| FACULTY OF ENGINEERING | Project | | | | Job Ref. | |
|------------------------|---------|------|----------|------|----------------|------|
| UNIVERSITY OF RUHUNA | Section | | | | Sheet no./rev. | |
| Learning Tool for | | | | | 1 | |
| Reinforced Concrete | Calc.by | Date | Chk'd by | Date | App'd by | Date |
| Design | | | | | | |

FLEXURAL REINFORCEMENT CALCULATION (EC2)

(FLANGE BEAM)

K' = 0.167

 $K = \frac{M}{bd^2 f_{cu}} =$

 $b_{eff} =$

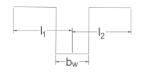
$$f_{cu} = Mpa$$

$$f_{v} = Mpa$$

$$L = m$$

$$M = kN/m$$

$$h_f = mm$$



$$Z = d \left\{ 0.5 + \sqrt{0.25 - \frac{K}{1.134}} \right\} =$$

But not greater than 0.95d

$$x = \frac{(d-z)}{0.4} =$$

$0.8x \le h_f$ $0.8x > h_f$

Slab thickness is taken as h_f , hence, neutral axis is in fact within the flange and the beam can be designed as a rectangular beam width.

$$(b_{eff} = b)$$

$$A_S = {}^M/_{0.87} f_{yK} Z$$
$$A_S =$$

So the rectangular stress block goes outside the flange region, and the neutral axis goes inside the web region. The above Z value is then not valid.

$$M_{uf} = 0.567 f_{ck} (b_{eff} - b_w) h_f (d - 0.5 h_f)$$

 $M_{uf} =$

$$K_f = \frac{M - M_{uf}}{f_{ck} b_w d^2}$$
$$K_f =$$

$$A_{S} = \frac{M_{uf}}{0.87 f_{yk} (d - 0.5 h_{f})}$$

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<u>Check for maximum reinforcement</u> $A_{S,max} = 0.04A_c =$

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