

Development of a Novel **Damage** Model for **Concrete** Subjected to **Creep**

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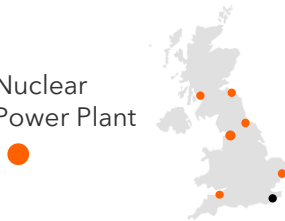


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1. Industrial and Project Context

Nuclear
Power Plant



nuclear power stations in UK
Online: **12 AGRs, 1 PWR**
Offline: **2 AGR**



Targeting Concrete pressure vessels:

- Safety assesment during LOCA
- In-service life assessments (30+ years of operations)
- Decommissioned (awaiting defueling)



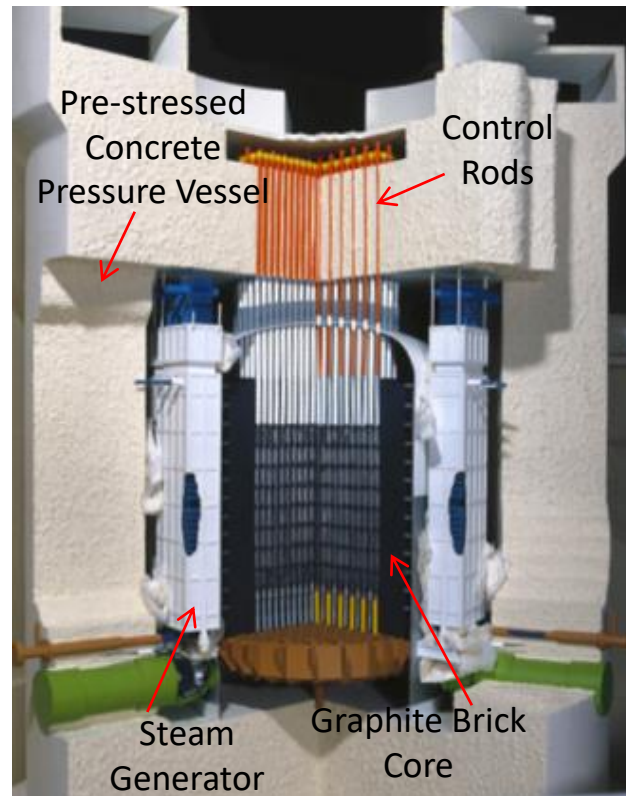
Exisiting models (in Mfront) :

- LITS (standalone),
- Damage (standalone),
- LITS+Damage (coupled)



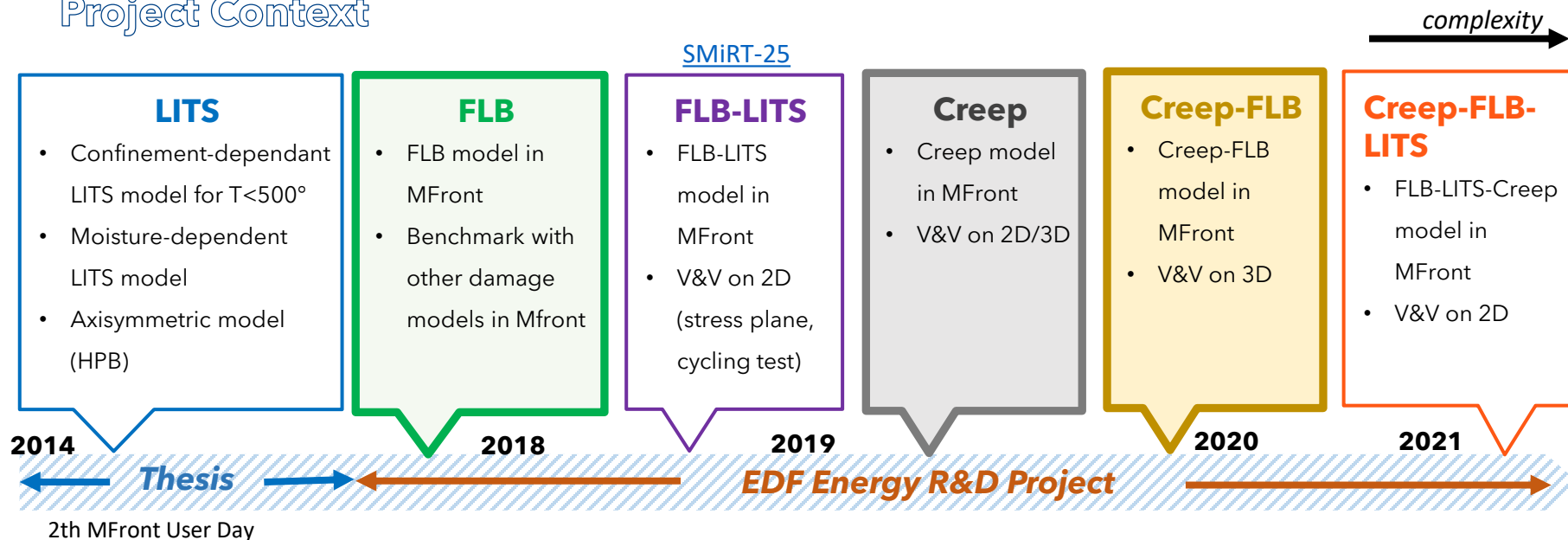
Objective :

- Highlighting the formation of cracks and assessing their effects on concrete vessels – *from cradle to grave*.
- Accounting for high temperature including Load Induced Thermal Strain (LITS) phenomenon and long-term creep



1. Industrial and Project Context

Project Context



<https://github.com/thelfer/tfel-doc/blob/master/MFrontUserDays/SecondUserDay/torelli-lits.pdf>

https://github.com/thelfer/MFrontGallery/blob/master/generic-behaviours/viscoplasticity/LoadInducedThermalStrain_Torelli2018.mfront

<http://tfel.sourceforge.net/FichantLaBorderieDamageBehaviour.html>

<https://github.com/thelfer/MFrontGallery/blob/master/generic-behaviours/damage/FichantLaborderieDamageBehaviour.mfront>

2 . The damage Model

Isotropic Damage Model

Use of an **Isotropic Damage model** applied to quasi-brittle material such as concrete

Damage models have the ability to numerically :

- **initiate** crack(s)
- **propagate** crack(s)

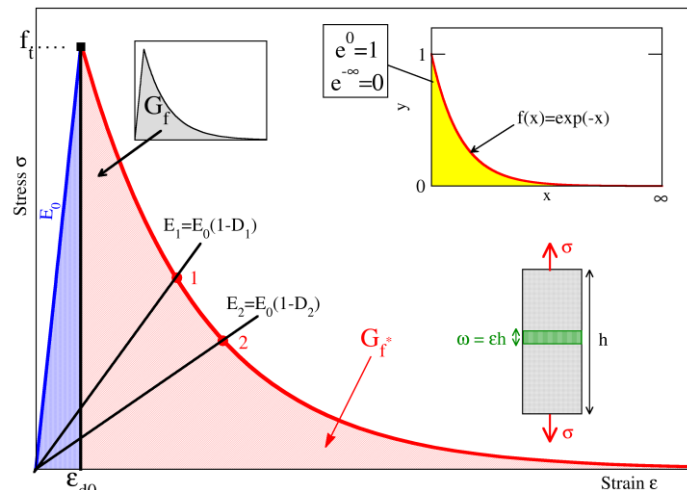
Concrete damage models should take into account :

- **Asymmetry** between tensile and compressive behaviour
- **Unilateral Effect** (also called crack closure effect)
- **Independency of the mesh** density/size

Fichant-La Borderie (FLB)

(extension of the well known **Mazars** model)

- **Variable scalar damage** D (if $D \sim 1$ it is a crack, if $D=0$ sane material)
- **Local damage model** with an energetic regularisation method
- Unilateral effect based on the stress tensor
- Can be coupled with plasticity (Matallah, 2009)¹ and/or creep (Saliba, 2013)²
- Support mesoscopic scale (aggregates, rebar...)



