

1 Measurement and Identification of the Nonlinear Dynamics of a Jointed
2 Structure Using Full-Field Data; Part I - Measurement of Nonlinear
3 Dynamics

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18 Abstract

19 Jointed structures are ubiquitous constituents of engineering systems; however, their dynamic
20 properties (e.g., natural frequencies and damping ratios) are challenging to identify correctly due
21 to the complex, nonlinear nature of interfaces. This research seeks to extend the efficacy of traditional
22 experimental methods for linear system identification (such as impact testing, shaker ring-
23 down testing, random excitation, and force or amplitude-control stepped sine testing) on nonlinear
24 jointed systems, e.g., half Brake-Reuß beam, by augmenting them with full-field data collected by
25 high-speed videography. The full-field response is acquired using high-speed cameras combined
26 with Digital Image Correlation (DIC), which enables studying the spatial-temporal dynamic char-
27 acteristics of the system. As this is a video-based experiment, additional constraints are attached
28 to the beam at the node points to remove the rigid body motion, which ensures that the beam is
29 in the view of the camera during the entire test. The use of a video-based method introduces new
30 sources of experimental error, such as noise from the high-speed camera's fan and electrical noise,
31 and so the measurement accuracy of DIC is validated using accelerometer data. After validating
32 the DIC data, the measurements are recorded for several types of excitation, including hammer
33 testing, shaker ringdown testing, fixed sine testing, and stepped sine testing. Using the DIC data
34 to augment standard nonlinear system identification techniques, modal coupling and the mode
35 shapes' evolution are investigated. The suitability of videography methods for nonlinear system
36 identification of nonlinear beams is explored for the first time in this paper, and recommendations
37 for techniques to facilitate this process are made. This article focuses on developing an accurate

38 data collection methodology as well as recommendations for nonlinear testing with DIC, which
39 paves the way for video-based investigation of nonlinear system identification. In Part-II [1] of this
40 work, the same data set is used for a rigorous assessment of nonlinear system identification with
41 full-field DIC data.

42 *Keywords:* Jointed Structures, Nonlinear System Identification, Nonlinear System Testing,
43 Digital Image Correlation (DIC), Full-field Measurements, Modal Interactions.

44 1. Introduction

45 Jointed structures are one of the most prevalent and vital constituents of numerous small to
46 large scale engineering constructs [2]. Besides their primary task of providing structural stiffness
47 and alignment to the structure, they are also known to be a source of nonlinearity, especially
48 in energy dissipation [3, 4], which influences the dynamic response of the assembly. Accurate
49 characterization and quantification of the nonlinear properties from the experiment are of great
50 importance for understanding the dynamic behavior of a jointed structure. There are many non-
51 linear system identification techniques [5, 6, 7, 8, 9], which have been developed to construct a
52 mathematical representation of the system of interest (usually in terms of amplitude-dependent
53 natural frequencies and damping ratios or a polynomial describing the system's behavior). Due
54 to the complexity of nonlinear dynamic behaviors (e.g., amplitude-dependent properties, jumps,
55 sub/super harmonics, and modal interactions), nonlinear dynamic testing demands more data and
56 effort than linear testing. The nonlinear dynamic test methods for mechanical systems can be cat-
57 egorized as free vibration response testing and forced vibration response testing based on external
58 forces.

59 The commonly used free-response testing includes hammer testing and shaker ringdown testing.
60 Hammer testing is a convenient method to obtain the frequency and damping properties for both
61 linear and nonlinear systems [10]. For a nonlinear system, the system properties depend on the
62 response amplitude, which necessitates that careful attention to the excitation levels be paid.
63 Even though the hammer testing is convenient to conduct without introducing additional mass
64 and stiffness, it excites the system with a broadband force, which results in most of the energy
65 not being transmitted to the mode of interest. Consequently, data extracted from impact hammer
66 testing spans a relatively small range of modal amplitudes, and often the nonlinearities of interest
67 are not fully engaged. Another way to obtain the free response is shaker ringdown testing [11], or
68 resonance decay testing [12], in which the system is excited by a sinusoid (or sinusoids) to steady-
69 state with a high response amplitude, then the force from the shaker is withdrawn. In this manner,
70 the identified instantaneous frequency, damping, and amplitude are from a single-mode response.
71 The benefit of this method, compared to hammer testing, is that it can excite a specific mode to a
72 high response level, and the responses are commonly dominated by this single mode. However, for
73 operating this test, the sudden removal of the excitation usually leads to an impulsive loading on
74 the structure, which may introduce the responses of the other modes. Moreover, for extracting the
75 instantaneous damping for mechanical joints, the measured damping will be considerably affected
76 if the shaker is attached to the structure due to an additional damping effect from the shaker [13].

77 The forced response testing includes stepped sine testing, phase resonance testing, and broad-
78 band excitation testing. At a constant output response amplitude level, mildly nonlinear systems
79 are approximated as linear systems, and the obtained frequency response function (FRF) from

80 stepped sine testing is used to calculate the frequency and damping ratio, namely a single point in
81 the backbone curve and damping-amplitude curve, respectively. Thus, amplitude-control stepped
82 sine testing [14] (or force-control stepped sine testing [15]) is a relatively reliable method to charac-
83 terize the nonlinear behavior based on FRFs. However, one disadvantage of this method is its low
84 efficiency since one sweep only obtains one point in the backbone curve and damping-amplitude
85 curve. Phase resonance testing (or force appropriation testing) [16], which tunes multiple shakers
86 to excite a single-mode according to the phase lag quadrature criterion, has been widely used
87 for large airplanes [17]. Based on the solid framework offered by the theory of Nonlinear Nor-
88 mal Modes (NNMs) [18], the phase resonance testing is extended to nonlinear systems [12] using
89 multi-harmonic excitation to isolate an individual NNM. However, this method may consume a
90 considerable amount of time in configuring the excitation forces. Control-based methods, such as
91 the control-based continuation scheme [19] and phase-locked loop [20, 21], are potentially more
92 robust to changes of stability and bifurcations. Compared to phase resonance testing, the phase-
93 locked loop method is shown to be highly efficient in adjusting the excitations. Additionally,
94 through proper feedback, the input force tends to be purely harmonic excitation by suppressing
95 the super harmonics caused by the interaction between the specimen and the excitation system.
96 However, since the state feedback is introduced, the effect of the control on the characteristics of
97 the original system is difficult to evaluate. Nonlinear subspace identification methods [22, 23, 24],
98 which are commonly implemented with broadband excitation testing, aim to construct a nonlin-
99 ear state-space model to reproduce the dynamic responses of the test system. The advantages of
100 this method are its ability to deal with multiple inputs and outputs data and to describe non-
101 linear behaviors without linearization. The deficiencies include its inapplicability to hysteretic
102 nonlinearities and determination of the excitation levels of broadband excitation [25].

103 Accelerometers are the typical response acquisition approach used in the aforementioned test
104 methods. Thus, most research extracts nonlinear behaviors through a limited number of distinct
105 points on the structure. As such, these methods often do not capture the influence of the non-
106 linearity on the deflection shapes of the structure and, consequently, the information provided to
107 existing nonlinear identification techniques is limited. The spatio-temporal distribution of the non-
108 linear properties has seldom been experimentally investigated. Recent advances in image-based
109 measurement techniques, together with improvements in high-speed cameras, shed light on the
110 appropriateness and accuracy of videography-based methods for measuring the response (both of
111 discrete points and of the full-field) of a linear structure compared to more traditional approaches
112 such as accelerometers and lasers [26, 27]. Significant advantages of videography include its low
113 cost (at least compared to scanning Laser Doppler Vibrometers (LDVs)), non-contact, ease of op-
114 eration, and multi-point vibration measurement from a pair of cameras [28]. The state-of-the-art
115 vision-based techniques can be categorized as the target-less method [29, 30], the point tracking
116 (PT) [31], and digital image correlation (DIC) [32, 33, 34], which are widely reported in review
117 papers [35, 36, 37, 38]. The target-less method, which is based on computer vision extraction, does
118 not require any surface preparation work, and is commonly used in structural health monitoring
119 of civil engineering structures [39]. Both the PT method and DIC require surface preparation and,
120 therefore, provide higher quality measurements [35]. In particular, DIC is a class of non-contact
121 methods that acquires images of an object and performs image analysis to extract the full-field
122 shape, deformation, and motion of the object [40, 41]. Compared with the PT method, which
123 tracks the motion of a single or discrete set of points, DIC can extract full-field responses [42], and
124 is advantageous when high spatial density is required [43]. In the context of structural dynamics,

125 DIC has already been used to study the slip and separation of jointed interfaces [44, 45], and the
126 full-field motions of small-scale and full-size engineering structures [41, 46].

127 The focus of this article is to augment the current nonlinear dynamic test methods with full-
128 field measurement to improve the identification of the nonlinear properties. The primary objective
129 of this paper will be three investigations: 1) how to augment full-field DIC data in nonlinear
130 system test techniques, with a particular focus on the spatial as well as temporal distributions of
131 the dynamic properties of the system, 2) how to select a test technique for a specific nonlinear
132 structure (e.g., a jointed structure) by considering its advantages and deficiencies, and 3) how can
133 the DIC data be used in identifying the modal interaction in jointed structures. Details about
134 the experimental setup, the test specimen, and the data processing procedure are presented in
135 Section 2. As this is a videography-based measurement on a beam with free boundary conditions,
136 constraints are attached as the node point to ensure that the beam stays in the field-of-view (FOV)
137 of the camera for the vibration experiment. The effect of such constraints on the free-response
138 testing and comparison of the results from different test methods using accelerometers are shown
139 in Section 3. In Section 4, the experimental studies using video-based data are conducted to track
140 the evolution of mode shapes with response levels. Additionally, modal interaction behaviors of
141 the test specimen are studied. Finally, the conclusions are given in Section 5.

142 2. Experimental Setup

143 Four different types of tests are conducted during the series of experiments, which are listed in
144 Fig. 1. Both free and forced vibration experiments are used to assess which testing strategies can
145 benefit the most from DIC data. In the free vibration experiments, shaker ringdown tests excite
146 only one particular mode with significant vibration amplitude; by contrast, impact hammer tests
147 excite multiple modes. In the forced vibration experiments, FRFs are obtained by force-control and
148 amplitude-control stepped sine testing. The acceleration and displacement response is recorded
149 for all experiments using a set of accelerometers and high-speed digital cameras.

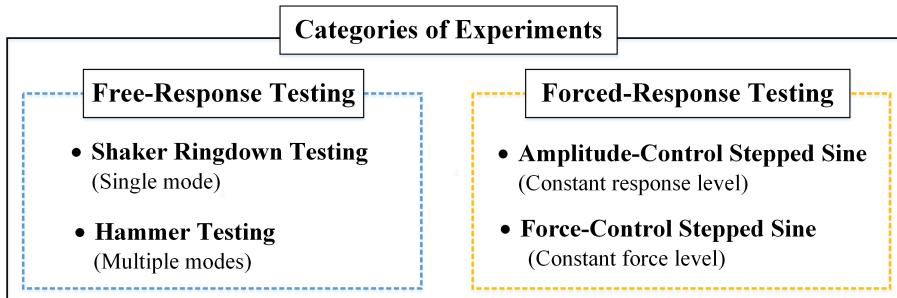


Figure 1: Overview of the different types of experiments.

150 2.1. Specifications of the Test Specimen

151 The half Brake-Reuß beam (HBRB) shown in Fig. 2 (a) is a modification of the Brake-Reuß
152 Beam (BRB) [47], in which the width of the BRB is reduced by half in order to decrease the
153 stiffness in the lateral bending direction (i.e., bending in the Y direction; this also reduces the
154 natural frequencies of interest) so that the vibration amplitude can be increased. This modification

in geometry is chosen to improve the functionality of the video-based data acquisition, which measures displacement and is limited by the frame rate and resolution of the camera. The HBRB contains a three-bolt lap joint and is manufactured using wire electrical discharge machining, with the interfaces ground to a $10\ \mu\text{m}$ finish. The geometric properties of this beam are given in Fig. 2 (b) and (c). Aspects including surface curvature, surface roughness, residual stress, prestress, and wear all contribute to the uncertainty of the dynamic characteristics of the jointed structures, which can be found in [2, 47]. To ensure the consistency and repeatability of the experiment, the HBRB is assembled using the process outlined in [48], which is also briefly mentioned as follows. A constant gap in the axial (X) direction is maintained using a 0.35 mm thick shim along the edges of the beam halves to ensure accurate alignment of the interface. The beam halves are bolted together with three stainless steel 5/16"-24 bolts, where the torque for each bolt is the same. The bolts are first tightened to 70% of the desired torque level using a torque wrench in the order of middle, right, and left, and then to the full level in the same order. The boundary conditions of the beam are approximated as free-free, which is achieved by hanging the beam using fishing lines at the two ends of the beam (0.5" from both ends) as shown in Fig. 4 (b). Since the assembly process (specifically, disassembling and reassembling) may affect the dynamic properties of the system, the nonlinear characteristics of the system are measured (via impact hammer tests) every time that the beam is reassembled in order to compare these characteristics across all experiments and to ensure consistency before undertaking a longer set of experiments. Additionally, in order to assess the condition of the beam after an experimental campaign, impact hammer tests are also performed before disassembly in order to quantify any changes in the system due to wear or degradation [49].

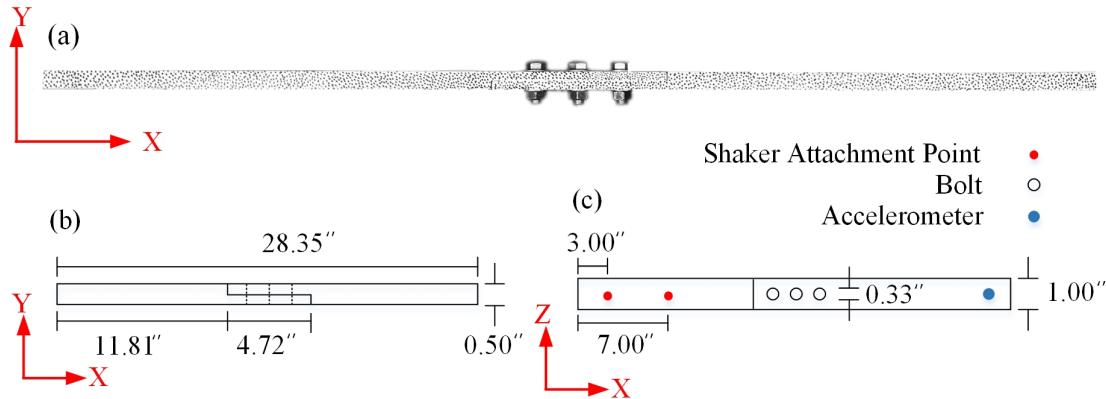


Figure 2: The geometry of the HBRB, shown are (a) the photo of the HBRB, (b) the top view of the HBRB, and (c) the front view of the HBRB.

