

GENERAL

Read structural drawings in conjunction with all other consultants' drawings, contract documents and specifications.

Check and verify all dimensions, elevations, quantities, and conditions with architectural drawings before commencing with any work. Notify architect and engineer of any discrepancies or inconsistencies. Any discrepancies not reported become the responsibility of the contractor.

The latest edition of the standards and codes referenced in these notes and drawings shall apply.

All dimensions and notes to take precedence over scale shown on plans, sections, and details.

In the event of discrepancies in the specifications, drawings or contract documents, the more stringent requirement shall apply. Contact Quantum Engineering for clarification.

The contractor shall inform Quantum Engineering during the bidding period of any discrepancies or omissions noted on the drawings or in the specifications. The engineer will provide written clarification.

Notes and details specified on the plans and details take precedence over those in the general structural notes, except for minimum requirements.

For conditions not explicitly shown, contractor shall immediately request clarification from Quantum Engineering.

Review of any work or portion thereof by the Engineer, shall not in any way relieve the contractor of any responsibility and obligation to comply with the contract documents and specifications.

Structural information on these drawings do not include any design or detailing of any waterproofing systems.

Architectural and building envelope design, civil, electrical, geotechnical, and mechanical engineering requirements are not included on these drawings and are the responsibility of others.

DESIGN LOADS

This structure has been designed in accordance with Part 4 and Part 9 of the BC Building Code 2012.

The structure has been designed to the following loads:

Uniformly distributed loads (specified loads $l_s, l_w, l_e = 1.0$):

| | LL | DL |
|--------|-------------------|----------------|
| Roofs: | 51.4psf (2.46kPa) | 15psf(0.72kPa) |
| Floor: | 40.0psf (1.9kPa) | 15psf(0.72kPa) |

Importance Category: Normal

Snow: Ss = 3.2kPa, Sr = 0.7kPa

Wind: Q50 = 0.50kPa

Seismic: Sa(0.2) = 0.72 Sa(0.5) = 0.52
Sa(1.0) = 0.30 Sa(2.0) = 0.16 PGA = 0.33
Rd/Ro = 3.0*1.7

CONSTRUCTION

Drawings show the completed structure only. The contractor is responsible for all temporary bracing for all building elements against construction loading conditions, for construction erection procedures, stability until the structure is complete, falsework, shoring, and underpinning of adjacent structures. These shall be designed by a registered Professional Engineer in accordance with WorkSafeBC regulations and required by local building authorities.

All construction to be in accordance with municipal by-laws and the British Columbia Building Code 2012, and WorkSafeBC regulations.

Contractors, suppliers, and subcontractors are responsible they are working with current drawings and for verification of this. All obsolete drawings should be discarded. Do not construct from these drawings unless marked "Issued for Construction".

The contractor is responsible for job site safety and conformance with WorkSafeBC regulations during construction.

All structural work and elements shall be protected during adverse weather.

Contractor to provide Quantum Engineering with a written submission of all proposed alternate products and systems and provide technical specifications if required for review and approval prior to construction.

The contractor shall immediately notify the engineer if site conditions differ from those anticipated or shown on the drawings for corrective or remedial work.

The contractor is responsible for all costs associated with the correction of deficiencies as directed by the Engineer.

FOUNDATIONS

In the absence of a geotechnical engineering report, the foundation design is based on the following assumptions:
Minimum Allowable Soil bearing capacity: 1500psf

Underside of all footings to be minimum 3'-6" below grade for frost cover or at the frost protection depth required by the geotechnical engineer.

Specifications regarding site preparation, excavation, backfill, compaction, fill materials, drainage, etc. are the responsibility of others.

Approval to pour concrete during a field review by Quantum Engineering does not imply assurance of and takes no responsibility for the assumed soil bearing capacity used in this design.

Excavations and soil conditions shall be inspected by the geotechnical engineer prior to construction of formwork. Field report and recommendations to be forwarded to Quantum Engineering Ltd.

Unless otherwise noted, do not backfill against retaining and basement walls until.

The concrete has cured for 14 days and is at 80% of its specified strength. The slab on grade on the lower, opposite side has cured for 7 days and, All intermediate and top floor slabs are constructed and the concrete has cured for 7 days minimum.

Provide minimum 6" layer of free draining compacted backfill under the slab-on-grade. Provide 6mil poly vapour barrier under all slabs-on-grade as indicated on drawings or as per architect or geotechnical engineer.

Footings to be centered under columns and walls unless noted otherwise. All dowels tie be tied in place prior to pouring.

FIELD REVIEWS

Quantum Engineering Ltd. provides field reviews only for the work shown on the structural drawings prepared by Quantum Engineering. These field reviews consist of a periodic review at the professional judgement of Quantum Engineering. The purpose is to ascertain that the work is in general conformance with the structural documents and drawings prepared by Quantum Engineering Ltd and to facilitate completion of the Letters of Assurance required the local Authority having Jurisdiction. These field reviews do not replace any required municipal inspections.

Field reviews are not carried out for the benefit of the contractor(s), nor does the field review make Quantum Engineering guarantor of the contractor(s) work. The contractor(s) is responsible for their own quality control and shall perform the work with good workmanship and in conformance with the contract documents.

The contractor shall provide minimum 24 hours notice to Quantum Engineering for field reviews of the following items:
a. Concrete reinforcement – prior to each concrete pour
b. Masonry reinforcement – prior to each concrete pour
c. Structural steel - prior to concealment
d. Wood framing – prior to concealment

Contractor is responsible for pre-inspecting the work and confirming completeness and conformity with the structural documents prior to field review by Quantum Engineering Ltd.

The work must be complete prior to field review and the contractor shall provide safe access for the engineer. Allow sufficient time for the field review and to execute any corrections.

Any work that is found to be incomplete, poorly executed, contains errors or omissions, unauthorized alterations, and requires additional field reviews and/or remedial design by the Engineer shall be at the expense of the contractor.

All works shall be accessible for review. Failure to provide required notification and accessibility may result in the Engineer requiring removal and replacement of the work at the contractor's expense.

The contractor shall notify the truss manufacturer to review the installation of all trusses and provide a sealed certificate for installation and manufacture prior to installation of any roofing.

Refer to materials sections for inspection requirements related to specific materials.

SECONDARY & NON-STRUCTURAL ELEMENTS

Quantum Engineering Ltd is not responsible for the structural design of non-structural and secondary building elements and their connection to the primary structure. These elements include, but are not limited to:

- All glazed component systems including windows, storefronts, curtain walls, skylights, canopies, and guards.
- Attached and free standing sign structure.
- Roofing systems and wall cladding systems.
- Interior non-load bearing walls
- Ceiling systems
- Movable curtain/door systems
- Storage shelving and racking systems
- Guards, handrails, and guard systems
- Elevators, escalators, lifts and dock levelers.

Secondary building components shall be designed to part 4 of the Building code for gravity and lateral loads, designed and inspected by the specialty Professional Engineer retained by the contractor. Specialty engineers to submit letters of assurance, sealed shop drawings, and perform appropriate field reviews.

Submit shop drawings to Quantum Engineering for review at least three weeks prior to fabrication. Shop drawings to indicate all design assumptions, loads, and loads imposed on building structure, and connection details.

Shop drawings must be signed and sealed by a BC registered Professional Engineer for structural design.

CONCRETE

Concrete design is in accordance with Part 9 of the BCBC 2012 and CAN/CSA-A23.3. All concrete work shall conform to the requirements of CAN/CSA-A23.1 and CAN/CSA-A23.2-04

| Concrete Mix Requirements: | 28 day strength (MPa) | Air (%) | Slump +3/4" | Exp class | Agg size |
|----------------------------|-----------------------|---------|-------------|-----------|----------|
| Footings | 25 | 4-7% | 3" | - | ¾" |
| Ext. columns & Walls | 25 | 4-7% | 3" | F2 | ¾" |
| Int. S.O.G | 32 | 1-4% | 3" | - | ¾" |
| Ext. S.O.G | 32 | 5-8% | 3" | C2 | ¾" |
| Suspended slab | 30 | 1-4% | 3" | - | ¾" |

All concrete shall be normal weight, 150 pcf, Type 10 cement, Type F flyash, use 20mm (3/4") aggregate for all concrete unless noted otherwise. Slumps noted are before the addition of superplasticizer. Do not add water in the field for workability beyond the amount specified in the mix design. Do not use admixtures other than superplasticizer, air entrainment admixture or standard water reducers without prior approval from engineer.

Contractor shall submit concrete mix designs to the material testing agency and the Engineer for review prior to commencing work.

Concrete will be rejected where time between batching and placing exceeds 2hrs.

Concrete testing shall be performed in accordance with CAN/CSA-A23.2-04 by an independent material testing agency retained by the owner. Conduct concrete tests (minimum of 3 test cylinders) for each mix design and pour, and for each 100 cubic meters. Test 1 cylinder at 7 days and 2 cylinders at 28 days. One additional cylinder in each set shall be field cured and tested at 7 days for all suspended slabs, lift up panels, and concrete poured in temperatures below 5 degrees Celsius. Submit copies of all test results to engineer, maximum 48 hours after test.

Provide ¾" chamfer on all exposed concrete corners. Unless noted otherwise all slabs and beams shall be cambered 1/8" for each 8ft of span.

All hot and cold weather concrete work shall conform to CSA A23.1. If the temperature is expected to be 5 degrees Celsius or colder within 3 days of placing concrete, the contractor shall submit provision for cold weather concrete work to the engineer for approval.

Contractor is responsible for design of formwork, shoring and reshoring of suspended concrete elements in conformance with WorkSafeBC regulations. Submit pour joint locations to Engineer for review prior to construction.

Do not remove forms for footings, walls and columns until the concrete strength has reached 10MPa and has cured for 24 hours minimum. Leave formwork in place for beam soffits, slabs and decks for a minimum of 10 days. Quantum Engineering to review the 7 day concrete test cylinder results to ensure tests meet or exceed 70% of the specified 28 day compressive strength.

Do not wet dowel reinforcing, anchor bolts and other embedded components in concrete. Accurately install and tie all reinforcing, embed plates, anchor bolts, etc. unless noted otherwise.

Remove all construction debris from forms or slab prior to pouring concrete.

CONCRETE REINFORCING

All concrete reinforcing shall be deformed bar conform to CAN/CSA-G30.18-M92. All reinforcing to be welded shall be grade 400w.

All reinforcing shall be clean, undamaged, and free of corrosion. Any questionable reinforcing shall be cleaned or replaced at the contractors expense to the engineers satisfaction.

Contractor shall provide reinforcing for all concrete elements shown on all drawings and specifications, and carrying bars as required for bar placement. Reinforcing shall also be provide in all unspecified concrete elements.

| Minimum laps of reinforcing unless indicated otherwise to be: | | | | |
|---|-------------|----------------|-------|-------|
| Bar size | Comp splice | Tension Splice | 25MPa | 30MPa |
| 10M | 12" | 18" | 18" | |
| 15M | 20" | 24" | 24" | |
| 20M | 24" | 30" | 28" | |
| 25M | 30" | 48" | 44" | |
| 30M | 36" | 58" | 52" | |
| 35M | 42" | 68" | 64" | |

No splices permitted without engineer's approval where the length of bars has been specified on the drawing.

All reinforcing to be accurately placed, chaired and securely tied to restrict displacement and to maintain the specified concrete cover during construction. Use concrete bricks to chair footing and slab on grade reinforcing. Space bricks at max 4ft grid to support slab-on-grade reinforcing. Chairs for reinforcing in exposed soffits or other areas exposed to weather, parking, swimming, and view shall be plastic or epoxy coated.

Reinforcing to have clear concrete cover as follows or as noted on the drawings:
a. Concrete cast against earth (bottom of footings): 3"
b. Formed concrete exposed to earth (sides and tops of footings: 2"
c. Beams, columns and plasters exposed to weather or earth: 2"
d. Interior face of walls: 1"
e. Beams and columns to stirrups/ties: 1.5"

All ends of walls shall have two extra 15M vertical bars or as noted on the drawings. All openings in slabs and walls to have two 15M extra bars extending 24" past the corners plus one 15M x 48" long diagonal each corner, unless noted otherwise.

Dowels in footings to match size and spacing of vertical bars above unless noted otherwise.

Unless shown otherwise:

- a. Top reinforcing in slabband to be centered over columns
- b. Top slab reinforcing to be centered over slabband
- c. Bottom reinforcing to be centered between supports except at end spans where all bars shall extend a minimum of 6" into exterior supports

Reinforcing steel on drawings shown thus,
Top Bars - Solid line
Bottom Bars - Broken Line

SHOP DRAWINGS

Refer to the prime consultants documents and drawings for items requiring shop drawings.

Submit no more than 4 hard copies of shop drawings and allow minimum of 10 working days for review by the Engineer.

Shop drawings shall clearly indicate the supplier's company, detailers' information, drawing date, material lists, member arrangement, dimensions, assembly information, applicable codes and standards, finish, etc.

The supplier, subcontractor and specialty engineer are responsible for dimensions, detailing, engineering design and field inspections of the installed components.

Hand sketched shop drawings will be rejected.

Shop drawings for structural components shall be signed and sealed by the supplier's specialty Professional Engineer.

The specialist engineer must be experienced with the associated component, registered in BC and in good standing with APEGBC. Upon completion of the work in the field, the specialty engineer shall submit to the general contractor and engineer a signed and sealed letters of assurance certifying conformity of the work to the contract documents.

Printing costs for shop drawings sent to Quantum Engineering by email or fax will be charged to the respective subcontractor, contractor and/or project owner.

WOOD FRAMING

Lumber grading to NLGA standard grading rules for Canadian lumber.

All framing details shall conform to BCB 2012 sections 9.23 and 9.24.

Provide continuous cross bridging between floor joists at maximum 7'-0" on center.

All framing members to be S-P-F, Kiln Dried, No.1/No.2 unless noted otherwise.

D,Fir-L shall not be used for studs, joists, beams, or rafters unless specifically noted.

All drop beams, lintels and flush beams to be minimum 2-2x10 KD SPF No.1/No.2 unless noted otherwise.

All built up beams and headers to be laminated together with 3 rows of 3 ½" nails at 16" o/c per lamination. Cantilevered beams greater than 2-ply nail together with 3 rows of 3 ½" nails at 6" o/c each ply.

All beam splices are to occur at supports, uno.

Where sheathing fastened to built up posts, fasten sheathing to each ply of the post with minimum 2 ½" nails at 6" o/c and laminate each ply with 2 rows of 3" nails at 8" o/c

| Laterally unsupported built up posts to shall be laminated as follows: | | |
|--|---------|--------------------------------|
| 2 ply | 2x4 | 3" nails at 8" o/c staggered. |
| | 2x6/2x8 | 2 rows 3" nails at 8" o/c. |
| 3ply | 2x4 | 4 ½" nails at 8" o/c staggered |
| | 2x6/2x8 | 2 rows 4 ½" nails at 8" o/c |
| 4-ply | 2x4 | 6" nails at 8" o/c staggered |
| | 2x6/2x8 | 2 rows of 6" nails at 8" o/c. |

Provide solid blocking @ 24" o/c at locations where bearing walls run parallel with the joist span.