¹ Matallah, M., La Borderie, C., 2009. Inelasticity-damage-based model for numerical modeling of concrete cracking. Engineering Fracture Mechanics 76, 1087-1108.

² Saliba, J. et al. (2013). Relevance of a mesoscopic modeling for the coupling between creep and damage in concrete. Mechanics of Time-Dependent Materials, 17(3):481-499.

2 . The damage Model : Fichant - La Borderie

FLB : Theory

1- No visco-elasticity-plasticity in decomposition of strains

$$\varepsilon_{ij}^{tot} = \varepsilon_{ij}^{el} \quad \text{tensor}$$

2- Damage occurs on elastic strains part when $\varepsilon_{eq} > \varepsilon_{d0}$

$$\varepsilon_{eq} = \sqrt{\sum_{i=1}^3 \langle \varepsilon_{ij}^{el} \rangle_+^2} > \varepsilon_{d0} = \frac{f_t}{E} \quad \text{scalar}$$

3- Computation of the damage (exponential law)

$$D = 1 - \frac{\varepsilon_{d0}}{\varepsilon_{eq}} \exp\left(B_t (\varepsilon_{d0} - \varepsilon_{eq})\right) \quad \text{scalar}$$

$$B_t = \frac{f_t h}{G_f - 0.5 \varepsilon_{d0} f_t h}$$

4- Computation of total final stress tensor (damaged one) - Unilateral effect (crack closure)

$$\sigma_{ij}^{tot} = \underbrace{(1 - D) \langle \tilde{\sigma}_{ij} \rangle_+}_{\text{Tensile State}} + \underbrace{(1 - D^\alpha) \langle \tilde{\sigma}_{ij} \rangle_-}_{\text{Compressive State}} \quad \text{tensor}$$

$$\tilde{\sigma}_{ij} = E_{ijkl}^0 : \varepsilon_{ij}^{el}$$

• 2 Elastic parameters :

- Young Modulus (E),
- Poisson's ratio (ν),

• 3 Fracture parameters :

- Fracture Energy (G_f),
- Tensile strength (f_t),

- Participation of the damage in compression (α),

• 1 Finite element parameter :

- Average element size at the Gauss Point (h).



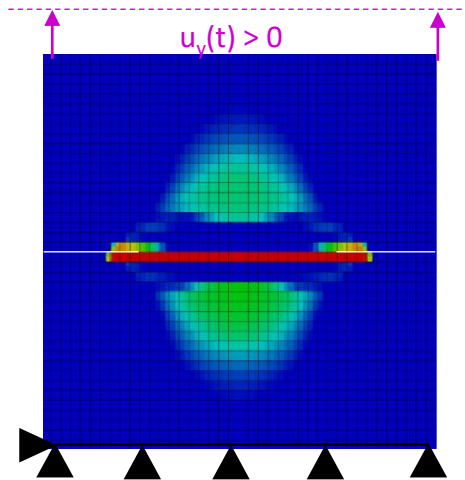
α : Factor linked to the damage in compressive state (Material Parameter)

- $\alpha = 1$: no unilateral effect - same damage in compression and tension
- $1 < \alpha < 1000$: effect on the level of the damage for the stiffness recovery
- $\alpha > 1000$: no damage in compression

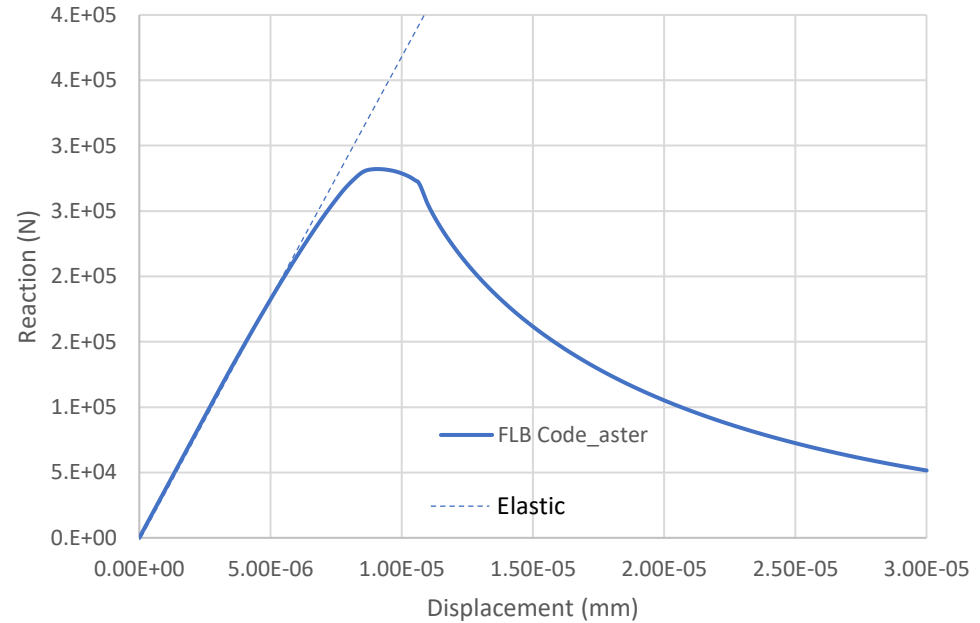
2 . The damage Model : Fichant - La Borderie

FLB : Damage Field

Example REV : 10x10 cm (Strain Plan)



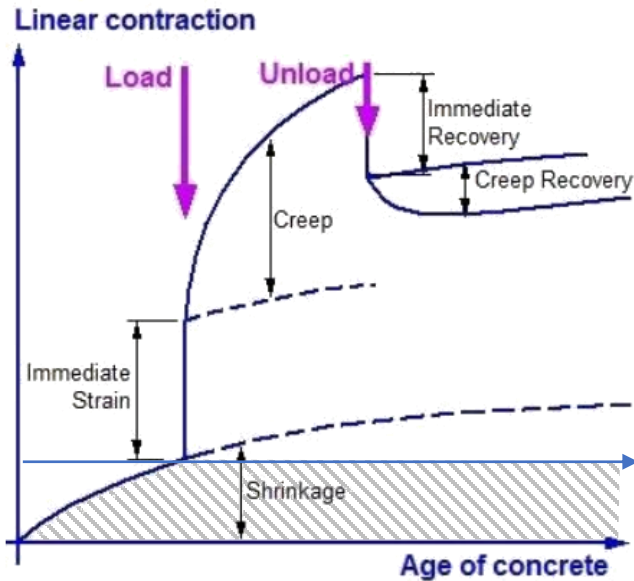
Damage field



Macroscopic Response

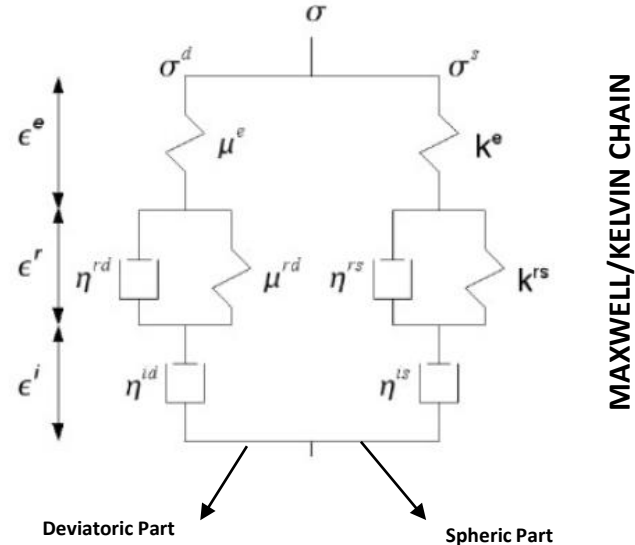
3 . The basic Creep Model : Burger

Basic Creep : Burger Model



Elastic and creep deformation of mass concrete under constant load followed by load removal