2.1.1. Linear Modal Properties

A series of linear modal tests are conducted for the specimen with three different torque levels of 10.2, 19.6, and 23.7 N·m. In each of these cases, only one accelerometer is used, which is attached at 1" from the right end of the beam. A roving hammer test is performed with 29 excitation points uniformly spaced 1" apart in the axial direction along the midline of the beam, with a low impact level in the range from 70 to 90 N. The identified natural frequencies and damping ratios obtained directly from the FRFs of the data for the three different torque levels are shown in Table

184 1. Consistent with measurements of other jointed systems [2, 47], the natural frequencies increase
 185 with the torque level, and the damping ratios decrease with torque level. Figure 3 shows the first
 186 three mode shapes for the 19.6 N·m bolt torque; no appreciable difference in the first three mode
 187 shapes are observed for the three different torque levels. Even though the beam has a geometric
 188 discontinuity at the joint location, it is difficult to discern this from the measured data. This is
 189 contrasted with the video-based data in Section 4.3.1.

Table 1: The natural frequencies f , in Hz, and damping ratios ξ , in %, of the HBRB at a low excitation level.

Torque level (N·m)	Mode 1		Mode 2		Mode 3	
	f_1 (Hz)	ξ_1 (%)	f_2 (Hz)	ξ_2 (%)	f_3 (Hz)	ξ_3 (%)
10.2	80.31	0.20	290.52	0.07	580.91	0.10
19.6	83.82	0.11	299.99	0.05	589.48	0.05
23.7	84.28	0.07	300.25	0.05	590.15	0.05

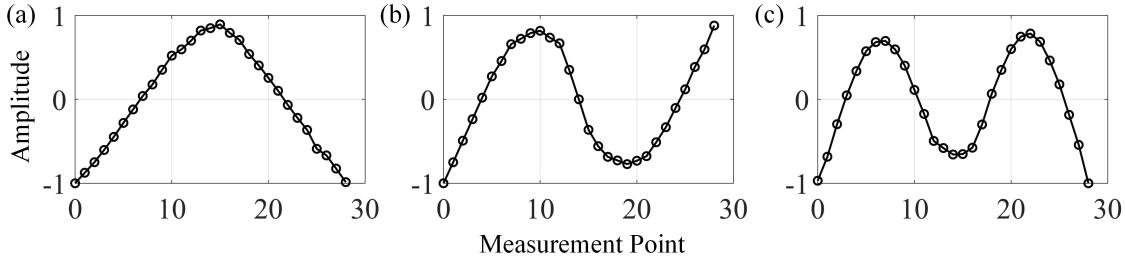


Figure 3: The first three mode shapes of the HBRB from roving hammer testing at a low impact level, shown are (a) mode 1, (b) mode 2, and (c) mode 3.

190 2.2. Video Acquisition Setup

191 Two high-speed cameras (Photron Mini UX100) are used to acquire the video data of the
 192 jointed beam during dynamic excitation. The experimental setup is shown in Fig. 4. The beam is
 193 suspended horizontally from a fixed bar using fishing lines connected to bungee cords with a total
 194 length of 1.65 m to simulate free-free boundary conditions. The swing frequency is approximately
 195 2.5 Hz, which is significantly lower than the first natural frequency (80Hz) of the beam. For
 196 the hammer tests and shaker ringdown tests, in order to improve the performance of the video-
 197 based data acquisition, additional constraints (bungees) are added to the beam. The effect of the
 198 constraints on the system is studied in detail in Section 3.1. Since the beam vibrates in the X-Y
 199 plane in Fig. 4, the cameras are positioned and aligned perpendicularly above the plane to capture
 200 the responses of the beam. The cameras are positioned such that the pixel to actual physical
 201 displacement conversion is consistent for all of the points on the beam (i.e., the plane of the beam
 202 and plane of the camera are parallel). A level is used to ensure that the beam and camera are
 203 parallel, and the distortion from the camera and lens is checked through 2D calibration via a Matlab
 204 toolbox called Camera Calibrator [50, 51]. Two cameras are placed 1 m above the top surface of
 205 the beam with 35 mm focal length lenses such that each camera records slightly more than one-half
 206 of the beam. For proper illumination, two 1000-watt halogen bulbs are placed symmetrically at the
 207 two sides of the beam. The sampling frequency is kept as 8000 frames per second (FPS) or higher

208 with the camera shutter speed of $16 \mu\text{s}$ to capture the high-frequency vibration. The cameras
 209 are synchronized by using a built-in triggering mechanism that starts recording for both cameras
 210 simultaneously. Using the videos from both of the cameras, the vibration profile of the whole beam
 211 can be obtained by stitching the two images obtained from the two cameras. The displacements
 212 recorded in the video are in pixels, which are converted to physical units by a conversion factor
 213 that is calculated by taking the length of the beam as the reference dimension and dividing the
 214 length by the number of pixels that the beam spans in the image. The measurement uncertainty
 215 from the DIC code and environment is evaluated through static analysis in Section 4.1, following
 216 which the noise level for the current test setup and displacement extraction code can be quantified.
 217 Responses at or below the noise threshold are discarded from the analysis.

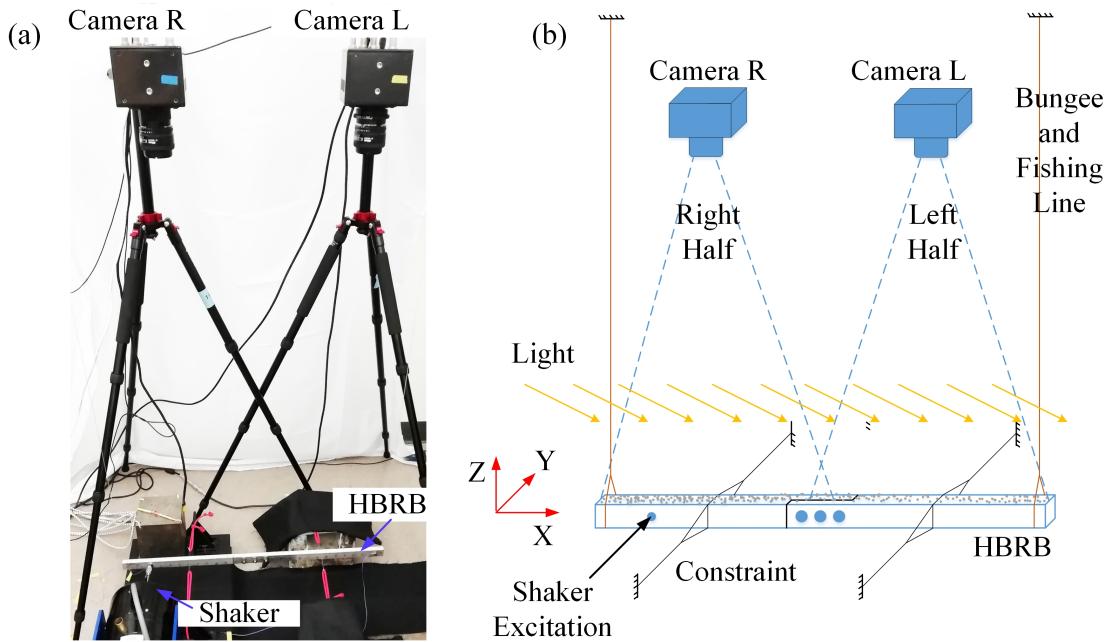


Figure 4: The test setup with the high-speed cameras. Shown are (a) the photo of the test setup, (b) the schematic diagram of the test setup.

218 As an example, one image pair from the left and right cameras are shown in Fig. 5. The images
 219 from the high-speed cameras are grayscale images with a depth of 12 bit. For better use of the
 220 limited memory of the high-speed cameras (16 GB) in capturing the full beam, the FOV of the
 221 cameras is fixed as 1280×312 . From Fig. 5, the two images have an overlap area of the middle
 222 bolt to ensure that the spatial motion profile of the whole beam can be reconstructed from these
 223 two independent images.

224 2.3. Specifications for Images Prior to Post Processing

225 To use DIC to measure the displacement of the beam, a speckle pattern is needed [52]. The top
 226 surface of the beam is painted with white matte paint as a primer. Black dots are marked randomly
 227 with a sharpie on the top surface of the whole beam, and each dark speckle spans three to five
 228 pixels [52]. Such speckles are drawn in a random manner in terms of spatial location as well as size
 229 to make all of the small image patches unique, which helps the DIC method to track properly. The
 230 recorded images are processed by the DIC software ‘Digital Image Correlation Engine’ (DICe) [53]

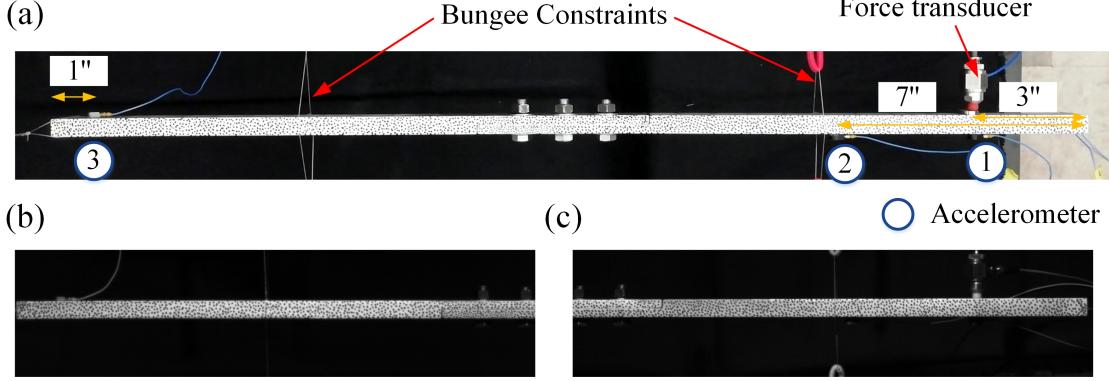


Figure 5: The images of the test beam, shown are (a) the top view of the HBRB, (b) the image from the left camera, and (c) the image from the right camera. The blue circles with numbers inside in (a) refer to the accelerometers.

231 to extract the displacement of the beam. The displacements are measured by creating overlapping
 232 square windows (or subsets) on the area of interest and tracking the motions of the subsets via
 233 image correlation. The displacement responses extracted by image correlation are the average
 234 responses of the subsets, which are commonly referred to as the responses at the center points. For
 235 a higher spatial resolution in the measurements, a smaller subset size is desirable; however, smaller
 236 subset sizes will bring about uncertainty during the image correlation and may cause more error
 237 for the extracted responses. In all of the tests, the length of the subset was set as 21 pixels (shown
 238 in Fig. 6; as each subset is square, this results in a total of 441 pixels inside the subset), and the
 239 overlap between the two subsets was set as 11 pixels. In Fig. 6 (a), the subsets in red were selected
 240 for DICe to calculate the displacement along the dashed red line. If the subsets are placed along
 241 the two sides of the interface, then the joint behaviors can be measured. In the previous work
 242 of the authors [45, 44], the FOV is focused on the jointed interface itself to measure the relative
 243 tangential and vertical motions of the interface under the dynamic excitations. Here, however, the
 244 focus is on the global motion of the system, and thus a coarser FOV is used compared to previous
 245 work.

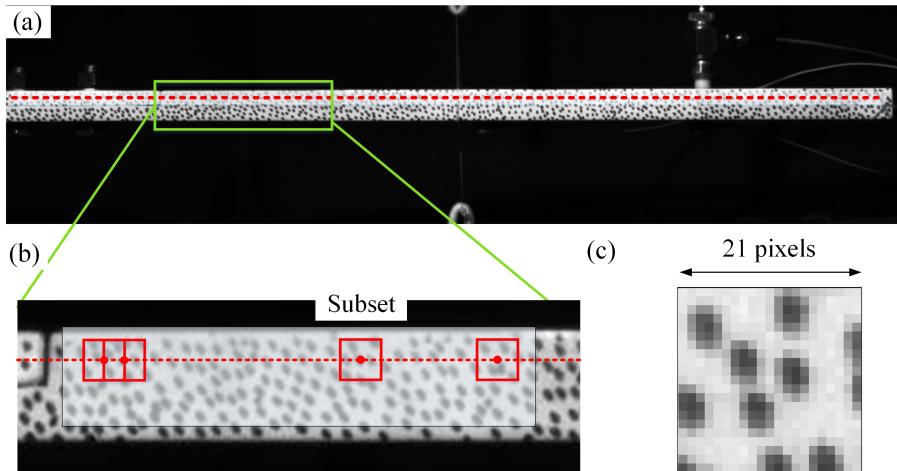


Figure 6: DIC settings for the measurements. Shown are (a) measurement locations on the beam, (b) a zoom-in of the beam, and (c) a subset.

246 2.4. Free Response Testing Experimental Setup

247 For the free-response testing, extra constraints (bungee cords) are added symmetrically at the
248 two sides of the beam to reduce the rigid body motion of the free-free beam. This is necessary
249 for the subsequent video-based measurements; without the additional constraints, the specimen
250 would move in and out of the FOV of the cameras (see the narrow FOV in Fig. 5 (b) and (c)). The
251 selection of the locations of the additional constraints and their effect on the system dynamics is
252 discussed further in Section 3.1.

