

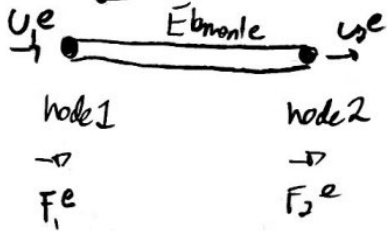
# Lecture 1

①

## ES 128 - Finite Element Analysis

1/28/2019

### Single bar



Governing Eqs: (Linear Elastic Materials)

1) Equilibrium:  $F_1^e + F_2^e = 0$

2) Constitutive Relation:  $\epsilon^e = E \epsilon^e$   
 $\epsilon^e = \frac{\delta^e}{L^e} = \frac{u_2^e - u_1^e}{L^e}$   
 $\sigma^e = \frac{F_2^e}{A^e} = E \epsilon^e$

$F_2^e = \frac{AE}{L^e} (u_2^e - u_1^e) = \frac{AE}{L^e} (u_2^e - u_1^e)$   
 $\hookrightarrow$  do the same for  $F_1^e$

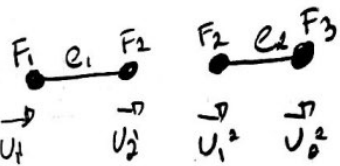
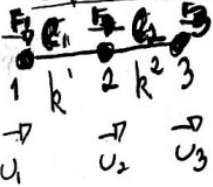
Write down in Matrix form

$$\begin{bmatrix} F_1^e \\ F_2^e \end{bmatrix} = \begin{bmatrix} k^e & -k^e \\ -k^e & k^e \end{bmatrix} \begin{bmatrix} u_1^e \\ u_2^e \end{bmatrix}$$

$\uparrow$   
 $F^e$  =  $k^e$   
 $\uparrow$   
 element force vector      element stiffness      element disp vector

$\hookrightarrow$  stiffness matrix is always symmetric, determinant of this matrix is 0, its singular. This is because there are no B.C. specified

### System of bars



$$\begin{bmatrix} F_1 \\ F_2 \\ F_3 \end{bmatrix} = \begin{bmatrix} k_1 & 0 & 0 \\ 0 & k_1 + k_2 & 0 \\ 0 & 0 & k_2 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix}$$

$\hookrightarrow$  create link  
 $u_1 = u_1$   
 $u_2 = u_2 = u_1$   
 $u_3 = u_3$

## Element Connectivity Chart

Elem	node 1	node 2
1	1	2
2	2	3

$$\begin{Bmatrix} F_1^1 \\ F_2^1 \\ 0 \end{Bmatrix} = k^1 \begin{bmatrix} 1 & -1 & 0 \\ -1 & 1 & 0 \\ 0 & 0 & 0 \end{bmatrix} \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \end{Bmatrix}$$

expanded stiffness matrix

$$\begin{Bmatrix} 0 \\ F_1^2 \\ F_2^2 \end{Bmatrix} = k^2 \begin{bmatrix} 0 & 0 & 0 \\ 0 & 1 & -1 \\ 0 & -1 & 1 \end{bmatrix} \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \end{Bmatrix}$$

↳ global displacement vector

We have 2 equations and now we need to put them together

Free body diagram



$$F_1 = F_1^1$$

$$F_2 = F_2^1, F_2^2$$

$$F_3 = F_3^2$$

$$\underline{F} = \begin{Bmatrix} F_1 \\ F_2 \\ F_3 \end{Bmatrix} = \begin{Bmatrix} F_1^1 \\ F_2^1 + F_2^2 \\ F_3^2 \end{Bmatrix}$$

↳ global force vector

$$\underline{F} = \left( k^1 \begin{bmatrix} 1 & -1 & 0 \\ -1 & 1 & 0 \\ 0 & 0 & 0 \end{bmatrix} + k^2 \begin{bmatrix} 0 & 0 & 0 \\ 0 & 1 & -1 \\ 0 & -1 & 1 \end{bmatrix} \right) \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \end{Bmatrix}$$

