

Structural Analysis and Finite Elements

Computer labworks – assembly and BC

Péter Z. Berke

Professor

Building, Architecture and Town Planning (BATir) Dept. CP194/2
Université libre de Bruxelles
E-mail: peter.berke@ulb.be

Louis Remes

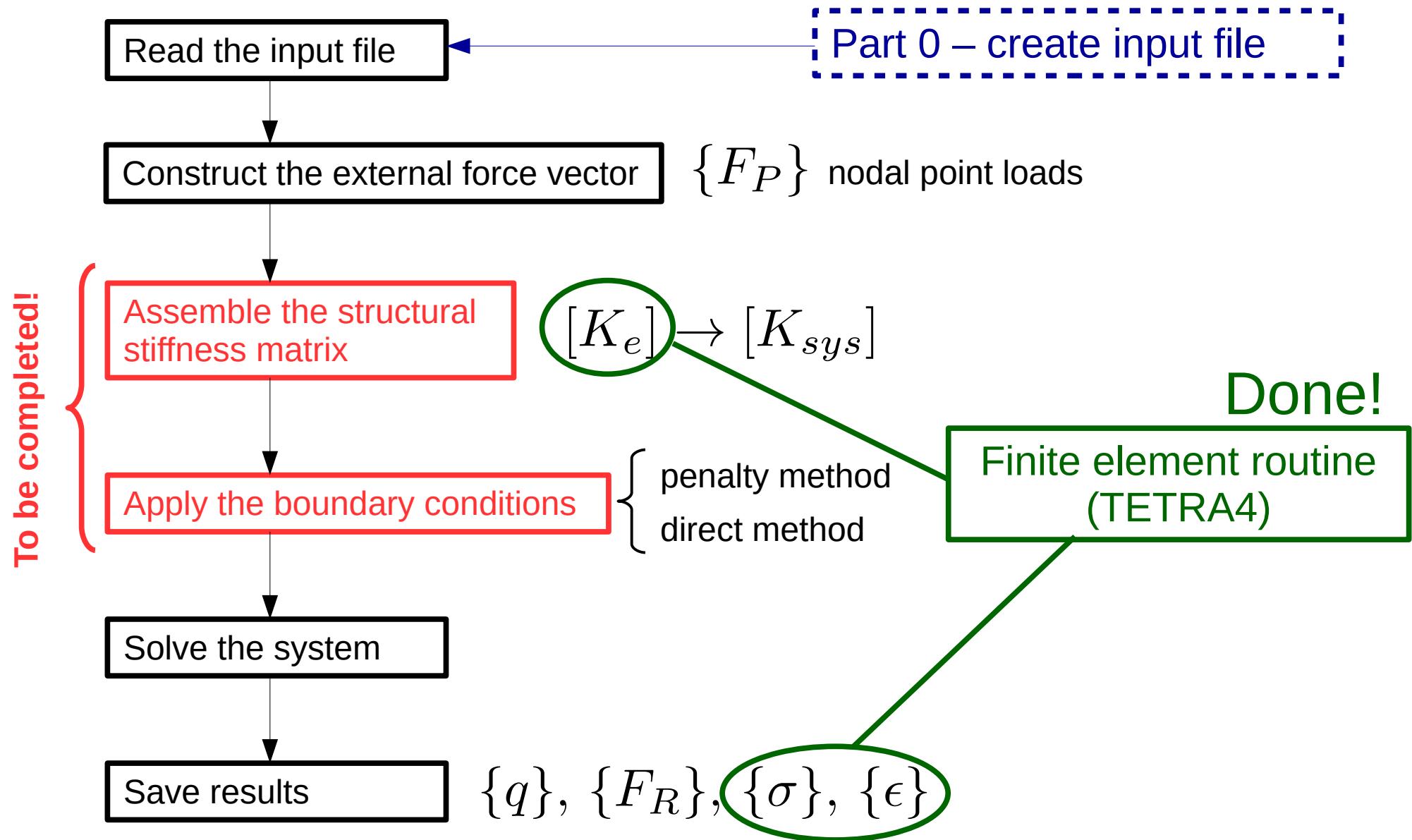
Teaching Assistant
BATir Dept.
Université libre de Bruxelles
E-mail: louis.remes@ulb.be

Noémi Mertens

Teaching Assistant
BATir Dept.
Université libre de Bruxelles
E-mail: Noemi.Mertens@ulb.be

Part I – MatLab FE code

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External force vector

Nodal point loads

$$\mathbf{f}_P = [ndof \times 1] \begin{bmatrix} f_P^1 \\ f_P^2 \\ f_P^3 \\ \dots \\ f_P^4 \end{bmatrix} \quad dofpos = [nnode \times 3] \begin{bmatrix} 1 & 2 & 3 \\ 4 & 5 & 6 \\ 7 & 8 & 9 \\ \dots & \dots & \dots \\ (3 \times nnode - 2) & (3 \times nnode - 1) & 3 \times nnode \end{bmatrix}$$

allows the identification/positioning of the system dof: $f_y^2 \rightarrow f_P^5$