All steel fasteners in contact with ACQ or CA pressure treated wood shall be either stainless steel or hot-dip galvanized in accordance with ASTM A653, G185 designations, or hot dip galvanized after manufacture in accordance with ASTM A123.

STRUCTURAL STEEL

Fabrication, erection, design & detailing shall be in accordance with CSA-S16. Welding shall be in accordance with CSA W47.1. General requirements in accordance with CAN3-G40.20. Workmanship to CAN3G40.21. Primer to CGSB 1-GP-40d requirements.

All welding shall conform to CSA W59, W55.3, and W186. All welding shall be performed by fabricators fully certified by the Canadian Welding Bureau to the requirements of W47.1. All welders shall be CWB certified.

Submit clear copies of the shop and welders' certificates to the Engineer along with the shop drawings.

Supplier shall confirm all dimensions and site conditions prior to fabrication.

Submit shop drawings for steel fabrications to Quantum Engineering and the architect for review prior to fabrication. Show all pertinent details, material specifications on the shop drawings. All shop drawings to be sealed and signed by the fabricators Professional Engineer for the connection design.

The Professional Engineer sealing the shop drawings shall be responsible for inspections the steel installation for conformance with their design and shop drawings. Upon completion the specialty engineer shall provide a Schedule S certifying substantial conformance to the Engineer of Record.

| Materials: | |
|-----------------------------------|-------------------|
| • All structural steel | 350W |
| • Hollow Structural sections, HSS | 350W, Class C |
| • Plates, angles, channels | 300W |
| • Structural Pipe | ASTM A53 (240MPa) |
| • Bars, Rods, sheets & misc metal | 300W |
| • Bolts, nuts and washers | ASTM A325 |
| • Anchor bolts | ASTM A307 |
| • Welds | E-70xx (480MPa) |

All edges and corners of connections shall be ground smooth.

Apply minimum one coat of shop primer to all steel work. Use weldable primer where further welding is anticipated.

Cap all open ends of pipes, tubes and HSS sections with 4.8mm (3/16") thick steel plates with seal welds all around.

Any steel subject to corrosion shall be hot dip galvanized. All hot dip galvanizing to be in accordance with CAN/CSA G164.

Refer to architectural, mechanical, and electrical drawings for roof top units and openings. Reinforce openings with welded L150x150x6 angles spanning to adjacent supports. Verify unit and opening sizes with shop drawings.

All beams to have minimum one 10mm (3/8") full height web stiffener on both sides centered about support/supported columns. Increase thickness of stiffeners for deeper beams to resist buckling and bending. Use double stiffeners where specified by the specialty engineer.

Provide a positive dead load camber to all simple span floor beams. Camber to be L/460, where L is the member length.

Grout under all baseplates with a non-shrink flowable, high-strength grout. Ensure full area is grouted.

Erection bracing during construction is the responsibility of the contractors.

SHEATHING & TIMBER

All D,Fir plywood shall conform with requirements in CAN/CSA O121 and OSB to CAN/CSA O437.

| Minimum sheathing thicknesses: | | |
|--------------------------------|-----------|--------------------|
| Location | Grade | Thickness(min) |
| Roof | D,Fir/OSB | ½" + H clips/9.5mm |
| Floor | D,Fir/OSB | 5/8" T&G/15.5mm |
| Walls | D,Fir/OSB | 3/8"/9.5mm |

Orient floor and roof sheathing with face grain perpendicular to joists. Stagger panel joints.

Minimum nailing of sheathing (UNO): 2 ½" nails at 6" o/c at sheet edges, 2 ½" nails at 12" o/c at intermediate support members. Staples are not permitted.

All sawn timber exposed to the exterior or in contact with concrete to be given a preservative treatment approved by the designer.

All sawn timber to be graded by a certified member of the National Lumber Grading Association, in accordance with the Canadian Lumber Standards Accreditation Board.

Minimum rough sawn timber grades to be No.1 unless noted otherwise.

Engineered Wood Products & Glulam Beams

Engineered wood products to be manufactured to CSA standards by Truss Joist (weyerhaeuser) or Louisiana Pacific.

Alternative manufacturers will be allowed if supporting documentation, sealed by a Professional Engineer registered in BC, are submitted to Quantum Engineering for review.

Supplier to provide layout drawings showing location and specifications for all engineered wood products.

Paralam (PSL) - Parallel Strand Lumber.
E = 2.0 x 10 psi / Fb = 2950 psi, Fc(perp) = 1365psi
E = 2.2 x 10 psi / Fb = 2950 psi

Gang-Lam (LVL) - Laminated Veneer Lumber.
E = 2.0 x 10 psi / Fb = 2950 psi, Fc(perp) = 1857psi
E = 1.8 x 10 psi / Fb = 2650 psi, Fc(perp) = 1365 psi
E = 1.9 x 10 psi

Fasten all laminations together as per manufacturer's specifications.

Glulam

All glulam beams shall be manufactured at a plant approved by the CSA under the requirements of CSA O177.

Upon request, supply the purchaser with a certificate confirming that the material has been manufactured in accordance with CSA O122 and according to the design requirements of CSA O86.1-M04.

PREFABRICATED WOOD TRUSSES / JOISTS

Design and fabricate trusses in accordance with:

- Part 4 of the Building code, CAN/CSA O86 and O86.1, and TPIC standards
- Design loads and design criteria as indicated on the structural drawings
- Western Wood Truss Association of British Columbia Quality Assurance Program

The truss manufacturer shall design trusses and joists for mechanical unit weights as specified by the mechanical consultant and contractor.

Live load deflections shall not exceed L/360, where L is the clear span of the truss.

The truss supplier's engineer shall design and supply all steel connections required for: Truss to truss connections, and truss to supporting structure connections.

Truss tie-down clips to be provided at ends of all trusses as specified by Engineer.

Supplier's truss systems engineer is responsible for the design of all bridging, blocking, bracing, and metal connections, required for the stability of the truss or joist.

All proposed site modifications and/or damage to trusses shall be reviewed by the supplier's truss systems Engineer who shall issue written instruction and repair details as required. Notify Quantum Engineering accordingly.

Suppliers shall provide the following submittals:

- Shop drawings sealed and signed by the truss supplier's truss systems Engineer who must be registered in BC.
- Truss layout showing location of trusses and all design loads including snow drift and unbalanced snow diagrams, and wind loads.
- Detailed design/fabrication of each truss, clearly showing design loads, member forces, deflections, camber details, lateral bracing etc.
- Truss erection instruction and details indicating required temporary and permanent bracing and connections.
- Site inspection report by the supplier's truss systems Engineer for the erection of the trusses.
- Letters of Assurance for design and installation of prefabricated wood trusses by the Supplier's truss systems engineer.

Refer to the Shop Drawings section of these notes for additional requirements for the submittals. Allow minimum 10 working days for review by the Engineer prior to beginning fabrication.

TYPICAL ABBREVIATIONS

| | |
|--------------------------------|----------------------------------|
| AB = anchor bolt | LL =live load |
| ALT = alternate | LSL = timberstrand |
| ARCH = architectural | LVL = microlam |
| BLL = bottom lower layer | MAX = maximum |
| BUL = bottom upper layer | MI = factored moment |
| BM = beam | MFR = manufacturer |
| B/S = both sides | Min = minimum |
| B/U = built up | NS = near side |
| CANT = cantilever | N STUD = nelson/shear stud |
| CONC = concrete | NTS = not to scale |
| CL = center line | o/c = on center |
| COL = column | PSL = Paralam |
| CONT = continuous | PT = pressure treated |
| CP = complete penetration | RS = rough sawn |
| CSK = countersink | REINF = reinforcing |
| C/W = complete or connect with | R/W = reinforce with |
| DL = dead load | SEL = select grade |
| DP = deep | SIM = similar |
| DWG = drawing | S.O.G. = slab on grade |
| EA = each | SST = Simpson Strong Tie |
| EE = each end | STAG = stagger |
| EF = each face | TLL = top lower layer |
| ELEV = elevation | TF = factored tension |
| EXT = exterior | T&G = tongue and groove |
| FDN = foundation | T.O. = top of |
| FO = face of | TUL = top upper layer |
| FS = far side | TRANS = transverse |
| GALV = hot dip galvanized | TYP = typical |
| GL = glulam | UDL = uniformly distributed load |
| H1E = hook one end | UNO = unless noted otherwise |
| H2E = hook two ends | U/S = underside |
| HDR = header | VERT = vertical |
| HORIZ = horizontal | Vf = factored shear |
| INT = interior | W/ = with |
| KD = kiln dried | W/O = without |
| LG = long | |



Quantum Engineering Ltd.
207 - 46167 Yale Road
Chilliwack, BC, V2P 2P2
Tel: 604.703.1605
Fax: 604.909.1963
Email: info@quantumeng.ca

Project:
KITWANGA RESIDENCE

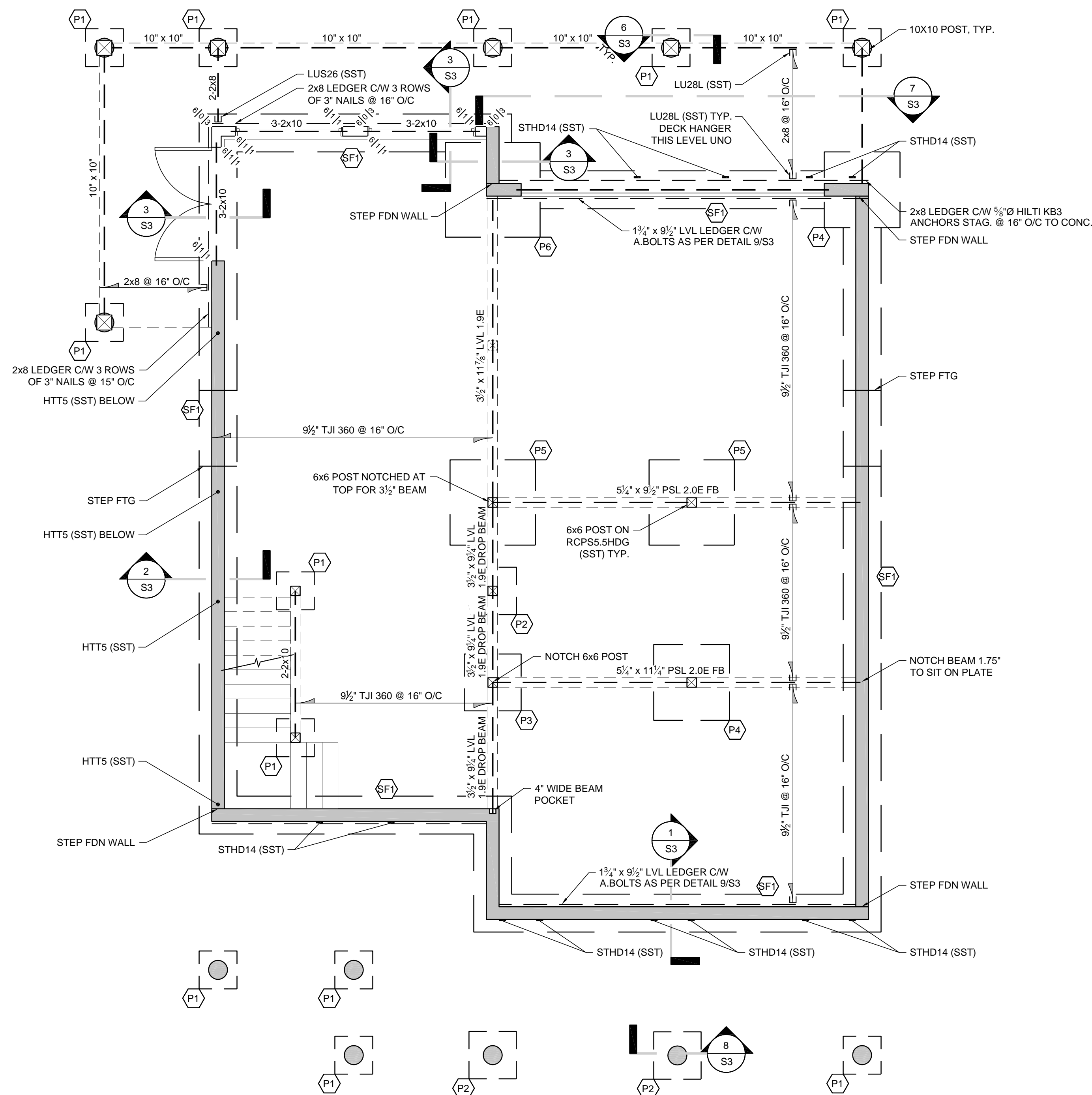
2939 STRANGWAY PL.
SQUAMISH, BC

Client:
TRINITY POST & PANEL

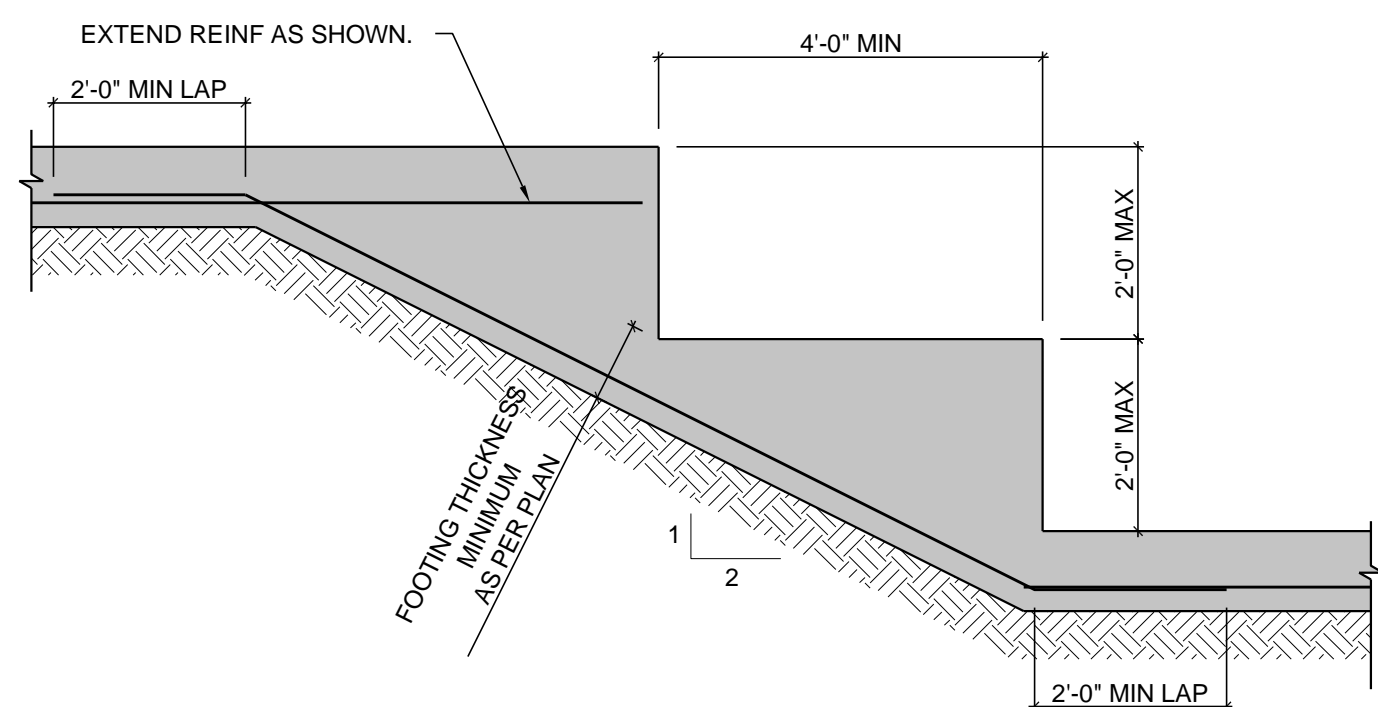
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STRUCTURAL NOTES