↳ global displacement vector -  $\underline{d}$

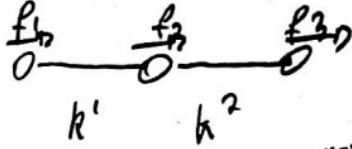
$\underline{K}$  - global stiffness matrix

$$\underline{F} = \underline{K} \underline{d} \Rightarrow \underline{K} = \sum K_{\text{expanded}}$$

$$\underline{\underline{K}} = \sum \underline{\underline{K}}_{\text{expander}} = \begin{pmatrix} q^1 & -q^1 & 0 \\ -q^1 & q^1 + q^2 & -q^2 \\ 0 & -q^2 & q^2 \end{pmatrix}$$

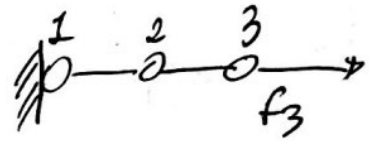
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## Lecture 2



$$\begin{bmatrix} f_1 \\ f_2 \\ f_3 \end{bmatrix} = \begin{bmatrix} \overset{K_{EE}}{k'} & \overset{K_{EF}}{-k'} & 0 \\ -k' & k' + k^2 & -k^2 \\ 0 & -k^2 & k^2 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix}$$

$\underset{K_{FE}}{k'}$        $\underset{K_{FF}}{k^2}$



$$\underline{F} = \underline{K} \underline{d}$$

### 2 Types of nodes

- Essential node: displacement is known  
E-nodes
- Free nodes: displacement is unknown  
F-nodes

$$\underline{d}_E = \begin{bmatrix} u_1 \end{bmatrix} \quad \underline{d}_F = \begin{bmatrix} u_2 \\ u_3 \end{bmatrix}$$

$$\underline{F}_E = \begin{bmatrix} f_1 \end{bmatrix} \quad \underline{F}_F = \begin{bmatrix} f_2 \\ f_3 \end{bmatrix}$$

$$\begin{bmatrix} \underline{F}_E \\ \underline{F}_F \end{bmatrix} = \begin{bmatrix} K_{EE} & K_{EF} \\ K_{FE} & K_{FF} \end{bmatrix} \begin{bmatrix} \underline{d}_E \\ \underline{d}_F \end{bmatrix}$$

We want to determine  $\underline{d}_F$  which is unknown.

$$\underline{F}_F = \underline{K}_{FE} \underline{d}_E + \underline{K}_{FF} \underline{d}_F$$

$$\underline{d}_F = \underline{K}_{FF}^{-1} (\underline{F}_F - \underline{K}_{FE} \underline{d}_E)$$

$$\underline{F}_E = \underline{K}_{EE} \underline{d}_E + \underline{K}_{EF} \underline{d}_F$$

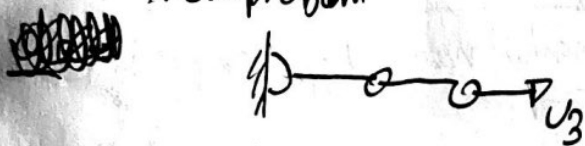
$$\underline{K}_{FF} = \begin{bmatrix} k^1 + k^2 & -k^2 \\ -k^2 & k^2 \end{bmatrix}$$

$$\underline{K}_{FE} = \begin{bmatrix} -k^1 \\ 0 \end{bmatrix} ; \underline{F}_F = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

$$\underline{d}_F = \begin{bmatrix} f_1 \\ f_3 \left( \frac{1}{k^1} + \frac{1}{k^2} \right) \end{bmatrix}$$

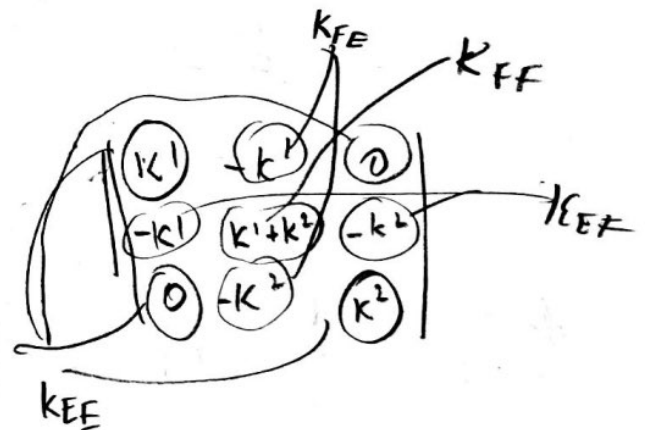
→ you can now use this to solve for reaction forces. This will give that reaction force  $f_1 = -f_3$  from force balance.

