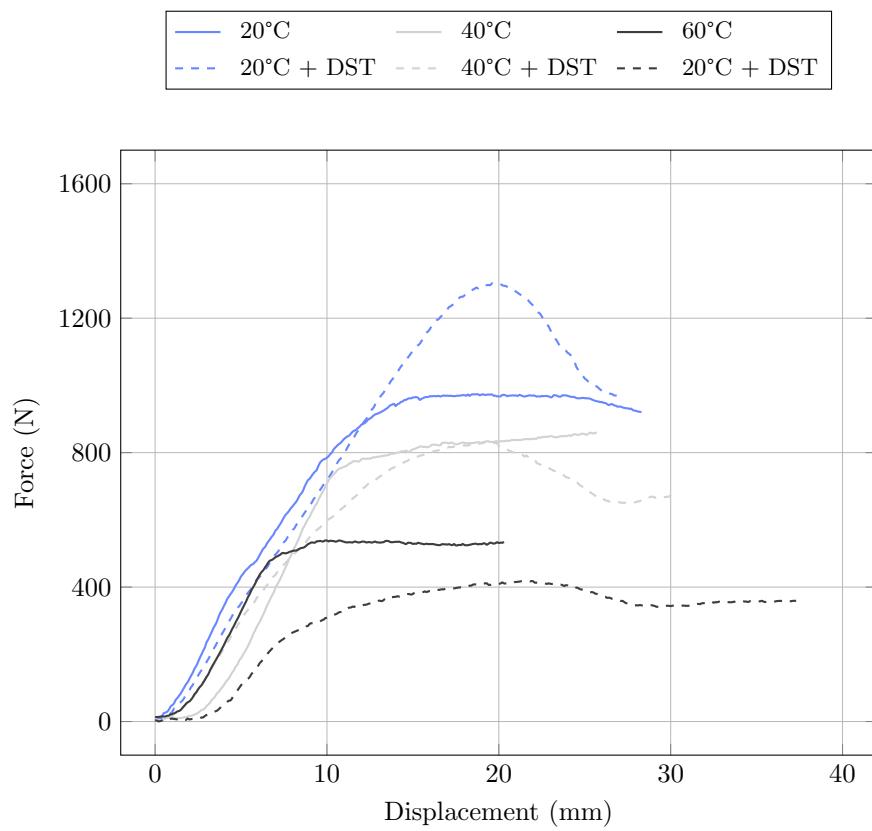


Figure 2.17 – Influence of the temperature on the sliding resistance for a 1.5 mm EPDM layer.
Results from [Tay15].

2.6.2 The sleeve system

Sleeves are major components in the structural system. The presented design is an important innovation compared with the composite gridshells built previously, where members were simply interrupted or overlapped (see [fig. 2.20a](#)). By establishing mechanical and architectural continuities between tubes, the sleeve brought the real behavior of the shell closer to its theoretical behavior (see [fig. 2.20b](#)).

The sleeve is a steel system that provides mechanical continuity between two adjacent composite tubes for both tension and bending. It is made of three parts : two connectors linked by a threaded rod (see [fig. 2.18](#)). Each connector is a 48.3×2.9 mm steel tube, slightly larger than the composite tubes it connects, with a welded M20 nut at one end. The connector is pinned to the composite tube with three 10 mm bolts. Some structural adhesive was also employed to fill the gaps and to guarantee good rigidity of the assembly. However, the sleeve is designed to ignore the contribution of the adhesive to the mechanical strength of the system. A M20 threaded rod links the two connectors. It allows tension forces and bending moments to pass from one tube to the other. It does not transfer any torsion.

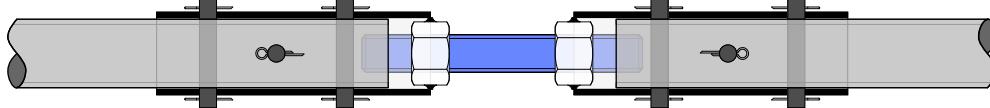


Figure 2.18 – Technical drawing of the sleeve system.

Mechanical behaviour

Tension forces are transferred from the composite tube to the connector through shear in the pins. Owing to a lower bearing resistance in the composite than in the steel, each of the three pins could be gradually loaded. When loading the system, initially, only one of the three pins is in contact with both the steel tube and the composite tube, because of inevitable minor manufacturing gaps. When the axial load is increased, this pin started to “eat” into the composite tube until the second pin also comes in contact. Thus, the axial load is transferred equally between the two pins. This scheme could work for more until another failure mode occurs. For this mode of composite failure, which prevailed in this case, the total bearing capacity of the assembly was thus three times the capacity of a single pin. This total bearing capacity can be calculated from the compressive strength, the composite thickness and the pin diameter :

$$F = 3 \times f_{u,c} \cdot d \cdot t = 3150 \text{ daN} \quad (2.4)$$

In the next section, tests carried out at the Navier laboratory to confirm the predicted value are presented. Bending moments were transferred through the threaded rod of the sleeve. This part was designed to reach the two following qualitative criterions simultaneously :

- Firstly, the bending stiffness of the rod should be roughly equivalent to the composite bending stiffness to preserve the curvature's continuity along the system. This continuity was of prime importance from an architectural point of view.

$$\frac{EI_{rod}}{EI_{gfrp}} \simeq 1 \quad (2.5)$$

- Secondly, the steel quality of the rod should be adjusted such that plastification begins when the composite tube tends to approach its maximum design stress (a third of the yield stress). Thus the rod acted as a “fuse” : if the curvature of the system reaches the maximum allowed curvature, the steel rod starts to plastify. The plastic hinge accumulates the rotation thus preventing the curvature to increase in the composite tubes.

$$\frac{M_{rod}^{elastic\ max}}{M_{gfrp}^{\sigma=133\ MPa}} \simeq 1 \quad (2.6)$$

In this project, the numerical values for the ratios in eq. (2.5) and (2.6) were 0.79 and 0.96 respectively.