253 2.5. Shaker Ringdown Testing

254 The experimental setup for shaker ringdown testing is shown in Fig. 5 (a). In addition to the
255 cameras, the acceleration responses and input forces (through a 267 N shaker) are recorded by
256 three accelerometers and a force transducer, respectively. The shaker is controlled by the signal
257 source inside the data acquisition system. In the experiment, the system is excited by a sinusoidal
258 signal that matches the resonant frequency of a target mode for a particular response level. When
259 the steady-state response is reached, the external force is removed from the system to observe the
260 free decay in that corresponding mode. Then, the amplitude-dependent frequency and damping
261 ratio curves can be estimated directly for that mode. In this case, a shaker is used to excite a
262 particular point of the HBRB, and when the beam reaches its steady-state, which can be observed
263 from the acceleration measurements, the stinger completely detaches from the beam, allowing the
264 vibration energy in the beam to decay freely.

265 The challenging part for the shaker ringdown testing is to separate the shaker and the test
266 specimen physically since the shaker adds mass and extra damping to the test structure. For
267 large systems [12], the shaker and system are not decoupled based on the assumption that the
268 shaker does not appreciably contribute to the system's dynamics. However, for the jointed HBRB
269 specimen, the shaker adds a significant amount of damping to the system due to its electromagnetic
270 mechanism and its relatively large size compared to the test specimen. Hence, a new setup, inspired
271 by [54], is devised that completely detaches the stinger from the beam specimen.

272 2.5.1. Shaker Decoupling Mechanism

273 The shaker decoupling mechanism consists of preloading the stinger against the beam instead of
274 rigidly connecting it. With a large enough preload, the stinger does not lose contact with the beam
275 during dynamic excitation. As shown in Fig. 7, the stinger is attached to the force transducer,
276 which in turn is attached to a rubber impact hammer tip. At the point of contact with the beam,
277 there is a tapped hole for forced-response shaker testing; a cap used for impact testing in similar
278 situations is fixed to this hole so that both the shaker ringdown testing and the forced-response
279 shaker testing are able to excite the system at the same location. The value of the DC offset
280 is chosen such that the stinger can retract 1.5 cm, which is sufficient to ensure that the stinger
281 assembly does not contact the beam during the subsequent ringdown. To decouple the stinger and
282 test specimen, the DC offset of -4 V is applied to the shaker control signal. To minimize disruption
283 to the vibration of the specimen, the DC offset is applied at a phase of 360 degrees where the
284 beam has the maximum velocity towards the opposite direction. This phase was determined as
285 experiments deduced that it had a minimal influence compared to other candidate phases. This
286 process is further described below.

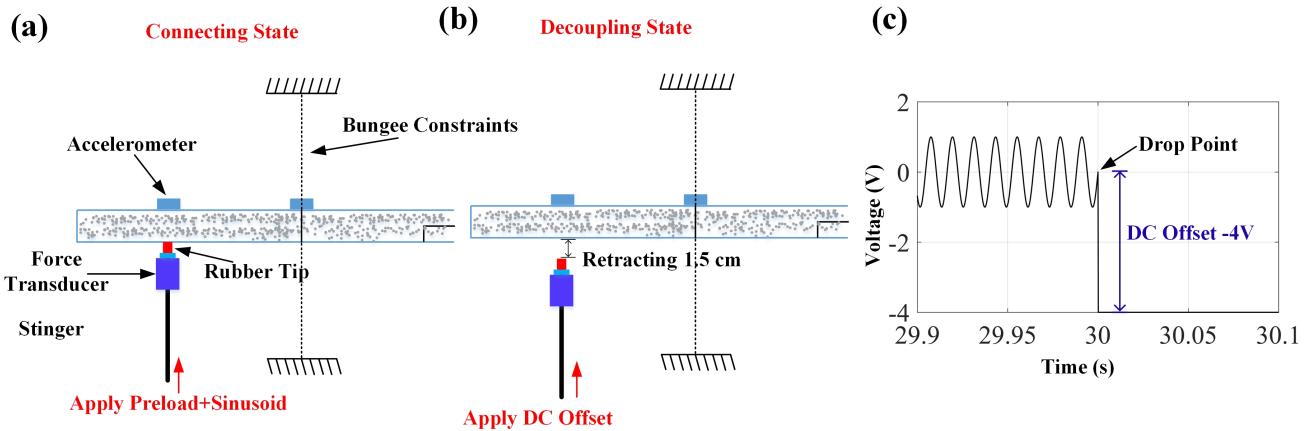


Figure 7: The decoupling mechanism of the stinger and the beam, shown are (a) the state with preload and sinusoidal excitation added, (b) the decoupling state where the stinger retracts, (c) the applied DC offset signal.

287 2.5.2. Shaker Ringdown Methodology

288 The steps for conducting shaker ringdown testing are shown in Fig. 8 and are summarized as:

- 289 • **Step 1: Estimation of natural frequency**

290 The approximate natural frequencies of the system are searched at a particular response
 291 level (e.g., Root Mean Square (RMS) of 5 g) using random broadband excitation. The
 292 modal parameters within the target frequency range are identified through the obtained
 293 FRFs using linear modal analysis principles.

- 294 • **Step 2: Driving point phase difference evaluation to determine the precise natural
 295 frequency**

296 Next, the shaker is driven with a fixed sine signal set at the estimated natural frequency
 297 and the target amplitude. The response of the accelerometer at the same location as the
 298 excitation point is recorded to estimate the phase between the force and excitation. For
 299 estimating the phase difference, the Cross Power Spectral Density (CPSD) (the Fast Fourier
 300 Transform (FFT) of the correlation function) is used, which can show the phase difference
 301 of two sinusoids directly. As shown in Fig. 8 (b), the phase difference between the force
 302 and response is recorded where the absolute value of the CPSD is a maximum. Such phase
 303 difference should be close to -90° if the sinusoidal forcing function frequency is close to the
 304 actual natural frequency.

- 305 • **Step 3: Adjustment of excitation frequency if the driving point phase difference
 306 is out of a preset threshold**

307 Due to the amplitude-dependent properties of the jointed structure, the identified frequency
 308 from random excitation testing is often not the exact natural frequency at the target exci-
 309 tation level. In this case, the phase difference is not close to -90° , and so the excitation
 310 frequency is adjusted by a frequency step (e.g., increments of 0.1 Hz during the test). When
 311 the phase difference is within the threshold, which is set as $\pm 5^\circ$ around -90° , the adjustment
 312 procedure stops. Fig. 8 (b) shows that the phase difference between the force and the accel-
 313 eration signal is -91.23° which is within the threshold limit. After completion of this step,

314 the obtained frequency and target excitation level is used for the input signal generation for
315 the shaker ringdown test.

316 • **Step 4: Detachment of shaker and recording of ringdown data**

317 After finding the excitation frequency at the desired response level, the DC offset signal is
318 generated and applied to the structure. As shown in Fig. 8 (c), the DC offset signal contains
319 two parts; in the first 30 s, there is a sinusoidal signal with frequency obtained from Step
320 3, and the last 30 s consist of the constant DC part, which does not have any motion. As
321 discussed above, the stinger is preloaded against the beam, and for the first 30 seconds, it
322 is used to transmit the shaker excitation to the beam such that the beam reaches a steady-
323 state. During this time, as the beam is constrained at the node points, the force transducer
324 is monitored to ensure that the stinger never loses a significant portion of its preload and is
325 able to maintain contact for the whole 30 seconds. The duration of 30 seconds is chosen to
326 ensure that the specimen achieves the steady-state response corresponding to a particular
327 target mode for that response amplitude level. At the designated time (30 seconds), the DC
328 offset is applied to the shaker's excitation signal, resulting in the stinger being decoupled
329 from the beam and being retracted 1.5 cm such that it never contacts the beam for the
330 remainder of the experiment, which allows the vibration in the structure to decay freely with
331 a single-mode component. The example response of the accelerometer at location 1 (see
332 Fig. 5 (a)) is shown in Fig. 8 (d). Just prior to the separation of the stinger from the beam,
333 the two cameras are triggered to capture the free responses of the whole beam for about 4
334 seconds. In this experiment, the excitation force-time history, the accelerometer data, and
335 the video is recorded. The sampling frequency for the video data is 5000 Hz for mode 1 and
336 8000 Hz for modes 2 and 3, and similarly, the sampling frequency for the accelerometer data
337 is 6400Hz for mode 1 and 12800Hz for mode 2 and 3. From this ringdown response, the
338 instantaneous frequency and damping ratio of the target mode can be calculated.

339 This procedure is followed to obtain the free responses of all of the first three modes. To
340 excite the target mode sufficiently, there are some studies to detect the optimal excitation location
341 using Fisher information [55] and modal force vector [56]. In this paper, the optimal excitation
342 location of the beam for each mode is estimated using the method proposed in [56]. This algorithm
343 determines the optimal location of the shaker to increase the vibration amplitude of a particular
344 mode. According to [56], for exciting modes 1, 2, and 3, the shaker should be attached at 3", 7",
345 and 7" from the right side of the beam, respectively.

346 *2.6. Impact Hammer Testing*

347 Hammer testing is a convenient and practical way to obtain the dynamic properties by analyzing
348 the responses after the hammer impacts the test specimen. The impact is a broadband excitation,
349 whose frequency range depends mainly on the material of the hammer tip. For this experiment,
350 the tip is made of plastic and has an effective excitation frequency range of 0 to 2800 Hz. The
351 same experimental setup used for the shaker ringdown is used for the hammer testing, except that
352 the shaker and stinger assembly is completely removed and never engages the HBRB.

353 The impact location is the same as the shaker attachment points used in the shaker ringdown
354 testing (3", 7", and 7" from the left side for mode 1, 2, and 3 respectively in Fig. 5 (a)), and the
355 excitation levels are 100, 300, and 700 N (though the 700 N case is not used in all experiments in

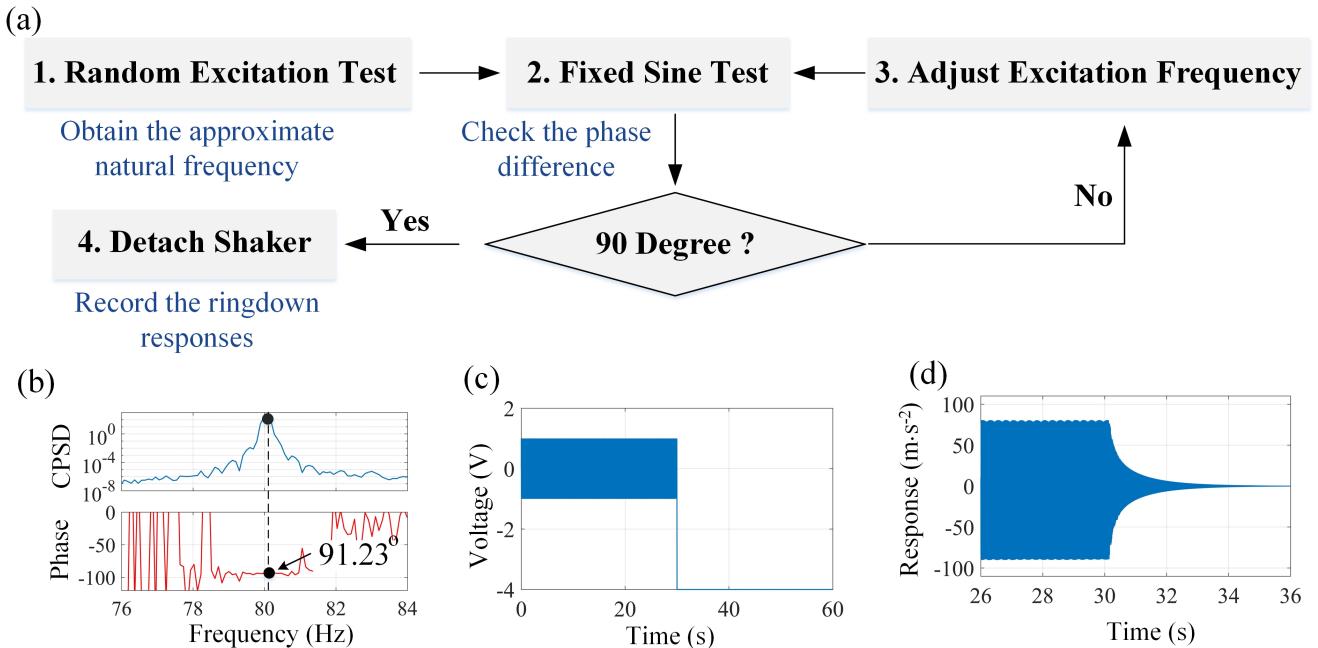


Figure 8: The flow chart of the shaker ringdown testing, shown are (a) the whole procedure for the shaker ringdown testing, (b) the cross-spectrum of the response and input force, (c) the input signal to the excitation system for detaching the shaker and test specimen, and (d) the free response of the system.

order to avoid excessive wear of the system) for mode 1, 2, and 3, respectively. Two repeats are conducted for each excitation level. The cameras capture the response of the entire beam from the initial impact until the beam dissipates the impact energy. Note that the camera data from a low excitation level has a relatively low signal-to-noise ratio (SNR) as low pixel-level displacement is not captured properly in the video-based measurement. Therefore, mainly the excitation level of 700 N is used for the hammer tests when the response is recorded by the cameras. The duration of the recording for the cameras is 4.5 s with a sampling frequency of 8000 Hz.

From each hammer test, the response of a single-mode (Fig. 9 (b)) is extracted from the raw acceleration data (Fig. 9 (a)) by reversing the time signal and then using a 4th-order Butterworth bandpass filter (with a bandwidth of 16 Hz) [57, 58]. The frequency-domain response of the filtered response and raw data are shown in Fig. 9 (c). The Peak Finding and Fitting (PFF) method [58] is then used to extract the instantaneous damping and frequency of the system, as shown in Fig. 9 (d) and (e). Due to friction in the joint interface, the natural frequency of the jointed structures decreases with increasing vibration amplitude (exhibiting the softening behavior expected from this type of jointed structure), while the damping ratio increases with the amplitude[2].