BETON_BURGER [R7.01.35]

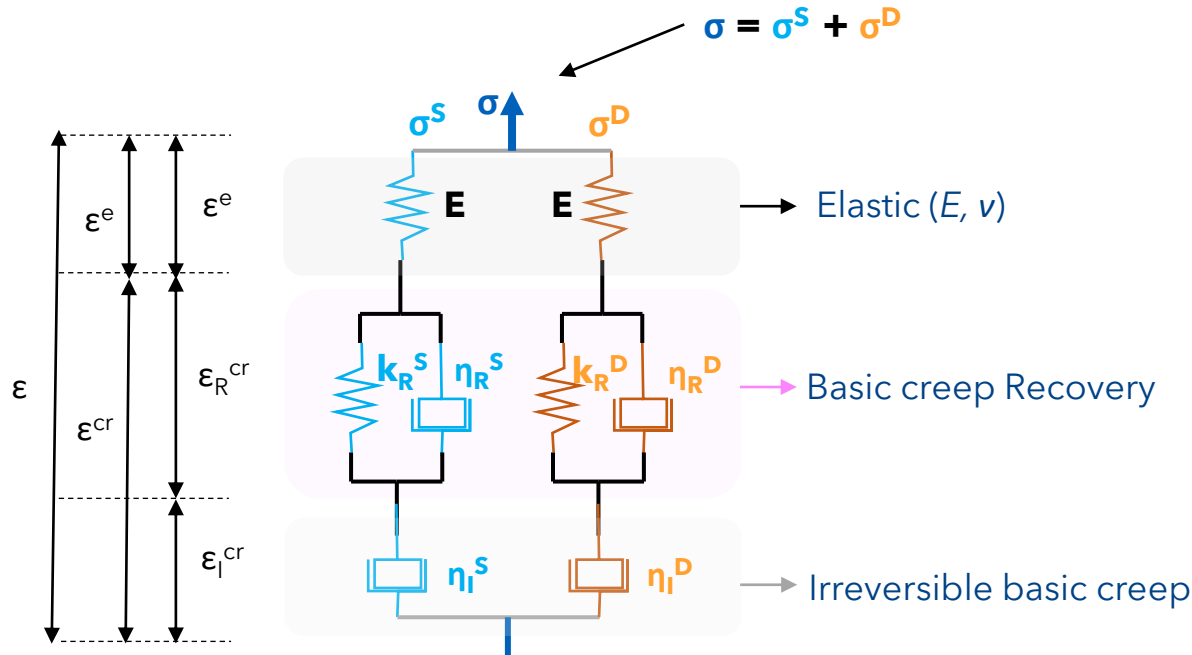


- Started point : **BurgerAgeing.mfront**
From verification code_aster test case mfron02c

→ Drying Creep has been removed

3 . The basic Creep Model : Burger

Basic Creep : Burger Model



• 6 Creep parameters :

$$\frac{k_R^S}{k_R^D} = \frac{\eta_R}{\eta_R^D} = \frac{\eta_I^S}{\eta_I^D} = \frac{(1+\nu)}{(1-2\nu)}$$

Bulk modulus $k_R^S = \frac{E}{3(1-2\nu)}$

η : viscosity (Pa.s)

k : rigidity (Pa)

• Creep parameters are dependant to the temperature:

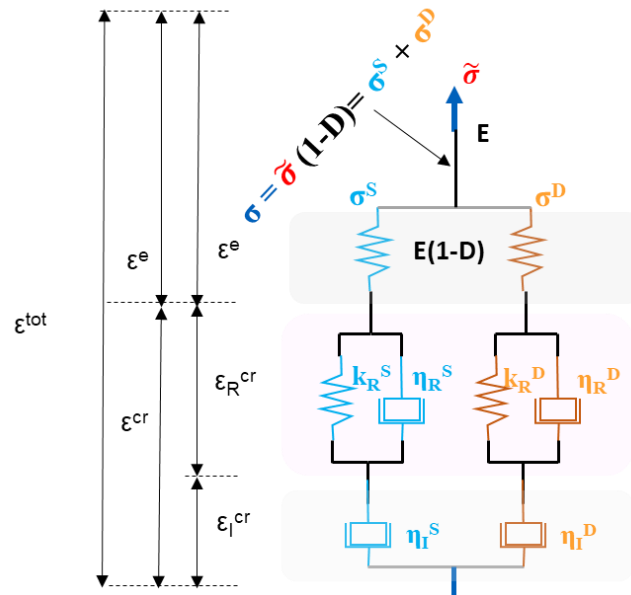
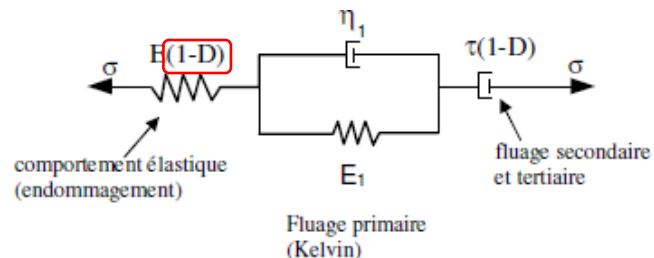
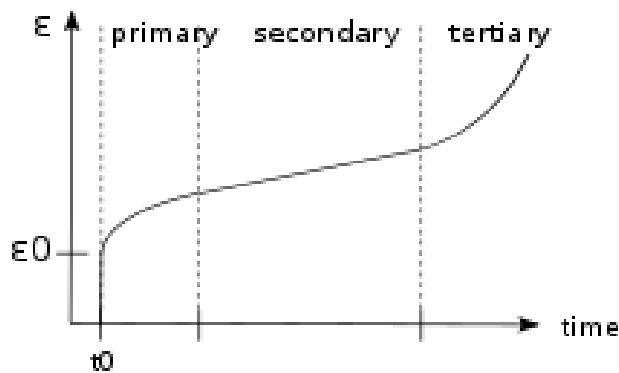
Thermal Factor

$$\times (T_0) e^{\frac{E_{ac}}{R} \left(\frac{1}{T} - \frac{1}{T_0} \right)}$$

4 . Coupling Damage and Creep

Burger-FLB Model

Introduction to damage



Introduction of a coupling Parameter : β

$$\epsilon_{eq} = \sqrt{\sum_{i=1}^3 \langle \epsilon_{ij}^{ela,0} \rangle_+^2 + \beta \sum_{i=1}^3 \langle \epsilon_{ij}^{creep} \rangle_+^2}$$

