

The reaction at A (horizontal) is obtained from the equation:

Exact: 
$$\epsilon_{22} = \frac{\delta}{h}$$
  $\epsilon_{11} = 0 = \frac{\sigma_{11}}{E} - \frac{V(\sigma_{12} + \sigma_{23})}{E(\sigma_{11} + \sigma_{22})}$ 

$$E \ E_{22} = \frac{E8}{h} = \sigma_{12} - \nu \ \frac{2\nu\sigma_{12}}{1-\nu} = \sigma_{22} \ \frac{1-\nu-2\nu^2}{1-\nu}$$

$$2\sigma_{II} = \frac{2\nu}{(1-\nu)} \frac{\pm \delta}{h(4\nu)(1-2\nu)}$$

The reaction HA is half the total load:

Multiply by suitable virtual deflection field Swa and integrate between "o" and "L":

Integrate by parts:

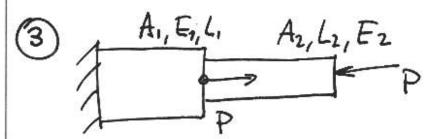
$$\int_{0}^{\infty} \left[ \left( \text{EI } w_{0}^{(i)} \right)^{2} \text{Sw}_{0}^{2} \, dx - \int_{0}^{\infty} \left( \text{EI } w_{0}^{(i)} \right)^{2} \text{Sw}_{0}^{2} \, dx + \int_{0}^{\infty} \left( \text{H } w_{0}^{2} \, \text{Sw}_{0}^{2} \, dx \right) \, dx$$

$$- \int_{0}^{\infty} \left[ \left( \text{EI } w_{0}^{(i)} \right)^{2} \text{Sw}_{0}^{2} \, dx - \int_{0}^{\infty} \left( \text{EI } w_{0}^{(i)} \right)^{2} \text{Sw}_{0}^{2} \, dx + \int_{0}^{\infty} \left( \text{H } w_{0}^{2} \, \text{Sw}_{0}^{2} \, dx \right) \, dx$$

The PVD reads:

Show dx = Show swodx oxxxl

H SWo s.t. SWo, SWo' satisfy the homogeneous essantial boundary conditions the natural boundary conditions are not enforced by the principle.



By equilibrium reaction at built-in end is "0", as is the normal force in the left member.
The exact solution is:

$$\sigma_1 = 0$$
  $\sigma_2 = \frac{P}{A_2}$ 

Finite element solution:

$$K^{e} = \frac{A^{e}E^{e}}{L^{e}}\begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$\begin{cases} O \\ O_{2} \\ O_{3} \end{cases} = \begin{cases} P \\ -P \end{cases}$$

$$K = \begin{bmatrix} A^{e}E^{e} \\ -1 & 1 \end{bmatrix}$$

$$\begin{cases} K_{22} U_2 + K_{23} U_3 = P \\ K_{32} U_2 + K_{33} U_3 = -P \end{cases}$$

$$K_{22} = K_{22}^{\odot} + K_{11}^{\odot} = A_{1}E_{1} + A_{2}E_{2}$$

$$K_{23} = K_{12}^{2} = \frac{-A_2 E_2}{L_2} = K_{32}$$

$$K_{33} = K_{22}^{2} = \underbrace{A_1 E_2}_{L_2}$$

Solving system:

$$(K_{22} + K_{52})U_2 + OU_3 = O \Rightarrow \boxed{U_2 = O}$$

$$K_{33} U_3 = -P$$

$$\rightarrow \boxed{U_3 = \frac{-PL_2}{A_2 E_2}}$$

Element 
$$0$$
  $U_1^0 = U_1 = 0$ ,  $U_2^0 = U_2 = 0$ 

$$U_2^0 = \phi_1 \circ + \phi_2 \circ = 0$$

$$\varepsilon^0 = 0$$
,  $\sigma^0 = 0$ 

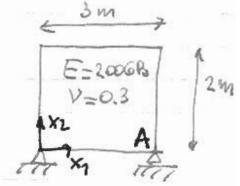
Element ② 
$$U_1 = U_2 = 0$$
  $U_2 = U_3 = \frac{-PL_2}{E_2A_2}$ 

$$U_1 = \emptyset, 0 + \emptyset_2 \left(\frac{-PL_2}{E_2A_2}\right), \quad \emptyset_2 = \frac{x}{L_2}$$

$$E_2 = \frac{-P}{E_2 A_2}$$
,  $\sigma_2 = \frac{-P}{A_2}$  Exact!