New problem



$$\underline{d}_E = \begin{bmatrix} u_1 \\ u_3 \end{bmatrix} \quad \underline{d}_F = \begin{bmatrix} u_2 \end{bmatrix}$$

$$\underline{F}_E = \begin{bmatrix} F_1 \\ F_3 \end{bmatrix} \quad \underline{F}_F = \begin{bmatrix} F_2 \end{bmatrix} = \begin{bmatrix} 0 \end{bmatrix}$$



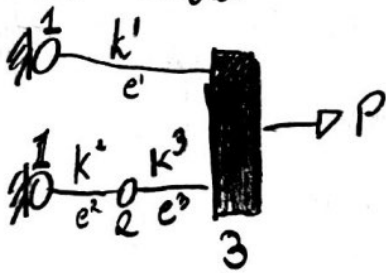
$K_{ij}$  force on  $i$  due to displacement of node  $j$

$$d_F = u_2 = \frac{1}{k_1 + k_2} (0 - [-k_1 - k_2]) \begin{bmatrix} 0 \\ u_3 \end{bmatrix}$$

$$= \frac{k^2 \bar{u}_3}{k_1 + k_2}$$

(3)

New Problem



$$\underline{K}^{(1)} = k_1 \begin{bmatrix} 1 & 3 \\ -1 & 1 \end{bmatrix}$$

$$\underline{K}^{(2)} = k_2 \begin{bmatrix} 1 & 2 \\ -1 & 1 \end{bmatrix}$$

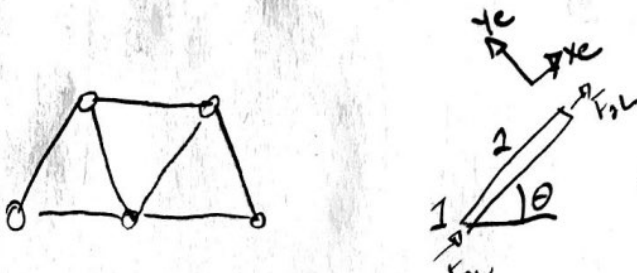
$$\underline{K}^{(3)} = k_3 \begin{bmatrix} 1 & 2 & 3 \\ -1 & 1 & 1 \end{bmatrix}$$

$$\underline{K} = \begin{bmatrix} 1 & 2 & 3 \\ K_{11} & K_{12} & K_{13} \\ K_{21} & K_{22} & K_{23} \\ K_{31} & K_{32} & K_{33} \end{bmatrix} = \begin{bmatrix} k_1 + k_2 & -k_2 & -k_3 \\ -k_2 & k_2 + k_3 & -k_3 \\ -k_3 & -k_3 & k_1 + k_3 \end{bmatrix}$$

$$\underline{d}_E = \begin{bmatrix} 0 \\ 0 \end{bmatrix} \quad \underline{d}_F = \begin{bmatrix} u_2 \\ u_3 \end{bmatrix}$$

$$\underline{F}_E = \begin{bmatrix} f_1 \\ f_2 \end{bmatrix} \quad \underline{F}_F = \begin{bmatrix} 0 \\ P \end{bmatrix}$$

$$\underline{d}_F = \underline{K}_{FF}^{-1} (\underline{F}_F - \underline{K}_{FE} \underline{d}_E) = \frac{1}{k_1 k_2 + k_2 k_3 + k_1 k_3} \begin{bmatrix} k_3 P \\ k_2 + k_3 P \end{bmatrix}$$



$$\begin{bmatrix} F_{1L}^e \\ F_{2L}^e \\ F_{3L}^e \end{bmatrix} = k_2 \begin{bmatrix} 1 & 1 \\ -1 & 1 \end{bmatrix} \begin{bmatrix} u_{1L}^e \\ u_{2L}^e \end{bmatrix}$$

$$\underline{F}_L^e = \underline{K}_L^e \underline{d}_L^e$$

$$\underline{d}^e = \begin{bmatrix} u_{1x}^e \\ u_{1y}^e \\ u_{2x}^e \\ u_{2y}^e \end{bmatrix} \quad \underline{F}^e = \begin{bmatrix} F_{1x}^e \\ F_{1y}^e \\ F_{2x}^e \\ F_{2y}^e \end{bmatrix}$$