Scale:
AS NOTED

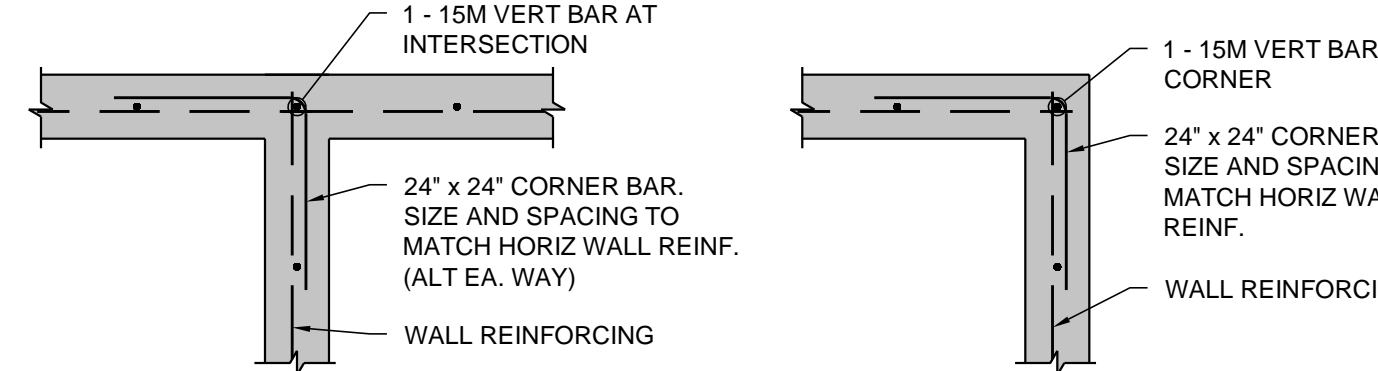
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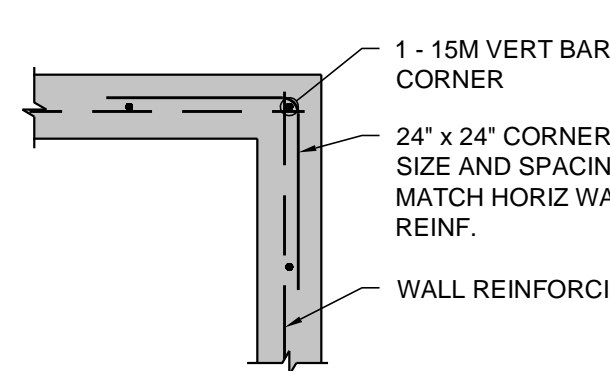
1 FOUNDATION PLAN + MAIN FLOOR FRAMING
SCALE: 1/4"=1'-0"



A TYP. STEPPED FTG SECTION
SCALE: 1/2"=1'-0"

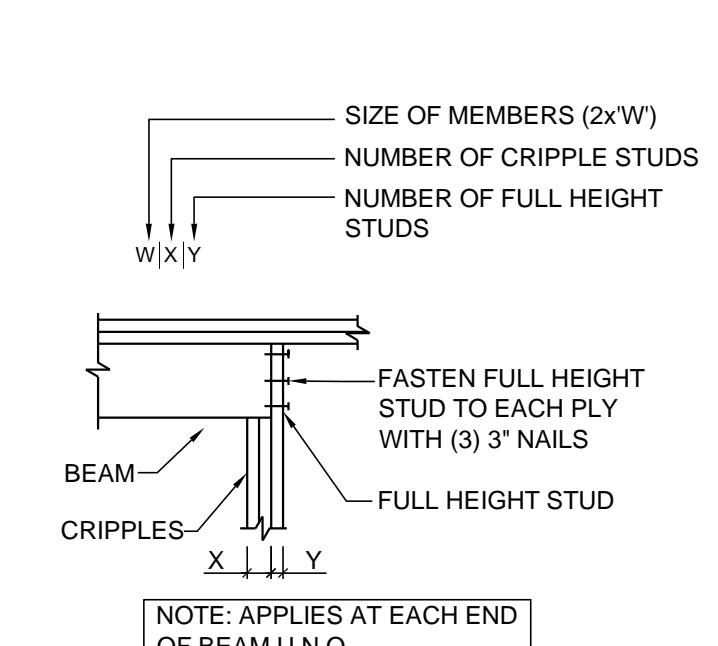


B TYP. 'T' JUNCTION DETAIL
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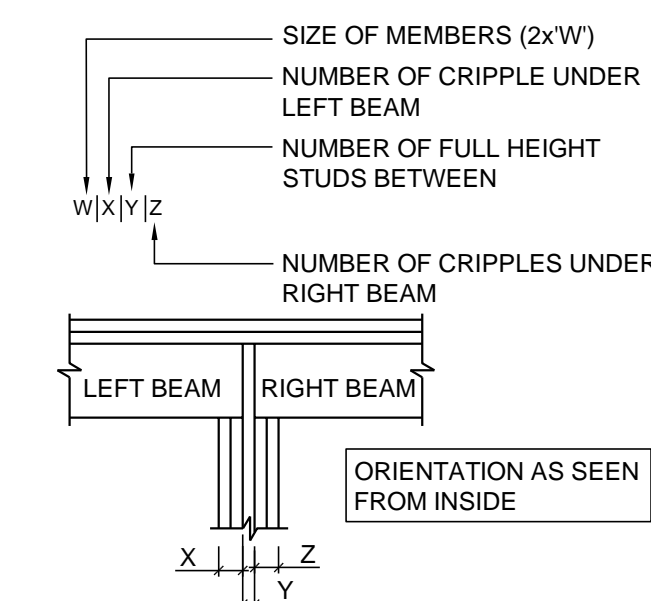


C TYP. CONC. CORNER DETAIL
SCALE: 1/2"=1'-0"

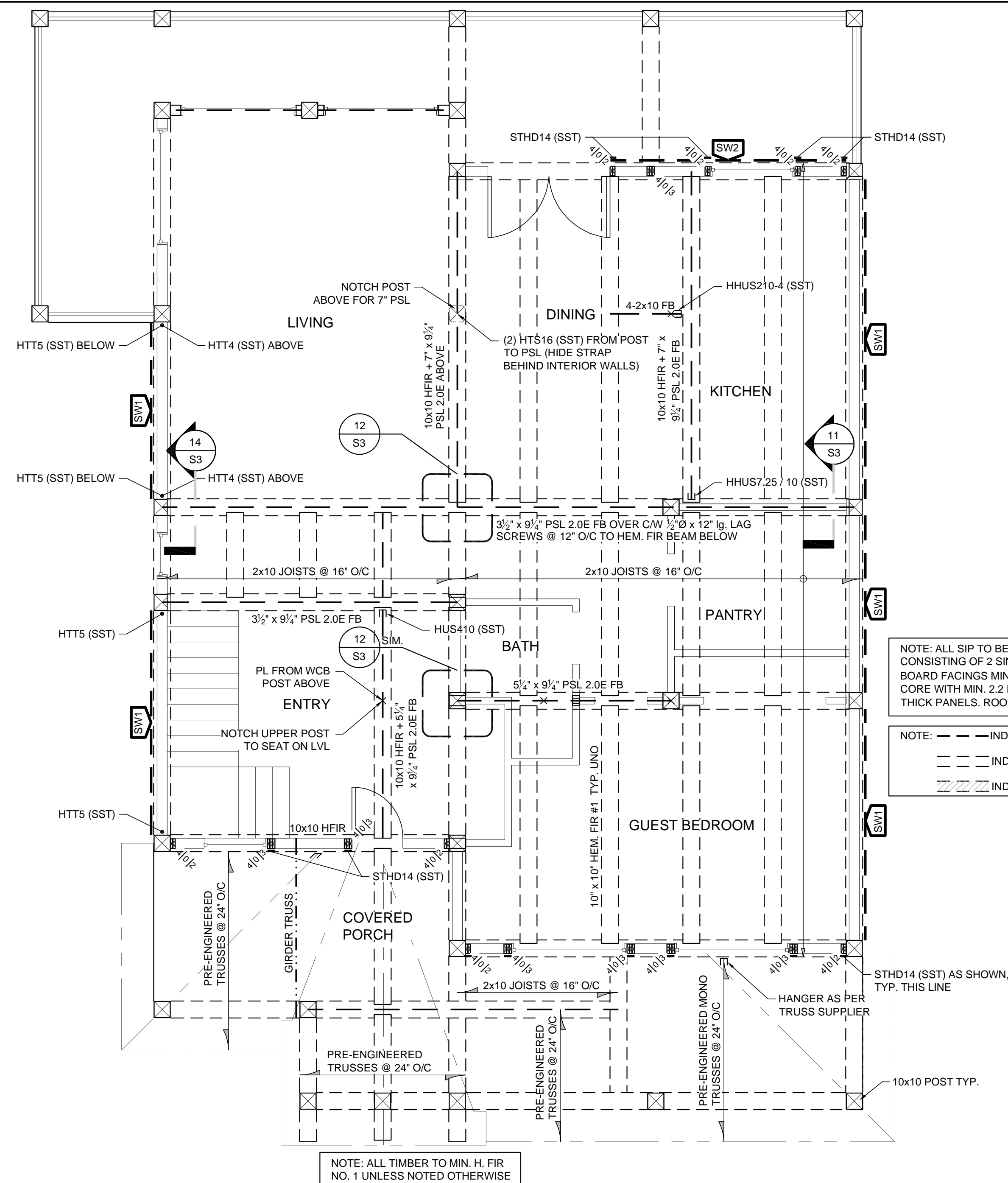
| FOOTING SCHEDULE | | |
|------------------|-----------------------------|---------------------|
| MARK | SIZE | REINFORCING |
| (SF1) | 2'-0" x 8" dp. STRIP | 2-15M CONT. |
| P1 | 2'-0" x 2'-0" x 8" dp. PAD | 2-15M x 20" lg. E/W |
| P2 | 2'-6" x 2'-6" x 8" dp. PAD | 3-15M x 26" lg. E/W |
| P3 | 3'-0" x 3'-0" x 10" dp. PAD | 3-15M x 32" lg. E/W |
| P4 | 4'-0" x 4'-0" x 10" dp. PAD | 4-15M x 44" lg. E/W |
| P5 | 4'-6" x 4'-6" x 10" dp. PAD | 5-15M x 50" lg. E/W |
| P6 | 5'-0" x 5'-0" x 12" dp. PAD | 5-15M x 56" lg. E/W |



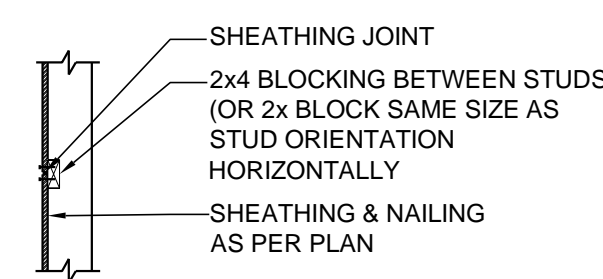
D TYP. BUILT-UP POST DETAIL
SCALE: 1/2"=1'-0"



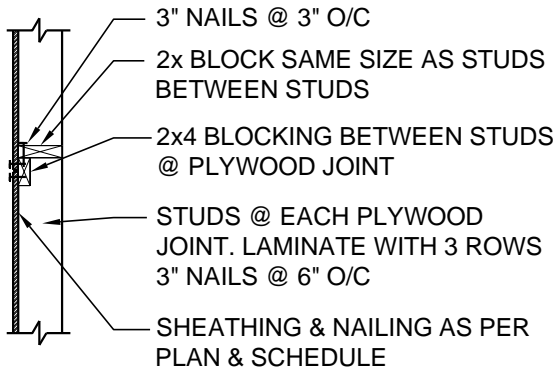
E TYP. BUILT-UP POST DETAIL
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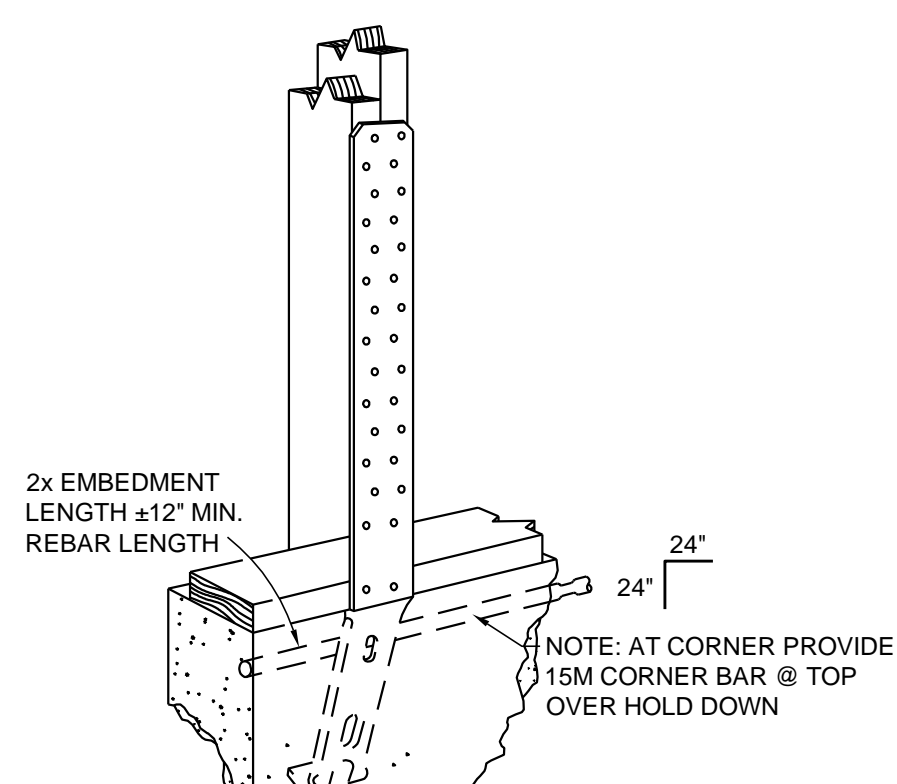
2 MAIN FLOOR PLAN + SECOND FLOOR FRAMING
SCALE: 1/4"=1'-0"