1. Initialize the nodal point load vector

$$\mathbf{f}_P = \mathbf{0}$$

2. Loop on nodf and fill nonzero terms

Assembly of the stiffness matrix

1. Initialize system stiffness matrix to sparse

$$\mathbf{K}_\sigma = \text{sparse}(ndof \times ndof)$$

2. Loop over all elements for element stiffness matrix

$$\mathbf{K}^e = \mathbf{B}^T \mathbf{H} \mathbf{B} V^e$$

Feed the right coordinates
for each element!

Feed the right material
properties for each element!

$$[\mathbf{K}_e]_{[12 \times 12]} \begin{bmatrix} u_1 \\ v_1 \\ \dots \\ v_3 \\ w_3 \end{bmatrix}_e = \begin{bmatrix} f_1^x \\ f_1^y \\ \dots \\ f_3^y \\ f_3^z \end{bmatrix}_e$$

Assembly of the stiffness matrix

2A) Element stiffness matrix:

$$\begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} & \dots & k_{112} \\ k_{21} & k_{22} & k_{23} & k_{24} & \dots & k_{212} \\ k_{31} & k_{32} & k_{33} & k_{34} & \dots & k_{312} \\ k_{41} & k_{42} & k_{43} & k_{44} & \dots & k_{412} \\ \dots & \dots & \dots & \dots & \dots & \dots \\ k_{121} & k_{122} & k_{123} & k_{124} & \dots & k_{1212} \end{bmatrix}_e \begin{bmatrix} u_1 \\ v_1 \\ w_1 \\ u_2 \\ \dots \\ w_3 \end{bmatrix}_e = \begin{bmatrix} f_1^x \\ f_1^y \\ f_1^z \\ f_2^x \\ \dots \\ f_3^z \end{bmatrix}_e$$

expresses how $f_{1,e}^z$ varies as a function of $v_{1,e}$

- quantity with a physical meaning
- same meaning in the system matrix
- “challenge”: find its correct position in \mathbf{K}_σ

Assembly of the stiffness matrix

2B) Positioning of element stiffness matrix terms in \mathbf{K}_σ

$k_{32, e}$ expresses how $f_{1, e}^z$ varies as a function of $v_{1, e}$

$$dofpos = \begin{bmatrix} 1 & 2 & 3 \\ 4 & 5 & 6 \\ 7 & 8 & 9 \\ \dots & \dots & \dots \\ (3 \times nnode - 2) & (3 \times nnode - 1) & 3 \times nnode \end{bmatrix}$$

$$\mathbf{f}_\sigma = \left\{ \begin{array}{c} f_1 \\ f_2 \\ \dots \\ \boxed{f_j} \\ \dots \\ f_n \end{array} \right\}$$

j^{th} line of \mathbf{K}_σ

$$\mathbf{u}_\sigma = \left\{ \begin{array}{c} u_1 \\ u_2 \\ \dots \\ \boxed{u_i} \\ \dots \\ u_n \end{array} \right\}$$

i^{th} column of \mathbf{K}_σ

Assembly of the stiffness matrix

$k_{32, e}$ expresses how $f_{1, e}^z$ varies as a function of $v_{1, e}$

$$\mathbf{u}_\sigma = \begin{Bmatrix} u_1 \\ u_2 \\ \dots \\ u_i \\ \dots \\ u_n \end{Bmatrix} \quad \text{ith column of } \mathbf{K}_\sigma$$

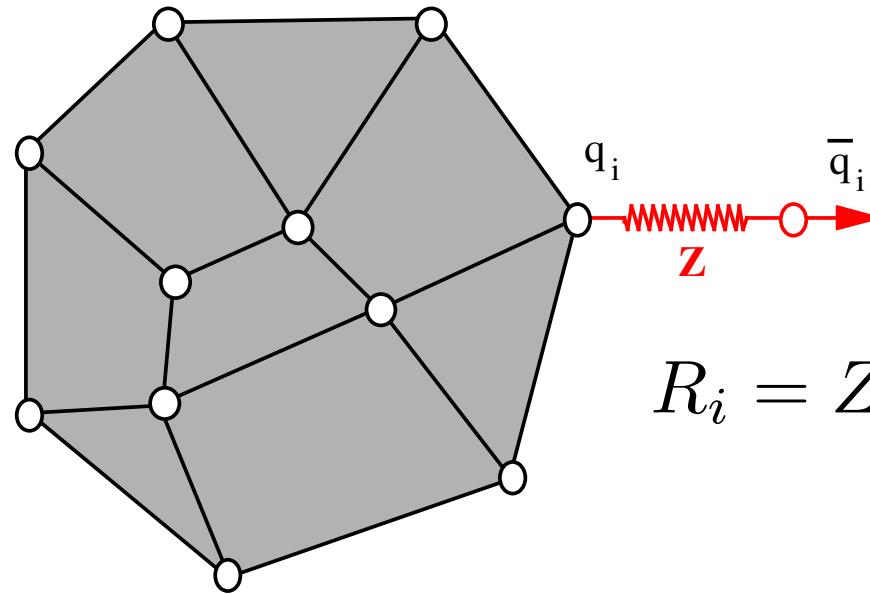
$$\mathbf{f}_\sigma = \begin{Bmatrix} f_1 \\ f_2 \\ \dots \\ f_j \\ \dots \\ f_n \end{Bmatrix} \quad \text{jth line of } \mathbf{K}_\sigma$$