$$u_L^e = u_{1x}^e \cos \theta + u_{1y}^e \sin \theta$$

$$u_{2L}^e = u_{2x}^e \cos \theta + u_{2y}^e \sin \theta$$

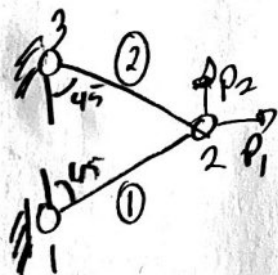
$$\begin{matrix} \downarrow \underline{T} \\ \begin{bmatrix} v_{1x}^e \\ v_{1y}^e \\ v_{2x}^e \\ v_{2y}^e \end{bmatrix} = \begin{bmatrix} \cos\theta & \sin\theta & 0 & 0 \\ 0 & 0 & \cos\theta & \sin\theta \end{bmatrix} \begin{bmatrix} v_{1x}^e \\ v_{1y}^e \\ v_{2x}^e \\ v_{2y}^e \end{bmatrix} \end{matrix}$$

$$\underline{T} \underline{F}^e = \underline{K}_L^e \underline{T} \underline{d}^e$$

$$\underline{F}^e = \underline{T}^{-1} \underline{K}_L^e \underline{T} \underline{d}^e$$

↑  
thermal  
 $\underline{T}^{-1}$

$$\underline{K}^e = k \begin{bmatrix} c^2 & cs & -c^2 & -cs \\ cs & s^2 & -cs & -s^2 \\ -c^2 & -cs & c^2 & cs \\ -cs & -s^2 & cs & s^2 \end{bmatrix}$$



Elem	node 1	node 2	$\theta$
1	1	2	45°
2	2	3	135°

$$\underline{K}^{(1)} = \frac{k}{2} \begin{bmatrix} 1 & 1 & -1 & -1 \\ 1 & 1 & -1 & -1 \\ -1 & -1 & 1 & 1 \\ -1 & -1 & 1 & 1 \end{bmatrix} \begin{matrix} 1x \\ 1y \\ 2x \\ 2y \end{matrix}$$

$$\underline{K}^{(2)} = \frac{k}{2} \begin{bmatrix} 2x & 2y & 3x & 3y \\ 1 & -1 & -1 & 1 \\ -1 & 1 & 1 & -1 \\ -1 & 1 & 1 & -1 \\ 1 & -1 & -1 & 1 \end{bmatrix} \begin{matrix} 2x \\ 2y \\ 3x \\ 3y \end{matrix}$$

(5)

$$\underline{\underline{K}} = \frac{k}{2} \begin{array}{c|cccccc} & 1x & 1y & 2x & 2y & 3x & 3y \\ \hline 1x & 1 & 1 & -1 & -1 & 0 & 0 \\ 1y & 1 & 1 & -1 & -1 & 0 & 0 \\ 2x & -1 & -1 & \boxed{1+1} & \boxed{1-1} & -1 & 1 \\ 2y & -1 & -1 & \boxed{-1} & \boxed{1+1} & 1 & -1 \\ 3x & 0 & 0 & -1 & 1 & 1 & -1 \\ 3y & 0 & 0 & 1 & -1 & -1 & 1 \end{array}$$

$$\underline{\underline{d}}_E = \begin{Bmatrix} u_{1x} \\ u_{1y} \\ u_{2x} \\ u_{2y} \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix} \quad \underline{\underline{d}}_F = \begin{Bmatrix} u_{3x} \\ u_{3y} \end{Bmatrix}$$

$$\underline{\underline{F}}_E = \begin{Bmatrix} F_{1x} \\ F_{1y} \\ F_{2x} \\ F_{2y} \end{Bmatrix} \quad \underline{\underline{F}}_F = \begin{Bmatrix} P_1 \\ P_2 \end{Bmatrix}$$

$$\underline{\underline{d}}_F = \underline{\underline{K}}_{FF}^{-1} (\underline{\underline{F}}_F - \underline{\underline{K}}_{FE} \underline{\underline{d}}_E)$$

$$= \begin{Bmatrix} u_{3x} \\ u_{3y} \end{Bmatrix} = \begin{Bmatrix} P_1/k \\ P_2/k \end{Bmatrix}$$