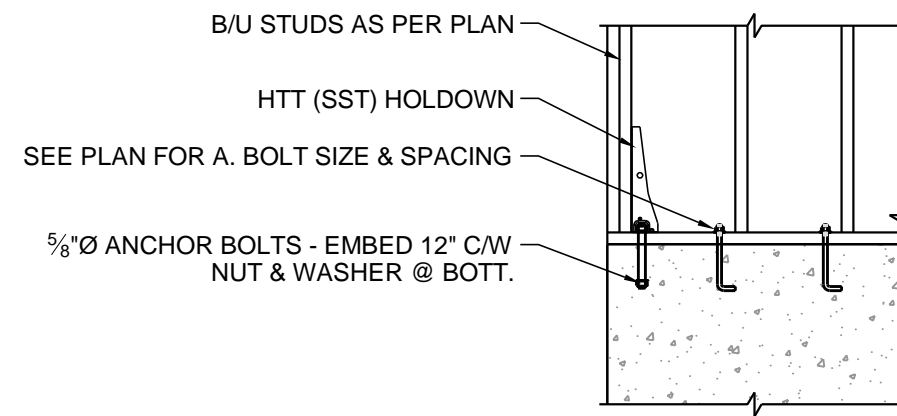
F TYP. SHEARWALL BLOCKING (SINGLE)
SCALE: 1/2"=1'-0"



G TYP. SHEARWALL BLOCKING (DOUBLE)
SCALE: 1/2"=1'-0"



H STHD10 INSTALLATION (SST)
SCALE: NOT TO SCALE



J TYP. HTT HOLD DOWN DETAIL
SCALE: 1/2"=1'-0"

NOTE: ALL SIP TO BE THERMOCORE BUILDING PANELS CONSISTING OF 2 SINGLE PLY ORIENTED STRAND BOARD FACINGS MIN. 7/16" THICK AND POLYURETHANE CORE WITH MIN. 2.2 PCF DENSITY. WALLS TO BE 4 1/2" THICK PANELS. ROOFS TO BE 6 1/2" THICK PANEL.

NOTE: --- INDICATES BEAM AT LOWER LEVEL
--- INDICATES INTERMEDIATE BEAM
--- INDICATES BEAM AT UPPER LEVEL

QUANTUM ENGINEERING LTD.
207 - 46167 Yale Road
Chilliwack, BC, V2P 2P2
Tel: 604.703.1605
Fax: 604.909.1963
Email: info@quantumeng.ca

| | | |
|-----------|--------------------|-------------|
| 1 | FOR PERMIT/CONSTR. | Aug. 13, 13 |
| NO. | DESCRIPTION | DATE |
| REVISIONS | | |

Project: **KITWANGA RESIDENCE**

2939 STRANGWAY PL.
SQUAMISH, BC

Client: **TRINITY POST & PANEL**

Title: **FOUNDATION PLAN
MAIN FLOOR FRAMING
MAIN FLOOR PLAN
2ND FLOOR FRAMING**

Scale: **AS NOTED**

Drawn: **MS**

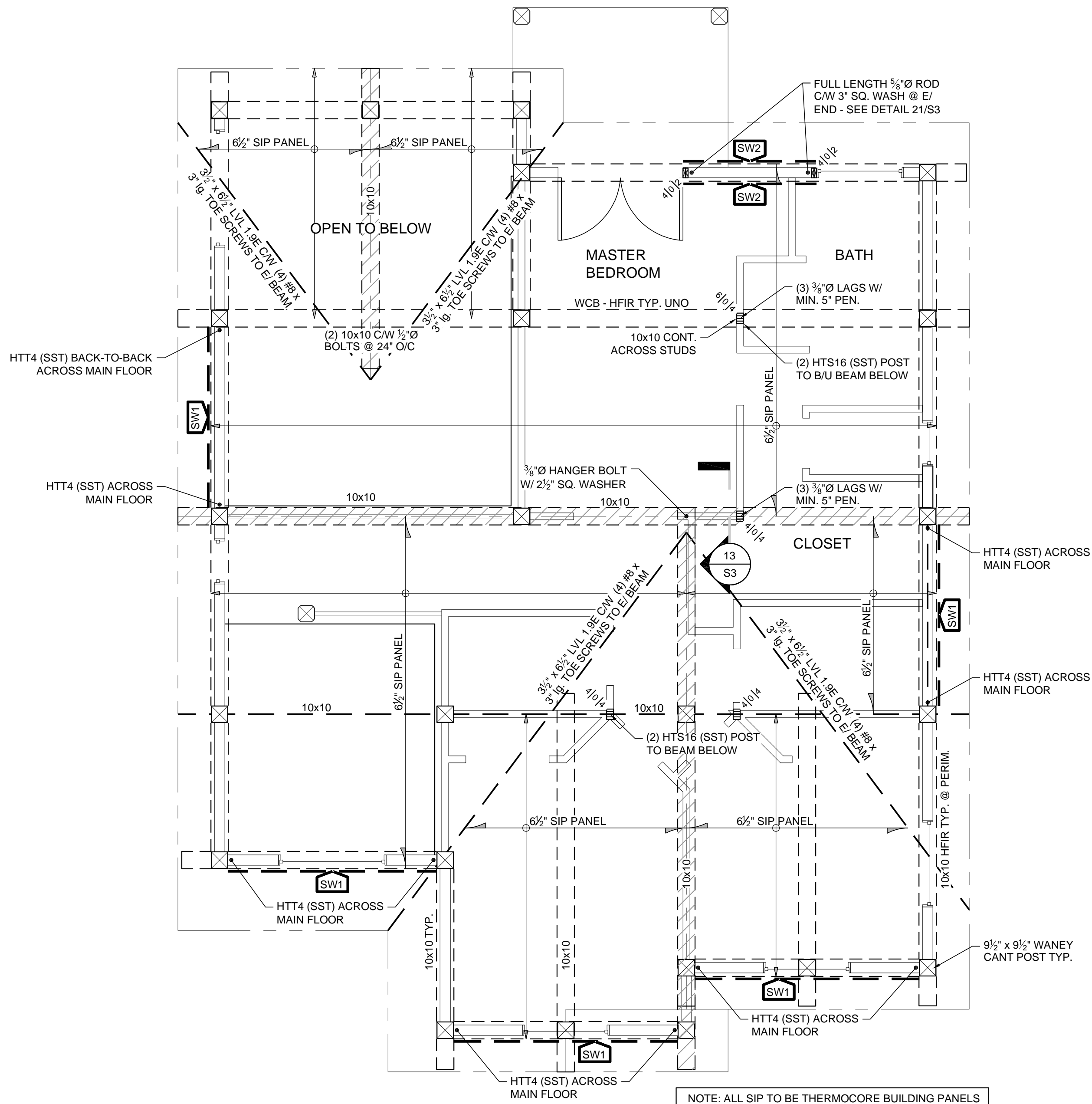
Checked: **JK**

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Printed: **August 13, 2013**

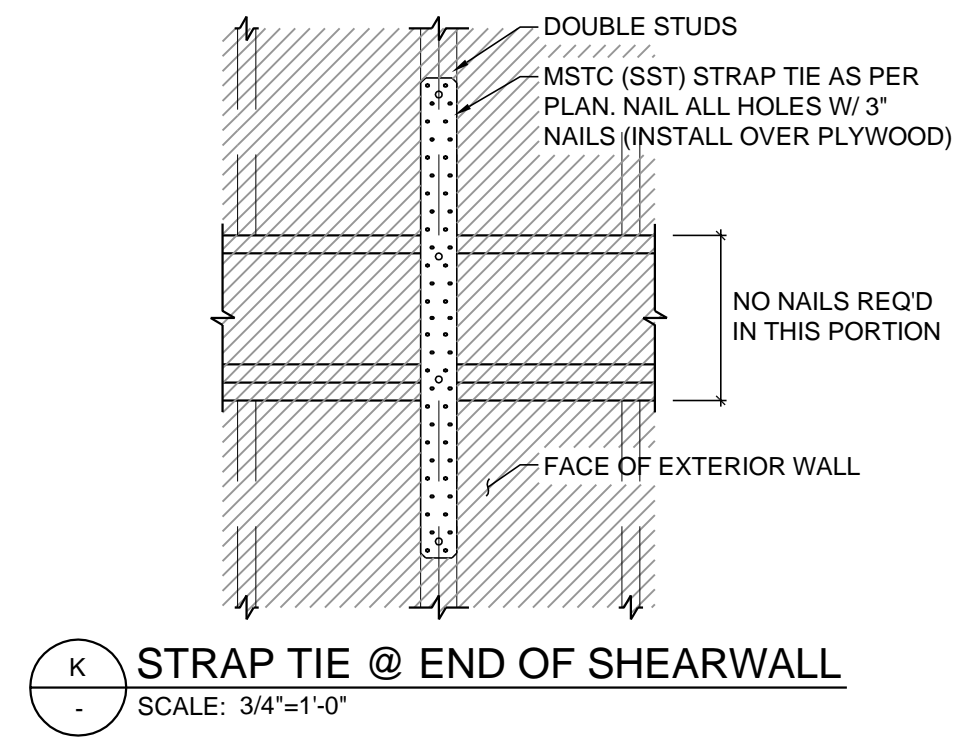
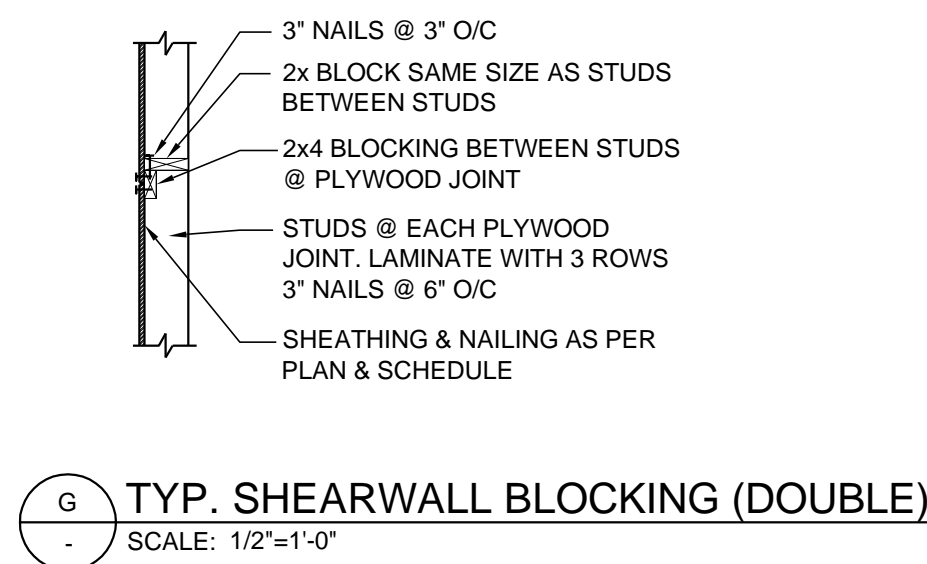
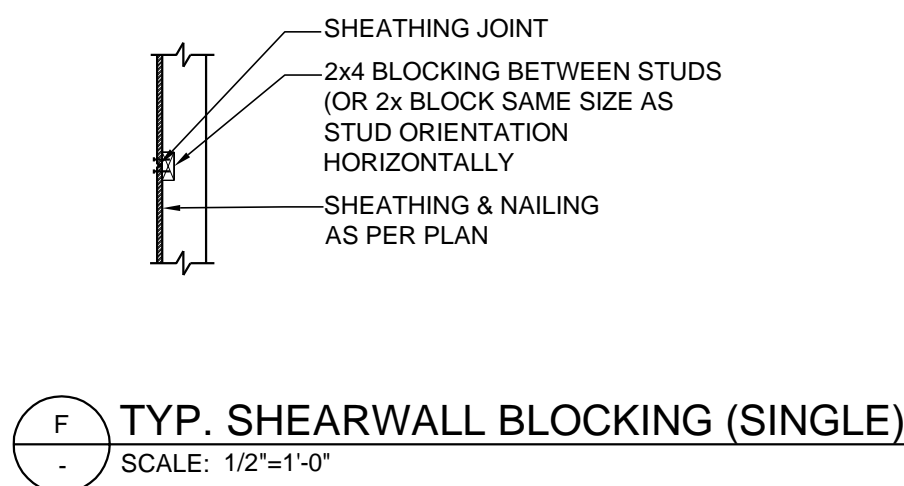
Project No: **13.207**

Sheet No: **S2.1**



| SHEARWALL SCHEDULE | | | | | |
|--------------------|---------------|----------|------------------|--------------|-------------|
| TYPE | PLYWOOD | BLOCKING | NAILS | NAILING EDGE | NAILING INT |
| SW1 | 7/16" OSB E/S | SINGLE | 2 1/2" x 0.128"Ø | 4" O/C E/S | 12" O/C E/S |
| SW2 | 7/16" OSB E/S | SINGLE | 2 1/2" x 0.128"Ø | 3" O/C E/S | 6" O/C E/S |

| HOLD DOWN SCHEDULE | |
|--------------------|----------------|
| MARK | MODEL |
| ① | MSTC48B3 (SST) |
| ② | MSTC66 (SST) |
| ③ | - |
| ④ | - |



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ENGINEERING LTD.
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Tel: 604.703.1605
Fax: 604.909.1963
Email: info@quantumeng.ca