4 . Coupling Damage and Creep

Burger-FLB Model : a visco-elasto-damage model

$$\varepsilon_{tot} = \varepsilon_{ela,0} + \varepsilon_{ij}^{creep}$$

BURGER

1- Computation of the effective stress $\tilde{\sigma}_{ij}$ under Mechanic and Thermal solicitation

2 - Computation of Creep strain ε_{ij}^{creep} based on deviatoric and spheric part of the the effective stress $\rightarrow \varepsilon_{ij}^{creep} = \varepsilon_R^S + \varepsilon_I^S + \varepsilon_R^D + \varepsilon_I^D$

3 - Computation of elastic strain ε_{ij}^{elas}

4 - Computation of equivalent strain ε_{eq}

β : effect of creep on the damage

$$\varepsilon_{eq} = \sqrt{\sum_{i=1}^3 \langle \varepsilon_{ij}^{ela,0} \rangle_+^2 + \beta \sum_{i=1}^3 \langle \varepsilon_{ij}^{creep} \rangle_+^2}$$

5- Estimation of potential damage D

$$D = 1 - \frac{\varepsilon_{d0}}{\varepsilon_{eq=f(\varepsilon_{ela,0})}} \exp \left(B_t (\varepsilon_{d0} - \varepsilon_{eq=f(\varepsilon_{ela,0})}) \right)$$

FLB

6- Computation of final stress σ_{ij}

$$\sigma_{ij} = (1 - D) \langle \tilde{\sigma}_{ij} \rangle_+ + (1 - D^\alpha) \langle \tilde{\sigma}_{ij} \rangle_-$$

$$\tilde{\sigma}_{ij} = E_{ijkl} : \varepsilon_{kl}^{ela,0}$$

7- Updating the effective stress $\tilde{\sigma}_{ij=f(\varepsilon_{ela,0})}$ that contains now the creep effect (ε_{ij}^{creep})

8- Updating the final damage D that contains now creep effects (ε_{ij}^{creep})

BURGER + FLB

9- Updating the final stress tensor $\sigma_{ij}(D, \varepsilon_{kl}^{ela,0}, \varepsilon_{ij}^{creep})$

4 . Coupling Damage and Creep

Internal Variable (3D case)

• first component of internal variable 'ElasticStrain'
• second component of internal variable 'ElasticStrain'
• third component of internal variable 'ElasticStrain'
• fourth component of internal variable 'ElasticStrain'
• fifth component of internal variable 'ElasticStrain'
• sixth component of internal variable 'ElasticStrain'
• Reversible_SphericalStrain
• Irreversible_SphericalStrain
• first component of internal variable 'Reversible_DeviatoricStrain'
• second component of internal variable 'Reversible_DeviatoricStrain'
• third component of internal variable 'Reversible_DeviatoricStrain'
• fourth component of internal variable 'Reversible_DeviatoricStrain'
• fifth component of internal variable 'Reversible_DeviatoricStrain'
• sixth component of internal variable 'Reversible_DeviatoricStrain'
• first component of internal variable 'Irreversible_DeviatoricStrain'
• second component of internal variable 'Irreversible_DeviatoricStrain'
• third component of internal variable 'Irreversible_DeviatoricStrain'
• fourth component of internal variable 'Irreversible_DeviatoricStrain'
• fifth component of internal variable 'Irreversible_DeviatoricStrain'
• sixth component of internal variable 'Irreversible_DeviatoricStrain'
• Damage


• first component of internal variable 'CreepStrain'
• second component of internal variable 'CreepStrain'
• third component of internal variable 'CreepStrain'
• fourth component of internal variable 'CreepStrain'
• fifth component of internal variable 'CreepStrain'
• sixth component of internal variable 'CreepStrain'
• Epsi_eq

4 . Coupling Damage and Creep


Quick View on the Mfront file

```
@Parser Implicit;  
@Behaviour burger_flb;  
@Algorithm NewtonRaphson_NumericalJacobian;  
@Theta 1.;  
@Epsilon 1.E-11;
```

```
@InitLocalVariables {  
  constexpr const auto T0 = temperature(273);  
  const auto T_ = T + theta * dT;  
  KRS_T = KRS * exp(Ea_R * (1.0/(T0 + T_) - 1.0/(T0 + Tref)));  
  KRd_T = KRd * exp(Ea_R * (1.0/(T0 + T_) - 1.0/(T0 + Tref)));  
  NRS_T = NRS * exp(Ea_R * (1.0/(T0 + T_) - 1.0/(T0 + Tref)));  
  NRd_T = NRd * exp(Ea_R * (1.0/(T0 + T_) - 1.0/(T0 + Tref)));  
  NIS_T = NIS * exp(Ea_R * (1.0/(T0 + T_) - 1.0/(T0 + Tref)));  
  NID_T = NID * exp(Ea_R * (1.0/(T0 + T_) - 1.0/(T0 + Tref)));  
  lambda = computeLambda(young, nu);  
  mu = computeMu(young, nu);  
}
```

```
@Integrator {  
  h_el = ELTSIZE1/coef1;  
  Bt = (h_el * ft) / (gf - (0.5 * e0 * h_el * ft));  
  constexpr const auto id = Stensor::Id();  
  //    
  // sig/(1-d): Effective stress applied to undamaged part of the concrete  
  
  const auto stresP = trace(sig/(1-d)) / 3.0;  
  Stensor stresD = (sig/(1-d)) - stresP * id;  
  
  fESPHR = dESPHR - (((stresP) - KRS_T * (ESPHR + dESPHR)) / NRS_T) * dt;  
  fEDEVR = dEDEVR - (((stresD) - KRd_T * (EDEVR + dEDEVR)) / NRd_T) * dt;  
  
  fESPFI = dESPFI - ((stresP) / (NIS_T * pow(t0 + 0.5 * dt, Alpha))) * dt;  
  fEDEVI = dEDEVI - ((stresD) / (NID_T * pow(t0 + 0.5 * dt, Alpha))) * dt;  
  
  fEF = dEF - (dEDEVR + dEDEVI + (dESPHR + dESPFI) * id);  
}
```

```
@ComputeFinalStress {  
  
  auto square_ppos = [] (const strain& v) { return v > 0 ? v * v : 0; };  
  constexpr const auto id = Stensor::Id();  
  const auto e_elas_f = eel.computeEigenValues();  
  const auto e_creep_f = EF.computeEigenValues();  
  
  e_eq = sqrt(  
    square_ppos(e_elas_f[0]) + square_ppos(e_elas_f[1]) + square_ppos(e_elas_f[2]) +  
    Beta * (square_ppos(e_creep_f[0]) + square_ppos(e_creep_f[1]) + square_ppos(e_creep_f[2]))  
  );  
  // ---> Official Theory  
  
  if (e_eq > e0) {  
    d = max(d, 1 - (e0 / e_eq) * exp(Bt * (e0 - e_eq)));  
  }  
  const auto sigma0 = lambda * trace(eel) * id + 2 * mu * (eel);  
  const auto sigma0_p = positive_part(sigma0);  
  const auto sigma0_n = sigma0 - sigma0_p;  
  // contrainte total  
  sig = (1 - d) * sigma0_p + (1 - pow(d, a)) * sigma0_n;  
}
```

```
//    
  
  // Computation of creep strain  
  const auto e_cr = eval(EF * theta * dEF);  
  
  // Computation of elastic strain  
  const auto e_elas = eval(eel + theta * deel);  
  
  // computation of eigenvalues of elastic and creep strain tensors  
  const auto e_vp = e_elas.computeEigenValues();  
  const auto e_cr_p = e_cr.computeEigenValues();  
  
  auto square_ppos = [] (const strain& v) { return v > 0 ? v * v : 0; };  
  
  // equivalent strain  
  e_eq = sqrt(  
    square_ppos(e_vp[0]) + square_ppos(e_vp[1]) + square_ppos(e_vp[2]) +  
    Beta * (square_ppos(e_cr_p[0]) + square_ppos(e_cr_p[1]) + square_ppos(e_cr_p[2]))  
  );  
  
  // Computation of damage d  
  
  if (e_eq > e0) {  
    d = max(d, 1 - (e0 / e_eq) * exp(Bt * (e0 - e_eq)));  
  }  
  const auto s = lambda * trace(e_elas) * id + 2 * mu * (e_elas);  
  
  // Decomposition of the stress tensor in positive and negative  
  const auto sp = positive_part(s);  
  const auto sn = s - sp;  
  
  // Computation of total stress  
  sig = (1 - d) * sp + (1 - pow(d, a)) * sn;  
  
  feel = deel - (deto - dEF);  
}
```