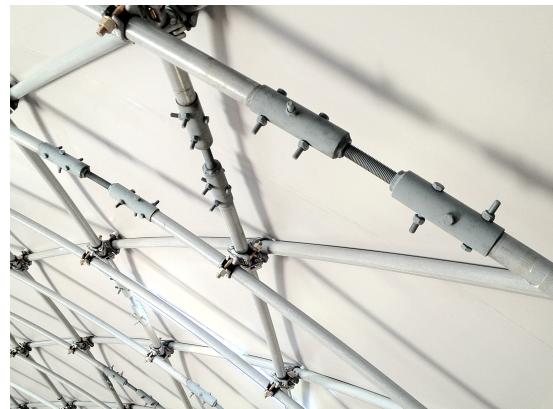
2.6.3 The membrane

The membrane is nothing but a tailor-made one-piece clothing manufactured to dress up the structure. It was prefabricated based on the 3D model of the shape computed during the formfinding process (see ??) and not on some on-site geometric survey. The technical properties of the PVC coated fabric can be found in table 2.2.

2.6.4 Foundations



(a) Solidays 2011



(b) Créteil 2013



(c) Continuity of curvature

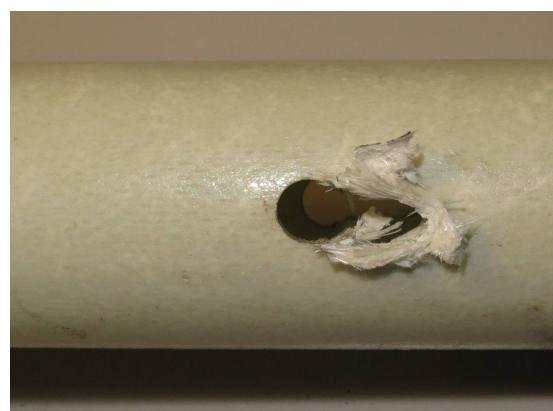


(d) Plastification threshold

Figure 2.19 – Different designs for the sleeve system.



(a) Tearing failure



(b) Contact compression failure

Figure 2.20 – Typical failure modes when testing the sleeve system in traction.

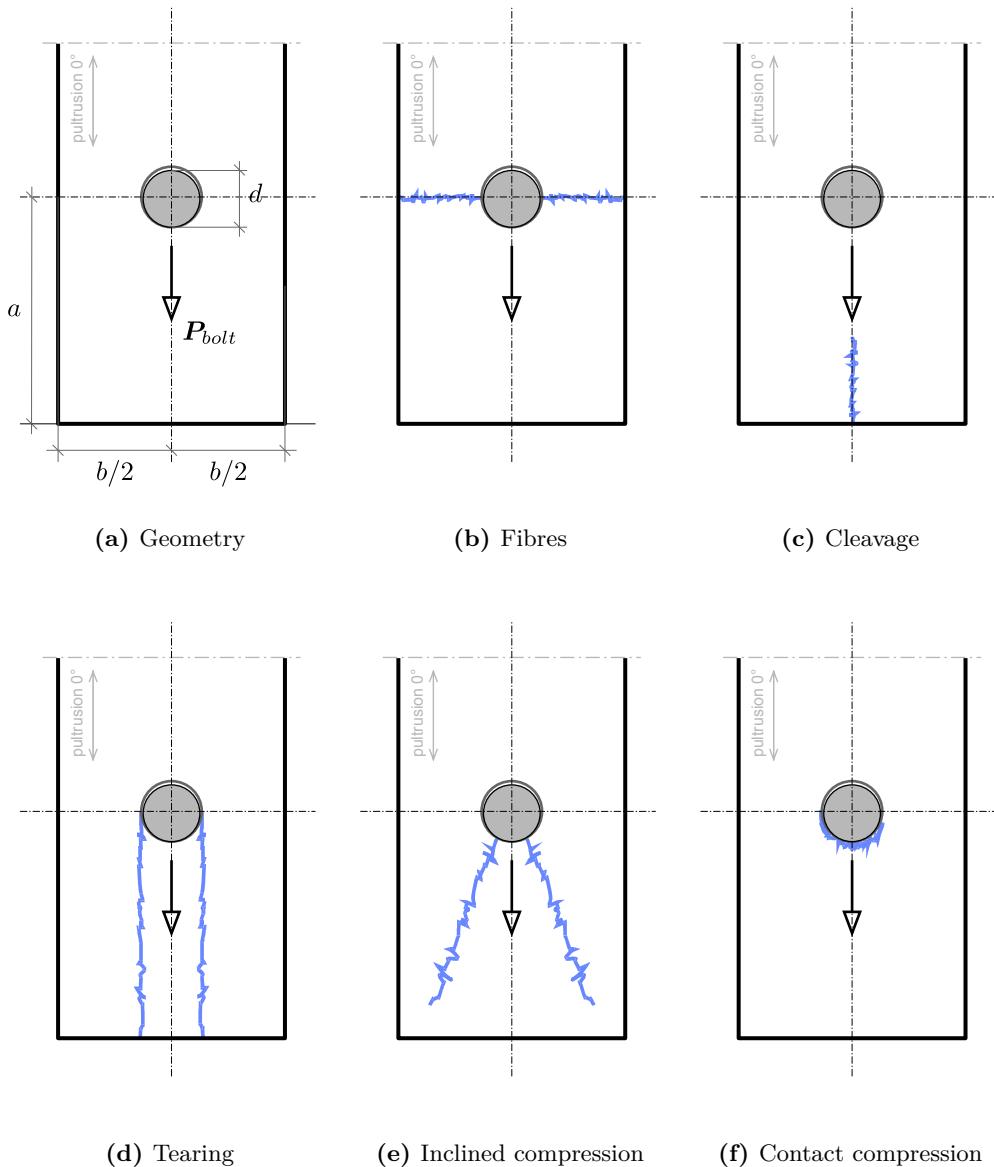


Figure 2.21 – Typical failure modes of a bolt in a pultruded element (0°).

