

TMR 4585 Specialization Course UWT

Thermal expansion behaviour

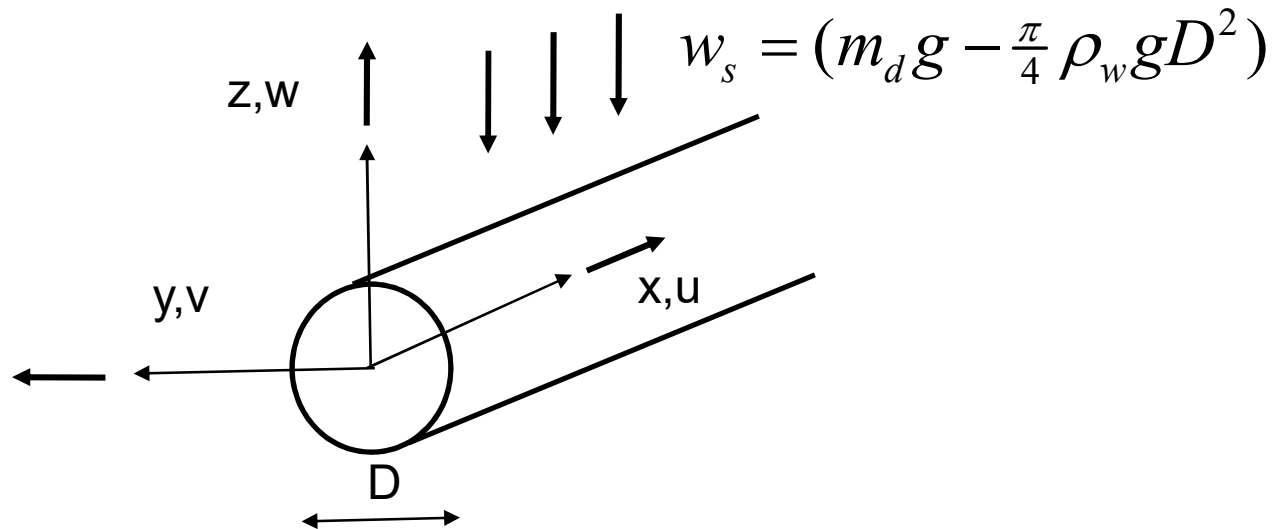
by
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Trondheim, 2019

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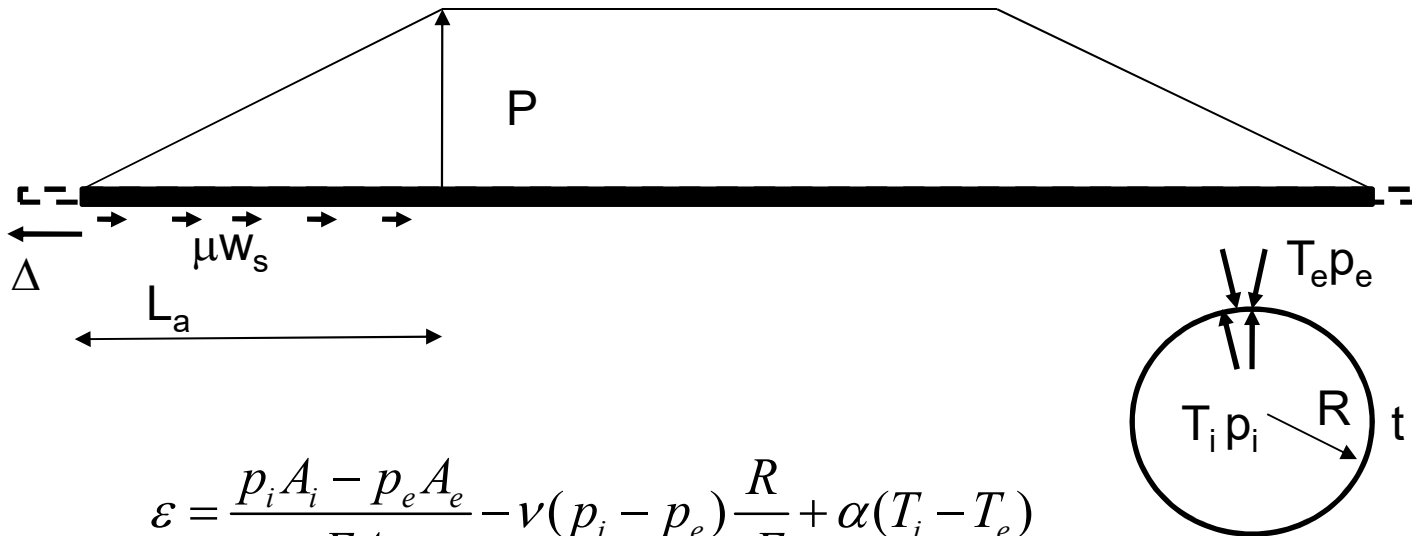
- **Pipeline thermal expansion**
- **What are the governing failure modes and critical sections?**
- **Pipeline global buckling**
 - Analytical evaluation
 - Finite element analysis