5 . Validation Study

Validation test-case : Uniaxial Study

Magazine of Concrete Research, 2005, 57, No. 10, December, 625-634

Experimental studies on creep of sealed concrete under multiaxial stresses

J. K. Kim,* S. H. Kwon,* S. Y. Kim,† Y. Y. Kim*

Korea Advanced Institute of Science and Technology; Chonnam National University

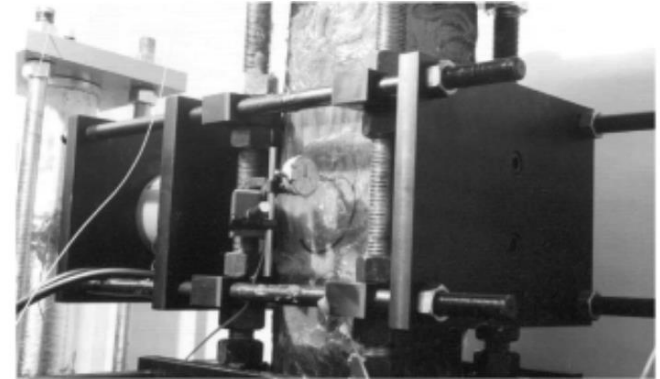
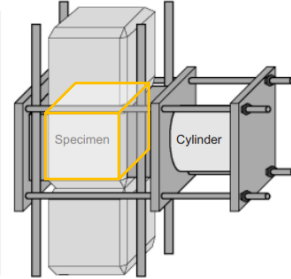
The majority of research studies and prediction models on the creep of concrete have been concerned with the uniaxial stress state. Research on creep under multiaxial stresses, however, has been relatively scarce even though concrete is subjected to multiaxial stresses in many structural members and structures. Although some experiments on the multiaxial creep of concrete have been performed, the results of these experiments, which mainly considered low-strength concrete, are controversial. Consequently, for safety standards and practice—need for a better understanding of the properties of concrete—more experimental data are needed on the multiaxial creep of concrete with various strengths. In this study, multiaxial creep tests with 17 cubic specimens (200 mm × 200 mm × 200 mm) were carried out for three different strengths of concrete, namely 26, 41 and 54 MPa, and creep strains were measured in three principal directions. From the measured strains, Poisson's ratio at initial loading was obtained, as was Poisson's ratio resulting from creep strains and Poisson's ratio resulting from the combined creep strains and strains. These Poisson's ratios were approximately equal for each concrete mix. The Poisson's ratio at initial loading and the Poisson's ratio for the combined strain increased slightly as the strength of the concrete increased. The volumetric creep strain and deviatoric creep strain were linearly proportional to volumetric stress and deviatoric stress, respectively.

Notation
 $\epsilon_1, \epsilon_2, \epsilon_3$ creep strains in 1, 2 and 3 directions
subscript indicates the three principal directions
 ϵ_1 creep strain in the 1 direction
 ϵ_2 specific creep in the loading axis under a uniaxial stress state
 ϵ_3 shear modulus for deviatoric creep at arbitrary time
 ϵ_4 bulk modulus for deviatoric creep at arbitrary time
 ϵ_5 concrete age under a sustained load
 ϵ_6 concrete age at initial loading
 ϵ_7 deviatoric strain in the 1 direction
 ϵ_8 volumetric strain

σ_1 shear stress on the octahedral plane
 σ_2 effective creep Poisson's ratio in the 1 direction
 σ_3 creep Poisson's ratio
 $\sigma_4, \sigma_5, \sigma_6$ applied stresses in 1, 2 and 3 directions
subscript indicates the three principal directions
 σ_7 deviatoric stress in the 1 direction
 $\sigma_8, \sigma_9, \sigma_{10}$ applied stresses in the 1, 2 and 3 directions
subscript means one of the three principal directions
 σ_{11} volumetric stress
 σ_{12} deviatoric stress
 σ_{13} shear stress on the octahedral plane

Introduction

Although many studies have examined the creep and shrinkage of concrete, these phenomena are not fully understood. Most research has been concerned with the uniaxial stress state, while research on creep of concrete under multiaxial stresses is relatively scarce, even though concrete is subjected to multiaxial stresses in many structural elements and structures such as concrete



Objective of this study:

⇒ Validation of the coupled law Burger-FLB by comparison with experimental data from Chonnam National University³

³Experimental studies on creep of sealed concrete under multiaxial stresses
J. K. Kim,* S. H. Kwon,* S. Y. Kim,† Y. Y. Kim*
Korea Advanced Institute of Science and Technology; Chonnam National University

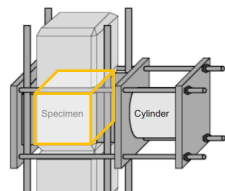
5 . V&V Study

Validation test-case : KIM Study

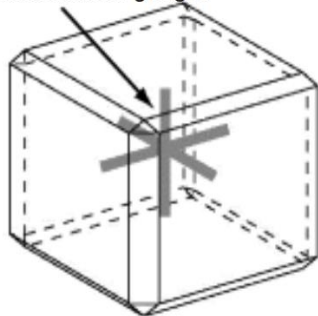
Details of Kim³ experimental test (Creep Only)

³Experimental studies on creep of sealed concrete under multiaxial stresses

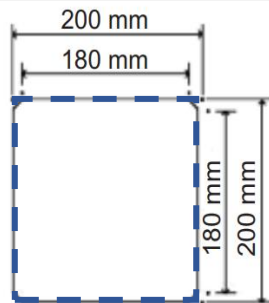
J. K. Kim,* S. H. Kwon,* S. Y. Kim,† Y. Y. Kim*
Korea Advanced Institute of Science and
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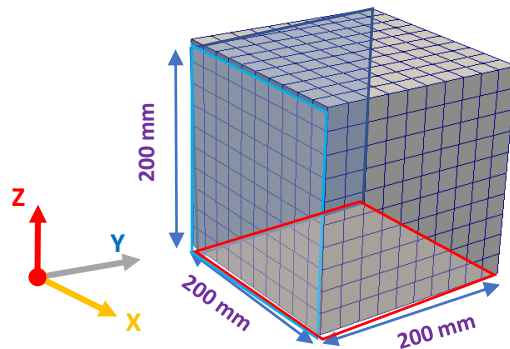
embedment gauges