2.7 Hygrothermal behavior

2.7.1 Temperature

During the first two years and a half of its service life, the building has shown that the thermal comfort was far from ideal as the membrane has very poor thermal properties (see [table 2.2](#)) :

- During winter, the comfort is ensured during mass by a forced-air heating system positioned on the equipments slab, few meters away from the main structure. This solution is adapted to infrequent occupations of the building. In that case, the energetic cost remains limited even if the solution is far from optimal because of the lack of insulation.
- During summer, the temperature raises very fast when the sun shines. The forced-air system is used to ventilate the interior volume. But the comfort level is rapidly insufficient as the interior temperature quickly exceeds 30 °C. The discomfort is amplified as the membrane gets very hot and radiates toward the inside, increasing the feeling of warm. Consequently, there was no choice but to scheduled the mass earlier at this period of the year. Cooling was not possible for economic reasons as the building was used only few hours a week.

At the time of writing, the building is being reconfigured and so its purpose is changing. A better thermal comfort is now required and solutions have to be found. Thus, a study on the thermal behavior of the structure has been done.⁷ The main results are gathered in this thesis. [fig. 2.22a](#) presents the monthly exterior temperatures observed at the site location from 8:00 AM to 7:00 PM, which corresponds to the new intended opening hours of the building. Note that the maximum value is given over one hour, that is the observed temperature exceeds this value during a one-hour-wide window in the day. The solar radiation is also given in [fig. 2.22b](#). Two scenarios are studied :

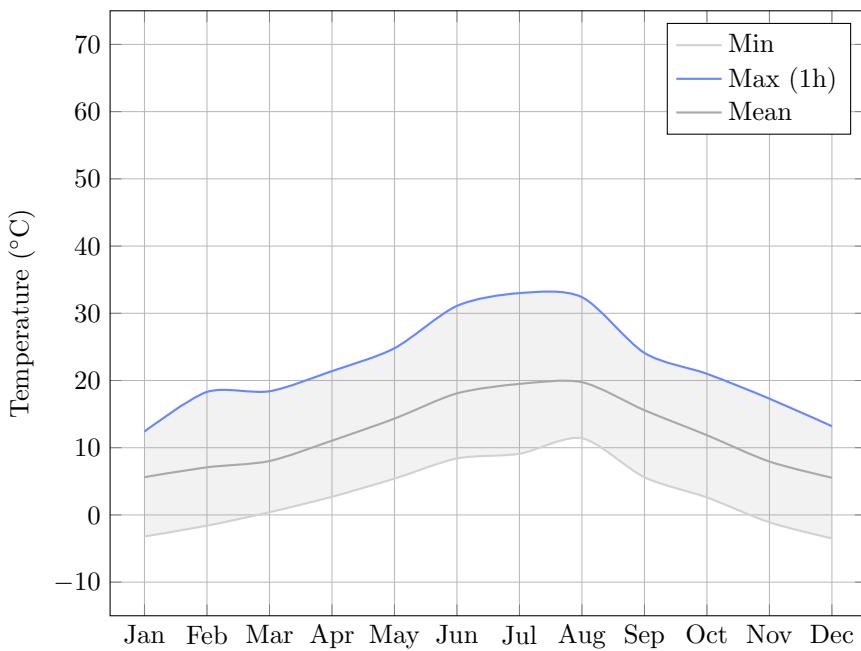
1. The structure is completely closed. No ventilation is put in place. The interior temperature can reach 70 °C (see [fig. 2.23a](#)).
2. The structure is ventilated but no cooling system is put in place. The maximum interior temperature is lowered significantly but it can still reach 50 °C (see [fig. 2.23b](#)).

This study confirms what the experience has shown : the temperature inside the building can be very high. Above 30 °C this is problematic for the comfort of the people. Above 50 °C and up to 70 °C it becomes problematic for the building itself. Indeed, this level of temperature is closed to the heat resistance of the membrane (see [table 2.2](#)) ; the interface layer faces a serious decrease of its capacity to resist to sliding ([fig. 2.17](#)) ; the creeping kinetics of GFRP tubes is speed up ([§2.5.1](#)).

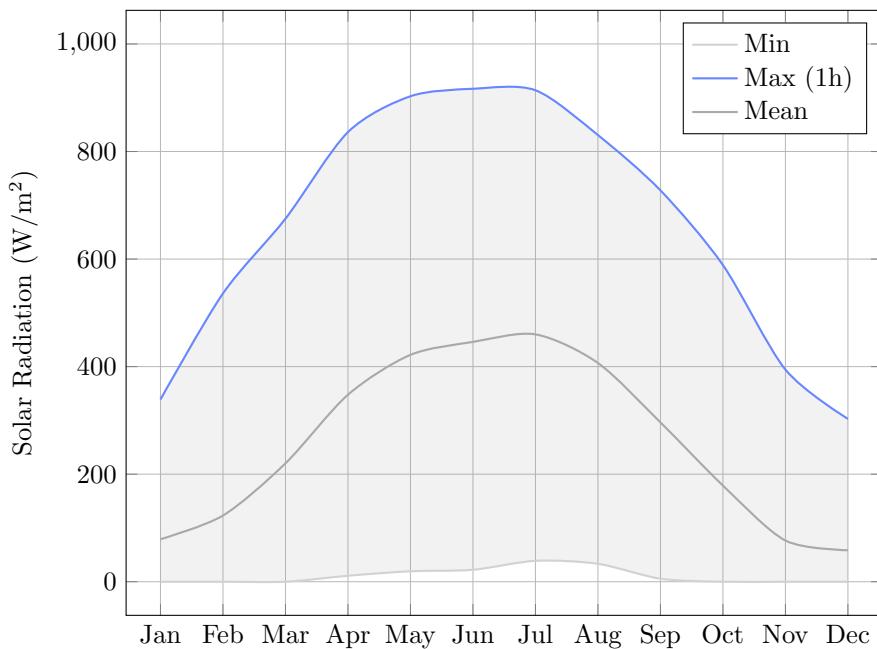
⁷This study was done in June 2017 by the design companies T/E/S/S and CHOULET for the reconfiguration project of the temporary cathedral.