Pipeline thermal expansion

Coordinate system



Pipeline thermal expansion



$$\varepsilon = \frac{p_i A_i - p_e A_e}{EA} - \nu(p_i - p_e) \frac{R}{tE} + \alpha(T_i - T_e)$$

$$\varepsilon \cong \frac{\Delta p R}{2Et} (1 - 2\nu) + \alpha \Delta T$$

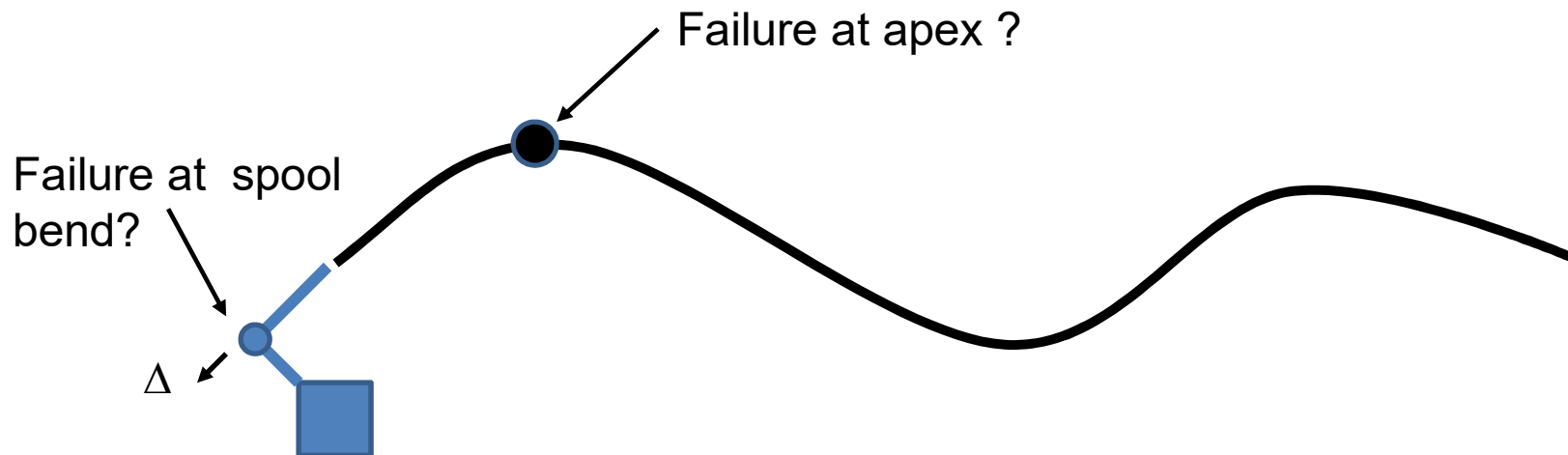
$$P \cong \Delta p \pi R^2 (1 - 2\nu) + 2\pi R t E \alpha \Delta T = \Delta p \pi R^2 (1 - 2\nu) + EA \alpha \Delta T$$

$$L_a = \frac{P}{\mu w_s} \quad \Delta = \int_0^{L_a} \left(\frac{P}{EA} - \frac{\mu w_s x}{EA} \right) dx = \frac{Pl_a}{2EA}$$

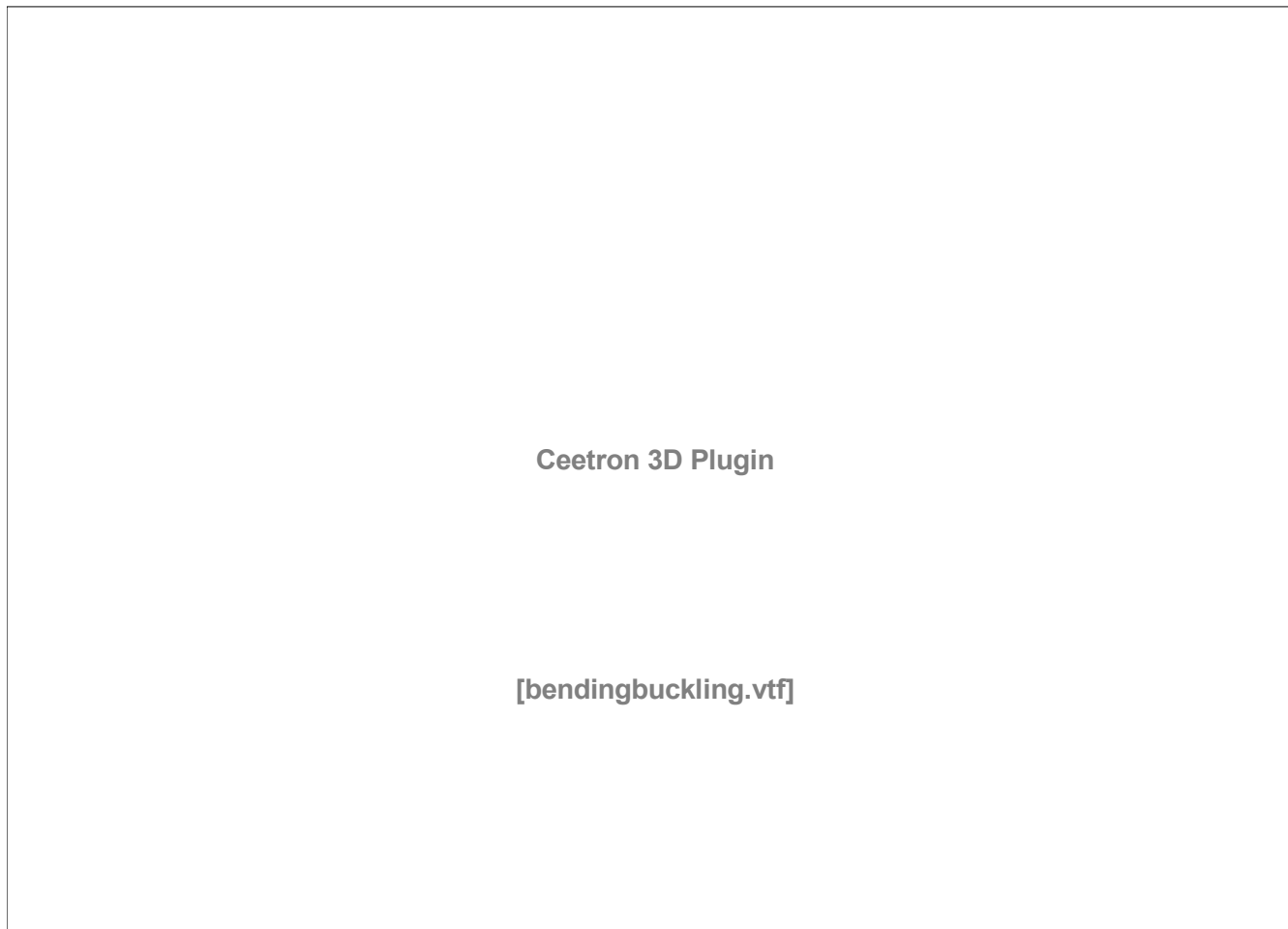
What are the governing failure modes and critical sections?