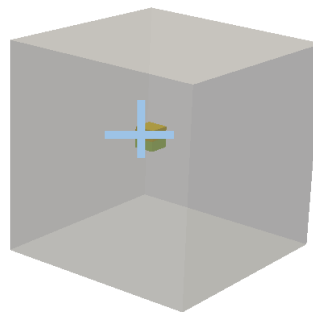
Gauge installation



Cross-section of test specimen

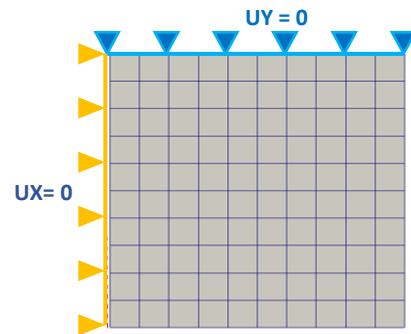


3D FEM

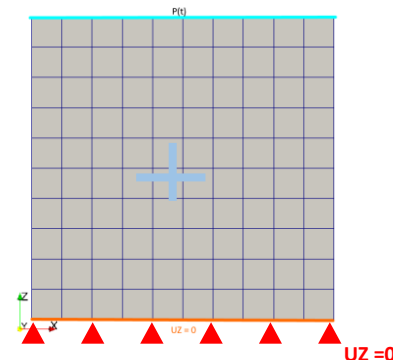


Gauge of FEM
(for FE Results)

Boundary Conditions



Top view



Lateral view

5 . V&V Study

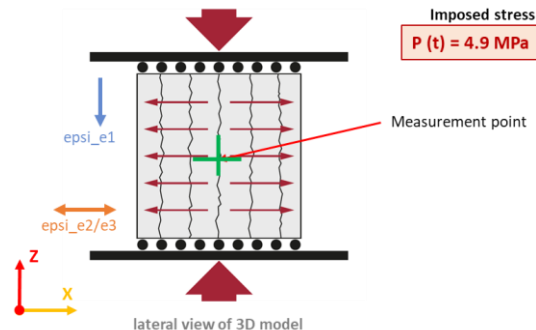
Validation test-case : KIM Study - Uniaxial

Material

- Burger parameters have been calibrated to achieve the strain from the Kim experiment (keeping in mind that Creep Poisson's ratio is keeping constant)

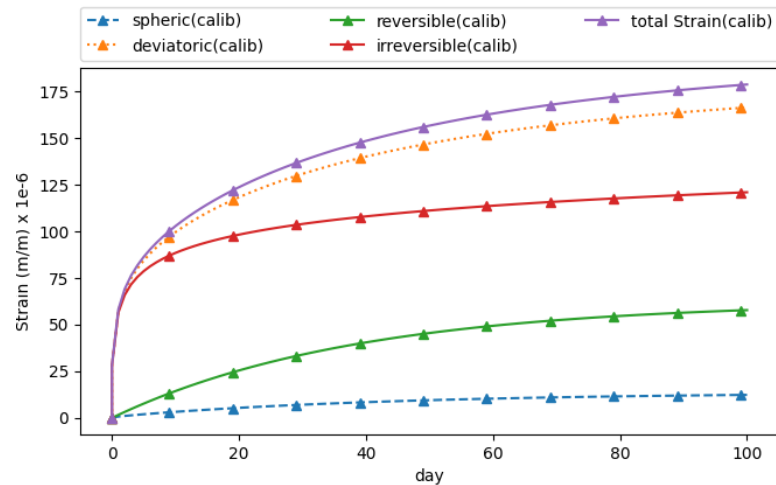
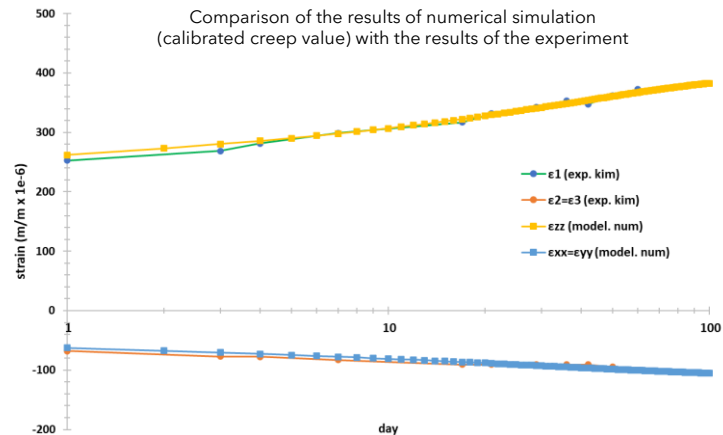
Calibrated creep parameters

NRD	=	2.07E+17
NID	=	2.33E+11
KRD	=	5.26E+10
KRS	=	1.28E+11
NRS	=	4.80E+17
NIS	=	1.57E+13



Analysis

- The values obtained with the simulation are in good agreement with the values of the results of the Kim
- the reconstruction of reversible and irreversible strains is physically correct

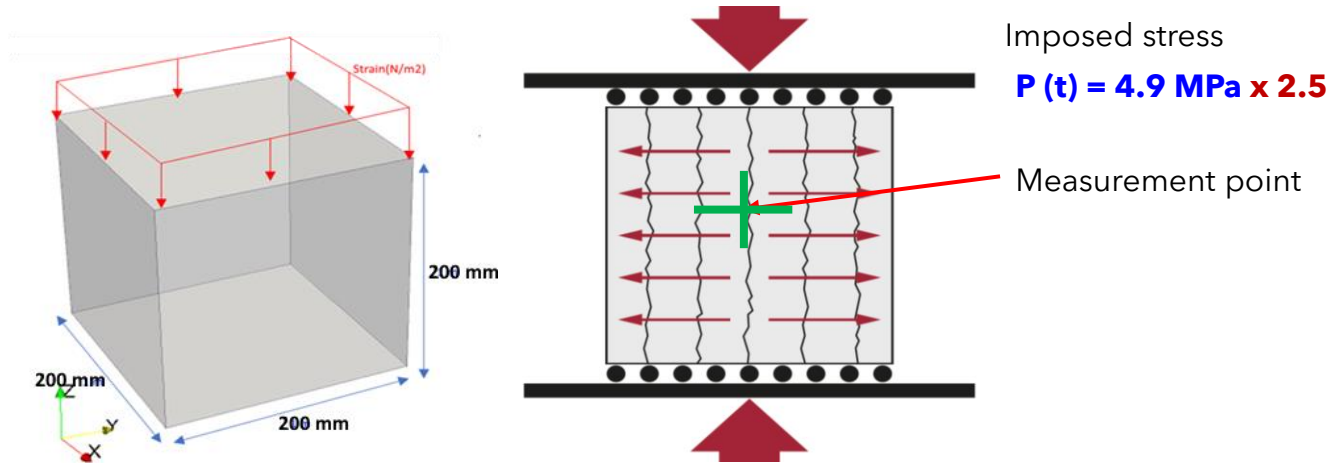


5 . V&V Study

Enable damage to KIM Study (verification) + Beta influence

Loading

The applied pressure is increased by a factor of 2.5 compared to the initial model (and experiment) to enable the damage brought by creep itself