2.7.2 Moisture

Condensation was also noticed in winter and shoulder season. Sometimes, droplets of water could fall abundantly and thus the wooden furnitures had to be protected. This phenomenon was particularly intense the first months because the concrete slab had not yet fully dry-out. To protect the structure and the furnitures, it was decided to maintain the inside temperature above 10 °C at all times.

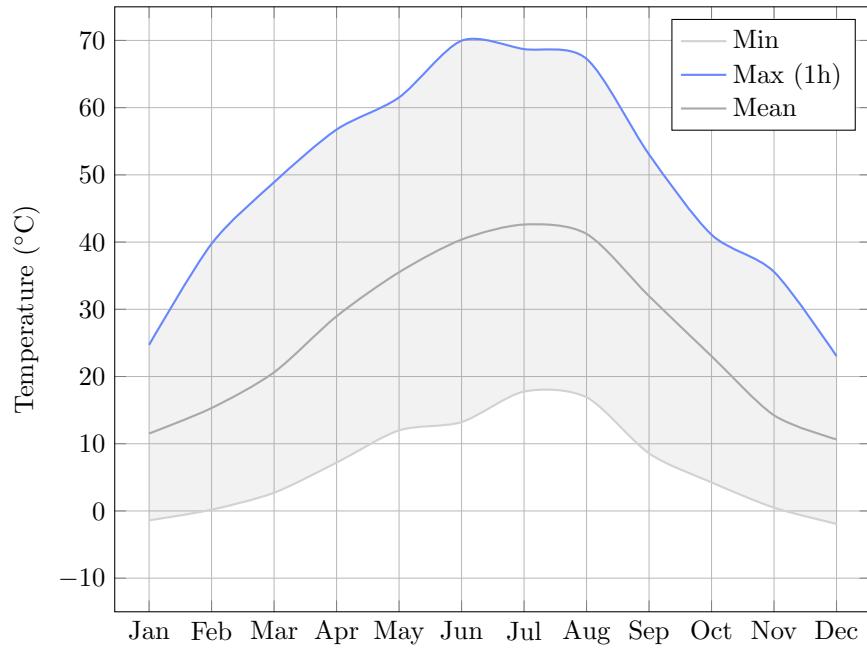


(a) Temperature.

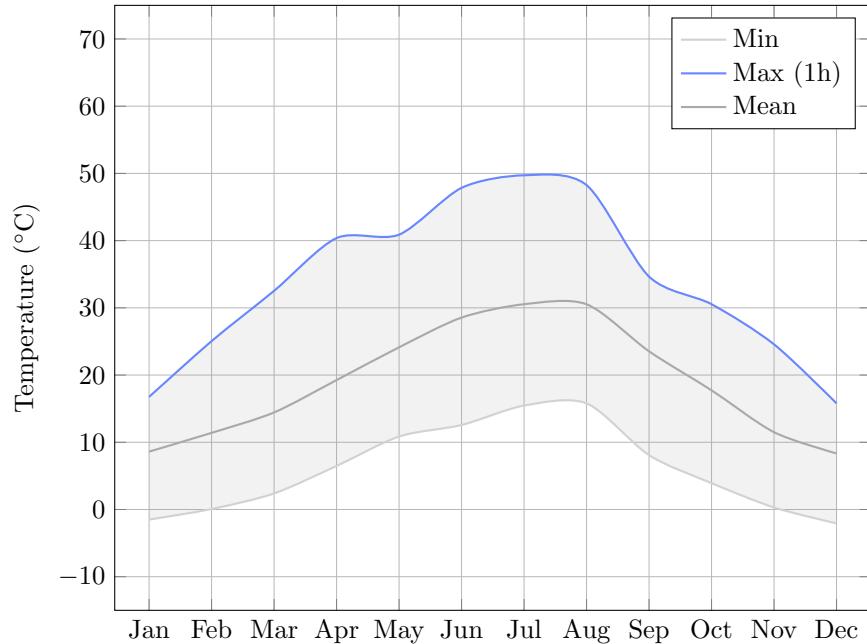


(b) Solar radiation.

Figure 2.22 – Weather data at site location during opening hours (8:00 AM to 7:00 PM).



(a) Without ventilation.



(b) With ventilation.

Figure 2.23 – Temperature inside the building during opening hours (8:00 AM to 7:00 PM).

2.8 Cost analysis

2.8.1 Overall cost for the client

The overall cost of the project – that is the amount of money paid by the client – was estimated to 324 000 € excluding taxes (see [fig. 2.24](#)). This price includes all the possible costs related to the construction of the project : the cost of the main building (masonry, doors, gridshell, envelop, fittings, heating, electricity, lightning, drainage, sewage, etc.), the cost of the service building, the cost of pedestrian pathways, the cost of the design studies, etc.