2.7. Forced Response Testing

For nonlinear jointed structures, amplitude- or force-control stepped sine testing is an effective technique to obtain the amplitude-dependent properties [14]. The backbone curves of the system can be directly calculated from the measured FRFs at different excitation amplitudes. A key difference between amplitude-controlled and force-controlled experiments is that amplitude-controlled experiments maintain a constant response spectrum, and force-controlled experiments maintain a constant force spectrum. The choice in amplitude- or force-controlled has ramifications for the

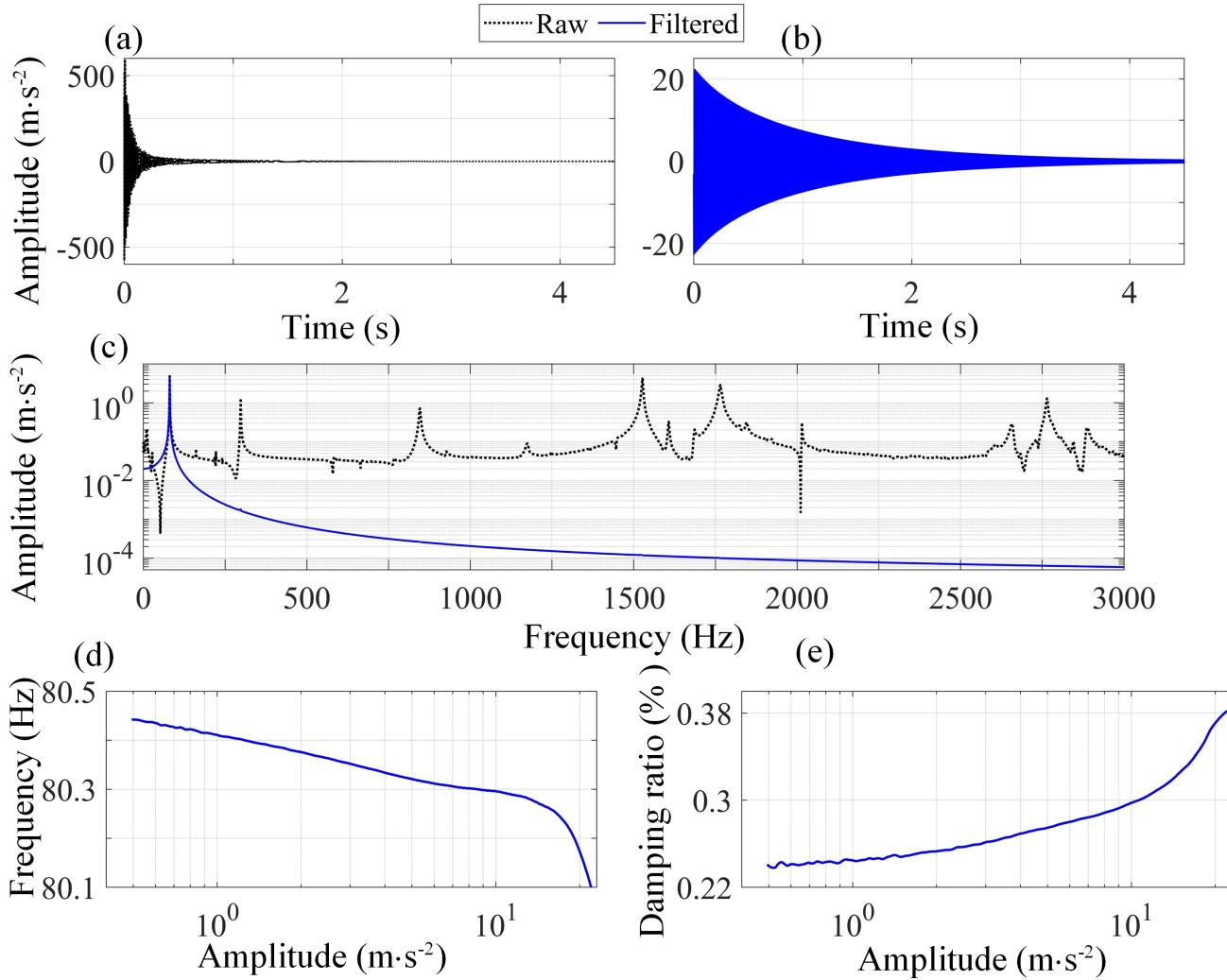


Figure 9: Typical results from hammer impact testing, showing (a) the raw data from the accelerometer, (b) the band pass filtered responses in the time domain, (c) the FFT of both the raw data and the band pass filtered responses, (d) the amplitude-dependent natural frequency, and (e) the amplitude-dependent damping ratio.

extraction of the backbone curves, as is discussed further in Smith et al. [59]. The employed control strategy can be treated as a closed-loop controller, which keeps the response or force level at the given frequency in an acceptable interval by changing the input current for the amplifier. Consequently, the input force may be distorted and contain higher harmonics of the excitation frequency.

The test setup is shown in Fig. 10. The force transducer connects the stinger and the beam through a screw connection. Hence, for these forced vibration tests, no additional constraints are required because the shaker's excitation amplitude is insufficient for the beam to go beyond the FOV of the cameras. Three accelerometers are placed at 1" from the left side, and 3" and 7" from the right free edge to measure the responses. For mode 1, the shaker is attached to the beam at 3" from the right side, which is close to the node point of mode 2 and 3; for mode 2 and 3, the shaker is attached to the beam at 7" from the right side, which is close to the node point of mode 1, to ensure that the energy from the shaker is focussed into the corresponding target mode. During the

391 test, a step of 0.05 Hz is used, and the specimen is excited for 3 s at each step to reach a steady
 392 state.

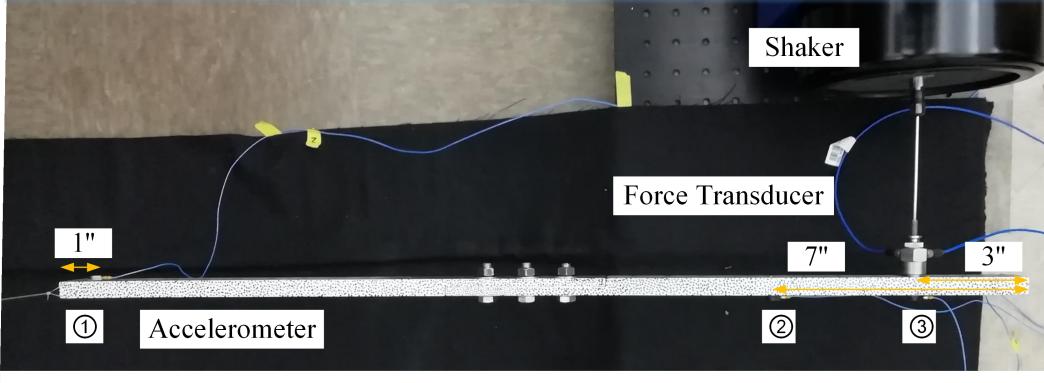


Figure 10: The test setup for the forced response testing.

393 2.7.1. Amplitude Control

394 During the amplitude-controlled stepped-sine testing, the driven point refers to the response
 395 measurement point with the exact same location as the excitation. The detailed settings for
 396 amplitude-controlled stepped-sine testing are shown in Table 2.

Table 2: Settings for amplitude-control stepped sine testing.

Test Mode	Driven Point	Sweep Range (Hz)	Step (Hz)	Controlled Amplitude (g)
1st	Point 3	79.0-81.0	0.05	0.5, 2, 4, 5, 6, 7, 8
2nd	Point 2	294.5-298.5	0.05	2, 4, 6, 8, 10
3rd	Point 2	576.8-582.8	0.05	2, 5, 10

397 The response levels of the driven point and excitation level are shown in Fig. 11 for the
 398 amplitude-control stepped sine test. Ideally, the response amplitude of the driven point should be
 399 a horizontal line with the set level during each stepped sine test. In Fig. 11 (a), (c), and (e), each
 400 dashed line is approximately constant, which means that the response level of the driven point is
 401 well controlled during the stepped sine test. For the excitation force, the lines corresponding to
 402 the force amplitude drop smoothly at the resonant frequency. The FRFs of modes 1, 2, and 3 for
 403 different amplitude responses are shown in Fig. 11 (b), (d), and (e), respectively. These curves
 404 are obtained by dividing the response curves by the force curves for each controlled level. As the
 405 response level increases, the amplitude of the FRF decreases. Since the amplitude of the FRF is
 406 inversely related to the damping ratio, the damping ratio of the structure increases with increasing
 407 response amplitude.

408 2.7.2. Force Control

409 For the force-control stepped sine test, the experimental setup is the same as that for the
 410 amplitude-controlled stepped-sine test. The force-controlled stepped-sine test is conducted from
 411 79 to 81 Hz with a step of 0.05 Hz and force levels of 0.2, 0.5, 1.0, and 1.5 N for mode 1. As

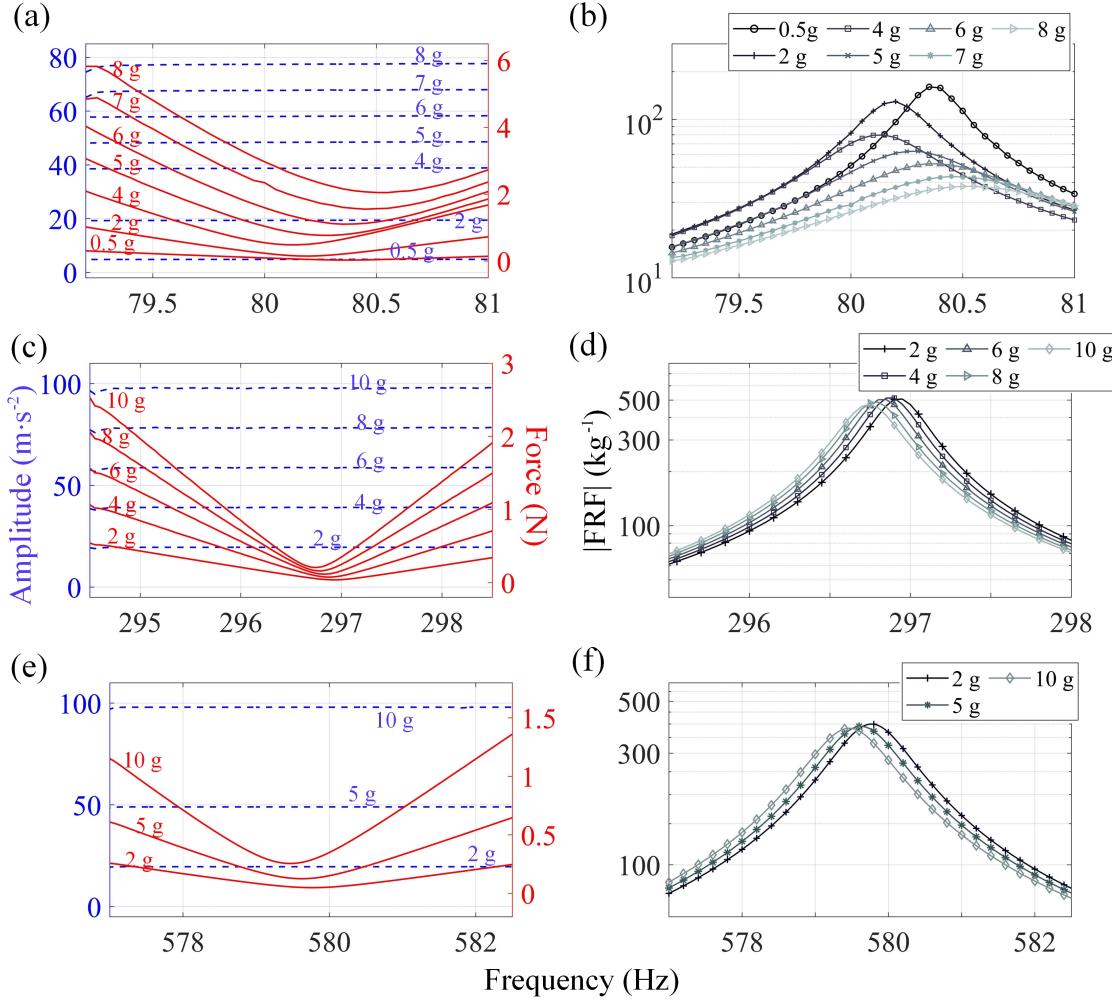


Figure 11: The response levels, excitation levels, and corresponding FRFs during the amplitude-control stepped sine testing, shown are response levels and excitation levels for (a) mode 1, (c) mode 2 and (e) mode 3; the corresponding FRFs for (b) mode 1, (d) mode 2, (f) mode 3. Subfigures (a), (c), and (d) has left y-label as amplitudes in m/sec^2 and right y-label as force in N. Similarly subfigures (b), (d), and (f) has y-label as —FRF— in kg^{-1} . xlabel of all the subfigures are Frequency in Hz. The dashed lines represent the response level for the amplitude-control stepped sine. The solid lines represent the excitation force levels.

412 shown in Fig. 12 (a), the excitation forces (solid lines) maintain almost the same level during the
 413 stepped-sine test, while the corresponding responses (dashed lines) show peaks. Compared to the
 414 dashed lines in Fig. 11 (a), the solid lines show more fluctuation. Specifically, for the 1.5 N control
 415 level in Fig. 12 (a), the lowest amplitude is 1.28 N, and the highest level is 1.62 N. Additionally,
 416 the solid red lines, which represent the response levels, are not smooth for this frequency band.
 417 In Fig. 12 (b), the height of the FRF decreases with increases in the excitation force level, which
 418 indicates an increase in damping for high response levels. Experiments for modes 2 and 3 were
 419 not conducted because of the clear advantage presented by the amplitude-controlled experiments,
 420 and because it is also very difficult to maintain a constant force level for the higher modes.