As the damage is homogeneous, no visual is being proposed

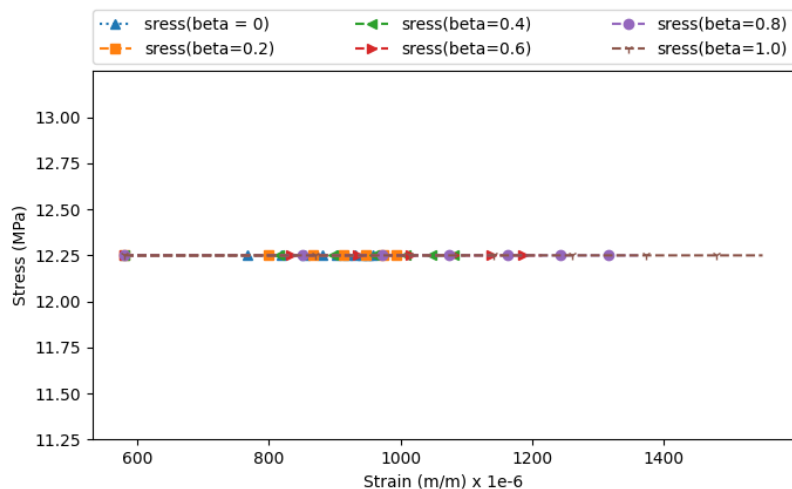
5 . V&V Study

Enable damage to KIM Study (verification) + Beta influence

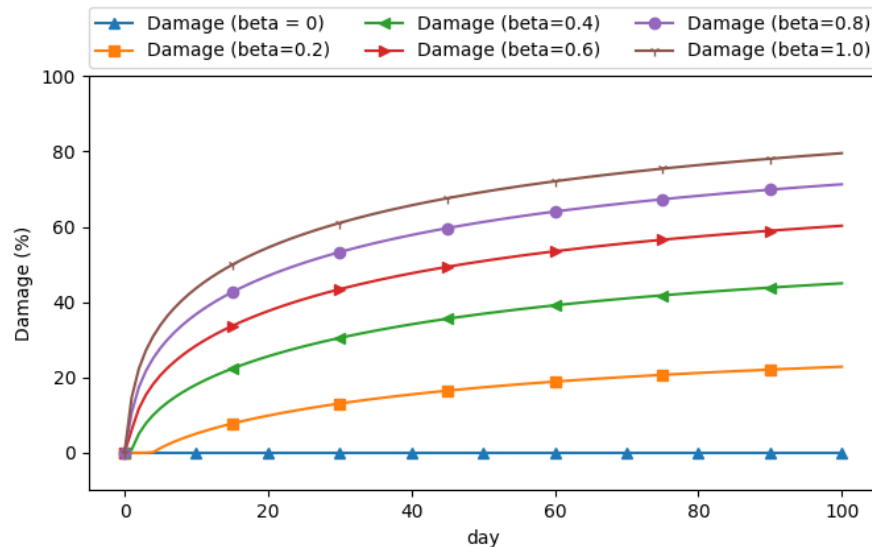
Full Name	Name	Value	Type	Model
Young Modulus	E	24010 MPa	Material	ELAS/FLB
Fracture Energy	Gf	60 J/mm ²	Material	FLB
Tensile Strength	f_t	4 MPa	Material	FLB
--	α	60	Material	FLB
Poisson's ratio	ν	0.168	Material	ELAS
viscosity associated with deviatoric	NRD	2.07E+17 Pa	Material	BURGER
viscosity connects associated with the deviatoric unrecoverable deformations	NID	2.33E+11 Pa.s	Material	BURGER
rigidity connects associated with deviatoric	KRD	5.26E+10 Pa	Material	BURGER
rigidity connects associated with spherical	KRS	1.28E+11 Pa	Material	BURGER
viscosity connects associated with spherical	NRS	4.80E+17 Pa.s	Material	BURGER
viscosity connects associated with the spherical unrecoverable deformations	NIS	1.57E+13 Pa.s	Material	BURGER
creep part of the damage Beta	Beta	0 - 1 (variation → 0 ; 0.2; 0.4; 0.6; 0.8;1)	Coupling	FLB-BURGER

5 . V&V Study

Enable damage to KIM Study (verification) + Beta influence



Applied Stress (total strain evolution)

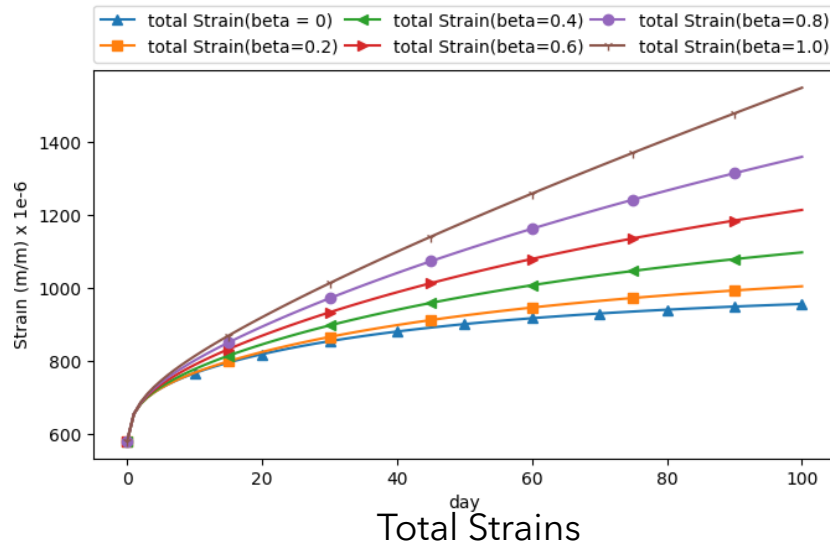
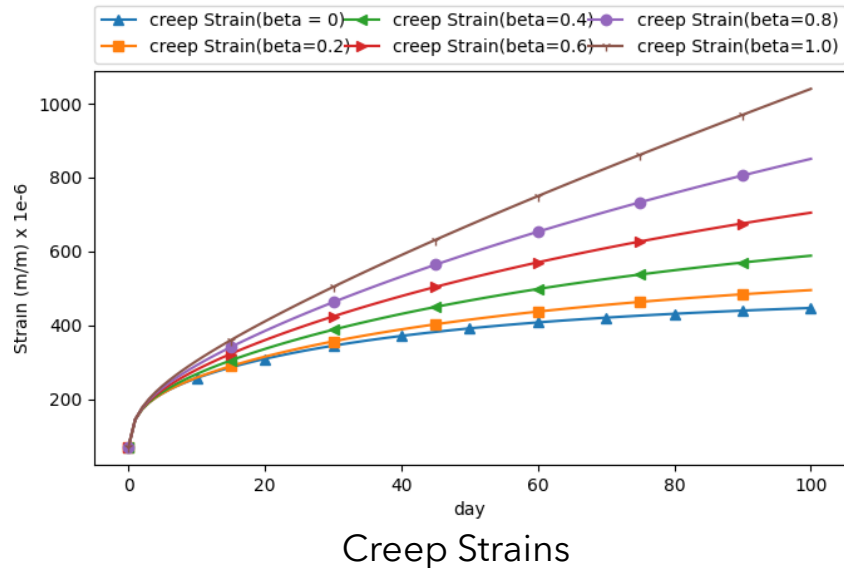