However, this cost does not take into account all the (free) man-hours spent by the volunteers to prefabricate, assemble, erect and brace the gridshell. The real cost of the gridshell system, when a cost is put on this labor, is estimated in [§2.8.2](#).

Moreover, this project required a lot of design studies and tests to verify the material properties and to validate the strength of key elements such as the swivel coupler with its EPDM layer, the sleeve system and the ground anchorage. The real cost of the studies was by far higher than what was really charged to the client and the difference must be regarded as an investment from the company T/E/S/S. In the same manner, people from the laboratory gave a consistent support during the construction stage as they were the only available experienced workers familiar with the construction of elastic gridshells in composite material [[DCB10](#), [BCTdP12](#)] and this labor was not charged back to the client.

The project was favorably accepted by the client based on the estimation that the cost of the gridshell would not be more expensive by 30% than renting a simple tent. The rental of a 400 m² tent with its floor was evaluated to 110 000 € for a period of time of 18 months. Retrospectively this target was met, especially as the cathedral was finally used for almost two years and a half, far more than the 18 months expected initially, and with no additional cost because the diocese owned the building

2.8.2 Cost details for the building

Here we present the cost details for the main building, that is the cathedral itself. We try to understand what is the true cost of the gridshell system in this particular project and we thus eliminate side costs (for instance the cost of fittings, the cost of the pedestrian pathways, the cost of the service building, etc.). The cost allocation is presented per square meter of covered area in [fig. 2.25](#). The total price for the building, excluding studies, is 473 €/m². It is composed of :

- 100 €/m² : the cost of masonry works (levelling, footings, slab, drainage) detailed in [table 2.6](#). This construction works were made by a professional contractor named BATEM.
- 248 €/m² : the cost of the superstructure (anchorages, gridshell, membrane covering and doors) detailed in [table 2.7](#). This price includes the labor of the volunteers

(35 €/hour) and all the costs associated to construction of the structure, including the renting of all the necessary equipments (cranes, aerial buckets, etc.).

- 126 €/m² : the cost of the envelope (lacing rod, fabric, installation) detailed in [table 2.7](#). This construction works were made by a professional contractor named ESMERY CARON.

Here, the global amount of studies was charged around 83 300 €, that is 248 €/m² (see [fig. 2.24](#)). This heavy cost was compensated by the fact that volunteers provided a lot of free labor (see [fig. 2.26](#)). In a more standard commercial context, the design process would be optimized too and the price of studies would go down to 15% to 20% of the price of the building, that is 70 to 95 €/m². This would bring the final price of the building to 550 €/m². This price is clearly high if only its sheltering capability is required regarding other technologies.. However, if more than sheltering is mandatory, the quality and singularity of the space created here is probably worth the price ; then this technology becomes a lot more affordable than existing traditional systems that can materialized free-forms.

2.8.3 Strengths and weaknesses

In this project the prefabrication process represents almost half of the cost in man-hours (see [fig. 2.26](#)). The manufacturing of the grid (cutting pipes, marking nodes, preassembling swivel couplers, sleeves and anchorages, etc.) could easily be automated. Composite materials such as GFRP are easy to cut, mill and drill. Small robot arms can do the job quickly with a better accuracy. This idea has been tested in a workshop at the Ecole des Ponts ParisTech in september 2016.⁸ More over, a numerical production process would allow to answer quickly to a variety of forms with the same equipment and industrial process.

Lots of man-hours are spent in the installation of the sleeve system (88 h). That represents 35 % of the man-hours spent in the grid prefabrication. This part should be reimplemented to allow a simpler and faster installation. Similarly, the connection should be redesigned to avoid the application of the EPDM protection layer and to allow a faster positioning and fastening as it represents 23 % of the man-hours spent in the prefabrication of the grid. This would also be a preponderant factor of improvement in the bracing stage although this cost is not detailed in [table 2.5](#).

At first sight it seems that the time spending assembling the grid on the construction site – which represents 22 % of the man-hours, see [fig. 2.26](#) – can not easily be reduced. However, the grid system could be divided into transportable modules. These modules would be preassembled in the factory to increase the speed and quality of the production and to minimize on-site works. Thanks to the intrinsic grid kinematic, modules can be folded for transportation. Once on site, modules are unfolded and connected to each other to form

⁸See the video of the construction of a 50 m² wooden gridshell in the workshop “Building Free Froms” : <http://thinkshell.fr/freeform-wooden-gridshell-2016/>.

the primary flat grid. This idea was tested successfully in two wooden gridshell projects of 50 m² each, with students of the Ecole National d'Architecture de Toulouse and Ecole National d'Architecture de Grenoble in June 2016.⁹

Bracing is yet another costly stage as it accounts for almost 30 % of the man-hours (see fig. 2.26). This work is not easily parallelizable as it requires working at height with proper lifting equipments such as cherry pickers. Thus almost a small and qualified team can do the job. For instance, on the gridshell of Créteil, the team was composed of 6 workers using two aerial lifts. This team spent three full days to complete this task, that is the same amount of days required to assemble the grid and lift it up. Several attempt have been made during this thesis to answer this problematic. The first attempt was to use a bidirectional cable network to brace the grid. The network is installed at the ground level before the grid is deformed. Thus, work at height is reduced to a minimum. The second attempt is a larger thought on the envelope of such structures and tries to tackle two issues with a thin fibre-reinforced concrete skin : the fact that bracing with a third direction of tubes is time consuming ; and the fact that membrane covering is not adapted for permanent buildings.

⁹Construction of two wooden gridshell pavilions : <http://www.lemoniteur.fr/article/a-toulouse-les-architectes-se-rassemblent-sous-le-meme-pavillon-32398196>.

