Plate Bending

not so long plate

previously have shown: $M := -D \cdot \frac{d^2}{dx^2} w$. this was for single axis bending.

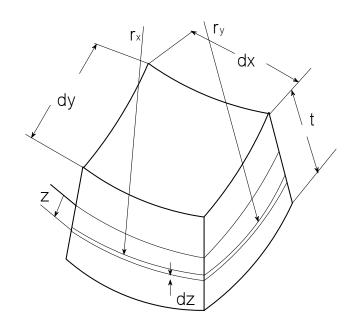
this relationship holds for the partial derivative in the respective direction fo both x and y;

assumptions:

plane cross section remains plane

small deflections $w_{max} < 3/4 t$

stress < yield



$$w = w(x, y)$$

$$\frac{1}{R_x} = -\frac{d^2}{dx^2} w(x,y)$$
 $\frac{1}{R_y} = -\frac{d^2}{dy^2} w(x,y)$

we are making no statement with respect to ϵ_{V} (or any other $\epsilon)$ as we did in long plate.

$$\begin{split} \epsilon_{\mathbf{X}} &\coloneqq \frac{1}{R_{\mathbf{X}}} \cdot \mathbf{z} & \epsilon_{\mathbf{X}} \coloneqq -\mathbf{z} \cdot \frac{d^2}{d\mathbf{x}^2} \mathbf{w}(\mathbf{x}, \mathbf{y}) & \epsilon_{\mathbf{y}} \coloneqq \frac{1}{R_{\mathbf{y}}} \cdot \mathbf{z} & \epsilon_{\mathbf{y}} \coloneqq -\mathbf{z} \cdot \frac{d^2}{d\mathbf{y}^2} \mathbf{w}(\mathbf{x}, \mathbf{y}) \\ \epsilon_{\mathbf{X}} &\coloneqq \frac{\sigma_{\mathbf{X}}}{E} - \frac{\mathbf{v} \cdot \sigma_{\mathbf{y}}}{E} & \epsilon_{\mathbf{y}} \coloneqq \frac{\sigma_{\mathbf{y}}}{E} - \frac{\mathbf{v} \cdot \sigma_{\mathbf{x}}}{E} & \mathbf{v} \cdot \boldsymbol{\epsilon}_{\mathbf{y}} & \mathbf{v} \cdot \boldsymbol{\epsilon}_{\mathbf{$$

$$\varepsilon_{\mathbf{x}} + \mathbf{v} \cdot \varepsilon_{\mathbf{y}} \text{ collect}, \sigma_{\mathbf{x}}, \mathbf{E} \rightarrow = \sigma_{\mathbf{x}} := \frac{\mathbf{E}}{1 - \mathbf{v}^2} \cdot (\varepsilon_{\mathbf{x}} + \mathbf{v} \cdot \varepsilon_{\mathbf{y}})$$

substituting

into

$$\epsilon_{\boldsymbol{y}} \coloneqq -z \frac{d^2}{d\boldsymbol{y}^2} w(\boldsymbol{x}, \boldsymbol{y}) \qquad \quad \epsilon_{\boldsymbol{x}} \coloneqq -z \cdot \frac{d^2}{d\boldsymbol{x}^2} w(\boldsymbol{x}, \boldsymbol{y}) \qquad \quad \boldsymbol{\sigma}_{\boldsymbol{x}} \coloneqq \frac{E}{1-\boldsymbol{v}^2} \cdot \left(\boldsymbol{\epsilon}_{\boldsymbol{x}} + \boldsymbol{v} \cdot \boldsymbol{\epsilon}_{\boldsymbol{y}} \right)$$

$$\sigma_x \to \frac{E}{1-\nu^2} \cdot \left(-z \cdot \frac{d}{dx} \frac{d}{dx} w(x,y) - \nu \cdot z \cdot \frac{d}{dy} \frac{d}{dy} w(x,y) \right) \quad \text{or} \ \dots \qquad \\ \sigma_x(z) \coloneqq -z \frac{E}{1-\nu^2} \cdot \left(\frac{d^2}{dx^2} w(x,y) + \nu \cdot \frac{d^2}{dy^2} w(x,y) \right) = 0$$

$$\text{similarly:} \qquad \sigma_y \coloneqq -z \frac{E}{1-v^2} \cdot \left(\frac{d^2}{dy^2} w(x,y) \, + \, v \cdot \frac{d^2}{dx^2} w(x,y) \right)$$