421 Amplitude-controlled or force-controlled stepped-sine testing is much more mature compared
 422 with shaker ringdown testing. However, in practice, this method may not be suitable for low
 423 response levels because the force required to excite the low-amplitude responses at the resonant

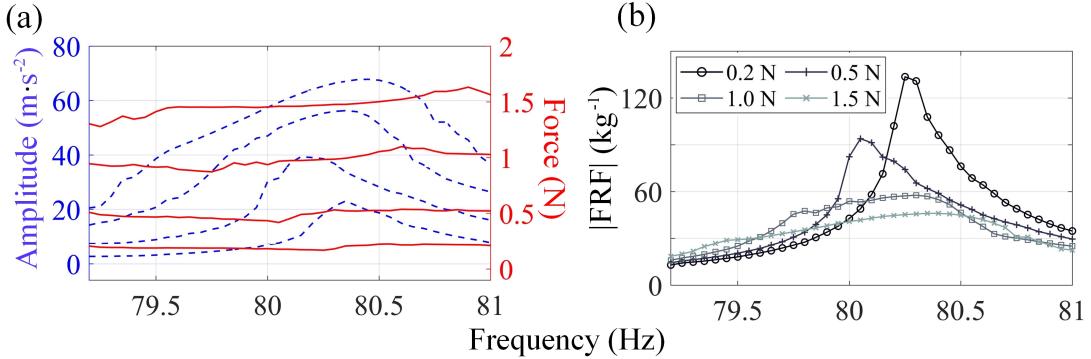


Figure 12: The response levels, excitation levels, and corresponding FRFs during the force-controlled stepped-sine, shown are (a) response levels and excitation levels, and (b) the corresponding FRFs. The dashed lines represent the response level for the amplitude-controlled stepped-sine. The solid lines represent the excitation force levels.

frequency may be contaminated by noise. Compared with force-controlled stepped-sine testing, the amplitude-controlled stepped-sine testing is more stable, and the measured force and response amplitude curves are much smoother. Due to these issues, only amplitude-control stepped sine testing is considered further.

3. Preliminary Analysis

The first set of experiments are analyzed without video data in order to characterize the system and to provide a basis for validating the video-based measurements. These experiments use accelerometer data to measure the response of the system.

3.1. Constraint Analysis

The first analysis focuses on determining if the additional constraints (to keep the beam within the FOV of the cameras) significantly affect the dynamics of the beam. For linear systems, adding constraints on the node points of any particular mode will not have any effect on that mode. However, as the HBRB is a nonlinear system, the mode shapes change with amplitude, and bifurcations in the dynamics may lead to modal interactions, including internal resonance and mode conversion. To study the effect of the constraints on the system and to find the placements that minimize these effects, hammer tests with different constraint locations on the beam were conducted.

The HBRB with each bolt torqued to 20 N·m is considered for this experiment. Figure 13 shows the test strategy for the beam. The excitation location and the accelerometer are placed 1" from the left end of the beam. The elastic bungees are attached to the beam symmetrically at a distance L from both the left and right sides. Here, L ranges from 1" to 13" with a step of 2". Hammer testing is conducted for each of the specified constraint locations, and the responses from the first three modes are extracted from the response. The backbone curves and the damping-amplitude curves measured for the different constraint locations are compared against the measurements from the unconstrained system.

Figure 14 shows the backbone curves and damping-amplitude curves for modes 1, 2, and 3. Each of the curves in these figures is obtained from the response with a different constraint location.

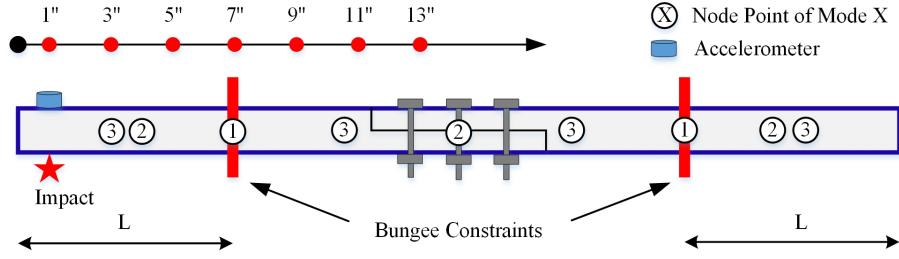


Figure 13: The experimental strategy of bungee constraints. The circles with numbers inside are the locations of the node points.

451 For mode 1 (see Fig. 14 (a) and (b)), compared with other constraint locations, the curves with
 452 the constraint at 7" match well with that of the free-free boundary condition where there is no
 453 additional constraint attached to the beam. This makes sense as 7" is the node point of mode 1
 454 (see Fig. 13); hence, constraining at 7" from both of the ends has the least effect on the frequency
 455 and damping ratio curves of mode 1. For the other constraint locations, the response exhibits much
 456 higher damping than for the optimal constraint location. The same conclusion for the frequency,
 457 as well as damping ratio, holds for the results of modes 2 and 3. Since the node point of mode 2
 458 is between 3" and 5", the backbone curves and damping-amplitude curves of constraints at these
 459 two locations are close to that without constraints. According to the results above, the constraint
 460 at the node point of a mode has the least effect on that mode. The applied constraint location for
 461 the free-response testing is shown in Table 3.

Table 3: Constraint location for the three modes.

Mode Order	1	2	3
Constraint Locations	7"	3.5"	3"

462 3.2. Result Comparison

463 With the optimal constraint locations identified, the next step is to compare each of the different
 464 testing methods. The amplitude-dependent frequency and damping ratios of the first three modes
 465 are extracted from the hammer tests with no constraint, the hammer tests with optimal constraint,
 466 the shaker ringdown tests with optimal constraint, and the amplitude-controlled stepped-sine tests.
 467 The torque level for each bolt is 10 N·m. The results are shown and compared with each other in
 468 Fig. 15.

469 The amplitude-dependent frequency and damping ratio curves are consistent with each other
 470 for all three modes in Fig. 15. Thus, the optimal constraint has little effect on the free-response
 471 testing, and all of the test methods are valid to extract the nonlinear properties. For the first
 472 mode, the amplitude-controlled stepped-sine tests and the shaker ringdown tests are able to excite
 473 the response to a much higher amplitude than the hammer tests. Moreover, the damping ratio
 474 shows a variation from 0.72% to 0.21% with amplitude dropping from $80 \text{ m}\cdot\text{s}^{-2}$ to $0.1 \text{ m}\cdot\text{s}^{-2}$, which
 475 is much higher than that of the second and third modes. Therefore, the nonlinearity has a smaller
 476 effect on the second and third modes than on the first mode.

477 Hammer tests are a good approach to obtain the nonlinear properties and their trend (softening
 478 or hardening) at relatively low amplitudes. In contrast, the amplitude-controlled stepped-sine tests

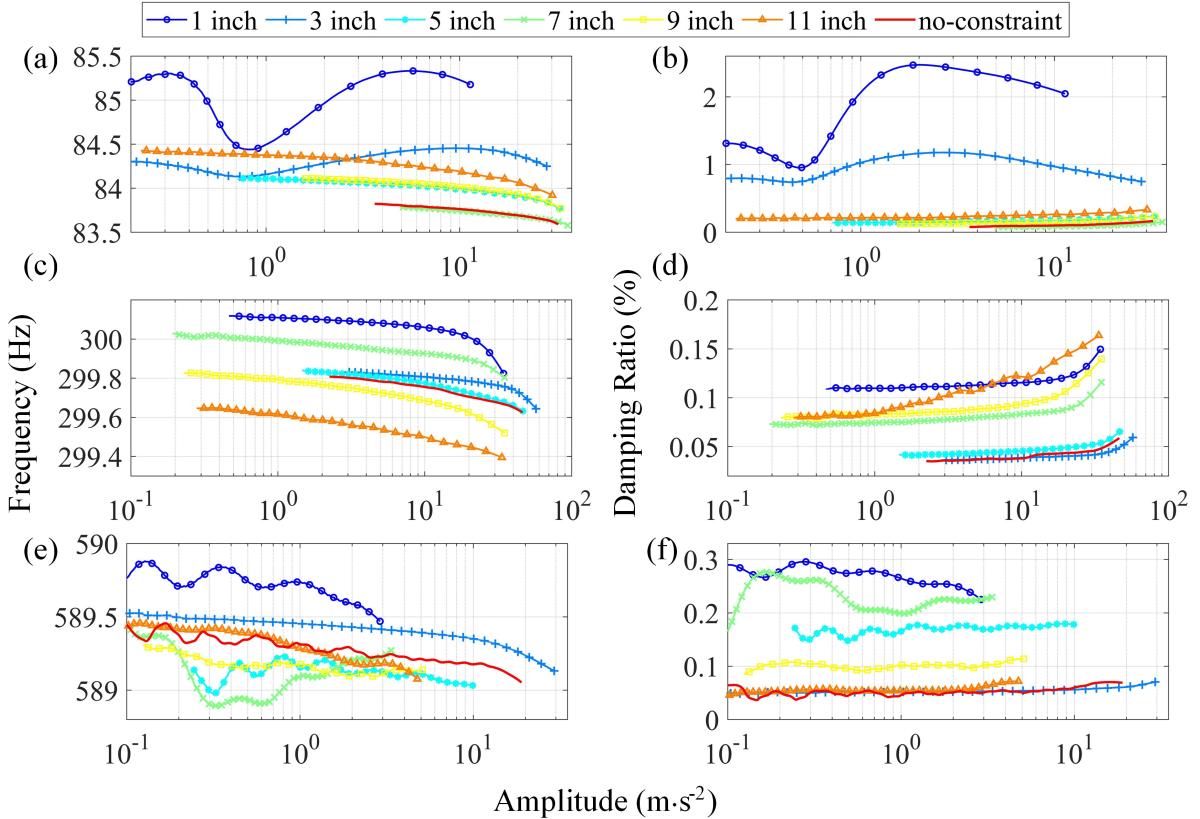


Figure 14: Backbone and damping-amplitude curves for different constraint locations, shown are backbone curves for (a) mode 1, (c) mode 2 and (e) mode 3, and damping-amplitude curves for (b) mode 1, (d) mode 2 and (f) mode 3.

show advantages in obtaining properties at high response levels but may not be suitable for the low response levels due to inaccuracies in the measurements of the force at low amplitudes, especially at the resonant frequency. The shaker ringdown tests show advantages in both exciting the target mode to high levels and recording that mode at low levels. Since a sinusoidal excitation is used, which has energy in a narrow band, the target mode (especially the first mode) can be excited to a level much higher than that from hammer tests. When the beam is separated from the shaker, the free decay response can be recorded from a high to a low response level (i.e., the noise level), which probes the nonlinearity in a wide range of response amplitude.

4. Experimental Study Using Video Data

The responses measured with accelerometer data in Section 3 are used to narrow the range of experiments conducted with videography. The process of recording, transferring to a computer, and processing the video data with the subsequent DIC analysis is time-consuming, so it is prudent to narrow the number of experiments conducted with videography. DIC data can provide dense spatial information of the beam; hence, it is uniquely suited to provide information about the entire mode shapes of the beam (as opposed to measurements at discrete points). An alternative approach might be the use of a scanning laser Doppler vibrometer (LDV) [27, 41]; however, an LDV is only able to measure normal motion, and the present study is concerned with planar motion.

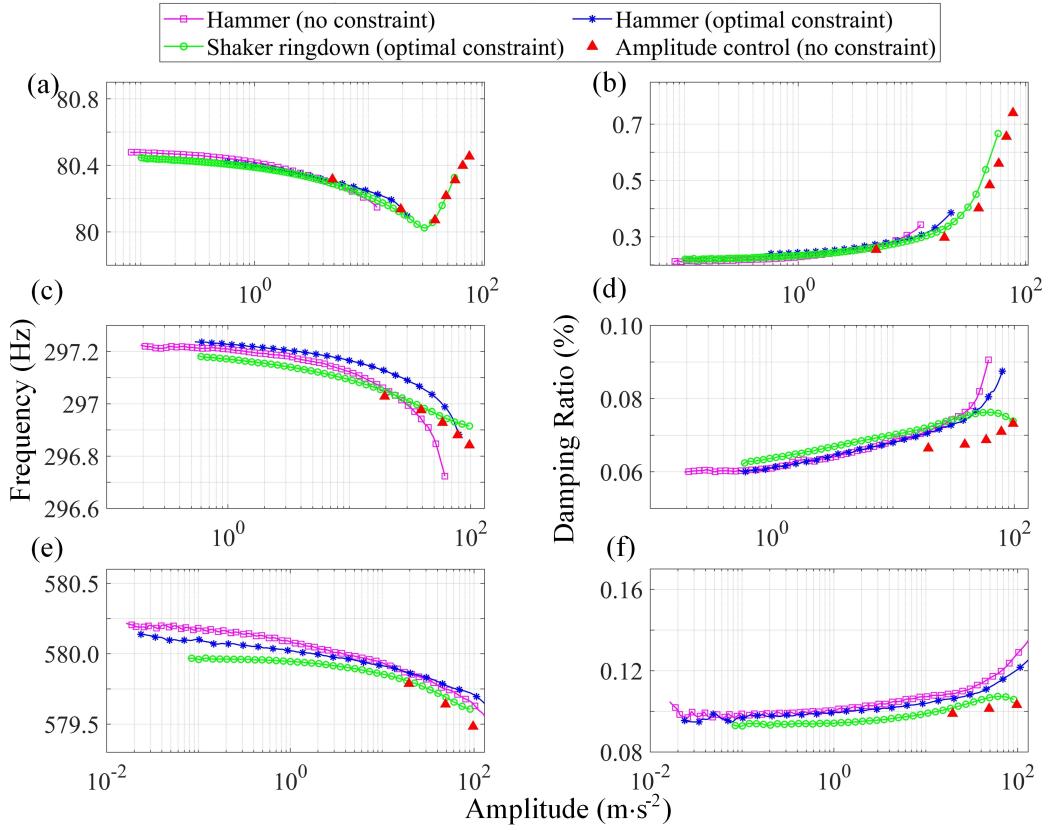


Figure 15: Backbone and damping-amplitude curves from different tests, shown are backbone curves for (a) mode 1, (c) mode 2 and (e) mode 3, and damping-amplitude curves for (b) mode 1, (d) mode 2 and (f) mode 3.

496 4.1. Noise Level Test

497 Prior to using the DIC data for any vibration test, the noise level for the camera-based measurement
 498 should be quantitatively assessed to evaluate the data quality. Hence, for this experiment,
 499 the HBRB is laid flat such that it is motionless. Without any excitation, the measured responses
 500 are ambient noise signals of the whole measurement system, including the camera (and its fan),
 501 tripod, light source, and the DIC method. Approximately 200 measurement points along the beam
 502 are tracked by the DIC technique. The signal is recorded for 2 s with a sampling frequency of 5000
 503 Hz.