The 4-node plane strain element shown is subjected to the constant stresses



Compute the displecement at A

$$\mathcal{E}_{22} = 0 \longrightarrow \mathcal{T}_{33} = \mathcal{V}(\mathcal{T}_{11} + \mathcal{T}_{22}) = \frac{90 \text{ MP}_3}{200 \text{ 10}^9}$$

$$\mathcal{E}_{44} = \frac{1}{E} \left[ \mathcal{T}_{11} - \mathcal{V}(\mathcal{T}_{22} + \mathcal{T}_{33}) \right] = \frac{143 \text{ 10}^6}{200 \text{ 10}^9}$$

$$2\mathcal{E}_{12} = \frac{\sqrt{12}}{G} = \frac{100 \, \text{MPs}}{200 \, \text{GPa}} \, 2 \, (1+03) = \frac{2.6}{2 \, \log^3}$$

$$L(3,0) = 3 E_{11} = \frac{3 \times 143}{200 \times 10^3} m = 0.21 \text{ Cm}$$

$$\begin{array}{c}
A = A_0 \left(1 - \frac{\times}{4L}\right) \\
A = A_0 \left(1 - \frac{\times}{4L}\right)
\end{array}$$

PVD: 
$$\int \sigma_{ij} \, \delta \epsilon_{ij} \, dv = F \, \delta u(i)$$

specializing to 1D:  $\int_{0}^{L} AGUT SE dx = F Su(L)$ 

$$\int_{0}^{L} A_{0}\left(1-\frac{X}{4L}\right) \left(\frac{72}{73}+\frac{24x}{73L}\right) \frac{F}{A_{0}} SE dx \stackrel{?}{=} FSu(L)$$

1) Su= ax (admissible, satisfies home. essential B. (. Su(o)=0).

PVD: 
$$\int_{0}^{L} \left(1 - \frac{x}{4L}\right) \left(\frac{7^{2}}{73} + \frac{24}{73} \frac{x}{L}\right) \frac{x}{L} dx \stackrel{?}{=} x^{1}$$

$$\frac{72}{73} - \frac{1}{4} \frac{72}{73} \frac{1}{2} + \frac{24}{73} \frac{1}{2} - \frac{24}{4 \times 73} \frac{1}{3} \stackrel{?}{=} 1$$

$$\frac{7}{8} \frac{72}{73} + \frac{24}{73} \left( \frac{1}{2} - \frac{1}{12} \right) \stackrel{?}{=} 1$$

$$\frac{\left(\frac{21}{8} + \frac{5}{12}\right) \frac{24}{73}}{\frac{63+10}{24}} \stackrel{?}{=} 1$$

$$\int_{0}^{L} \left(1 - \frac{X}{4L}\right) \left(\frac{72}{73} + \frac{24}{73} \frac{X}{L}\right) \frac{30(x^{2})}{L^{3}} dx \stackrel{?}{=} 0.1 \times \frac{3}{3}$$

$$\frac{72}{73} \frac{1}{3} - \frac{1}{4} \frac{72}{73} \frac{1}{4} + \frac{24}{73} \frac{1}{4} - \frac{1}{4} \frac{24}{73} \frac{1}{5} = \frac{1}{3}$$

$$\frac{72}{73} \left( \frac{1}{3} - \frac{1}{16} \right) + \frac{24}{73} \left( \frac{1}{4} - \frac{1}{20} \right) \stackrel{?}{=} \frac{1}{3}$$

$$\frac{72}{73} \frac{13}{48} + \frac{24}{73} \frac{1}{5} \stackrel{?}{=} \frac{1}{3}$$

$$\frac{24}{73}\left(\frac{3\times13}{4816}+\frac{1}{5}\right)\stackrel{?}{=}\frac{1}{3}$$

$$\frac{13}{3} + \frac{24}{73} \cdot \frac{1}{5} = \frac{1}{3}$$
PVD not satisfied stresses not in equilibrium !!
$$\frac{1}{3} \left( \frac{3 \times 13}{13} + \frac{1}{13} \right) \stackrel{?}{=} \frac{1}{3}$$
PvD not satisfied stresses not in equilibrium !!

$$\frac{24}{73} \left( \frac{65 + 16}{80} \right) = \frac{243}{730} = 0.3328 \neq \frac{1}{3}$$