

Calculation Description

234rwesdfsdv

Concrete Dowels Calculation

According to Norwegian "Betongelementboken"
Calculation Date: 10/15/2025, 9:46:52 AM

INPUT PARAMETERS

Load Parameters

$V_{Ed} = 27.5$ kN
 $f_{dowel} = 1.4$

Material Properties

$f_{ck} = 35$ MPa
 $f_{yk} = 500$ MPa
 $\alpha_{cc} = 0.85$
 $\gamma_c = 1.5$
 $\gamma_s = 1.15$
n-factor = 14

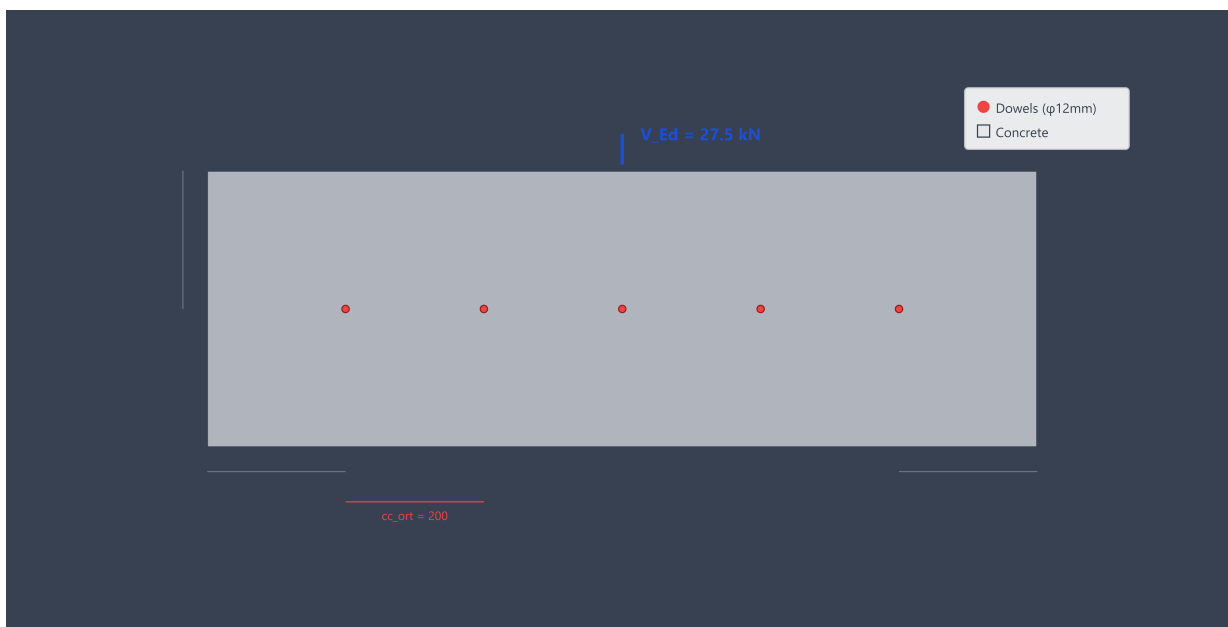
Dowel Configuration

Diameter = 12 mm
 $n_{V,parallel} = 1$
 $c/c_{parallel} = 200$ mm
 $n_{V,ortagonal} = 5$
 $c/c_{ortagonal} = 200$ mm

Edge Distances

Eccentricity = 0 mm
 $a_1 = 200$ mm
 $a_{2,h} = 200$ mm
 $a_{2,v} = 200$ mm

PLOT



RESULTS SUMMARY

$V_{Rd} = 66.9$ kN

Shear Capacity of Dowel Group

Concrete Utilization

41.1%

$v_c = V_{Ed} / V_{Rd}$

Steel Utilization

19.4%

$v_s = V_{Ed} / V_{Rd,s}$

Bending Utilization

39.5%

$m_s = M_{max} / M_{Rd,s0}$

DETAILED CALCULATIONS

Basic Calculations

$$\begin{aligned}n_{V,tot} &= n_{V,parallel} \times n_{V,ortagonal} = 1 \times 5 = 5 \\A_s &= \pi \times \varnothing^2 / 4 = \pi \times 12^2 / 4 = 113.1 \text{ mm}^2 \\f_{cd} &= \alpha_{cc} \times f_{ck} / \gamma_c = 0.85 \times 35 / 1.5 = 19.83 \text{ MPa} \\f_{yd} &= f_{yk} / \gamma_s = 500 / 1.15 = 434.78 \text{ MPa} \\\sigma_{cd} &= 3 \times f_{cd} = 3 \times 19.83 = 59.5 \text{ MPa}\end{aligned}$$

Steel Shear Capacity

$$V_{Rd,s} = (n_{V,tot} \times A_s \times f_{yd}) / \sqrt{3} = 141.9 \text{ kN}$$

Plastic Moment Capacity

$$M_{Rd,s0} = f_{yd} \times \varnothing^3 / 6 = 434.78 \times 12^3 / 6 = 0.125 \text{ kNm}$$

Concrete Shear Capacity (Single Dowel)

$$\begin{aligned}V_{Rd,e} &= 1.5 \times \sqrt{((f_{cd} \times e \times \varnothing)^2 + f_{cd} \times f_{yd} \times \varnothing^4)} - 1.5 \times f_{cd} \times e \times \varnothing \\V_{Rd,e} &= 20.06 \text{ kN} \\V_{Rd,c0} &= V_{Rd,e} = 20.06 \text{ kN}\end{aligned}$$

Distance Factors

$$\begin{aligned}k_a &= (\min(n \times \varnothing, a_1) - \varnothing) / (n \times \varnothing - \varnothing) = 1 \\k_s &= (a_{2,v} + (n_{V,ort}-1) \times c_{Cort} + a_{2,h}) / (3 \times \min(n \times \varnothing, a_1)) = 2.381 \\k &= \min(n_{V,ortagonal}, k_a \times k_s) = 2.381 \\\psi_{f,V} &= 1\end{aligned}$$

Final Shear Capacity

$$\begin{aligned}V_{Rd} &= f_{dowel} \times k \times \psi_{f,V} \times V_{Rd,c0} \\V_{Rd} &= 1.4 \times 2.381 \times 1 \times 20.06 = 66.9 \text{ kN}\end{aligned}$$

Utilization Checks

$$\begin{aligned}\text{Concrete: } v_c &= V_{Ed} / V_{Rd} = 27.5 / 66.9 = 41.1\% \\ \text{Steel: } v_s &= V_{Ed} / V_{Rd,s} = 27.5 / 141.9 = 19.4\% \\ \text{Bending: } m_s &= M_{max} / M_{Rd,s0} = 0.05 / 0.125 = 39.5\%\end{aligned}$$