504 The time histories of all of these measurement points are shown in Fig. 16 (a). Even though
 505 the beam is static, all of the points show that the response varies within ± 0.02 pixels in the time
 506 history, which is considered to be the noise for this measurement system. The frequency content
 507 of the noise responses is shown in Fig. 16 (b). Two dominant peaks are observed at 120 Hz and
 508 146.2 Hz, which are separated from the natural frequencies of the test beam. The peak at 120 Hz
 509 is due to the electrical noise, and the peak at 146.2 Hz is due to the rotation of the fan inside the
 510 high-speed cameras.

511 For proper quantification of the noise level of the measurement system, statistical analysis is
 512 conducted using the DIC data. The probability density function (PDF) of the noise signal is
 513 represented by the histogram of the recorded signals, which is also fitted by Gaussian distribution
 514 in Fig. 16 (c). From the cumulative distribution function (CDF) of this noise profile, according

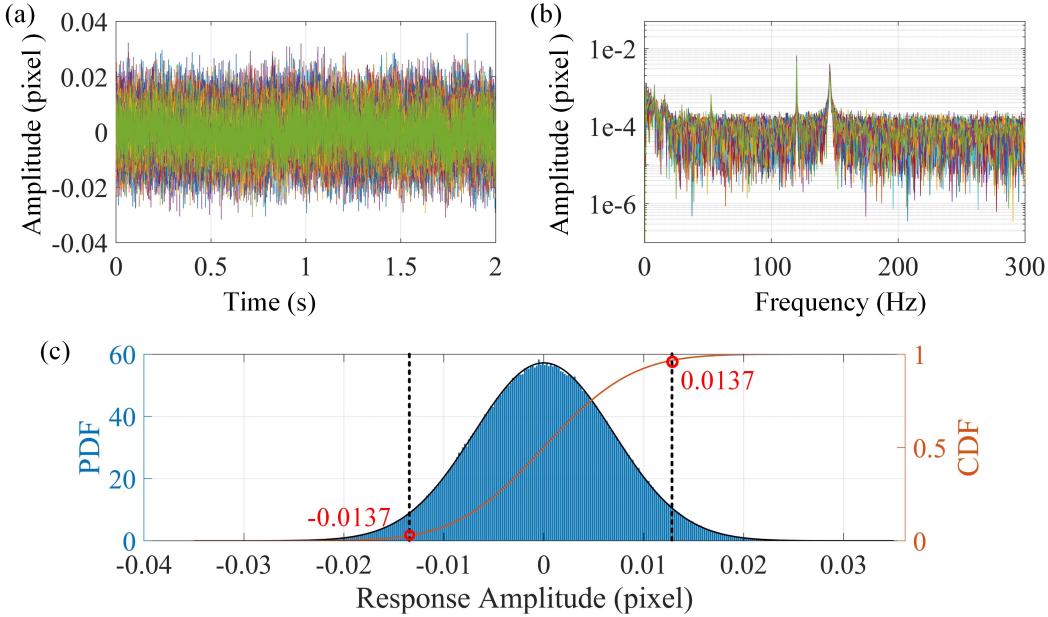


Figure 16: The static responses in time and frequency domain, shown are (a) time history of noise responses, (b) frequency content of noise responses, and (c) noise level determination from DIC data.

to the 95% confidence interval (dashed lines), the noise level is found to be 0.0137 pixel which is $3.9 \mu\text{m}$ in terms of the physical unit (as 1 pixel equals to 0.285 mm, details can be found in Section 2.2). Hence, if the response level is below $3.9 \mu\text{m}$, the raw data is not valid for further analysis.

4.2. Validation of DIC Data with Accelerometer Data

To show that the responses obtained from DIC in these experiments are accurate enough for characterizing the nonlinearity, the instantaneous frequency and damping ratio curves using the DIC data are compared with the curves obtained from the accelerometer data. For this comparison task, test results from a shaker ringdown experiment are used, where the bolt torque used to assemble the HBRB is 10 N·m.

Displacement time histories of the beam are recorded in a dense manner, i.e., 205 measurement points along the beam, which is shown in Fig. 17 (a). Owing to the high spatial resolution of the DIC measurement, the motion of the whole beam can be reproduced from this figure. Since the beam is released at the resonance frequency of the 1st mode, the mode shape of the 1st mode can be seen from any time instant of the response. The recorded response from the accelerometer (accelerometer 1 in Fig. 5) is shown in Fig. 17 (b), and the response from DIC at this location is shown in Fig. 17 (c). Since the SNR decreases with the decay of the response, the recorded response from DIC is truncated at 3.5 s. The low-frequency oscillation seen in Fig. 17 (c) is due to the rigid body sway mode of the constrained system (as acceleration scales with frequency squared, this is not visible in the acceleration data).

The amplitude-time-frequency curve and amplitude-time-damping curve are extracted from the accelerometer data (see Fig. 17 (b)) and DIC data (Fig. 17 (c)) at the same location of the beam. The responses are reverse filtered [57] by a 4th-order Butterworth bandpass filter, and then the filtered response is used to extract the instantaneous frequency, damping ratio, and

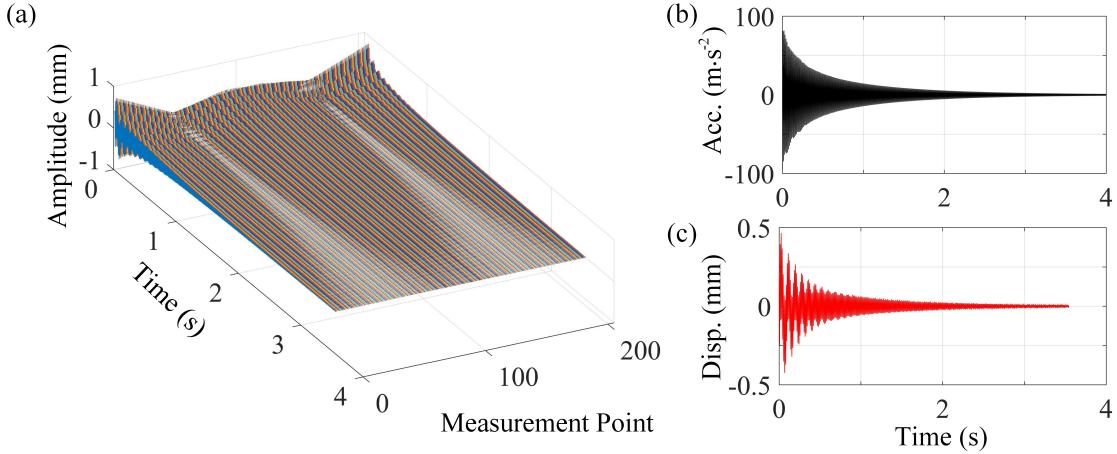


Figure 17: The recorded responses of the beam from DIC and accelerometer, shown are (a) all the recorded responses from DIC, (b) the acceleration time history from the accelerometer, (c) the recorded displacement response at the location with the accelerometer.

amplitude by the PFF method (details can be found in [58] and part II of this paper [1]). The amplitude for acceleration is transferred to amplitude in displacement by dividing by the square of the frequency. The extracted amplitude-time curves and amplitude-time-damping curves from these two data acquisition methods are compared in Fig. 18.

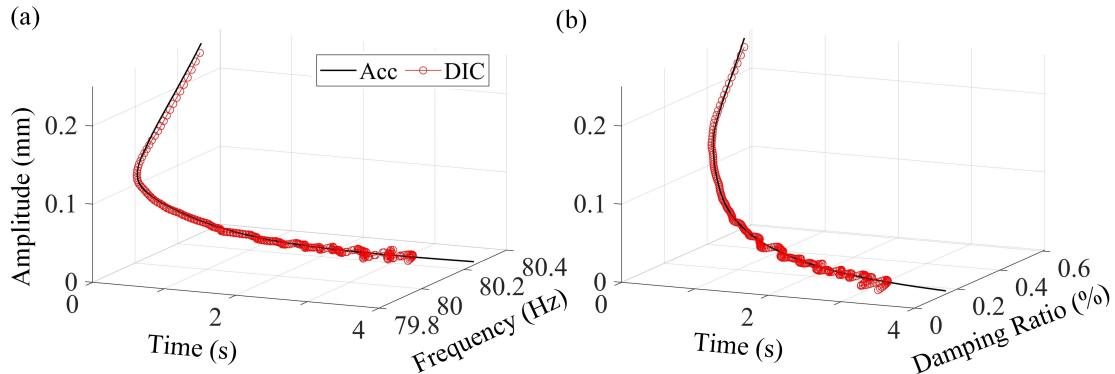


Figure 18: The comparison of the DIC results and the results from the accelerometer, shown are (a) instantaneous frequency and (b) instantaneous damping ratio.

From Fig. 18, it is observed that the amplitude-time-frequency curves and amplitude-time-damping curves are comparable for these two measurement techniques. Therefore, it can be concluded that DIC can be used for further data processing, and as it gives dense measurement data, it is beneficial in obtaining the spatial distribution of nonlinear properties. At low amplitudes (lower than 0.04 mm), the fluctuation from DIC results can be seen because of the video noise in low displacement, but it still shows the same trend with acceleration results.

4.3. Quantification of the Structure's Nonlinearities

A fundamental question in the analysis of nonlinear structures (specifically bolted assemblies) is whether the structure is strongly nonlinear or weakly nonlinear. Weakly nonlinear structures are

552 amenable to certain modeling assumptions that make numerical analysis more efficient [60]. Central
 553 among the assumptions made are that the mode shapes do not change during excitation and that
 554 there is no appreciable modal coupling. By contrast, models of strongly nonlinear structures make
 555 no such assumptions and often exhibit amplitude evolving mode shapes [61] as well as coupling
 556 between modes [62]. As a result, these modeling approaches can be more computationally intensive
 557 as higher fidelity models of the joint are necessitated.

558 4.3.1. Mode Shape Evolution

559 The evolution of the mode shapes as a function of response amplitude is investigated by using
 560 the DIC-based full-field measurement data from fixed sine tests in which the system is driven at
 561 resonance. First, the amplitude-control stepped-sine testing (in Section 2.7.1) is conducted to find
 562 the natural frequency at the target amplitude level, which shows a 90° phase shift between the
 563 excitation force and the acceleration response. Next, the fixed sine testing is conducted at the
 564 resonance frequency and response level, and the high-speed cameras are triggered to capture the
 565 steady-state vibration of the beam.

566 The first mode is excited, and the response levels are set as 0.5, 2, 4, 8, and 10 g for accelerometer
 567 3 in Fig. 10. The extracted deflection shape at the highest response amplitude level is shown in
 568 Fig. 19 (a). Since the phase shift between the excitation force and the acceleration response is 90° ,
 569 the deflection shape is the mode shape at the response level. With the increase of the response
 570 level from 0.5 to 10 g, the displacement at the left edge of the deflection shape, U_{tip} , increases
 571 from approximately 0.03 mm to 0.53 mm. The discontinuous slope can be seen at the edge of the
 572 interface (i.e., just to the left of the dashed line indicating the displacement of the middle of the
 573 beam, U_{mid}), which is caused by the opening and closing of the joint during the vibration. The
 574 discontinuous slope would have appeared on the other side of the interface if the opposite side
 575 of the beam were measured. In order to evaluate the variation of the mode shape, the deflection
 576 ratios between two locations (U_{tip} and U_{mid} in Fig. 19 (a)) at different response levels are shown
 577 in Fig. 19 (b). Due to the low SNR for the 0.5 g response level, the ratio for 0.5 g in Fig. 19 (b)
 578 is not in line with the other results. The ratio generally shows an increasing trend from -0.973 to
 579 -0.914 with a change of the mode shape of 6%.

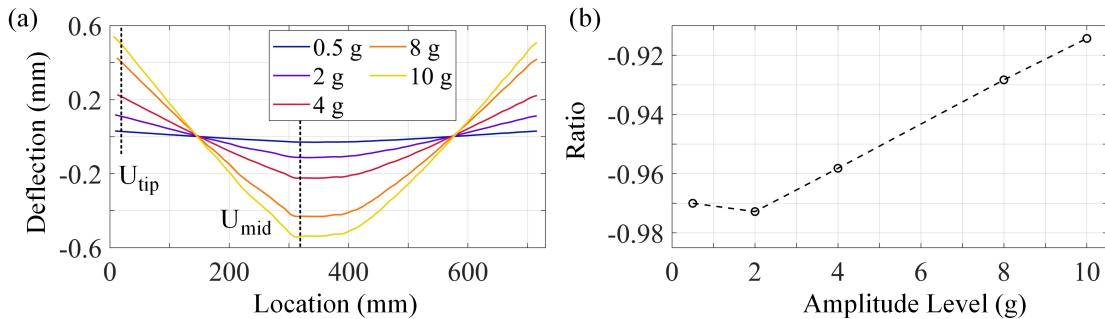


Figure 19: The deflection of the beam and its variation at different response levels, (a) the deflection of the beam at the fixed sine excitation of different levels, (b) the variation of the ratio between the tip and the middle of the beam.

580 4.3.2. Modal Interactions

581 The modes for linear structures are independent (one mode will not affect others, and there
 582 is no energy transfer between them). However, for nonlinear systems, some modes are coupled,
 583 and energy transfer is observed from one mode to another under specific conditions (such as
 584 the natural frequencies at a given amplitude being integer multiples of each other). To study
 585 the modal interaction in jointed structures, responses under steady-state sinusoidal excitation are
 586 used, where the experiment combines the fixed sine testing, amplitude-control stepped sine testing
 587 (in Section 2.7.1), and DIC technique.

588 The test setup is shown in Fig. 10. Three accelerometers are attached to the beam, 1" from the left end, 7" and 3" from the right end. The bolt torque level is kept as 23.7 N·m. The fixed sine
 589 and amplitude-control stepped-sine excitations are used to provide steady-state responses. The
 590 excitation location is 3" from the right end, which is the node point of mode 3 and is close to
 591 the node point of mode 2. Therefore, mode 1 can be sufficiently excited, and modes 2 and 3 are
 592 depressed in this excitation condition.