- **Failure mode concerns at inline buckle apex and at expansion spool bends:**

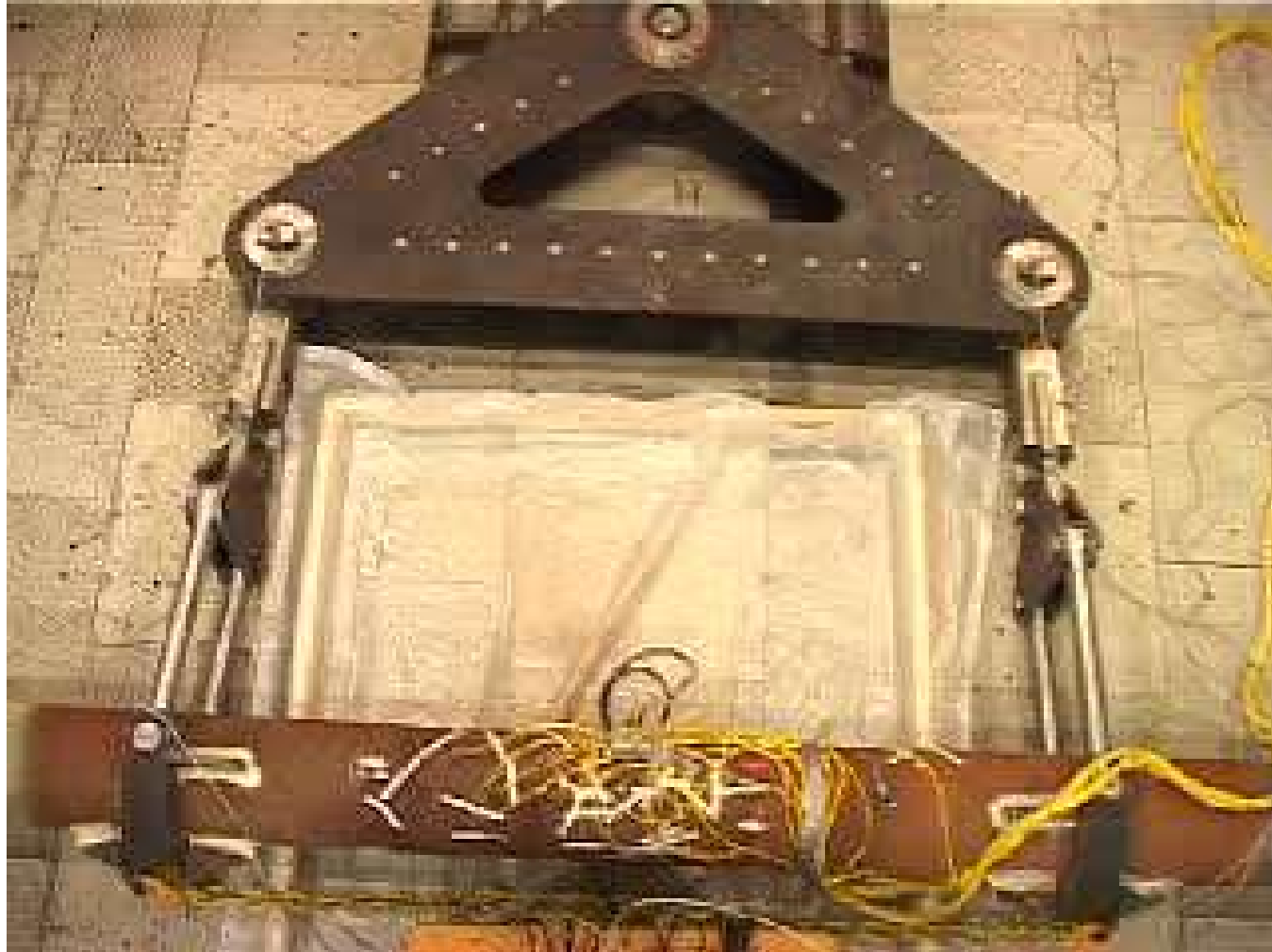
- Local buckling due to bending and external/internal pressure
- Ovalization
- Fracture
- Low Cycle Fatigue