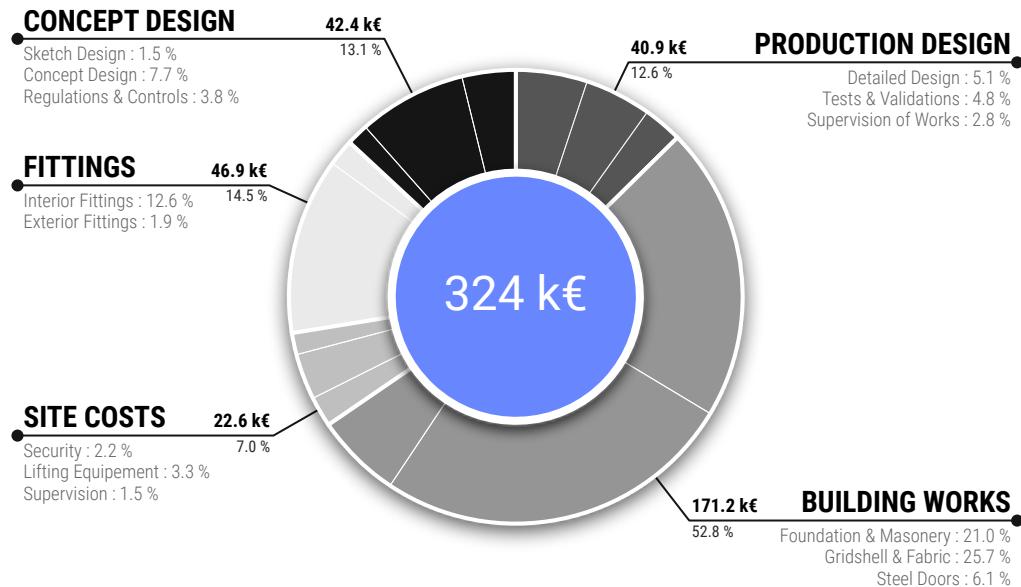


Figure 2.24 – Cost allocation for the whole project. This is the estimated overall final cost charged to the client. Prices are given excluding taxes (V.A.T).

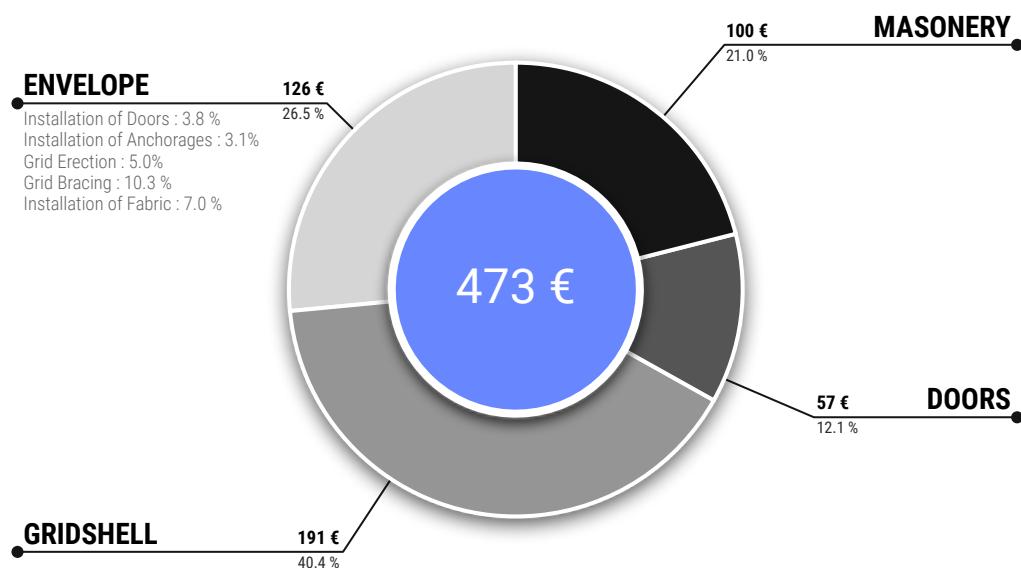


Figure 2.25 – Cost allocation per square meter of covered area. The cost of design is not included as it would not be representative. Prices are given excluding taxes (V.A.T).

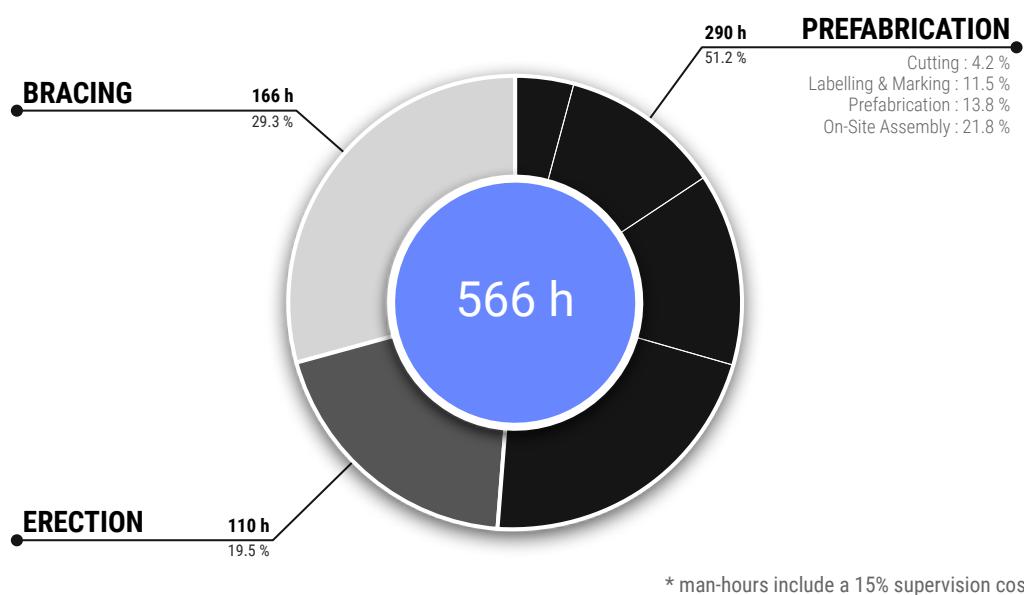


Figure 2.26 – Allocation of the man-hours spent by the volunteers on the fabrication of the gridshell. A 15% increase is considered to take into account coordination and supervision of the individual tasks. See [table 2.5](#) for detailed data.