593 To better understand the vibration behavior, the time history of the accelerometer located at
 594 1" from the left end is shown in a short time window in Fig. 20 (a). Since this accelerometer is not
 595 close to the node point of any of the first 3 modes, it should show the resulting response of the first
 596 three modes. For the results of Fig. 20 specifically, the excitation frequency is 84 Hz, which is the
 597 first modal frequency of the beam, and the excitation level is 10 g. Even though the input signal
 598 is sinusoidal, the response contains two main frequency components that are composed of the
 599 excitation frequency and the natural frequency of mode 3 (approximately 588 Hz). In Fig. 20 (b),
 600 the responses are shown after filtering with a 4th order Butterworth bandpass filter with a middle
 601 frequency of 84 Hz (curve 1) and 588 Hz (curve 2). Curve 3 indicates the difference between the
 602 raw response and the responses of curve 1 and curve 2 added together. The double reverse filter
 603 [58] is used here to guarantee no phase shift for the filtered responses. Even though energy is only
 604 put into mode 1 (due to both the excitation frequency and excitation location), a considerable
 605 amount of energy flows into mode 3.
 606

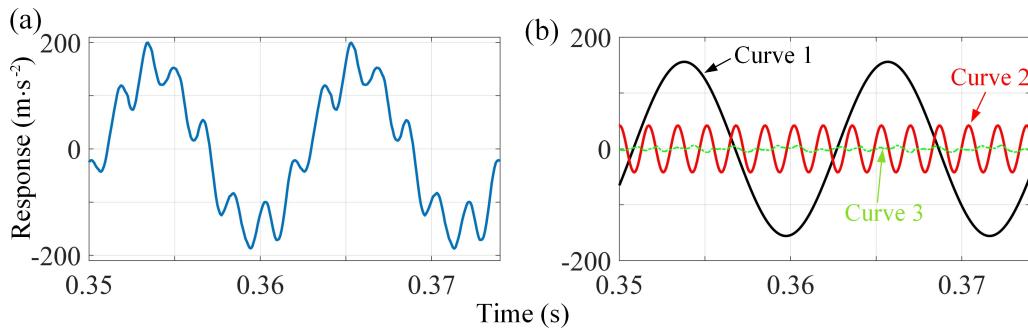


Figure 20: The test setup and the recorded signal from the blue accelerometer, shown are (a) the measured raw response from accelerometer 1 (1 inch from the left end), (b) the decomposed responses.

607 The modal coupling observed in Fig. 20 is further explored via the DIC measurements. Along
 608 the length of the beam, 205 measurement points are recorded with the high speed cameras during
 609 steady state excitation. From these measurements, the filtered responses at 84 and 588 Hz are
 610 shown in Fig. 21. The resulting shapes are the same as for the first and third mode shapes,
 611 providing clear evidence that even though energy is being put into only mode 1, the nonlinear

612 nature of the system has transferred energy to mode 3.

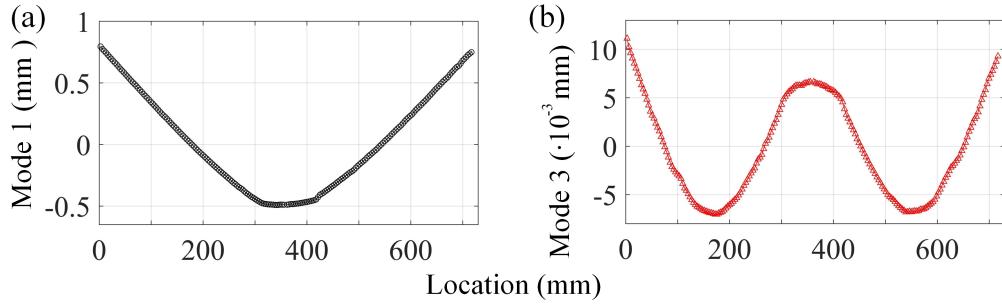


Figure 21: The extracted mode shapes from DIC data, shown are (a) the displacement distribution of component of 84 Hz, (b) the displacement distribution of component of 588 Hz.

613 To further explore the interaction of the modal coupling and excitation amplitude, an amplitude-
614 control stepped sine test is executed with response amplitudes of 0.5, 1, 2, 4, 6, 10, and 12 g. Here,
615 the response amplitude refers to the steady response amplitude of accelerometer 3 (at the location
616 of the excitation) in Fig. 10. The excitation frequency is from 83 to 85 Hz (in the vicinity of the
617 frequency of mode 1) with a step of 0.05 Hz. For each step, the excitation duration is set as 3 s to
618 reach a steady state.

619 The variation of the response levels of the mode 1 and mode 3 components measured by the
620 tip accelerometer with respect to the controlled level of mode 1 is shown in Fig. 22 (a) and (b),
621 respectively. Since the response level of mode 1 is controlled during the stepped-sine testing, the
622 response curves in Fig. 22 (a) are approximately constant amplitudes. By contrast, in Fig. 22 (b),
623 each response curve shows a peak for the mode 3 amplitude near an excitation frequency of 84 Hz
624 during the stepped-sine testing. The frequency of the peak shows a small range of variation across
625 the different response levels, which is highlighted by a black dashed line in Fig. 22 (b).

626 The highest amplitudes of mode 1 and the peak values of the amplitude of mode 3 with different
627 control levels are extracted from Fig. 22 (a) and (b). The peak values are further normalized by
628 dividing the corresponding control level, and the results are shown in Fig. 22 (c). For mode 1,
629 the amplitude ratios are approximately constant (around 2) with the increase of the controlled
630 response level. By contrast, the amplitude ratio curve of mode 3 shows a different trend. The
631 response level of mode 3 is lower than 10% of the input level for low response levels (0 to 4 g).
632 Increasing the input level above 4 g, however, significantly changes the behavior as the response
633 level jumps to 50% of the input level and remains at this percentage as the input is increased
634 further. Thus, there is both a frequency and an amplitude dependence to the existence of modal
635 coupling in this system.

636 The precise conditions for modal coupling are elucidated through a series of amplitude-controlled
637 stepped-sine tests in which the bolt torques are varied (10 N·m and 19.6 N·m, in addition to the
638 above-described results for 23.7 N·m). It is observed that modal interactions only happen for a
639 bolt torque level of 23.7 N·m, as shown in Table 4. In accordance with the nonlinear dynamics
640 conditions for modal coupling, this is explained by the 23.7 N·m case being the only one in which
641 the third natural frequency is an integer multiple of the first natural frequency. For all other
642 bolt torques investigated, this condition is not met, as shown in Table 4. In other words, as this
643 system is nonlinear, some energy is dissipated through the super-harmonics of the mode being

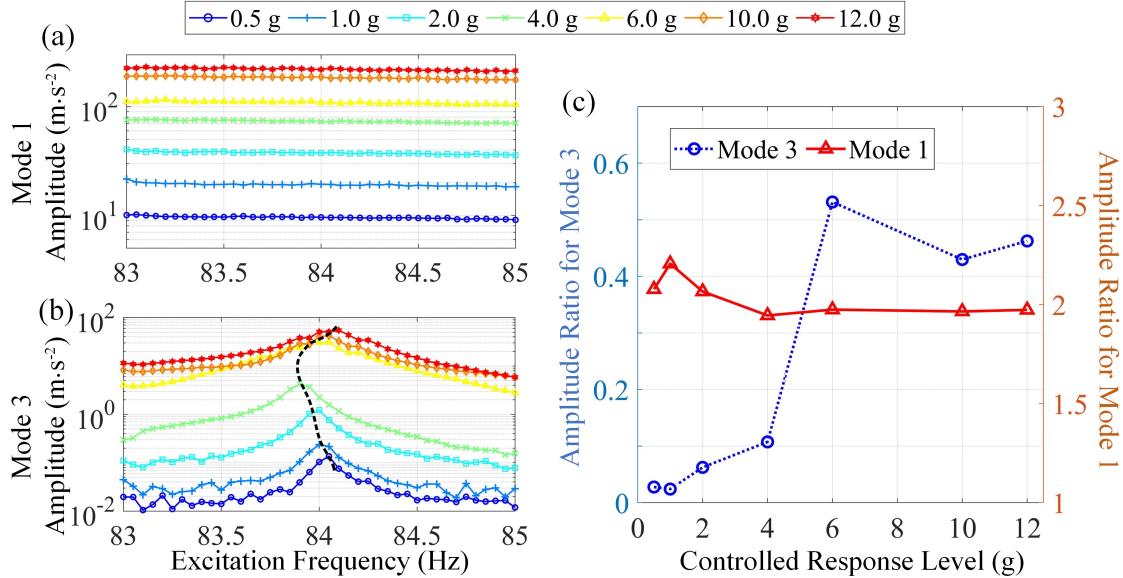


Figure 22: Response levels of mode 1 and mode 3 during the amplitude-controlled stepped-sine testing with different response levels, shown are (a) the amplitude of mode 1 at each step, (b) the amplitude of mode 3 at each step, and (c) the normalized peak values of mode 1 and mode 3 for different controlled levels.

excited. This dissipation is amplified when one of the super-harmonic frequencies is exactly equal to another modal frequency [18, 63].

Table 4: Modal interaction for different bolt torque levels.

Torque (N·m)	f_1 (Hz)	f_2 (Hz)	f_3 (Hz)	f_2/f_1	f_3/f_1	Modal Interaction
23.7 N·m	83.77-84.28	299.81-300.23	589.14-590.16	3.56-3.58	6.99-7.05	Yes
19.6 N·m	82.97-83.70	299.25-299.96	588.08-589.42	3.56-3.62	7.03-7.10	No
10.2 N·m	79.80-80.03	290.05-290.51	580.21-580.90	3.61-3.66	7.22-7.28	No

5. Conclusions

In this paper, a series of tests are conducted to characterize the dynamic behavior of an assembly that is joined together by a three-bolt lap joint, termed the HBRB. First, the system is characterized using conventional approaches based on accelerometer data. This initial characterization served the dual purpose of both providing a baseline model of the nonlinearity and identifying specific regions that would benefit from additional and more expensive, video-based measurements. DIC was subsequently used to measure a dense, spatio-temporal profile of the displacement of the system during dynamic excitation for multiple types of tests: hammer impact, shaker ringdown, force-control stepped sine sweeps, and amplitude-control stepped sine sweeps. The DIC measurements are extensively validated both from assessments of the noise floor in static tests and through direct comparison with the results of the analysis based on accelerometer data. The DIC data is then used to assess the nonlinear behavior of the system, specifically how the

658 mode shapes evolve with response amplitude and the conditions necessary for modal coupling to
659 occur. Specific conclusions from this study include:

660 • DIC is capable of capturing the full-field response of HBRB with sufficient accuracy. In this
661 approach, each camera captures the motion of a half beam to increase the spatial measurement
662 density, and then the spatial data from two cameras are stitched together to obtain the responses
663 of the whole beam. For the current experimental setup, the measurement noise of DIC responses,
664 which comes from the measurement system (including measurement environment, the camera, and
665 DIC method), is about 0.0137 pixels, corresponding to $3.9 \mu\text{m}$.

666 • A novel shaker ringdown testing setup is introduced in this paper. After the shaker excites
667 a particular mode of the HBRB to steady-state, the stinger is detached from the beam, and the
668 free responses of this mode are recorded. With a suitable excitation location, the target mode can
669 be excited to a higher level than hammer testing, which can provide a wider range for amplitude-
670 dependent natural frequency and damping ratio curves.

671 • Both force-controlled and amplitude-controlled stepped sine tests are used. The amplitude-
672 control tests proved to be a more accurate method to obtain the frequency response functions.

673 • The identified amplitude-dependent natural frequency and damping ratio curves are compa-
674 rable for accelerometer and DIC data. The size of the field-of-view for the camera has a great
675 effect on the measurement accuracy of the DIC technique. For the same free response testing, DIC
676 results show consistency with that from accelerometer data, but the accuracy may be lower than
677 acceleration data due to the large field of view (0.36 m in length for 1280 pixels).

678 • The mode shapes can be extracted from DIC data. With dense spatial data from DIC,
679 the mode shapes of the beam is extracted from the fixed sine testing where the phase difference
680 between the response and the force is 90° at the set response level. Specifically, the ratio of two
681 points on mode 1 shows a 6% variation as the response levels increase from 0.5 g to 10 g.

682 • A modal interaction is observed for the HBRB when the third natural frequency is an integer
683 multiple of the first natural frequency (in this case, a factor of 7). During the forced response
684 testing, the beam is excited at the node point of mode 3 by the frequency of the first mode.
685 However, there is a significant component of mode 3 in the responses. From the DIC data, the
686 deflection shapes corresponding to the excitation frequency and 7 times of the excitation frequency
687 are extracted, which show similarity with the mode shapes of modes 1 and 3. Additionally,
688 through amplitude-controlled stepped sine testing, the modal interaction behavior is affected by
689 the excitation frequency and response level, which becomes evident for high response levels (more
690 than 4 g for mode 1).