Bending induced local buckling at apex

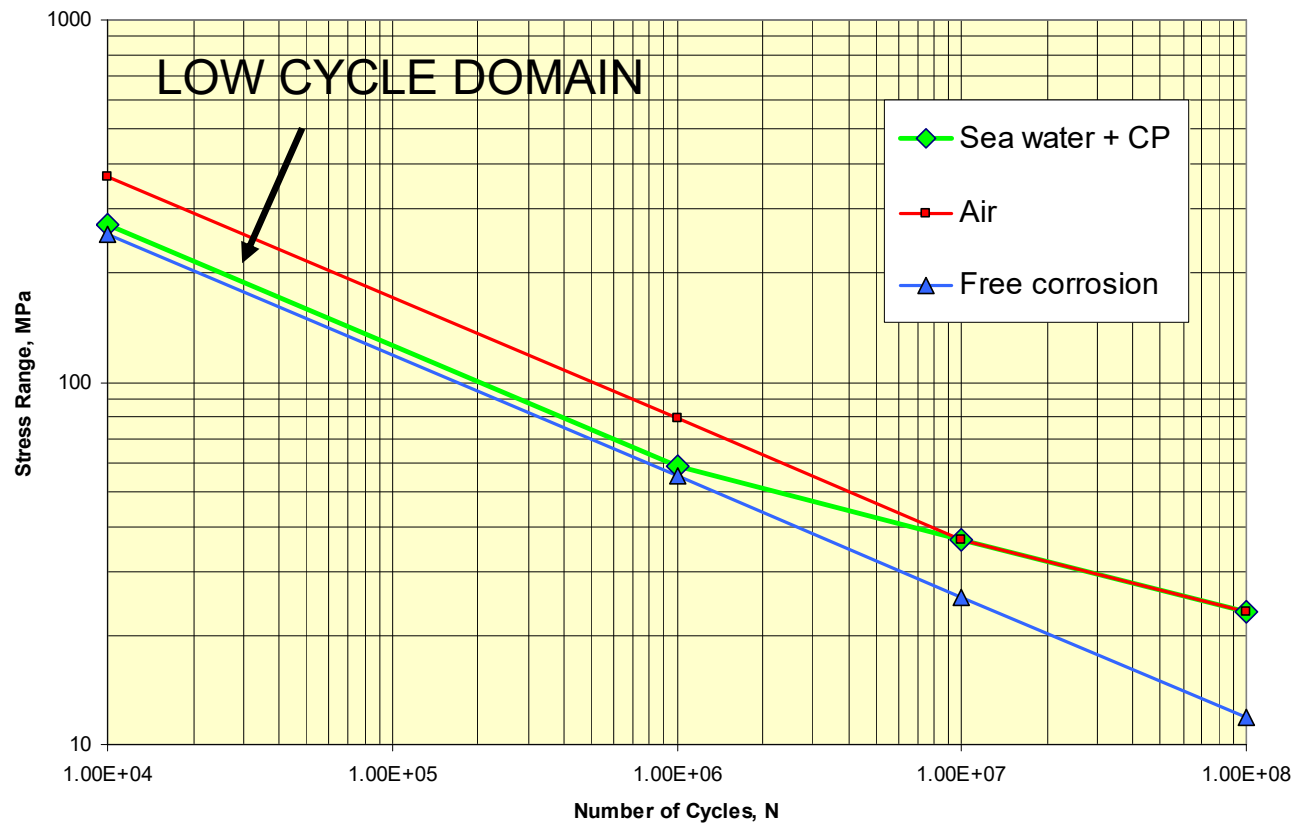


Fracture due to bending moment at buckling apex



Fatigue due to cyclic bending moment at buckling apex (cyclic shut-down and start-up)

Comparison between different environments, Curve F1 (DNV RP-C203)
Comparison of S-N curves, F1 curve (DNV RP-C203)

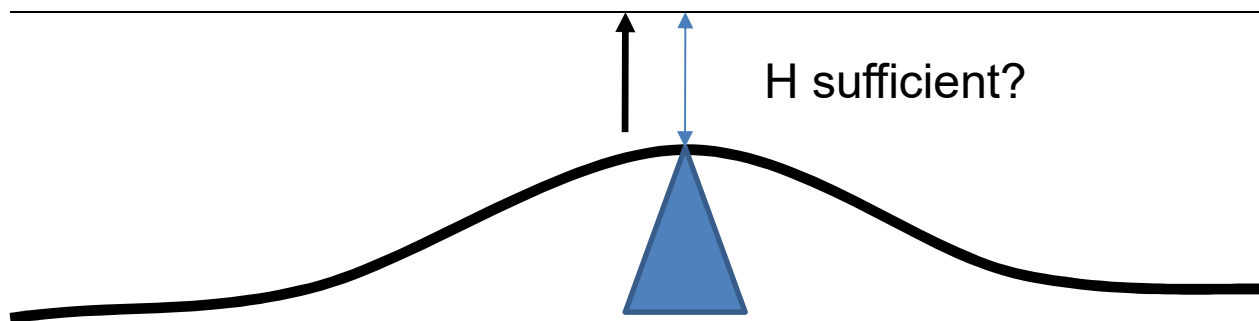


Pipeline global buckling

- Pipelines resting exposed on seabed – often referred to as "snaking"