Project:
KITWANGA RESIDENCE
2939 STRANGWAY PL.
SQUAMISH, BC

Client:
TRINITY POST & PANEL

Title:
**2ND FLOOR PLAN
ROOF FRAMING**

Scale: AS NOTED

Drawn: MS

Checked: JK

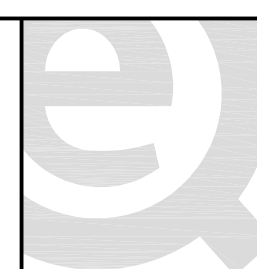
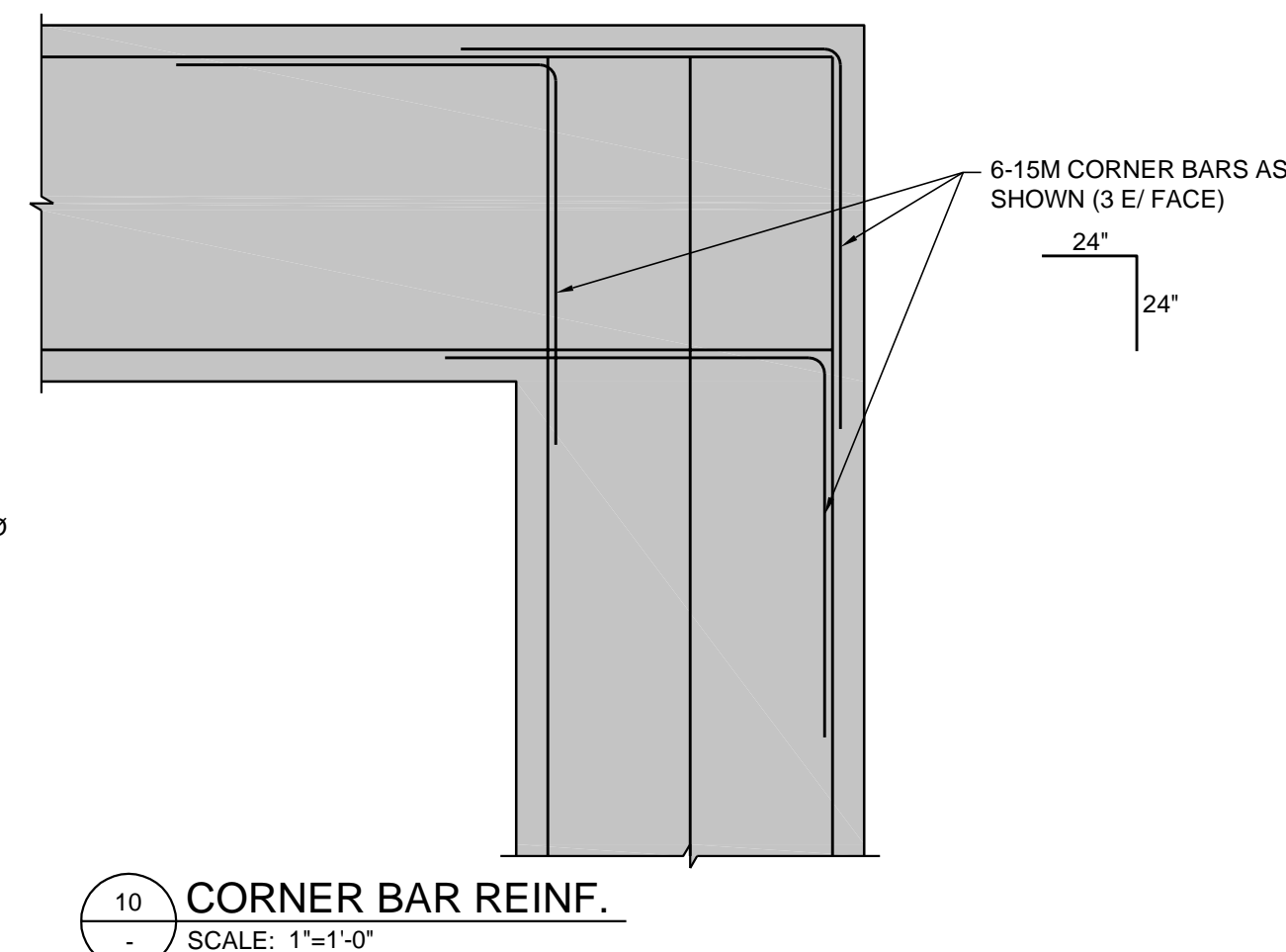
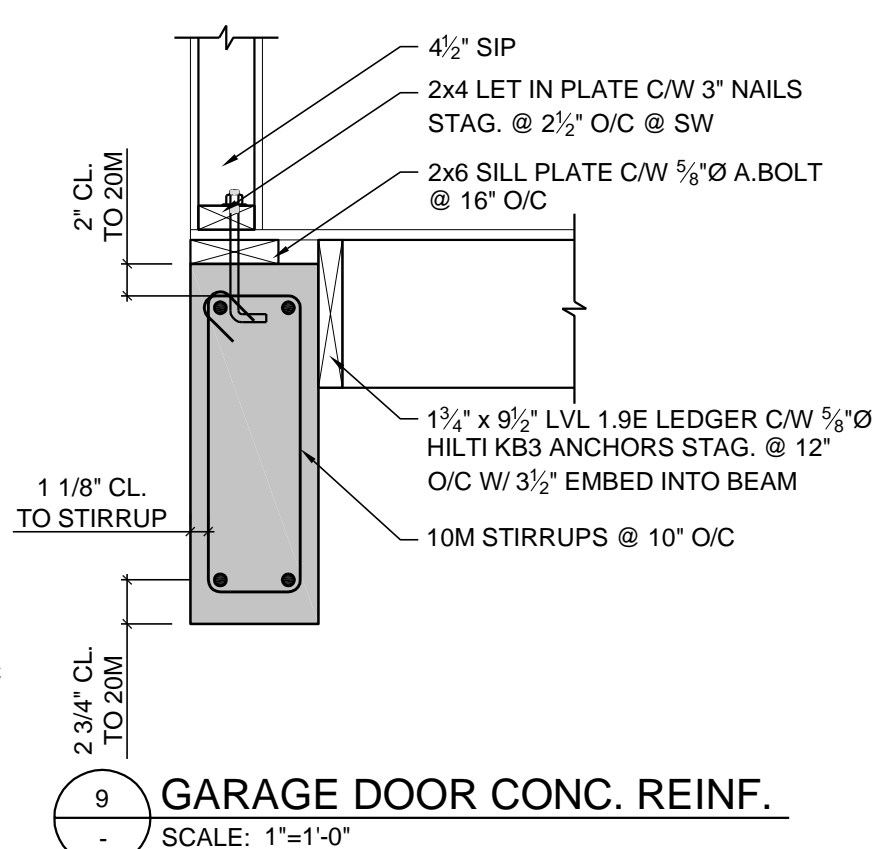
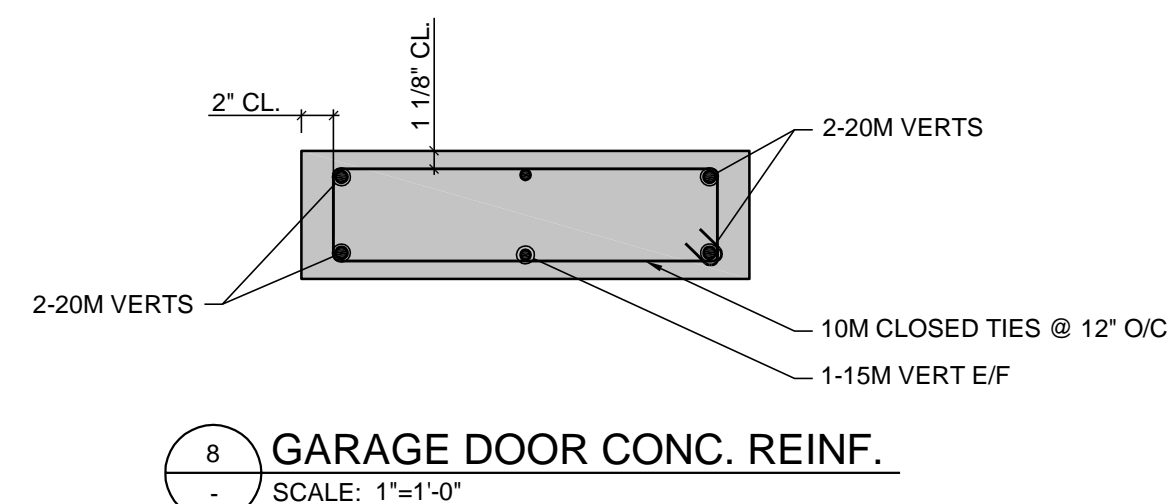