Stiffness contribution to the system stiffness matrix (update formula):

$$\mathbf{K}_\sigma(j, i) = \mathbf{K}_\sigma(j, i) + k_{32, e}$$

naturally considers all element contributions to the stiffness matrix

Application of BC – penalty method



$$R_i = Z (\bar{q}_i - q_i)$$

from pdof

$$\begin{bmatrix} K_{11} & \dots & K_{1i} & \dots & K_{1n} \\ \dots & \dots & \dots & \dots & \dots \\ K_{i1} & \dots & K_{ii} & \dots & K_{in} \\ \dots & \dots & \dots & \dots & \dots \\ K_{n1} & \dots & K_{ni} & \dots & K_{nn} \end{bmatrix} \begin{Bmatrix} q_1 \\ \dots \\ q_i \\ \dots \\ q_n \end{Bmatrix} = \begin{Bmatrix} f_1 \\ \dots \\ f_i + R_i \\ \dots \\ f_n \end{Bmatrix}$$

Application of BC – penalty method

$$\begin{bmatrix} K_{11} & \dots & K_{1i} & \dots & K_{1n} \\ \dots & \dots & \dots & \dots & \dots \\ K_{i1} & \dots & K_{ii} & \dots & K_{in} \\ \dots & \dots & \dots & \dots & \dots \\ K_{n1} & \dots & K_{ni} & \dots & K_{nn} \end{bmatrix} \begin{Bmatrix} q_1 \\ \dots \\ q_i \\ \dots \\ q_n \end{Bmatrix} = \begin{Bmatrix} f_1 \\ \dots \\ f_i + R_i \\ \dots \\ f_n \end{Bmatrix}$$

$$R_i = Z (\bar{q}_i - q_i)$$

from pdof

$$\begin{bmatrix} K_{11} & \dots & K_{1i} & \dots & K_{1n} \\ \dots & \dots & \dots & \dots & \dots \\ K_{i1} & \dots & K_{ii} + Z & \dots & K_{in} \\ \dots & \dots & \dots & \dots & \dots \\ K_{n1} & \dots & K_{ni} & \dots & K_{nn} \end{bmatrix} \begin{Bmatrix} q_1 \\ \dots \\ q_i \\ \dots \\ q_n \end{Bmatrix} = \begin{Bmatrix} f_1 \\ \dots \\ f_i + Z \bar{q}_i \\ \dots \\ f_n \end{Bmatrix}$$

Application of the BC – direct method

1. Group known terms on RHS

$$\left[\begin{array}{cccc} K_{11} & \dots & 0 & \dots & K_{1n} \\ \dots & \dots & 0 & \dots & \dots \\ K_{i1} & \dots & 0 & \dots & K_{in} \\ \dots & \dots & 0 & \dots & \dots \\ K_{n1} & \dots & 0 & \dots & K_{nn} \end{array} \right] \left\{ \begin{array}{c} q_1 \\ \dots \\ q_i \\ \dots \\ q_n \end{array} \right\} = \left\{ \begin{array}{c} f_1 - K_{1i} \bar{q}_i \\ \dots \\ f_i - K_{ii} \bar{q}_i \\ \dots \\ f_n - K_{ni} \bar{q}_i \end{array} \right\}$$

2. “Remove” equation corresponding to the known quantity

$$\left[\begin{array}{ccccc} K_{11} & \dots & 0 & \dots & K_{1n} \\ \dots & \dots & 0 & \dots & \dots \\ 0 & 0 & 1 & 0 & 0 \\ \dots & \dots & 0 & \dots & \dots \\ K_{n1} & \dots & 0 & \dots & K_{nn} \end{array} \right] \left\{ \begin{array}{c} q_1 \\ \dots \\ q_i \\ \dots \\ q_n \end{array} \right\} = \left\{ \begin{array}{c} f_1 - K_{1i} \bar{q}_i \\ \dots \\ \bar{q}_i \\ \dots \\ f_n - K_{ni} \bar{q}_i \end{array} \right\}$$

$$q_i = \bar{q}_i$$