

# MECÂNICA DE ESTRUTURAS AEROESPACIAIS

## Problems of practical classes of the chapter BENDING OF THIN PLATES

### Problem 7.1

A plate 10 mm thick is subjected to bending moments  $M_x$  equal to 10 N/mm and  $M_y$  equal to 5 N/mm. Calculate the maximum direct stresses in the plate.

Ans.  $\sigma_{x,\max} = \pm 600 \text{ N/mm}^2$ ,  $\sigma_{y,\max} = \pm 300 \text{ N/mm}^2$ .

### Solution

Substituting for

$((1/\rho_x) + (\nu/\rho_y))$  and  $((1/\rho_y) + (\nu/\rho_x))$  from Eqs

$$M_x = D \left( \frac{1}{\rho_x} + \frac{\nu}{\rho_y} \right) \quad M_y = D \left( \frac{1}{\rho_y} + \frac{\nu}{\rho_x} \right)$$

respectively in Eqs

$$\left. \begin{aligned} \sigma_x &= \frac{Ez}{1 - \nu^2} \left( \frac{1}{\rho_x} + \frac{\nu}{\rho_y} \right) \\ \sigma_y &= \frac{Ez}{1 - \nu^2} \left( \frac{1}{\rho_y} + \frac{\nu}{\rho_x} \right) \end{aligned} \right\}$$

gives

$$\sigma_x = \frac{Ez}{1 - \nu^2} \frac{M_x}{D} \quad \text{and} \quad \sigma_y = \frac{Ez}{1 - \nu^2} \frac{M_y}{D}$$

Hence, since, from Eq.

$$D = \int_{-t/2}^{t/2} \frac{Ez^2}{1 - \nu^2} dz = \frac{Et^3}{12(1 - \nu^2)}$$

Eqs for  $\sigma_x$  and  $\sigma_y$  become

$$\sigma_x = \frac{12z M_x}{t^3} \quad \sigma_y = \frac{12z M_y}{t^3}$$

The maximum values of  $\sigma_x$  and  $\sigma_y$  will occur when  $z = \pm t/2$ .

Hence

$$\sigma_x(\max) = \pm \frac{6M_x}{t^2} \quad \sigma_y(\max) = \pm \frac{6M_y}{t^2}$$

Then

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$$\sigma_x(\text{max}) = \pm \frac{6 \times 10 \times 10^3}{10^2} = \pm 600 \text{ N/mm}^2$$

$$\sigma_y(\text{max}) = \pm \frac{6 \times 5 \times 10^3}{10^2} = \pm 300 \text{ N/mm}^2$$

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### Problem 7.2

For the plate and loading of problem **P.7.1** find the maximum twisting moment per unit length in the plate and the direction of the planes on which this occurs.

Ans. 2.5 Nm/mm at  $45^\circ$  to the  $x$  and  $y$  axes.

### Solution

From Eq.

$$M_t = \frac{(M_x - M_y)}{2} \sin 2\alpha + M_{xy} \cos 2\alpha$$

and since  $M_{xy} = 0$

$$M_t = \frac{M_x - M_y}{2} \sin 2\alpha$$

$M_t$  will be a maximum when  $2\alpha = \pi/2$ , i.e.  $\alpha = \pi/4$  ( $45^\circ$ ). Thus, from Eq. (i)

$$M_t(\max) = \frac{10 - 5}{2} = 2.5 \text{ Nm/mm}$$

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### Problem 7.3

The plate of the previous two problems is subjected to a twisting moment of 5 Nm/mm along each edge, in addition to the bending moments of  $M_x = 10$  Nm/mm and  $M_y = 5$  Nm/mm. Determine the principal moments in the plate, the planes on which they act and the corresponding principal stresses.

Ans. 13.1 Nm/mm, 1.9Nm/mm,  $\alpha = -31.7^\circ$ ,  $\alpha = +58.3^\circ$ ,  $\pm 786$  N/mm<sup>2</sup>,  $\pm 114$  N/mm<sup>2</sup>.