as in bending (applied in each of x and y direction): see figure Hughes 9.3 (below) the lower case m denotes moment per unit length note that the designation is changed: the subscript on the m refers to the direction of axial stress

$$m_{X} := \int_{\frac{-t}{2}}^{\frac{t}{2}} \sigma_{X}(z) \cdot z \, dz$$

$$m_{y} := \int_{\frac{-t}{2}}^{\frac{t}{2}} \sigma_{y} \cdot z \, dz$$

$$m_{x} \rightarrow \frac{1}{12} \cdot t^{3} \cdot E \cdot \frac{\frac{d \ d}{dxdx} w(x,y) \ + \ v \cdot \frac{d \ d}{dydy} w(x,y)}{-1 \ + \ v^{2}}$$

$$m_y \rightarrow \frac{1}{12} \cdot t^3 \cdot E \cdot \frac{\frac{d}{dy} \frac{d}{dy} w(x,y) + v \cdot \frac{d}{dx} \frac{d}{dx} w(x,y)}{-1 + v^2}$$

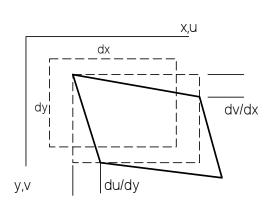
with
$$D := \frac{E \cdot t^3}{12 \cdot \left(1 - v^2\right)}$$

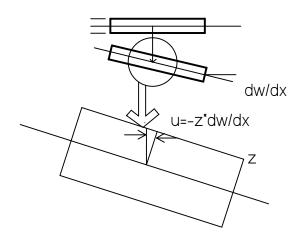
$$m_{x} := -D \cdot \left[\frac{d^{2}}{dx^{2}} w(x,y) + \nu \left(\frac{d^{2}}{dy^{2}} w(x,y) \right) \right]$$

$$m_{y} := -D \cdot \left(\frac{d^{2}}{dy^{2}} w(x,y) + v \cdot \frac{d^{2}}{dx^{2}} w(x,y) \right)$$

in the general plate case there may exist a twisting moment my we need to derive a similar expresssion for this moment

from basic shear strain relationships: $\gamma := \frac{d}{dv}v + \frac{d}{dv}u$; $\tau := G\gamma$ and $\tau := G\frac{d}{dv}v + \frac{d}{dv}u$





from the geometry of the slope of w in each direction (x and y): $u := -z \cdot \frac{d}{dx} w(x,y)$ and $v := -z \cdot \frac{d}{dy} w(x,y)$

 $=> \tau := -2 \cdot G \cdot z \cdot \frac{d}{dx dy} \frac{d}{dx dy} w(x, y)$ and the twisting moment per unit length is determined by:

$$m_{xy} := -\int_{-\frac{t}{2}}^{\frac{t}{2}} \tau \cdot z \, dz \qquad m_{xy} \to \frac{1}{6} \cdot t^3 \cdot G \cdot \frac{d}{dy} \frac{d}{dx} w(x, y)$$

minus sign comes from sense of mxy + and $\tau xy(+)^*z(+)$ see figure Hughes 9.3

using: G:=
$$\frac{E}{2 \cdot (1 + \mathbf{v})}$$
 => m_{xy} := $\frac{d}{dx} \left(\frac{d}{dy} w(x,y) \right) \cdot \frac{Et^3}{12 \cdot (1 + \mathbf{v})}$

multiply and divide by (1-v) => $m_{xy} := \frac{d}{dx} \left(\frac{d}{dy} w(x,y) \right) \cdot \frac{Et^3}{12 \cdot (1+y)} \cdot \frac{(1-y)}{(1-y)}$

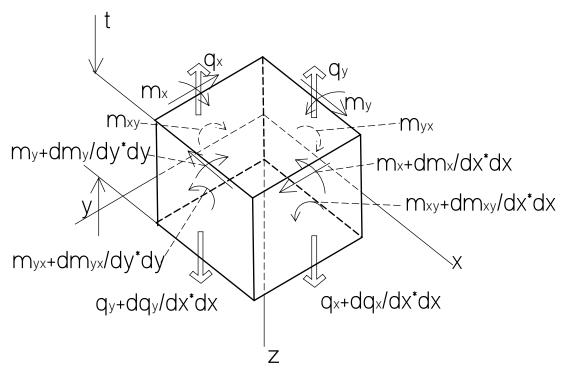
$$m_{xy} \coloneqq \frac{d}{dx} \left(\frac{d}{dy} w(x,y) \right) \cdot \frac{E t^3}{12 \cdot (1+\nu)} \cdot \frac{\left(1-\nu\right)}{\left(1-\nu\right)}$$