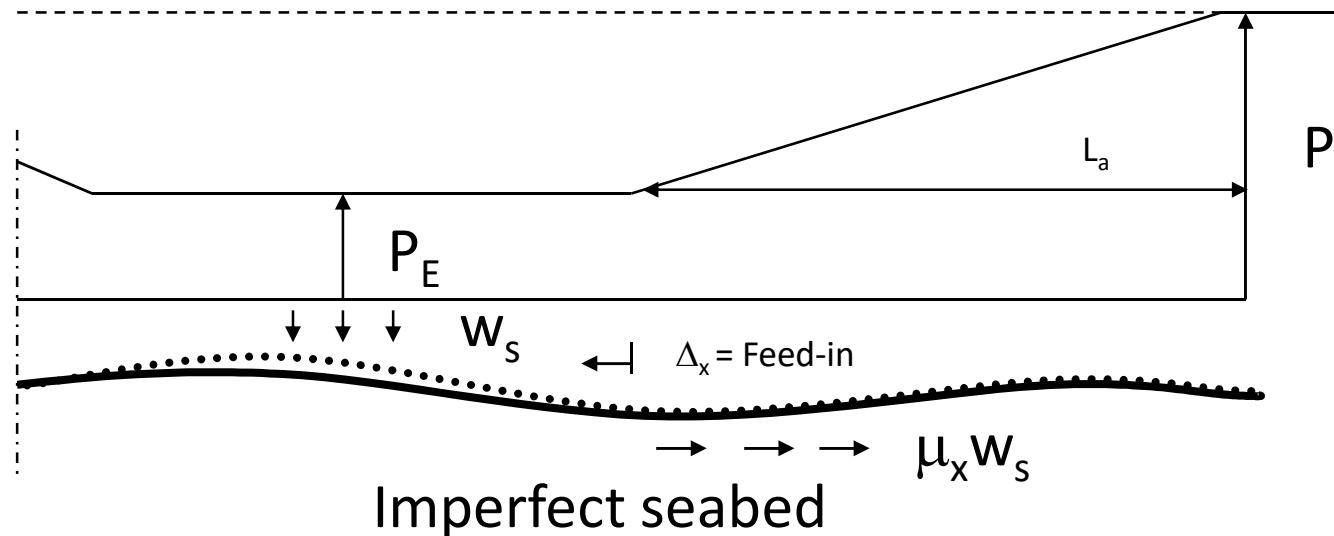


- Buried pipelines – buckling in vertical plane often referred to as "upheaval buckling"



Pipeline global buckling – exposed pipelines (no burial)

- Pipelines resting on seabed and exposed to high temperatures and pressures may buckle as a bar (Euler Buckling)
- Due to seabed irregularities the buckling may be localized at the point of max imperfections:
 - The pipe first undergoes uplift at crown of imperfection
 - Having lost contact at the crown of imperfection the pipe will then buckle laterally
- To avoid excessive strains it may be necessary to perform rock installation to increase axial friction and reduce feed-in



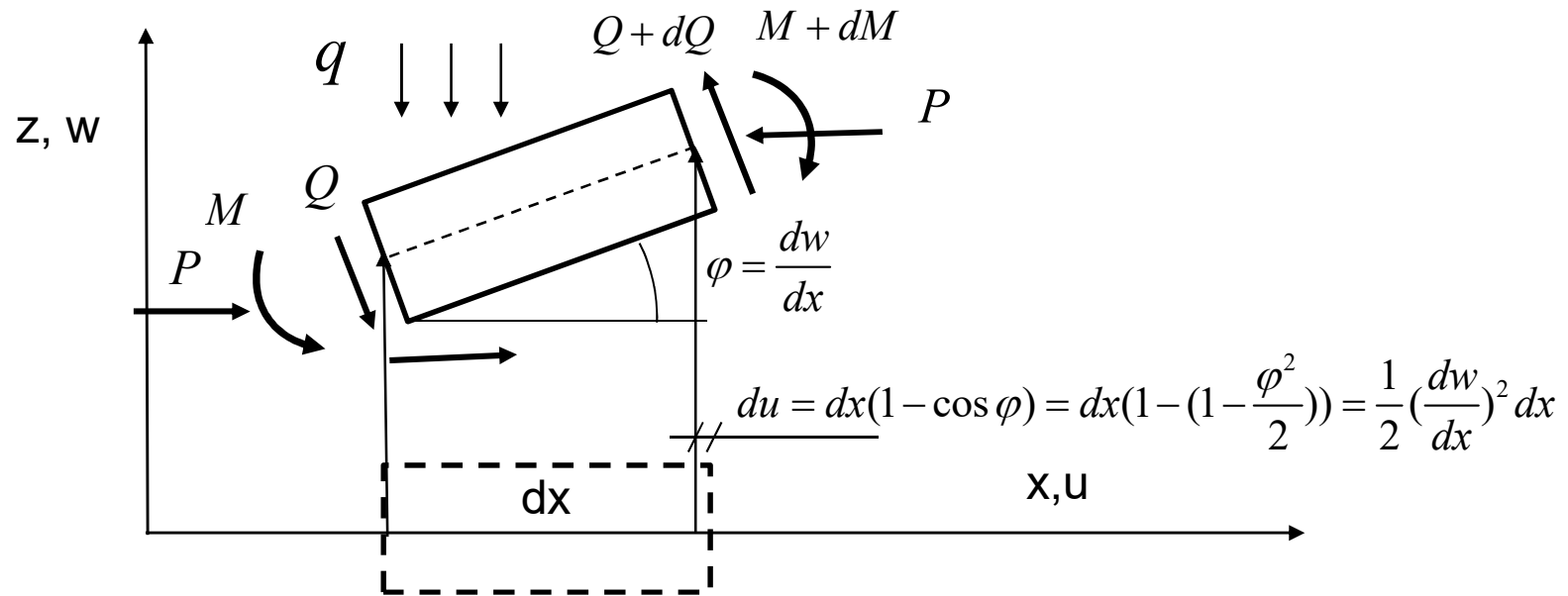
Pipeline global buckling- by FEM

Ceetron 3D Plugin

[roughsnaking.vtf]

Pipeline global buckling – analytical calculation

Differential equation of an Euler Bernoulli beam
exposed to transverse load and an axial
compressive force:



Pipeline global buckling – analytical calculation

Differential equation of an Euler Bernoulli beam exposed to transverse load and an axial compressive force based on equilibrium in moment and in z-direction:

$$\frac{d^4 w}{dx^4} + k^2 \frac{d^2 w}{dx^2} = \frac{q}{EI} \quad (M = -EI \frac{d^2 w}{dx^2} \quad k^2 = \frac{P}{EI})$$

$$Q = P \frac{dw}{dx} - \frac{dM}{dx} = P \frac{dw}{dx} + EI \frac{d^3 w}{dx^3}$$

$$w = w_h + w_p = C_1 + C_2 x + C_3 \cos kx + C_4 \sin kx + \frac{qx^2}{2P}$$

$$\Delta_x = \frac{1}{2} \int_0^l \left(\frac{dw}{dx} \right)^2 dx$$

Pipeline global buckling – analytical calculation

Pipeline resting on uneven seabed – uplift phase

Symmetry line

Boundary conditions:

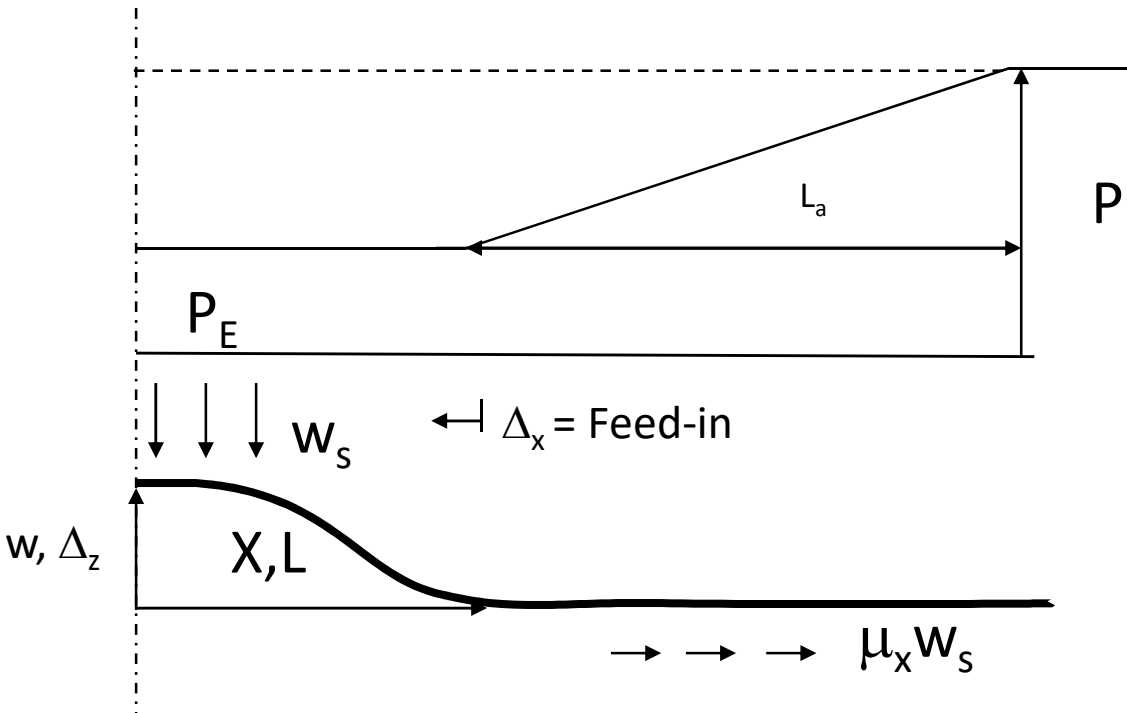
$$1: \frac{dw}{dx} \Big|_{x=0} = 0$$

$$2: \frac{dw}{dx} \Big|_{x=l} = 0$$

$$3:w|_{x=l} = 0$$

$$4: \frac{d^2 w}{dx^2} \Big|_{x=l} = 0$$

$$5:Q|_{x=0} = 0$$



Pipeline global buckling – analytical calculation

Pipeline resting on uneven seabed – uplift phase:

Results:

$$1: \Delta_z = 15.7 \frac{w_s EI}{P_E^2} = 0.0385 \frac{w_s L^4}{EI}$$

$$2: P_E = 20.25 \frac{EI}{l^2} = 3.98 \sqrt{\frac{w_s EI}{\Delta_z}}$$

$$3: M_y \Big|_{x=0} = M_{\max} = 5.6 \frac{w_s EI}{P_E}$$

$$4: Q_z \Big|_{x=l} = ql$$

$$5: \Delta_x = 37.4 \frac{w_s^2 EI^{\frac{3}{2}}}{P_E^{\frac{7}{2}}}$$

Pipeline global buckling – analytical calculation

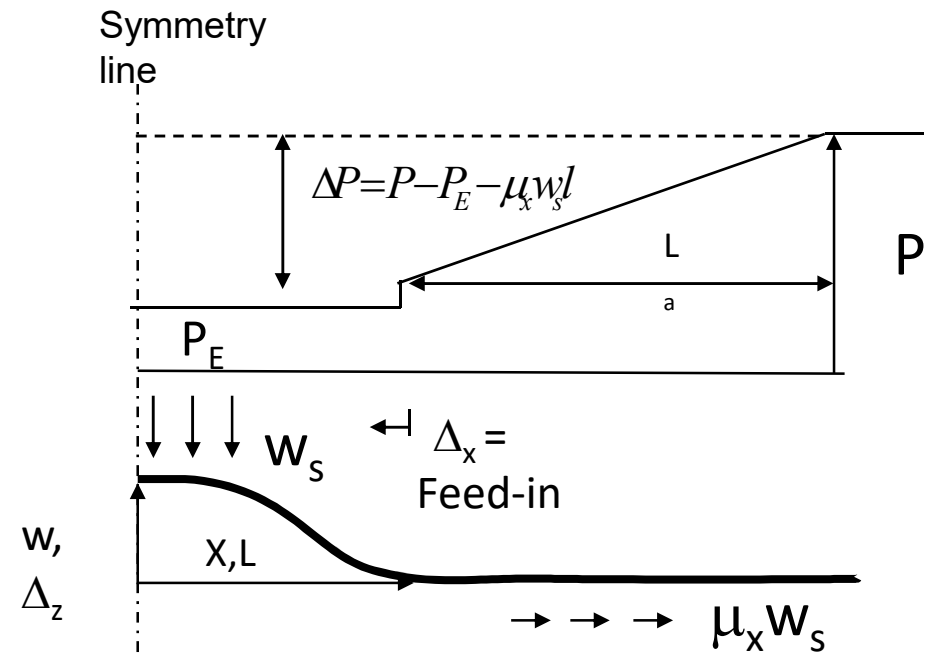
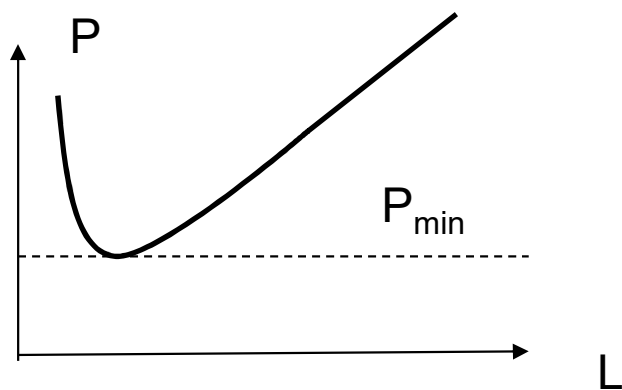
Pipeline resting on uneven seabed – uplift phase