Damage Evolution

→ More beta is close to one, more the damage linked the basic creep is taking into account

5 . V&V Study

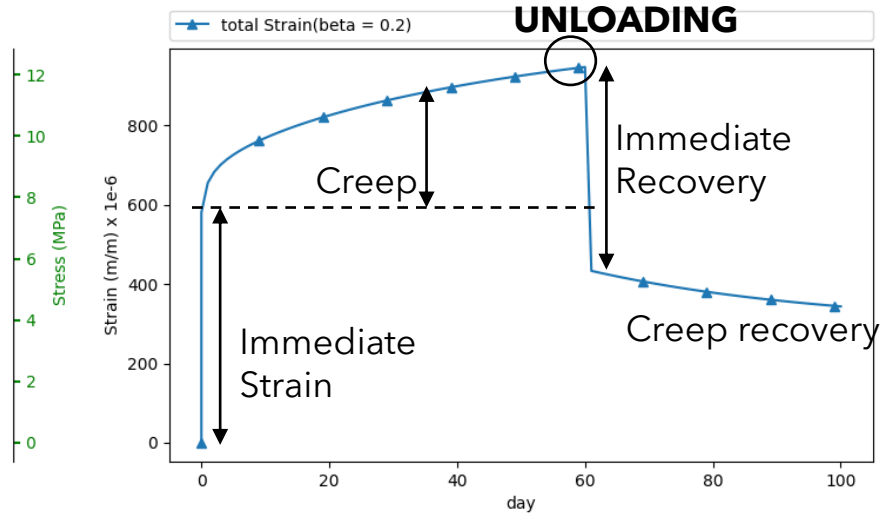
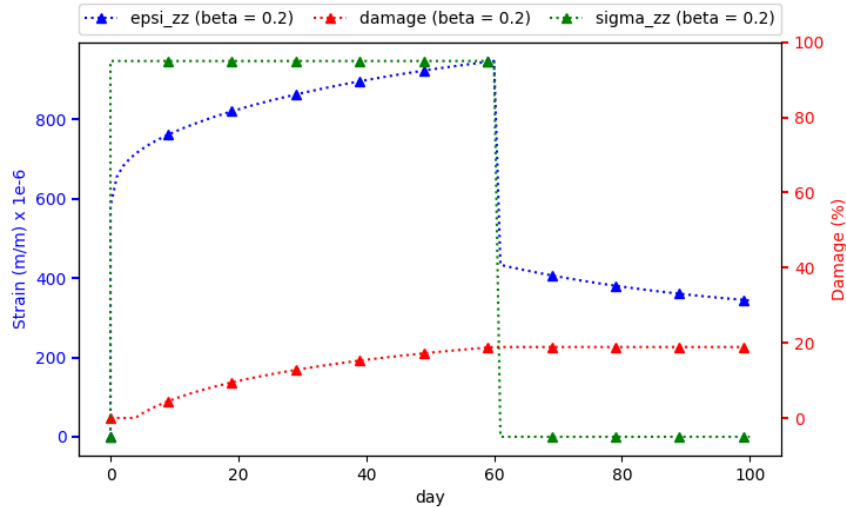
Enable damage to KIM Study (verification) + Beta influence



→ More beta is close to one, more the damage is and more the strain is

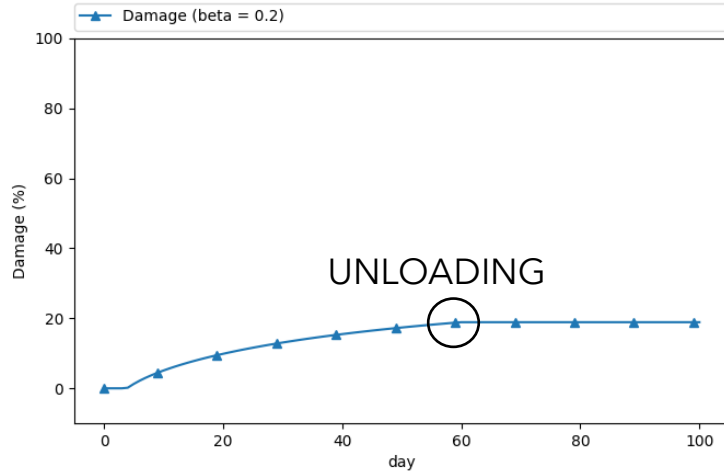
5 . V&V Study

Enable damage to KIM Study (verification) : Beta set to 0.2 + Unloading

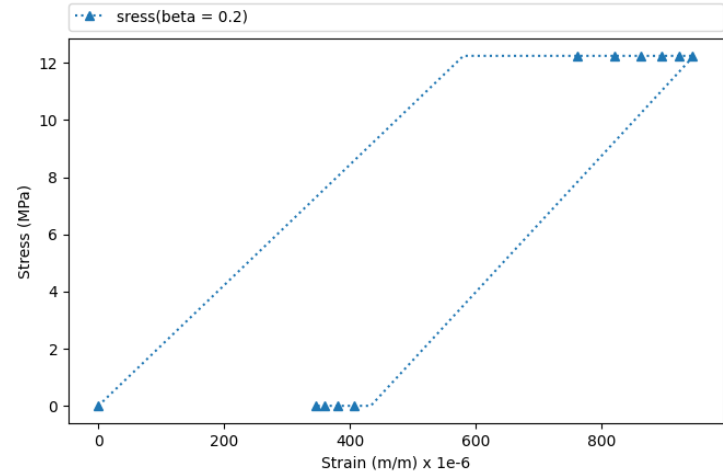


5 . V&V Study

Enable damage to KIM Study (verification) : Beta set to 0.2 + Unloading



Damage Evolution



Macroscopic Response

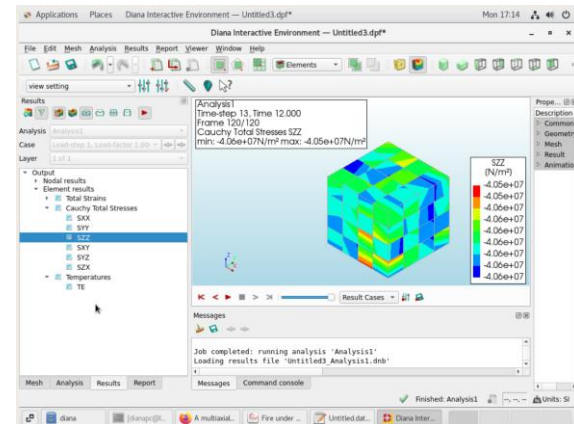
6 . Conclusion and Perspective

Conclusion

- A coupled model has been developed : Damage + Basic creep (tested under code_aster)
- This models contains few parameters (experimentally accessible with calibration)
- Burger parameters influences have been analysed (*not showed in that presentation*)
- The coupling with Thermal creep has not been fully validated yet (the mfront is ready)
- Primary results are encouraging : Burger_FLB gives a good accordance to the basic creep theory

Perspective

- Extend to an industrial test-case (HPB Pre-stressed Vessel)
- CEA developed Mfront/Diana FEA interface to support UK supply chain
- Tested by EDF Energy UK Engineer through Diana Mfront Interface



Questions time !

THANKS FOR YOUR ATTENTION