### Solution

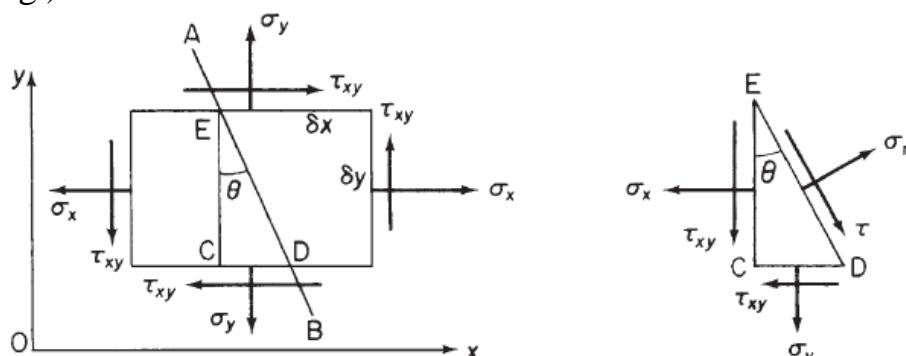
The relationship between  $M_n$  and  $M_x$ ,  $M_y$  and  $M_{xy}$  in Eq.

$$M_n = M_x \cos^2 \alpha + M_y \sin^2 \alpha - M_{xy} \sin 2\alpha$$

and between  $M_t$  and  $M_x$ ,  $M_y$  and  $M_{xy}$  in Eq.

$$M_t = \frac{(M_x - M_y)}{2} \sin 2\alpha + M_{xy} \cos 2\alpha$$

are identical in form to the stress relationships in Eqs (in accordance with the given fig.)



$$\sigma_n = \sigma_x \cos^2 \theta + \sigma_y \sin^2 \theta + \tau_{xy} \sin 2\theta \quad \tau = \frac{(\sigma_x - \sigma_y)}{2} \sin 2\theta - \tau_{xy} \cos 2\theta$$

Therefore, by deduction from Eqs

$$\sigma_I = \frac{\sigma_x + \sigma_y}{2} + \frac{1}{2} \sqrt{(\sigma_x - \sigma_y)^2 + 4\tau_{xy}^2} \quad \sigma_{II} = \frac{\sigma_x + \sigma_y}{2} - \frac{1}{2} \sqrt{(\sigma_x - \sigma_y)^2 + 4\tau_{xy}^2}$$

we have

$$M_I = \frac{M_x + M_y}{2} + \frac{1}{2} \sqrt{(M_x - M_y)^2 + 4M_{xy}^2} \quad M_{II} = \frac{M_x + M_y}{2} - \frac{1}{2} \sqrt{(M_x - M_y)^2 + 4M_{xy}^2}$$

Further, Eq.

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$$M_t = \frac{(M_x - M_y)}{2} \sin 2\alpha + M_{xy} \cos 2\alpha$$

gives the inclination of the planes on which the principal moments occur, i.e. when  $M_t = 0$ . Thus

$$\tan 2\alpha = -\frac{2M_{xy}}{M_x - M_y}$$

Substituting the values  $M_x = 10$  Nm/mm,  $M_y = 5$  Nm/mm and  $M_{xy} = 5$  Nm/mm in all derived Eqs gives

$$M_I = 13.1 \text{ Nm/mm}$$

$$M_{II} = 1.9 \text{ Nm/mm}$$

$$\alpha = -31.7^\circ \text{ or } 58.3^\circ$$

The corresponding principal stresses are obtained directly from Eqs

$$\sigma_x(\max) = \pm \frac{6M_x}{t^2} \quad \sigma_y(\max) = \pm \frac{6M_y}{t^2}$$

Hence

$$\sigma_I = \pm \frac{6 \times 13.1 \times 10^3}{10^2} = \pm 786 \text{ N/mm}^2$$

$$\sigma_{II} = \pm \frac{6 \times 1.9 \times 10^3}{10^2} = \pm 114 \text{ N/mm}^2$$