Results:

$$\Delta_x = \frac{\Delta P^2}{2EA\mu_x w_s} = 37.4 \frac{w_s^2 EI^{\frac{3}{2}}}{P_E^{\frac{7}{2}}} = 1.022 \cdot 10^{-3} \frac{w_s^2 l^7}{EI^2} \Rightarrow$$

$$P(l) = 20.25 \frac{EI}{l^2} + \mu_x w_s l + 4.52 \cdot 10^{-2} \frac{w_s}{EI} \sqrt{EA\mu_x w_s l^7}$$

$$P_{\max} \cong \Delta p \pi R^2 (1 - 2\nu) + 2\pi R t E \alpha \Delta T = \Delta p \pi R^2 (1 - 2\nu) + EA \alpha \Delta T$$



If $P_{\min} > P_{\max} \Rightarrow$ No buckling!!

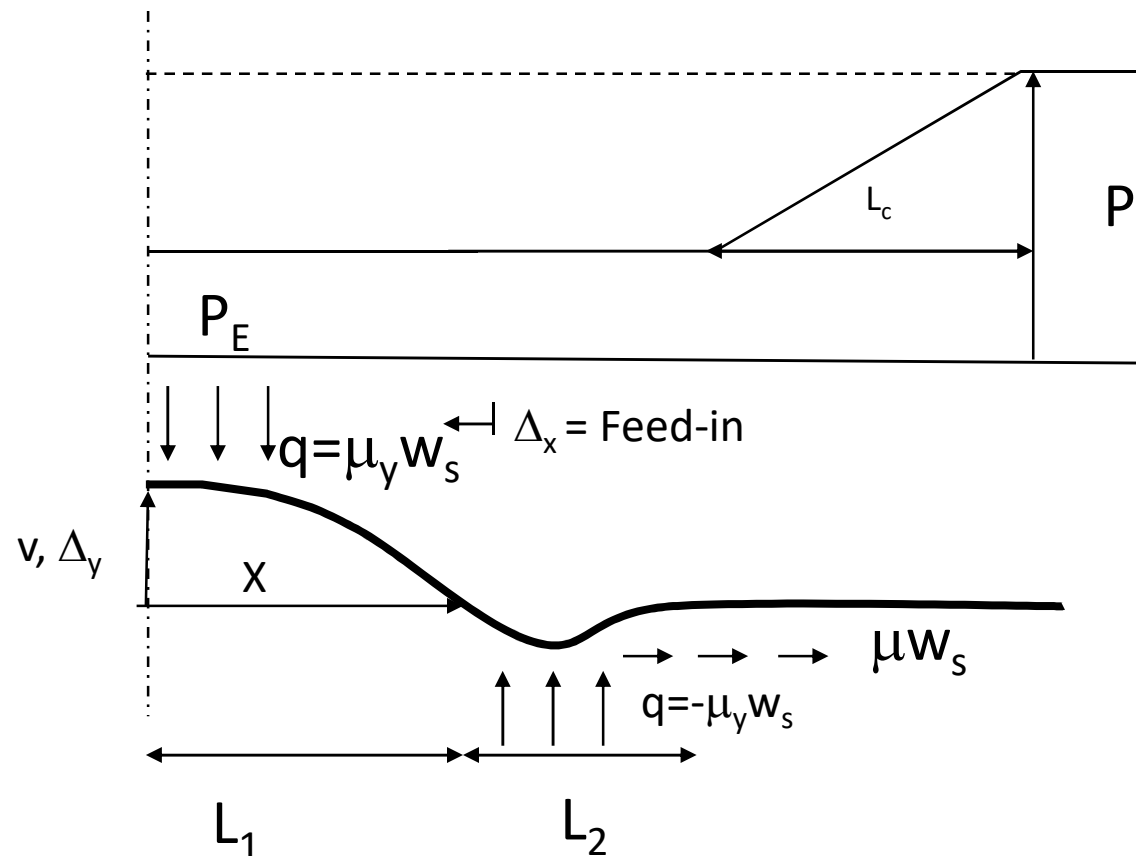
Pipeline global buckling – analytical calculation

Pipeline resting on uneven seabed – Horizontal buckling

Boundary conditions:

Symmetry line

- 1: $\left. \frac{dv_1}{dx} \right|_{x=0} = 0$
- 2: $v_1|_{x=l_1} = 0$
- 3: $\left. \frac{d^2v_1}{dx^2} \right|_{x=l_1} = \left. \frac{d^2v_2}{dx^2} \right|_{x=0}$
- 4: $Q_{1y}|_{x=0} = 0$
- 5: $v_2|_{x=0} = 0$
- 6: $v_2|_{x=l} = 0$
- 7: $\left. \frac{dv_2}{dx} \right|_{x=l_2} = 0$
- 8: $\left. \frac{d^2v_2}{dx^2} \right|_{x=l_2} = 0$
- 9: $Q_{1y}|_{x=l_1} = Q_{2y}|_{x=0}$