Item	Unit Task		Man-Hours	
	Worker	Duration	Quantity	Hours
Workstation “Cutting”	4			20.53
Pick a raw tube from the stock	2	1' 00"	176	5.87
Mark it and cut it at right length	2	2' 00"	176	11.73
Put it into the labelling stock	2	0' 30"	176	2.93
Workstation “Labelling”	5			56.64
Pick a tube from the labelling stock	2	1' 00"	176	5.87
Label it at start and end	2	1' 00"	176	5.87
Mark the position of connection collars	1	0' 30"	2260	18.83
Mark the position of sleeves	1	0' 30"	250	2.08
Mark the position of anchorages	1	0' 30"	127	21.06
Put it into the prefabrication stock	2	0' 30"	176	2.93
Workstation “Prefabrication”	6			67.75
Pick a tube from the prefabrication stock	2	1' 00"	176	5.87
Put the EPDM ribbon	1	0' 30"	2260	18.83
Prefix the swivel collar on the tube	1	0' 30"	565	4.71
Glue the sleeves	3	2' 00"	250	25.00
Drill pin holes for the sleeves	1	1' 00"	250	4.16
Fix sleeve pins	1	1' 30"	250	6.25
Put it into the final stock	2	0' 30"	176	2.93
Workstation “Site Assembly”	12			107.34
Connect the sleeves with steel rods	5	5' 00"	125	52.08
Pick a tube and position it in the grid	2	3' 00"	176	17.60
Install swivel couplers (HV)	1	2' 00"	565	18.83
Controlled tightening of couplers (HV)	1	1' 00"	1130	18.83
Workstation “Grid Erection”	12	8:00' 00"	1	96.00
Workstation “Grid Bracing”	6	8:00' 00"	3	144.00
Grid prefabrication				252.00
Grid erection				96.00
Grid bracing				144.00
Cost of supervision (15%)				74.00
Total				566.00

Table 2.5 – Man-hours spent by the volunteers on the fabrication of the gridshell. A 15% increase is considered to take into account coordination and supervision of the individual tasks. See [fig. 2.26](#) for a graphical representation of these data.

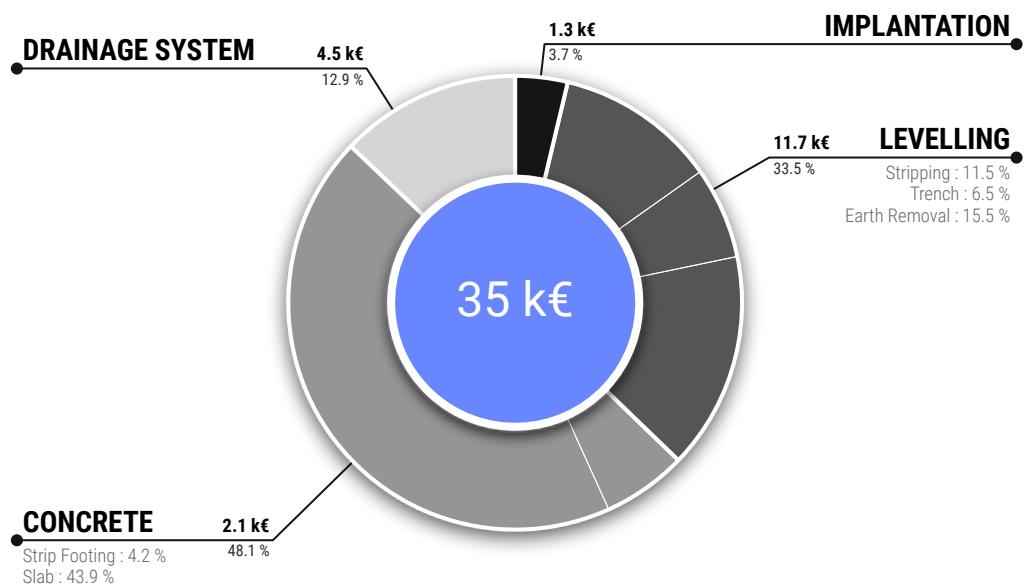


Figure 2.27 – Cost allocation for masonry works. Prices are given excluding taxes (V.A.T). Only costs associated to the structure are reported here ; for instance the works for the equipment slab and the pathways are omitted. See [table 2.6](#) for detailed data.