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700 **Data Access**

701 The data used in the paper is available at <https://github.com/mattiacenedese/BRBtesting>.

702 **References**

- 703 [1] M. Jin, G. Kosova, M. Cenedese, W. Chen, A. Singh, D. Jana, M. R. W. Brake, C. W.
704 Schwingshackl, S. Nagarajaiah, K. Moore, and J. P. Noël. Measurement and identification of
705 the nonlinear dynamics of a jointed structure using full-field data; Part II - Nonlinear System
706 Identification. *Under Review*, 2021.
- 707 [2] M. R. W. Brake. *The Mechanics of Jointed Structures: Recent Research and Open Challenges
708 for Developing Predictive Models for Structural Dynamics*. Springer, 2017.
- 709 [3] L. Gaul and R. Nitsche. The role of friction in mechanical joints. *Applied Mechanics Reviews*,
710 54(2):5173–84, 2001.
- 711 [4] C. W. Schwingshackl, D. Di Maio, I. Sever, and J. S. Green. Modeling and validation of the
712 nonlinear dynamic behavior of bolted flange joints. *Journal of Engineering for Gas Turbines
713 and Power*, 135(12):122504, 2013.
- 714 [5] G. Kerschen, K. Worden, A. F. Vakakis, and J. C. Golinval. Past, present and future of nonlin-
715 ear system identification in structural dynamics. *Mechanical Systems and Signal Processing*,
716 20(3):505–592, 2006.
- 717 [6] J. P. Noel and G. Kerschen. Nonlinear system identification in structural dynamics: 10 more
718 years of progress. *Mechanical Systems and Signal Processing*, 83:2–35, 2017.
- 719 [7] SL. Brunton, JL. Proctor, and JN. Kutz. Discovering governing equations from data by sparse
720 identification of nonlinear dynamical systems. *Proceedings of the national academy of sciences*,
721 113(15):3932–3937, 2016.
- 722 [8] Z. Lai and S. Nagarajaiah. Sparse structural system identification method for nonlinear dy-
723 namic systems with hysteresis/inelastic behavior. *Mechanical Systems and Signal Processing*,
724 117:813–842, 2019.
- 725 [9] K. Worden, RJ Barthorpe, EJ Cross, N Dervilis, GR Holmes, G Manson, and TJ Rogers.
726 On evolutionary system identification with applications to nonlinear benchmarks. *Mechanical
727 Systems and Signal Processing*, 112:194–232, 2018.
- 728 [10] M. Jin, M. R. W. Brake, and H. Song. Comparison of nonlinear system identification methods
729 for free decay measurements with application to jointed structures. *Journal of Sound and
730 Vibration*, 453:268–293, 2019.
- 731 [11] P. A. Atkins, J. R. Wright, and K. Worden. An extension of force appropriation to the
732 identification of non-linear multi-degree of freedom systems. *Journal of Sound and Vibration*,
733 237(1):23–43, 2000.

- 734 [12] M. Peeters, G. Kerschen, and J. Golinval. Modal testing of nonlinear vibrating structures
 735 based on nonlinear normal modes: experimental demonstration. *Mechanical Systems and*
 736 *Signal Processing*, 25(4):1227–1247, 2011.
- 737 [13] C. W. Schwingshakl, C. Joannin, L. Pesaresi, J. S. Green, and N. Hoffmann. *Test method*
 738 *development for nonlinear damping extraction of dovetail joints*, pages 229–237. Springer,
 739 2014.
- 740 [14] S. Catalfamo, S. A. Smith, F. Morlock, M. R. W. Brake, P. Reuß, C. W. Schwingshakl, and
 741 W. Zhu. *Effects of experimental methods on the measurements of a nonlinear structure*, pages
 742 491–500. Springer, 2016.
- 743 [15] G. Gloth and M. Sinapius. Influence and characterisation of weak non-linearities in swept-sine
 744 modal testing. *Aerospace Science and Technology*, 8(2):111–120, 2004.
- 745 [16] J. R. Wright, J. E. Cooper, and M. J. Desforges. Normal-mode force appropriation—theory
 746 and application. *Mechanical Systems and Signal Processing*, 13(2):217–240, 1999.
- 747 [17] J. Lau, B. Peeters, J. Debillie, Q. Guzek, and T. Kahlmann. *Ground Vibration Testing Master*
 748 *Class: modern testing and analysis concepts applied to an F-16 aircraft*, volume 1 of *Advanced*
 749 *Aerospace Applications*. Springer New York, 2011.
- 750 [18] G. Kerschen, M. Peeters, J. Golinval, and A. F. Vakakis. Nonlinear normal modes, part i: A
 751 useful framework for the structural dynamicist. *Mechanical Systems and Signal Processing*,
 752 23(1):170–194, 2009.
- 753 [19] L. Renson, A. Gonzalez-Buelga, D. A. W. Barton, and S. A. Neild. Robust identification of
 754 backbone curves using control-based continuation. *Journal of Sound and Vibration*, 367:145–
 755 158, 2016.
- 756 [20] S. Peter, R. Riethmüller, and R. I. Leine. *Tracking of Backbone Curves of Nonlinear Systems*
 757 *Using Phase-Locked-Loops*. Springer International Publishing, 2016.
- 758 [21] M. Scheel, S. Peter, R. I. Leine, and M. Krack. A phase resonance approach for modal testing
 759 of structures with nonlinear dissipation. *Journal of Sound and Vibration*, 435:56–73, 2018.
- 760 [22] J. P. Noël and G. Kerschen. Frequency-domain subspace identification for nonlinear mechan-
 761 ical systems. *Mechanical Systems and Signal Processing*, 40(2):701–717, 2013.
- 762 [23] S. L. Lacy and D. S. Bernstein. Subspace identification for non-linear systems with measured-
 763 input non-linearities. *International Journal of Control*, 78(12):906–926, 2005.
- 764 [24] S. Marchesiello and L. Garibaldi. A time domain approach for identifying nonlinear vibrating
 765 structures by subspace methods. *Mechanical Systems and Signal Processing*, 22(1):81–101,
 766 2008.
- 767 [25] J. P. Noel, A. F. Esfahani, G. Kerschen, and J. Schoukens. A nonlinear state-space approach
 768 to hysteresis identification. *Mechanical Systems and Signal Processing*, 84:171–184, 2017.

- 769 [26] C. Warren, C. Nieszrecki, P. Avitabile, and P. Pingle. Comparison of frf measurements and
770 mode shapes determined using optically image based, laser, and accelerometer measurements.
771 *Mechanical Systems and Signal Processing*, 25(6):2191–2202, 2011.
- 772 [27] P. L. Reu, D. P. Rohe, and L. D. Jacobs. Comparison of DIC and LDV for practical vibration
773 and modal measurements. *Mechanical Systems and Signal Processing*, 86:2–16, 2017.
- 774 [28] W. Chen, X. Zang, S. Wu, M. Jin, and H. Song. Gradient-based point tracking method and
775 its application in the modal test of a solar array model. *Measurement*, 2020.
- 776 [29] S. Bhowmick, S. Nagarajaiah, and Z. Lai. Measurement of full-field displacement time his-
777 tory of a vibrating continuous edge from video. *Mechanical Systems and Signal Processing*,
778 144:106847, 2020.
- 779 [30] S. Bhowmick and S. Nagarajaiah. Identification of full-field dynamic modes using continuous
780 displacement response estimated from vibrating edge video. *Journal of Sound and Vibration*,
781 489:115657, 2020.
- 782 [31] Wei Chen, Mengshi Jin, Jiasheng Huang, Yuanchang Chen, and Hanwen Song. A method to
783 distinguish harmonic frequencies and remove the harmonic effect in operational modal analysis
784 of rotating structures. *Mechanical Systems and Signal Processing*, 2021.
- 785 [32] TC Chu, WF Ranson, and Michael A Sutton. Applications of digital-image-correlation tech-
786 niques to experimental mechanics. *Experimental mechanics*, 25(3):232–244, 1985.
- 787 [33] Bing Pan, Kemao Qian, Huimin Xie, and Anand Asundi. Two-dimensional digital image
788 correlation for in-plane displacement and strain measurement: a review. *Measurement science
789 and technology*, 20(6):062001, 2009.
- 790 [34] Michael A Sutton, Jean Jose Orteu, and Hubert Schreier. *Image correlation for shape, motion
791 and deformation measurements: basic concepts, theory and applications*. Springer Science &
792 Business Media, 2009.
- 793 [35] Javad Baqersad, Peyman Poozesh, Christopher Nieszrecki, and Peter Avitabile. Photogram-
794 metry and optical methods in structural dynamics—a review. *Mechanical Systems and Signal
795 Processing*, 86:17–34, 2017.
- 796 [36] D. M. Feng and M. Q. Feng. Experimental validation of cost-effective vision-based structural
797 health monitoring. *Mechanical Systems and Signal Processing*, 88:199–211, 2017.
- 798 [37] Billie F Spencer Jr, Vedhus Hoskere, and Yasutaka Narazaki. Advances in computer vision-
799 based civil infrastructure inspection and monitoring. *Engineering*, 5(2):199–222, 2019.
- 800 [38] T. J. Beberniss and D. A. Ehrhardt. High-speed 3D digital image correlation vibration mea-
801 surement: Recent advancements and noted limitations. *Mechanical Systems and Signal Pro-
802 cessing*, 86:35–48, 2017.
- 803 [39] Debasish Jana and Satish Nagarajaiah. Computer vision-based real-time cable tension esti-
804 mation in Dubrovnik cable-stayed bridge using moving handheld video camera. *Structural
805 Control and Health Monitoring*, 28(5):e2713, 2021.

- 806 [40] B. Pan, K. Qian, H. Xie, and A. Asundi. Two-dimensional digital image correlation for in-
807 plane displacement and strain measurement: a review. *Measurement science and technology*,
808 20(6):062001, 2009.
- 809 [41] D. A. Ehrhardt, M. S. Allen, S. F. Yang, and T. J. Beberniss. Full-field linear and non-
810 linear measurements using continuous-scan laser doppler vibrometry and high speed three-
811 dimensional digital image correlation. *Mechanical Systems and Signal Processing*, 86:82–97,
812 2017.
- 813 [42] T. Bregar, K. Zaletelj, G. Cepon, J. Slavic, and M. Boltezar. Full-field FRF estimation from
814 noisy high-speed-camera data using a dynamic substructuring approach. *Mechanical Systems
815 and Signal Processing*, 150:12, 2021.
- 816 [43] A. Singh and K. J. Moore. Characteristic nonlinear system identification of local attachments
817 with clearance nonlinearities. *Nonlinear Dynamics*, 102(3):1667–1684, 2020.
- 818 [44] W. Chen, M. Jin, I. Lawal, M. R. W. Brake, and H. Song. Measurement of slip and separation
819 in jointed structures with non-flat interfaces. *Mechanical Systems and Signal Processing*,
820 134:22, 2019.
- 821 [45] M. Brøns, T. A. Kasper, G. Chauda, S. W. B. Klaassen, C. W. Schwingshackl, and M. R. W.
822 Brake. Experimental Investigation of Local Dynamics in a Bolted Lap Joint Using Digital
823 Image Correlation. *Journal of Vibration and Acoustics*, 142(5), 07 2020.
- 824 [46] P. Poozesh, J. Baqersad, C. Niegrecki, P. Avitabile, E. Harvey, and R. Yarala. Large-area pho-
825 togrammetry based testing of wind turbine blades. *Mechanical Systems and Signal Processing*,
826 86:98–115, 2017.
- 827 [47] M. R. W. Brake, C. W. Schwingshackl, and P. Reuss. Observations of variability and re-
828 peatability in jointed structures. *Mechanical Systems and Signal Processing*, 129:282–307,
829 2019.
- 830 [48] T. Dossogne, T. W. Jerome, D. P. T. Lancereau, S. A. Smith, M. R. W. Brake, B. R. Pacini,
831 P. Reuss, and C. W. Schwingshackl. *Experimental assessment of the influence of interface
832 geometries on structural dynamic response*, pages 255–261. Springer, 2017.
- 833 [49] A. Fantetti, L. R. Tamatam, M. Volvert, I. Lawal, L. Liu, L. Salles, M. R. W. Brake, C. W.
834 Schwingshackl, and D. Nowell. The impact of fretting wear on structural dynamics: Experi-
835 ment and simulation. *Tribology International*, 138:111–124, 2019.
- 836 [50] Z. Zhang. A flexible new technique for camera calibration. *IEEE Transactions on Pattern
837 Analysis and Machine Intelligence*, 22:1330–1334, 2000.
- 838 [51] S. Urban, J. Leitloff, and S. Hinz. Improved wide-angle, fisheye and omnidirectional camera
839 calibration. *ISPRS Journal of Photogrammetry and Remote Sensing*, 108:72–79, 2015.
- 840 [52] P. Reu. All about speckles: Speckle size measurement. *Experimental Techniques*, 38(6):1–2,
841 2014.
- 842 [53] D. Z. Turner. Digital image correlation engine (DICe) reference manual. *Sandia Nat. Lab.,
843 Livermore, CA, USA, Sandia Rep. SAND2015-10606 O*, 2015.

- 844 [54] T Mace, J Taylor, and CW Schwingshakl. A novel technique to extract the modal damping
845 properties of a thin blade. In *Topics in Modal Analysis & Testing, Volume 8*, pages 247–250.
846 Springer, 2020.
- 847 [55] D. Jana, S. Mukhopadhyay, and S. Ray-Chaudhuri. Fisher information-based optimal input
848 locations for modal identification. *Journal of Sound and Vibration*, 459:114833, 2019.
- 849 [56] W. Chen, M. S. Jin, and H. W. Song. Optimal configuration of shakers for phase resonance
850 testing using modal parameters. *Proceedings of the Institution of Mechanical Engineers Part*
851 *C-Journal of Mechanical Engineering Science*, 233(16):5676–5690, 2019.
- 852 [57] H. Goyder. Signal processing methods for determining the properties of bolted joints. In
853 *ASME 2015 International Design Engineering Technical Conferences and Computers and In-*
854 *formation in Engineering Conference*, pages V008T13A024–V008T13A024. American Society
855 of Mechanical Engineers, 2015.
- 856 [58] M. Jin, W. Chen, M. R. W. Brake, and H. Song. Identification of instantaneous frequency
857 and damping from transient decay data. *Journal of Vibration and Acoustics*, 142(5), 2020.
- 858 [59] S. A. Smith, M. R. W. Brake, and C. W. Schwingshakl. On the characterization of nonlin-
859 earities in assembled structures. *Journal of Vibration and Acoustics*, 142(5):11, 2020.
- 860 [60] D. R. Roettgen and M. S. Allen. Nonlinear characterization of a bolted, industrial structure
861 using a modal framework. *Mechanical Systems and Signal Processing*, 84:152–170, 2017.
- 862 [61] N. N. Balaji, W. Chen, and M. R. W. Brake. Traction-based multi-scale nonlinear dynamic
863 modeling of bolted joints: Formulation, application, and trends in micro-scale interface evo-
864 lution. *Mechanical Systems and Signal Processing*, 139:32, 2020.
- 865 [62] M. Krack. Nonlinear modal analysis of nonconservative systems: Extension of the periodic
866 motion concept. *Computers and Structures*, 154:59–71, 2015.
- 867 [63] K. J. Moore, M. Kurt, M. Eriten, D. M. McFarland, L. A. Bergman, and A. F. Vakakis.
868 Direct detection of nonlinear modal interactions from time series measurements. *Mechanical*
869 *Systems and Signal Processing*, 2017.