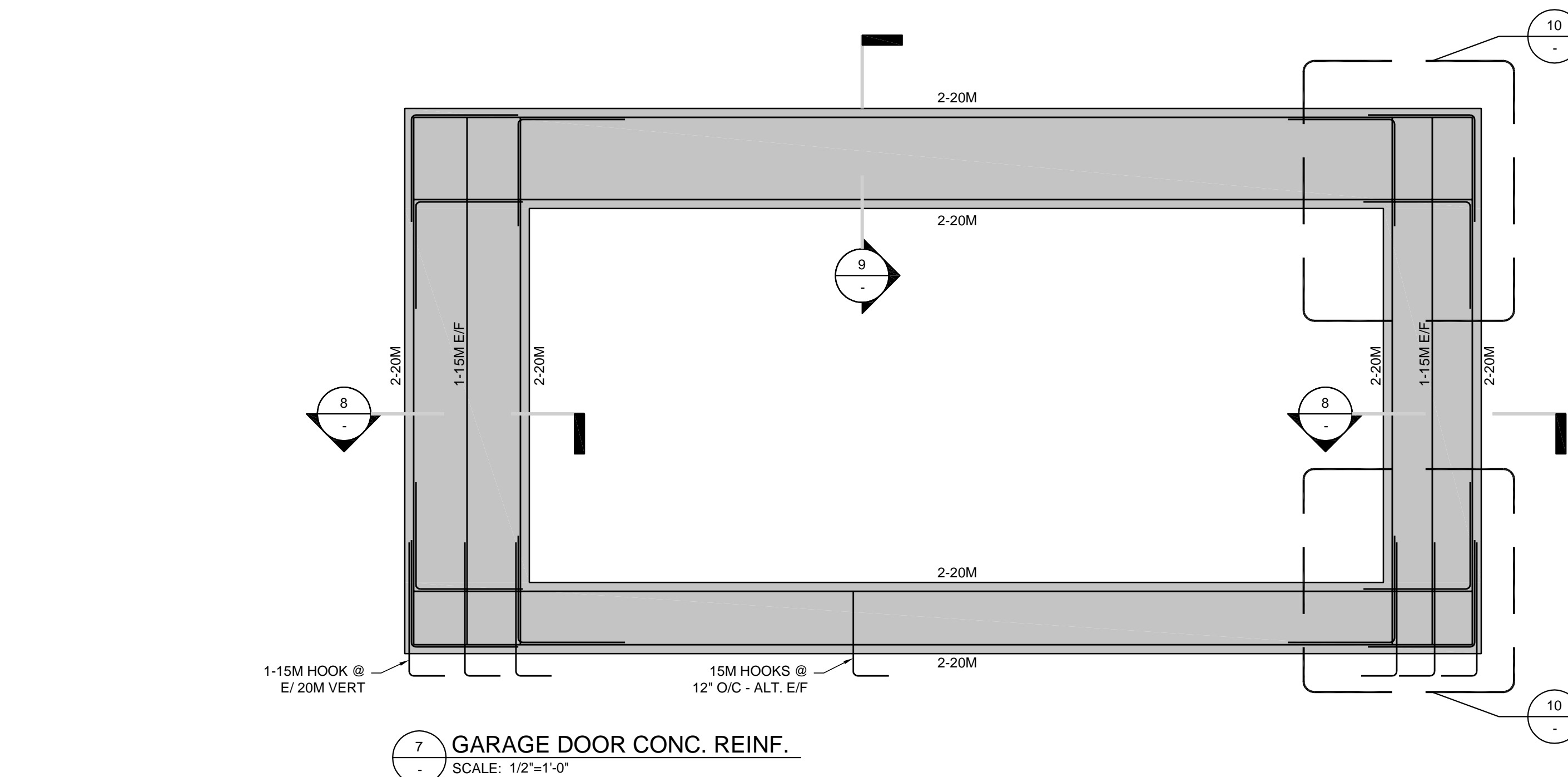
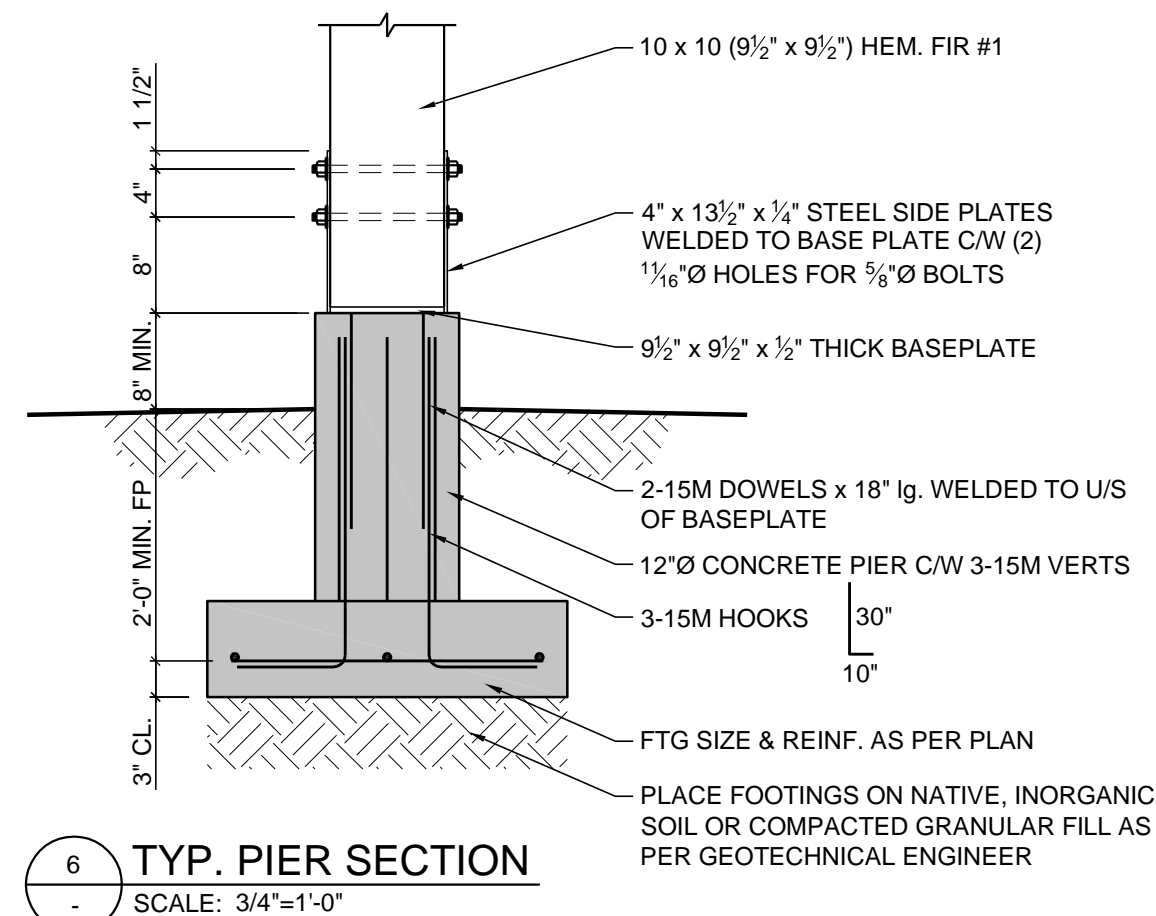
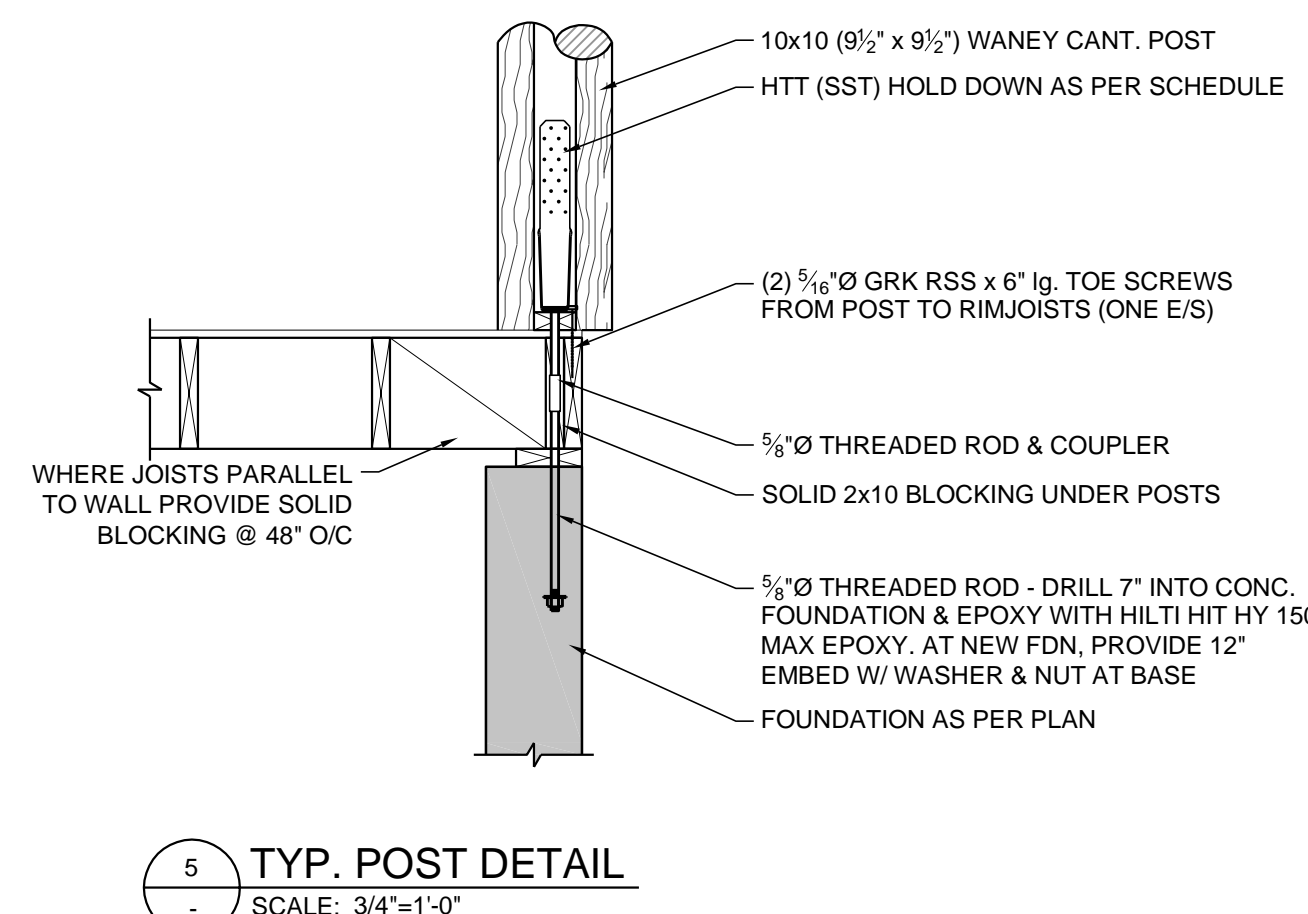
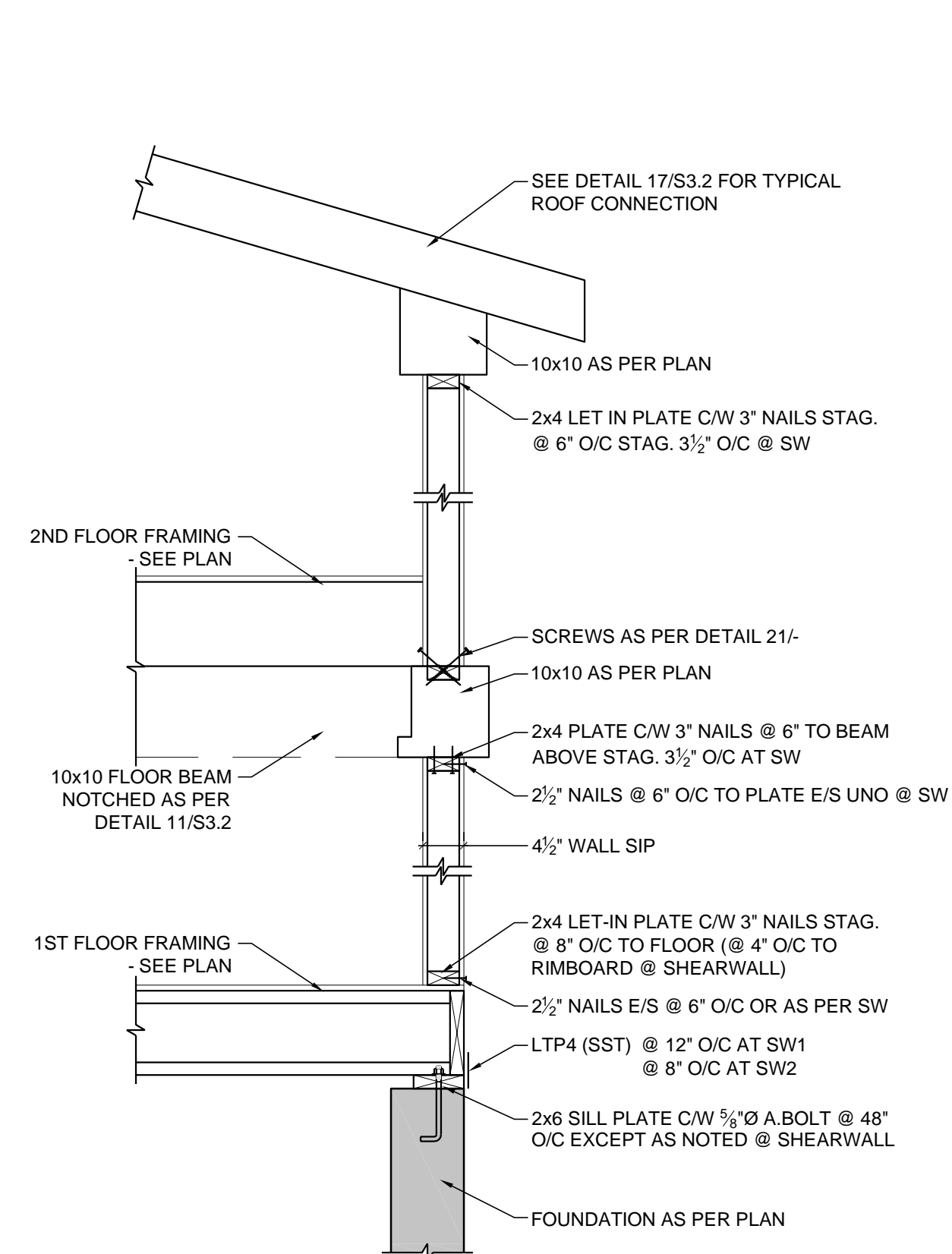
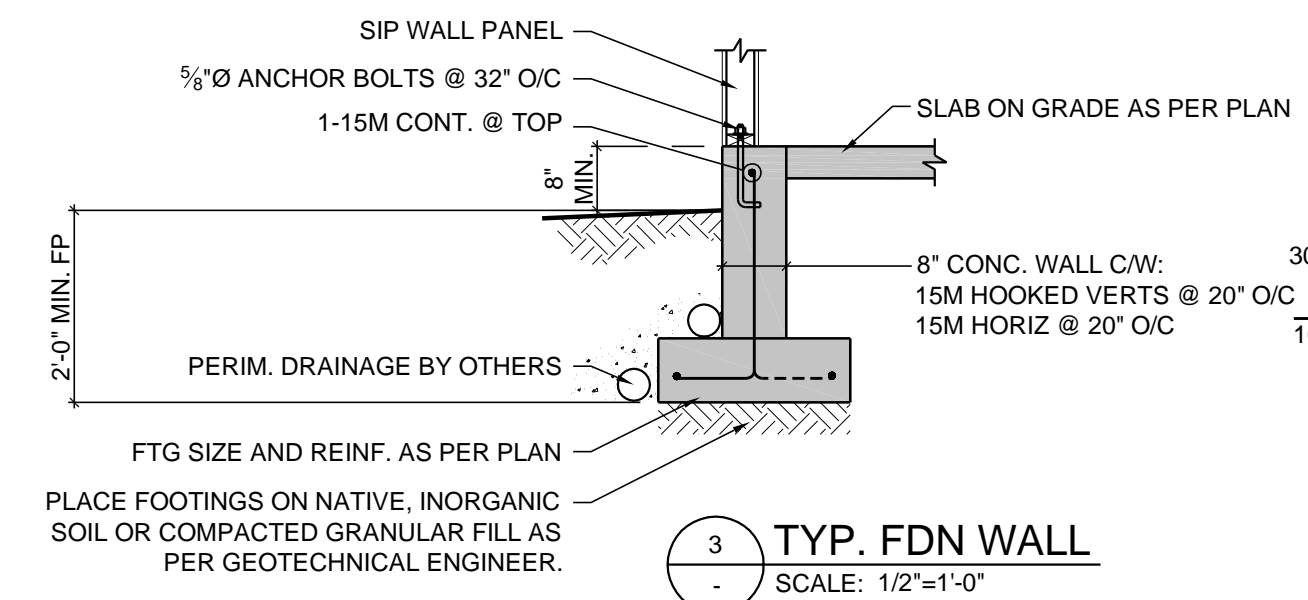
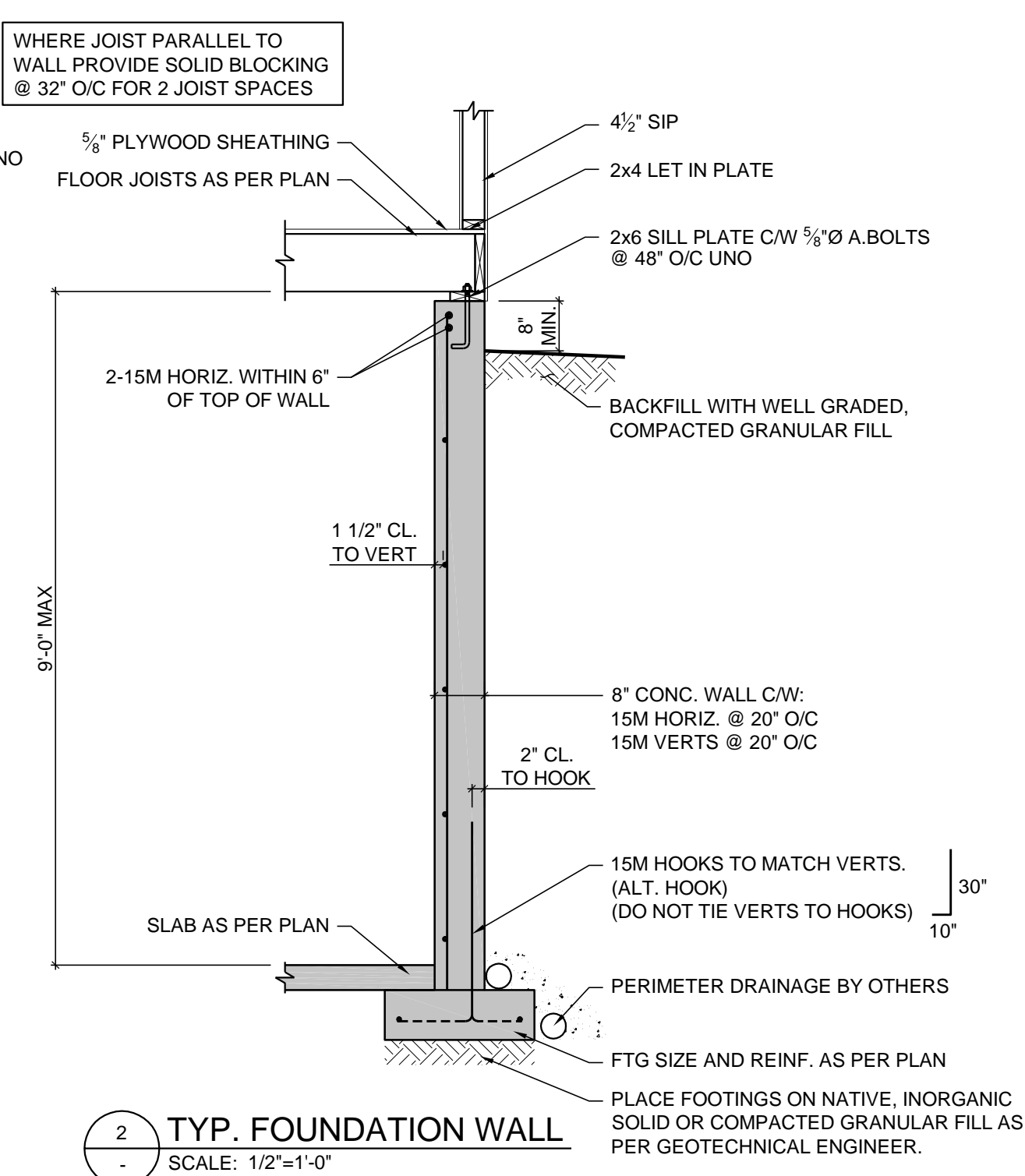
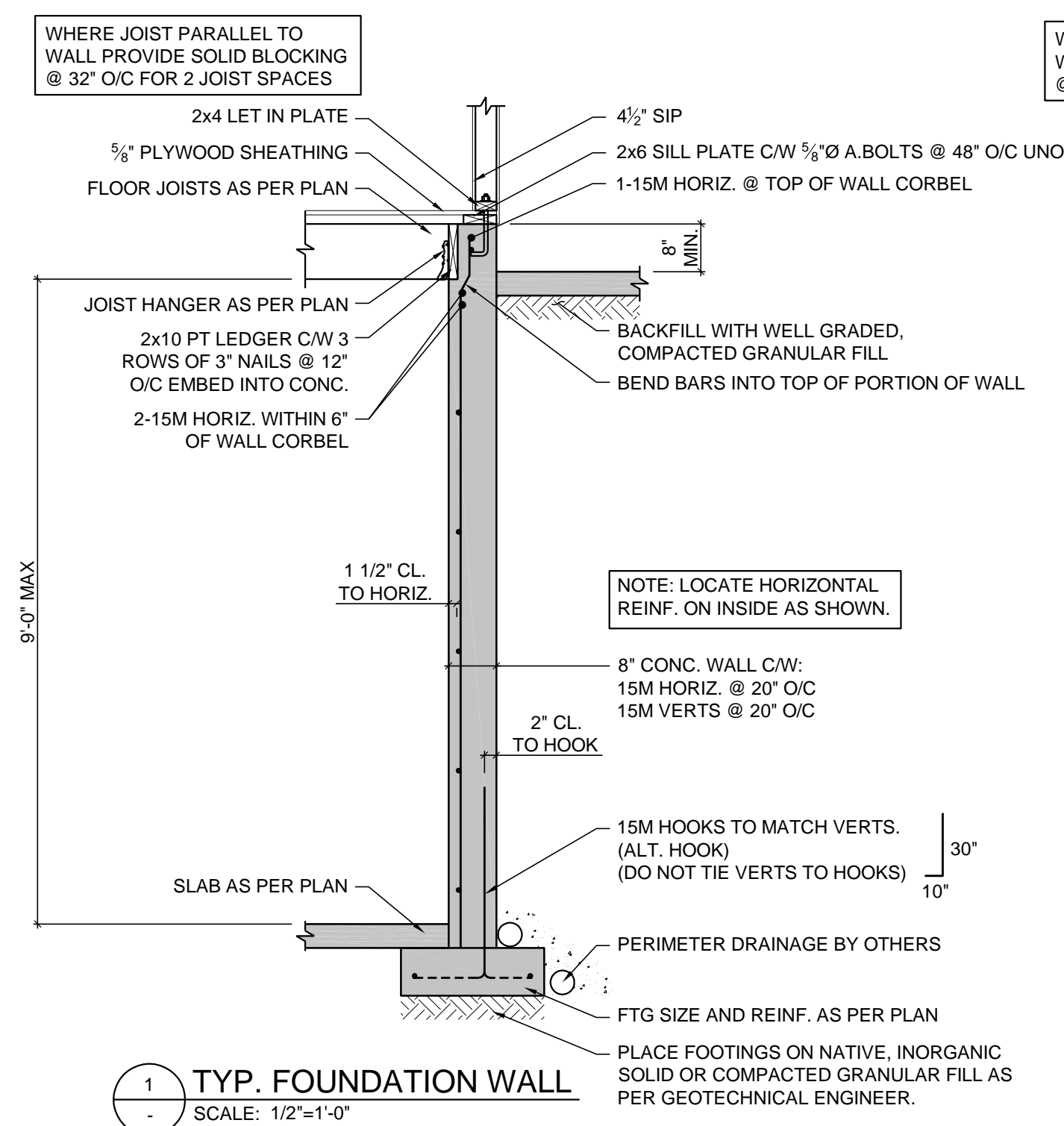
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Seal:

Project No: **13.207**

Sheet No: **S2.2**

Printed: August 13, 2013



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| 1 | FOR PERMIT/CONSTR. | Aug. 13, 13 |
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Tel: 604.703.1605
Fax: 604.909.1963
Email: info@quantumeng.ca

Project:

KITWANGA RESIDENCE

2939 STRANGWAY PL.
SQUAMISH, BC

Client: TRINITY POST & PANEL

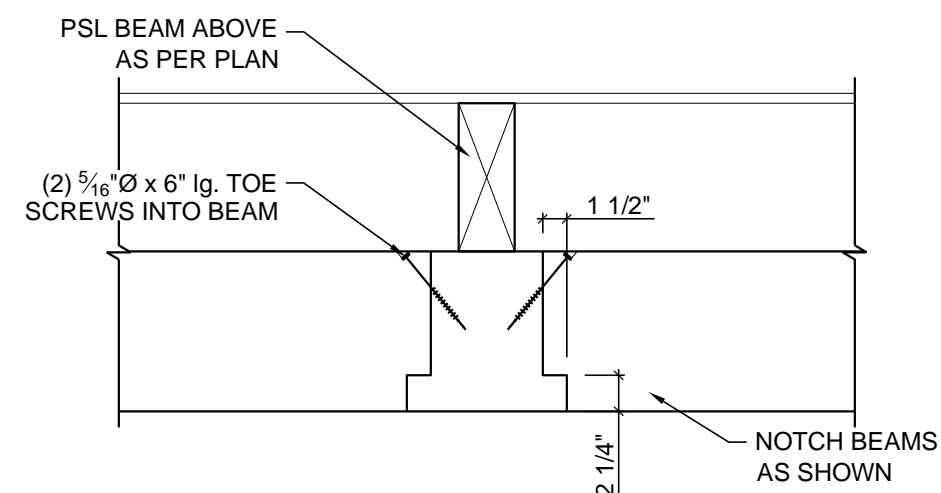
Title: **SECTIONS + DETAILS**

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| Scale: | AS NOTED |
| Drawn: | MS |
| Checked: | JK |
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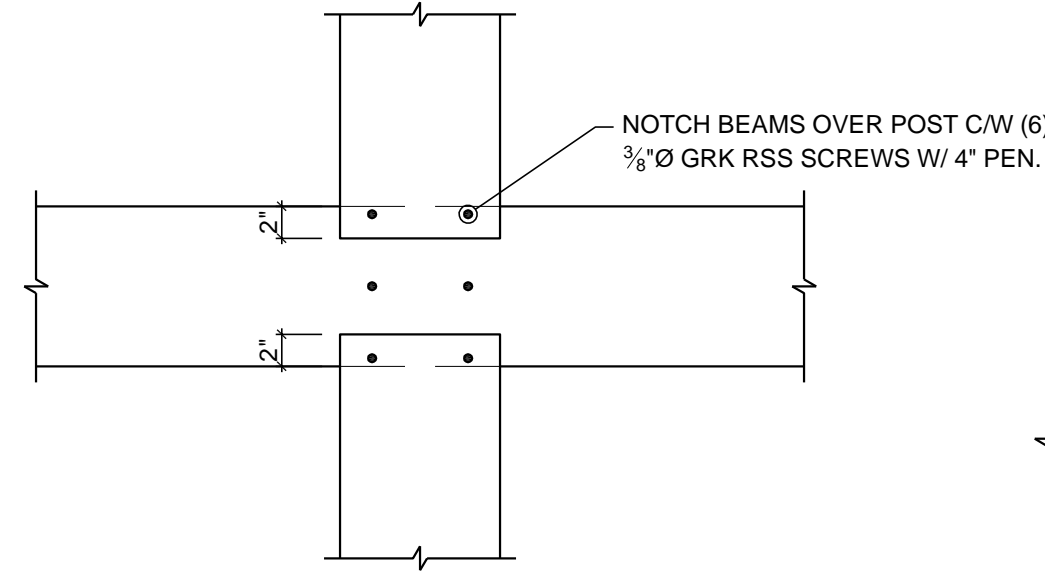
Project No: **13.207**

Sheet No: **S3.1**

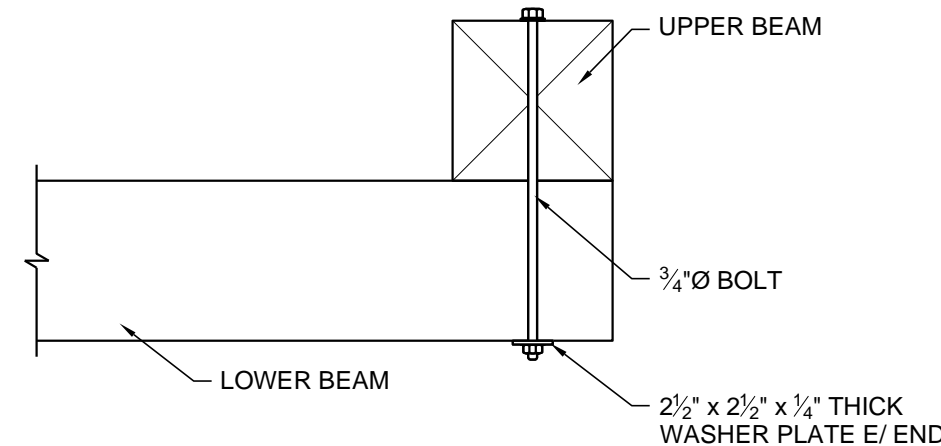
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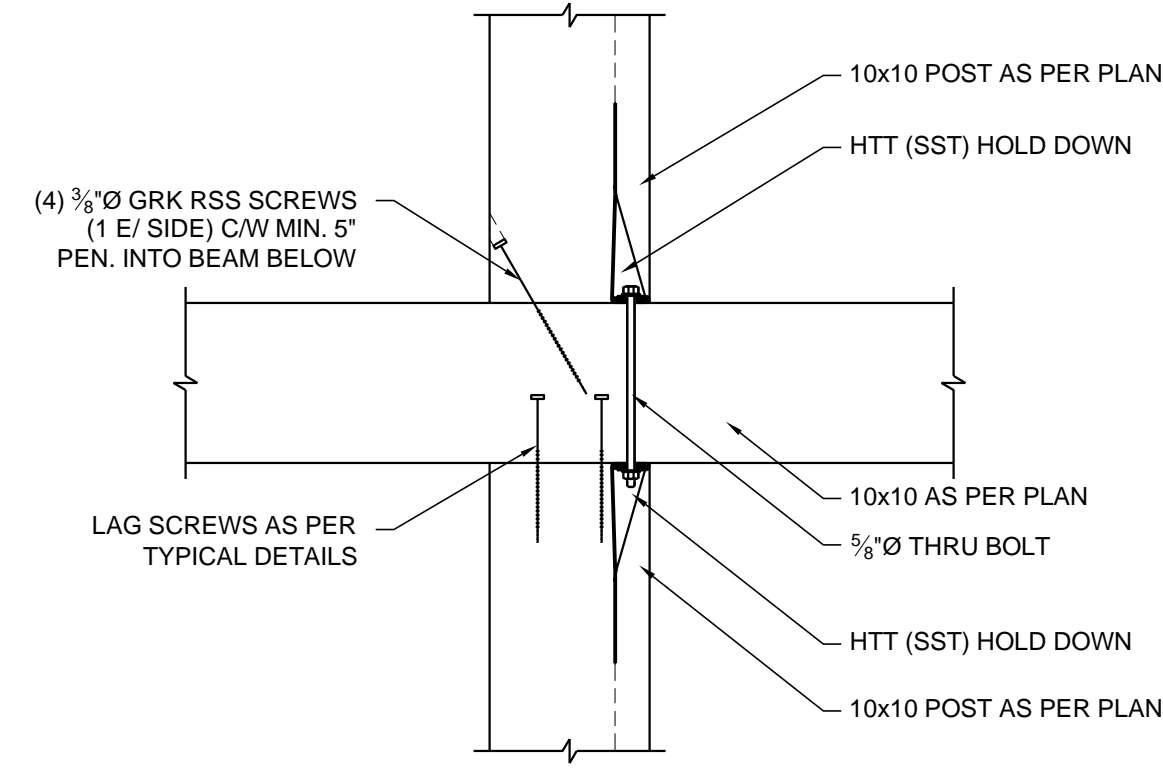
11 TYP. NOTCH DETAIL
SCALE: 1"=1'-0"



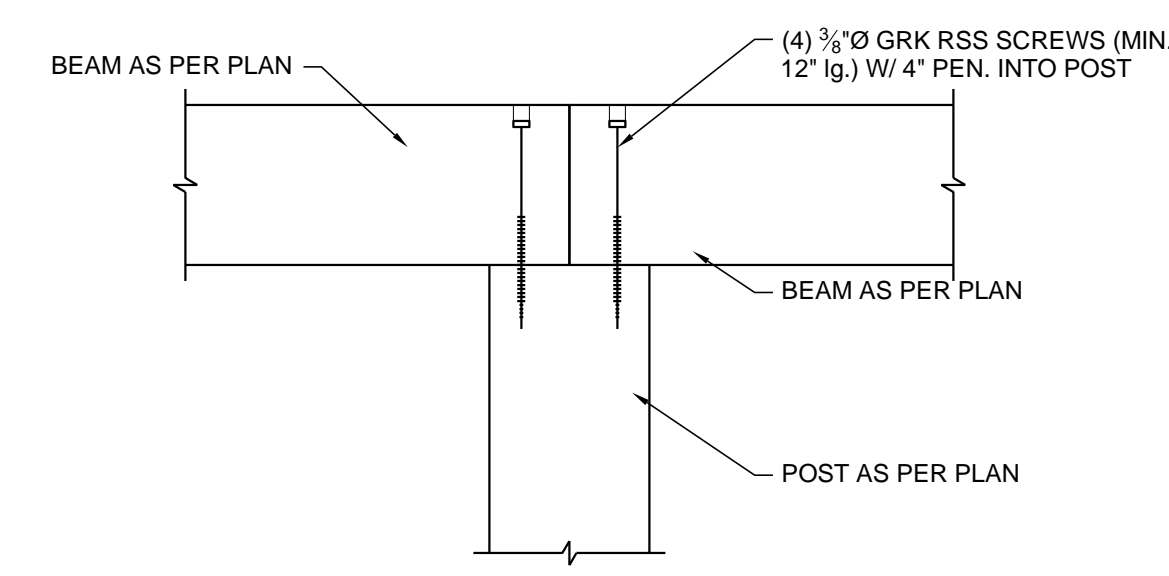
12 TYP. NOTCH DETAIL OVER POST
SCALE: 1"=1'-0"



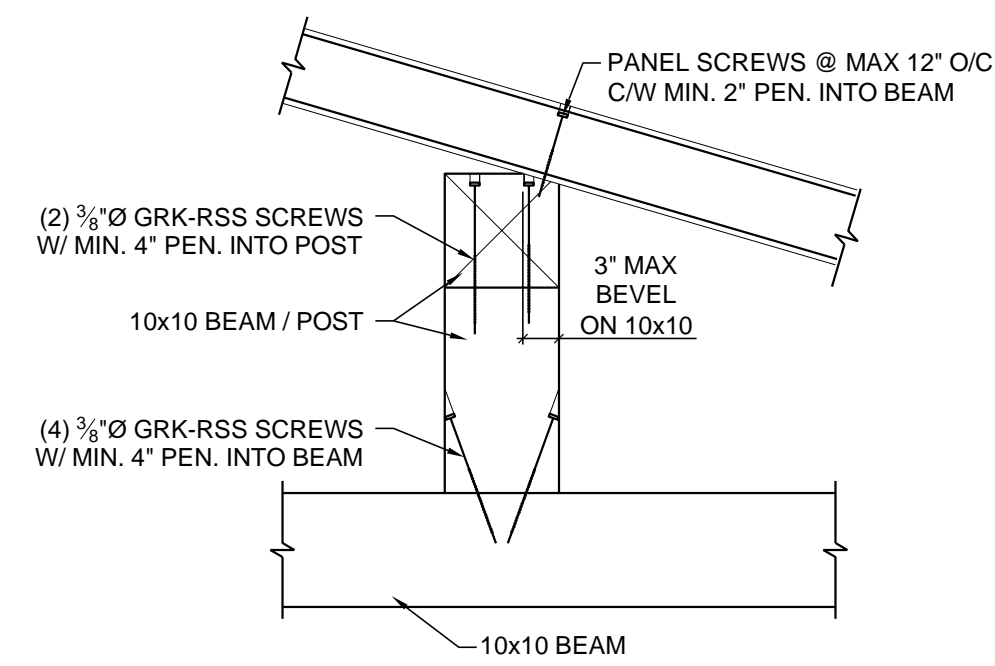
13 BEAM HANGER DETAIL
SCALE: 1"=1'-0"