Item	Unit	Quantity	U.P. (€)	Price (€)
Building implantation		1	1 300.00	1 300.00
Leveling works				11 680.00
Top soil stripping	m ²	400	10.00	4 000.00
Trench for concrete strip footing	ml	76	30.00	2 280.00
Earth removal	m ³	180	30.00	5 400.00
Concrete				17 400.00
Concrete strip footing (200 kg/m ³ steel)	ml	70	30.00	2 100.00
Concrete slab (x2 welded wire mesh)	m ²	340	45.00	15 300.00
Drainage systems				4 500.00
French drain (x2 Ø100 mm pipes)	ml	70	30.00	2 100.00
Precast concrete inspection chamber		1	400.00	400.00
Drain line (PVC Ø125 mm pipe)	ml	30	30.00	900.00
Pre-assembled channel drain	ml	10	110.00	1 100.00
Masonry works	€/m ²	350	100	34 880.00

Table 2.6 – Cost details for masonry works. Prices are given excluding taxes (V.A.T). Only costs associated to the structure are reported here ; for instance the works for the equipment slab and the pathways are omitted. See [fig. 2.27](#) for a graphical representation of these data.

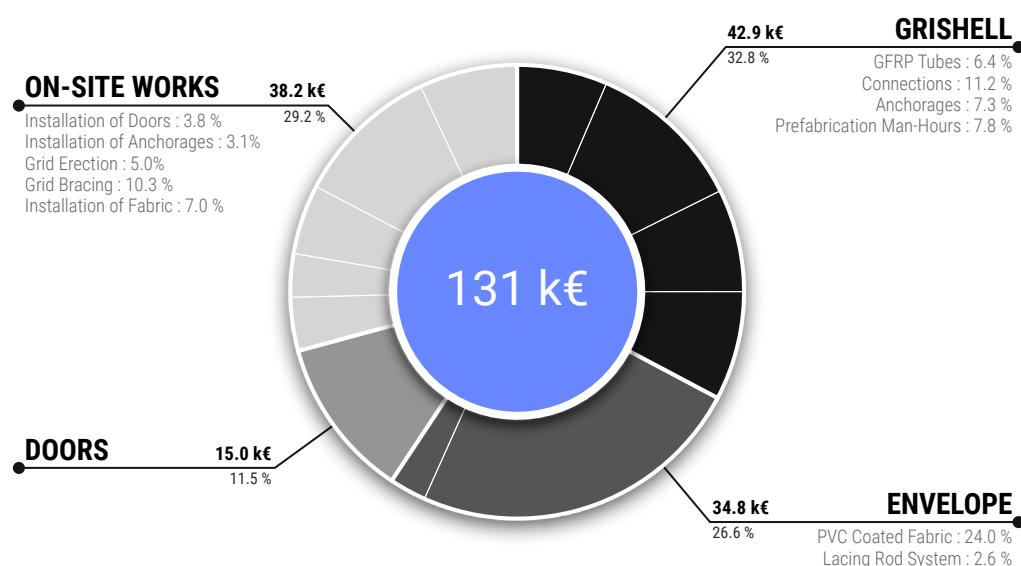


Figure 2.28 – Cost allocation for the superstructure. On-site works are isolated to identify pure manufacturing costs of the gridshell, the envelope and the doors. To this end, the cost of the man-hours provided by the volunteers to prefabricate the grid has been assessed and allocated. See [table 2.7](#) for detailed data.

Item	Unit	Quantity	U.P. (€)	Price (€)
Manufacturing of the gridshell				
GRFP tube (Ø42 mm)	ml	2304	3.66	8 433
Swivel connector (42x42 mm)		1295	3.95	5 115
Swivel connector (42x49 mm)		135	4.18	564
EPDM layer (1302x40x1.5 mm ribbon)		2775	0.36	1131
Welded steel sleeve system		150	50.00	7 500
ARALDIT 2047 glue (480 ml cartridge)		8	45.00	360
Ground anchorage (welded steel)		120	80.00	9 600
Man-hours (prefabrication)	h	290	35.00	10 150
Manufacturing of the envelope				
GFRP lacing rod (Ø32 mm)	ml	96	14.00	1 134
Steel clip for the rod		125	15.00	1 875
Swivel collar (Ø34 mm)		120	3.50	420
PVC coated fabric	m ²	550	50.00	27 500
Option for transparent inclusion		12	320.00	3 840
Manufacturing of the steel doors				
Main door		1	10 000.00	10 000
Small door		1	5 000.00	5 000
On-site works of installation				
Installation of the doors		1	5 000.00	5 000
Installation of the anchorages and clips	h	90	45.00	4 050
Grid erection	h	110	35.00	3 850
cranes (x2 35T)	h	24	110.00	2 640
Grid bracing	h	167	45.00	7 515
aerial bucket (x2)		1	6 000.00	6 000
Installation of fabric		1	9 165.00	9 165
Total	€/m ²	374	363	130 842
Cost of structure			265	92 622
Cost of installation			109	38 220

Table 2.7 – Cost details for the superstructure. On-site works are isolated to identify pure manufacturing costs of the gridshell, the envelope and the doors. To this end, the cost of the man-hours provided by the volunteers to prefabricate the grid has been assessed and allocated. Prices are given excluding transport costs and excluding taxes (V.A.T.). Spare quantities are included. See fig. 2.28 for a graphical representation of these data.

2.9 Conclusions

This paper has presented the different steps for the design of a gridshell in composite material : a Temporary Cathedral at Crêteil, in Paris suburb, in 2013. The first step was the optimization of the shape in order to avoid local concentrations of curvature. The second step showed a tool to automatically mesh a surface using the compass method. With this tool, the orientation of the mesh is studied according to structural and architectural criterions. The last steps showed the structural analysis of the gridshell and how to get the as-built geometry from the analysis model. Architecturally, the structure offers a very interesting space where the textual richness of the tubes against the membrane accentuates the reading of the complex curved surfaces.

This project demonstrates that gridshells in composite material are suitable for constructing freeform buildings. However, the long-term behavior of these materials needs to be better characterized to extend their lifespan.

At the moment, further developments are conducted by the laboratory to take account for torsional effects and non axisymmetric sections in such structures, as it is studied in [BAK13].

2.10 Acknowledgements

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