$$m_{xy} := \frac{d}{dx} \left(\frac{d}{dy} w(x,y) \right) \cdot \frac{E t^3}{12 \cdot \left(1 - v\right)} \cdot \left(1 - v\right) = > \qquad m_{xy} := D \cdot \left(1 - v\right) \left[\frac{d}{dx} \left(\frac{d}{dy} w(x,y) \right) \right]$$

due to the complimentary nature of shear stress $~\tau_{xy}\!\coloneqq\tau_{\text{yx}}$ and due to the sign convention for + moment $m_{XY} := -\mathbf{m}_{YX}$

$$=> m_{yx} := -D \cdot (1 - \nu) \left[\frac{d}{dx} \left(\frac{d}{dy} w(x,y) \right) \right]$$

now for equilibrium of the dx, dy, dz segment: see Hughes figure 9.3 forces and moments in a plate (below)



Hughes figure 9.3 forces and moments in a plate

 α in this context is the shear force per unit length on each face: $q_x^{\ \ *}$ the distance dy is the force on the x face etc...

equilibrium of vertical forces => p is the lateral (+z direction) load per unit area - not shown on figure

$$\left[q_{x} + \left(\frac{d}{dx} q_{x} \right) \cdot dx \right] \cdot dy - q_{x} \cdot dy + \left[q_{y} + \left(\frac{d}{dy} q_{y} \right) \cdot dy \right] \cdot dx - q_{y} \cdot dx + p \cdot dx \cdot dy = 0 \qquad \Longrightarrow \qquad \qquad \frac{d}{dx} q_{x} + \frac{d}{dy} q_{y} + p = 0$$

moments wrt the x axis =>

$$\left[m_{xy} + \left(\frac{d}{dx} m_{xy} \right) \cdot dx \right] \cdot dy - m_{xy} \cdot dy + m_y \cdot dx - \left[m_y + \left(\frac{d}{dy} m_y \right) \cdot dy \right] \cdot dx + q_y \cdot dx \cdot dy = 0$$

$$= 0$$

and similarly: taking moments wrt the y-axis =>

$$\begin{split} &\frac{d}{dy}m_{yx}+\frac{d}{dx}m_{x}-q_{x}&=0\\ &\text{or ... since} \qquad m_{xy}:=-\textbf{m}_{yx} \qquad -\!\!\left(\!\frac{d}{dy}m_{xy}\!\right)+\frac{d}{dx}m_{x}-q_{x}&=0 \end{split}$$

substituting the moment relations

$$q_x := -\!\!\left(\!\frac{d}{dy}m_{xy}\right) + \frac{d}{dx}m_x \ \text{ and } \quad q_y := -\!\!\left(\!\frac{d}{dx}m_{xy}\right) + \frac{d}{dy}m_y$$

into the shear equation => $\frac{d}{dx} \left[-\left(\frac{d}{dy} m_{xy}\right) + \frac{d}{dx} m_{x} \right] + \frac{d}{dy} \left[-\left(\frac{d}{dx} m_{xy}\right) + \frac{d}{dy} m_{y} \right] + p = 0$

$$\frac{d^{2}}{dx^{2}}m_{x} - 2 \cdot \frac{d}{dx}\frac{d}{dy}m_{yx} + \frac{d^{2}}{dy^{2}}m_{y} + p = 0$$

substituting the relations fro m_x , m_y and m_{xy} above =>

$$\frac{d^2}{dx^2} \left[-D \cdot \left[\frac{d^2}{dx^2} w(x,y) + v \left(\frac{d^2}{dy^2} w(x,y) \right) \right] \right] - 2 \cdot \frac{d}{dx} \left[\frac{d}{dy} D \cdot \left(1 - v \right) \left[\frac{d}{dx} \left(\frac{d}{dy} w(x,y) \right) \right] \right] + \frac{d^2}{dy^2} \left[-D \cdot \left(\frac{d^2}{dy^2} w(x,y) + v \cdot \frac{d^2}{dx^2} w(x,y) \right) \right]$$

$$= -D$$

the terms with v cancel and the result is: $\frac{d^4}{dx^4}w(x,y) + 2 \cdot \frac{d^2}{dx^2} \frac{d^2}{dy^2}w(x,y) + \frac{d^4}{dy^4}w(x,y) = \frac{p}{D}$

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