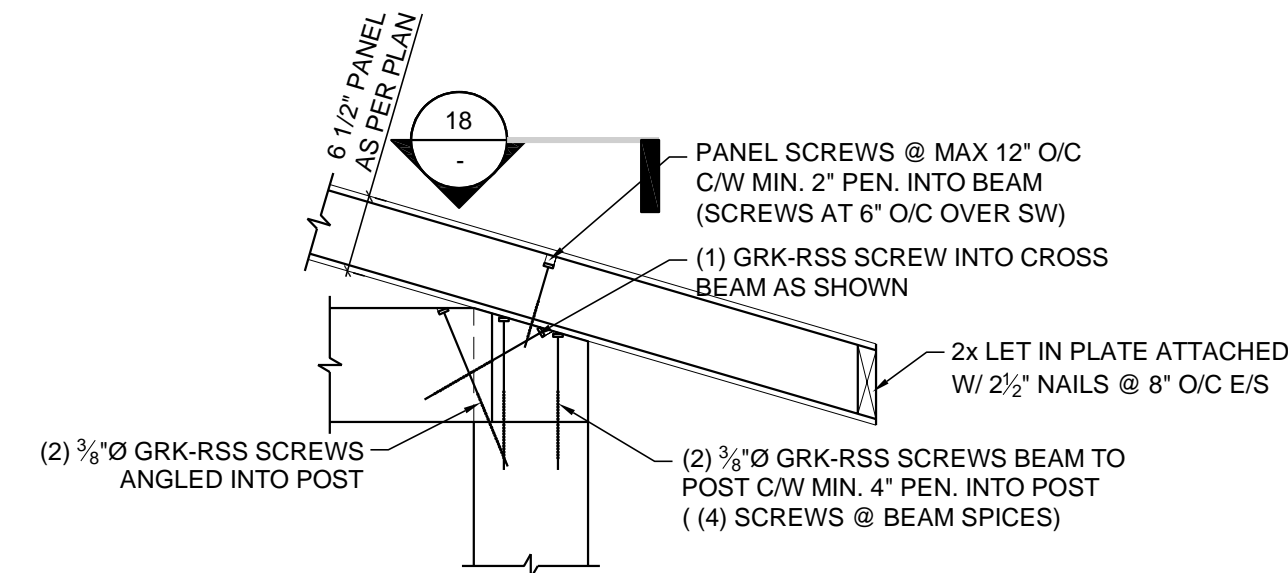
14 TYPICAL DOUBLE HTT HOLD DOWN ACROSS BEAM
SCALE: 1"=1'-0"



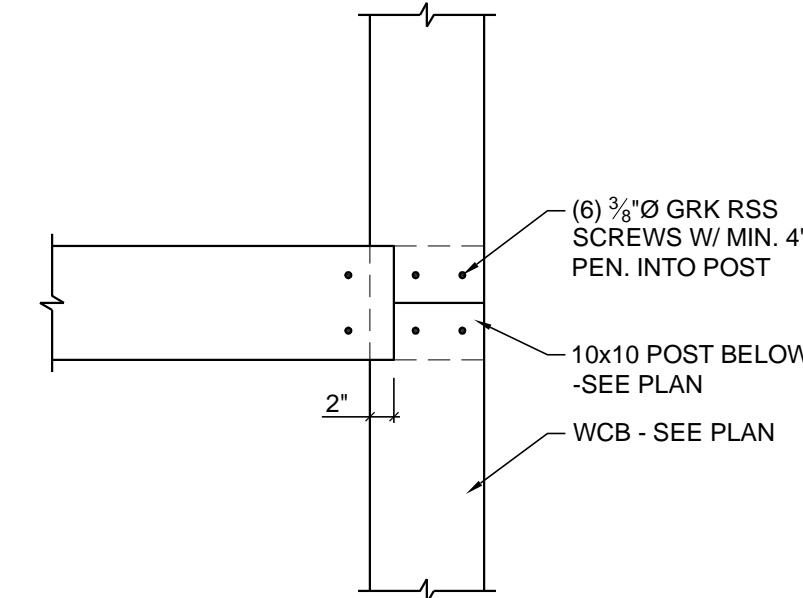
15 TYP. BEAM TO POST CONNECTION
SCALE: 1"=1'-0"



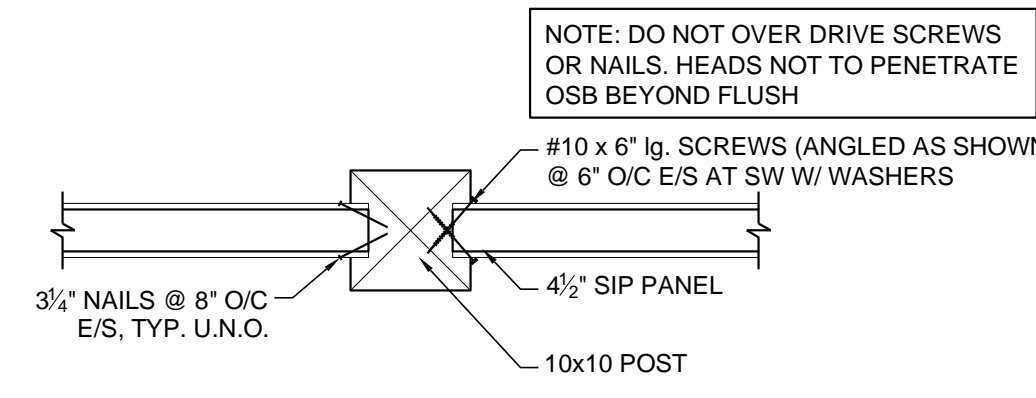
16 TYPICAL ROOF CONNECTION DETAIL @ PURLINS
SCALE: 3/4"=1'-0"



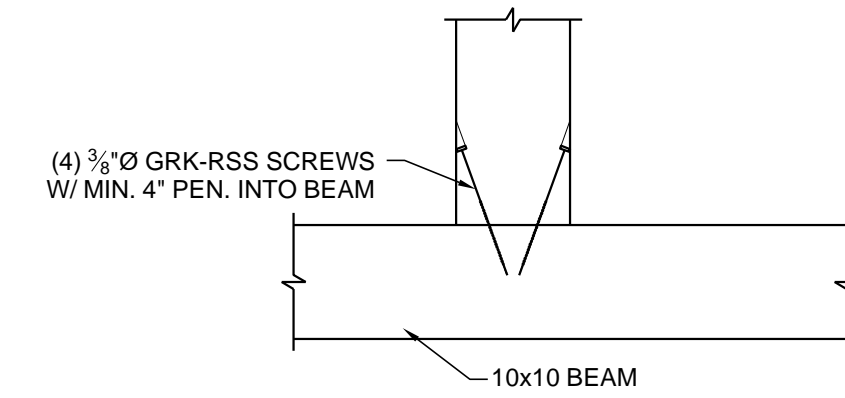
17 TYPICAL ROOF DETAIL
SCALE: 3/4"=1'-0"



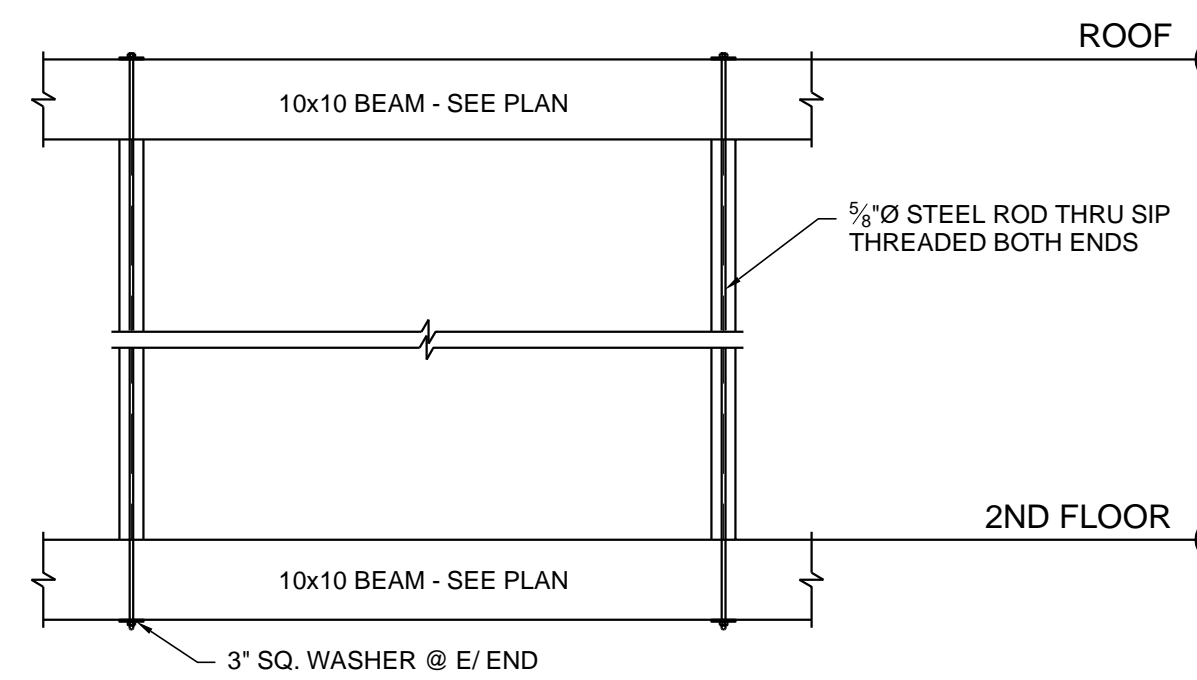
18 POST-BEAM CONNECTION DETAIL
SCALE: 3/4"=1'-0"



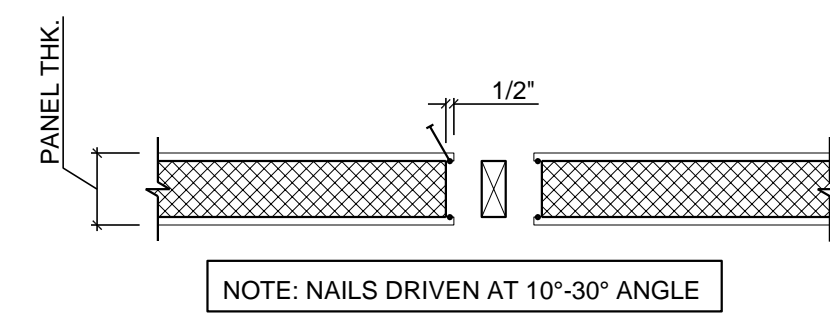
19 TYP. SIP - POST CONNECTION
SCALE: 3/4"=1'-0"



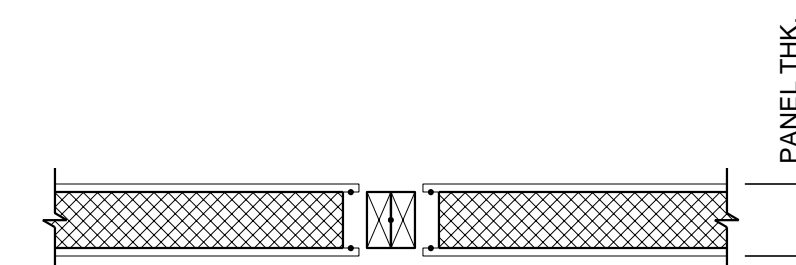
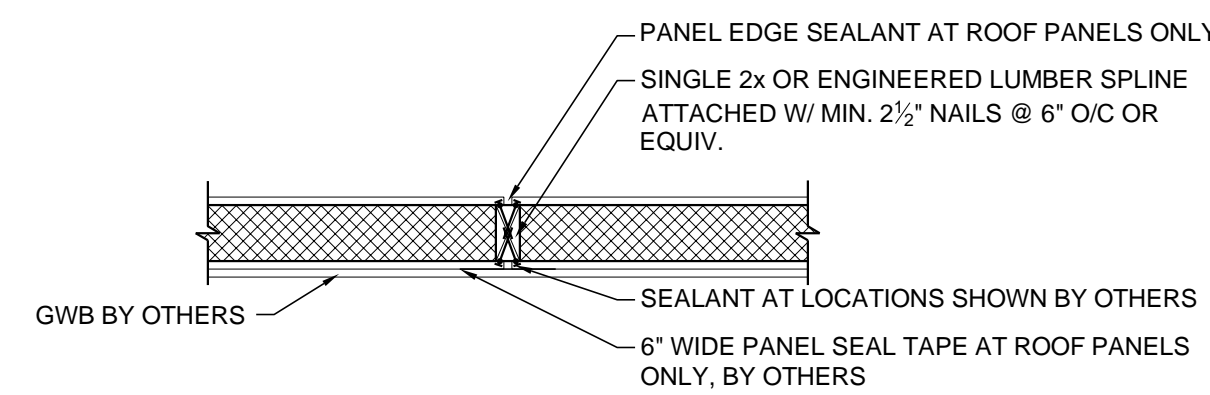
20 TYPICAL POST TO BEAM CONNECTION
SCALE: 3/4"=1'-0"



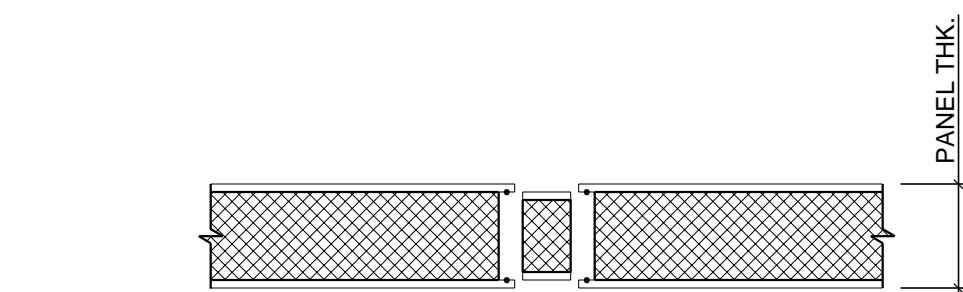
21 FULL LENGTH ROD DETAIL
SCALE: 1/2"=1'-0"



22 TYPICAL SIP PANEL DETAIL
SCALE: 1"=1'-0"



23 TYPICAL SIP PANEL DETAIL
SCALE: 1"=1'-0"



24 TYPICAL SIP PANEL DETAIL
SCALE: 1"=1'-0"



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Client:
TRINITY POST & PANEL

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SECTIONS + DETAILS

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Seal:

Project No: **13.207**

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