Pipeline global buckling – analytical calculation

Pipeline resting on uneven seabed – horizontal buckling:

Results:

$$1: \Delta_{1y} = 11.94 \frac{\mu_y w_s EI}{P_E^2}$$

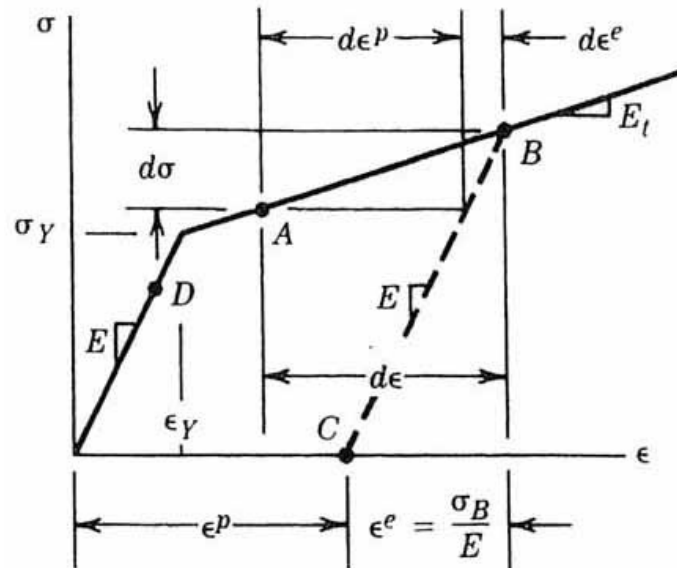
$$2: P_E = 3.35 \sqrt{\frac{\mu_y w_s EI}{\Delta_{1y}}}$$

$$3: M_{1y} \Big|_{x=0} = M_{\max} = 4.89 \frac{\mu_y w_s EI}{P_E}$$

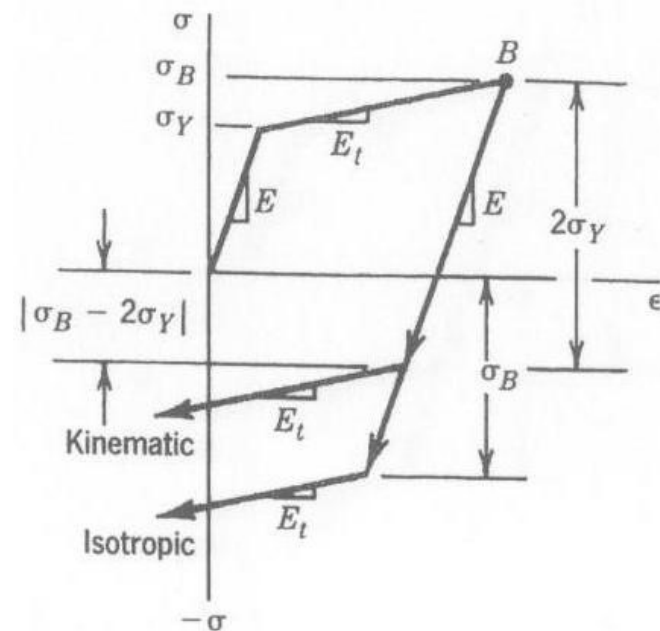
$$4: \Delta_x = 32.54 \frac{\mu_y^2 w_s^2 EI^{\frac{3}{2}}}{P_E^{\frac{7}{2}}}$$

Pipeline global buckling - Finite element analysis

- Large displacement kinematics (non-linear geometry)
- Pipe elements with elastoplastic material properties including the multiaxial stress effects (non-linear material)
- Seabed contact elements to describe the pipe/seabed interaction



a) Stress-strain plot in uniaxial stress, idealized as two straight lines, where σ_Y is the stress at first onset of yielding.



b) Kinematic and isotropic hardening rules.

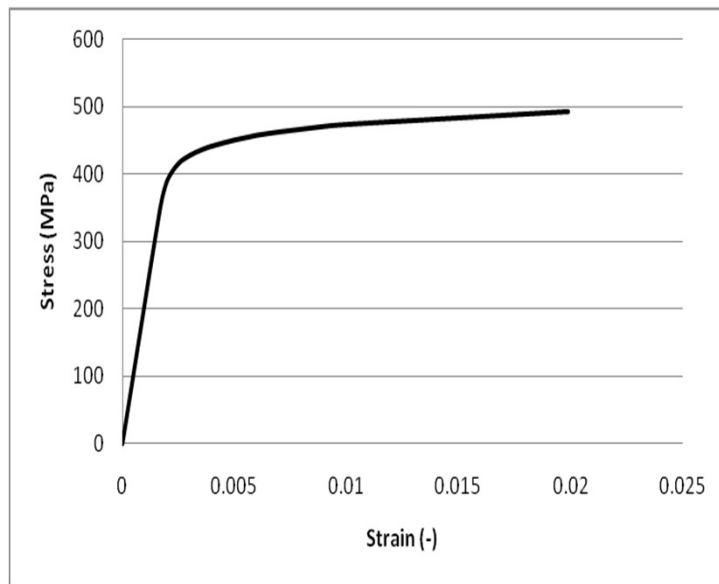
Figure 12.17 One-dimensional stress-strain relationships.

Elastic-plastic material behaviour in uniaxial stress :

- 1) **The yield criterion** : $|\sigma| = \sigma_Y$ (σ_Y tensile yield strength)
- 2) **A hardening rule**,
 - unloading occurs from point B to point C (Fig. a)
 - reverse loading from B: yield stress (?) (Fig. b)
 - isotropic or kinematic
- 3) **Flow rule** $d\sigma = E_t d\epsilon$, ($d\sigma = E d\epsilon$ - before yielding occurs)

Material models

Steel X65



Soil/Roller properties

