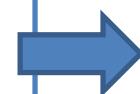


Structural idealization

2020

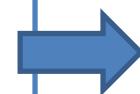
Principle

stringers have small cross-section compared with the complete section

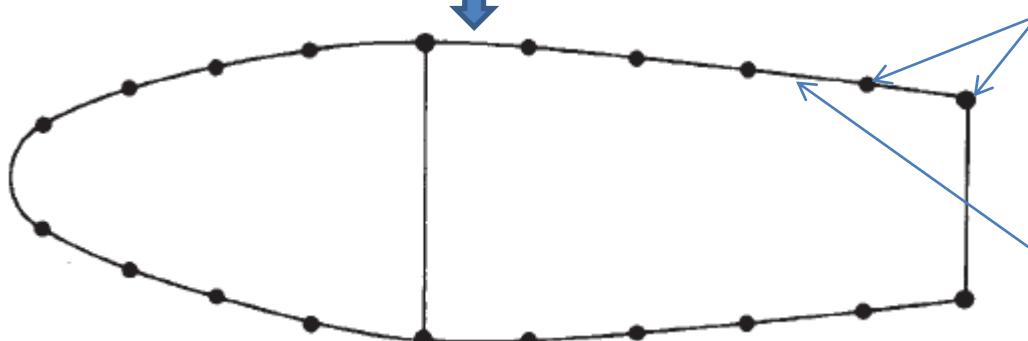
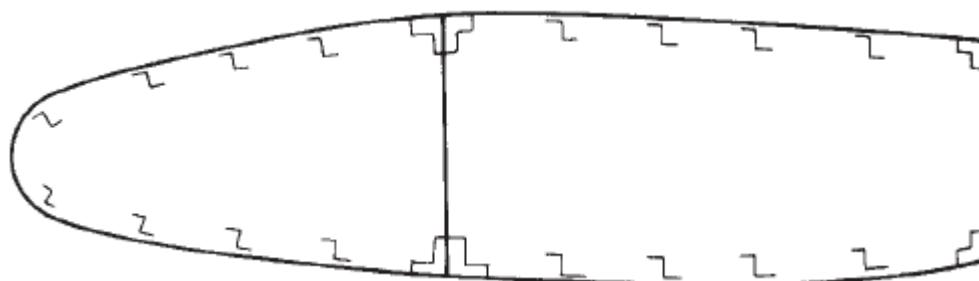


variation of stress over the cross-section of a stringer is small

the difference between the distances of the stringer centroids and the adjacent skin from the wing section axis is small



the σ is constant over the stringer cross-sections



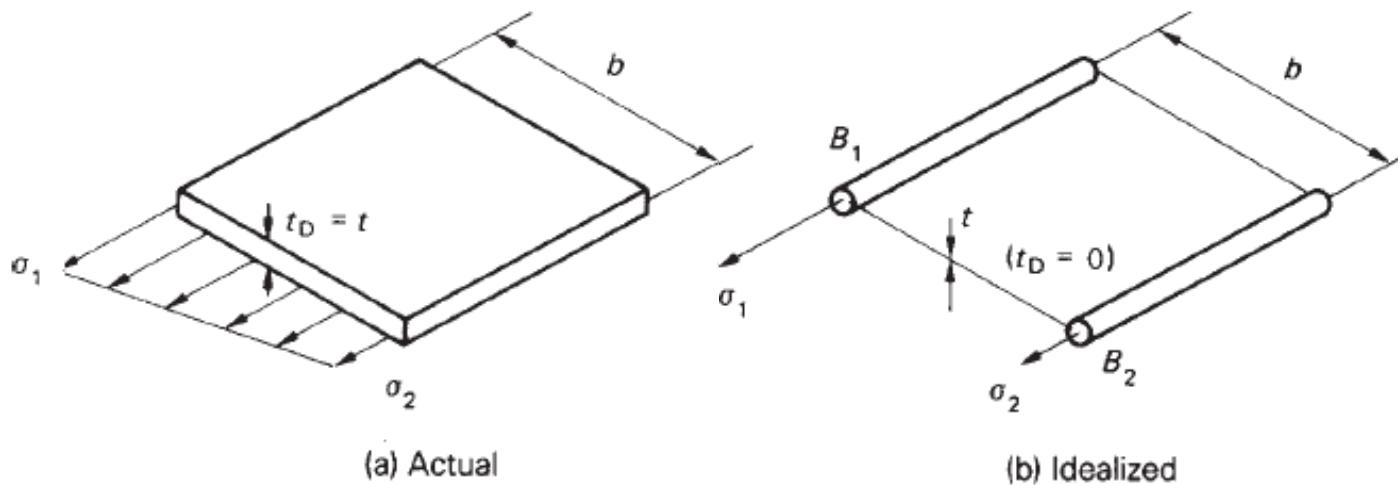
Boom:

- $\sigma = \text{const}$;
- located in mid-line of skin;
- carries only direct stresses.

Skin is effective only in shear (although can carry some of the direct stresses)

Idealization of a panel

Idealize the panel into a combination of direct stress carrying booms and shear stress only carrying skin:



Taking moments about the right-hand edge of each panel:

$$\sigma_2 t_D \frac{b^2}{2} + \frac{1}{2}(\sigma_1 - \sigma_2)t_D b \frac{2}{3}b = \sigma_1 B_1 b$$

different loading conditions require different idealizations of the same structure

$$B_1 = B_2 = t_D b / 6 \quad \begin{matrix} \sigma_1 / \sigma_2 = -1 \\ \text{bending moment} \end{matrix} \quad \left\{ \begin{array}{l} B_1 = \frac{t_D b}{6} \left(2 + \frac{\sigma_2}{\sigma_1} \right) \\ B_2 = \frac{t_D b}{6} \left(2 + \frac{\sigma_1}{\sigma_2} \right) \end{array} \right\} \quad \begin{matrix} \sigma_1 / \sigma_2 = 1 \\ \text{axial load} \end{matrix} \quad B_1 = B_2 = t_D b / 2$$

Idealization of part of a wing section

Idealize, if

- booms carry direct stresses;
- panels carry shear stresses;
- section resists bending moments in vertical plane;
- booms can be applied at the spar/skin junctions.

Solution:

$$\text{Area of the booms: } B_1 = \frac{t_D b}{6} \left(2 + \frac{\sigma_2}{\sigma_1} \right)$$

$$B_1 = 300 + \frac{3 \cdot 400}{6} \left(2 + \frac{\sigma_6}{\sigma_1} \right) + \frac{2 \cdot 600}{6} \left(2 + \frac{\sigma_2}{\sigma_1} \right) \quad \frac{\sigma_6}{\sigma_1} = -1$$

angle spar 1-6 skin 1-2

$$B_1 = B_6 = 1050 \text{ mm}^2$$

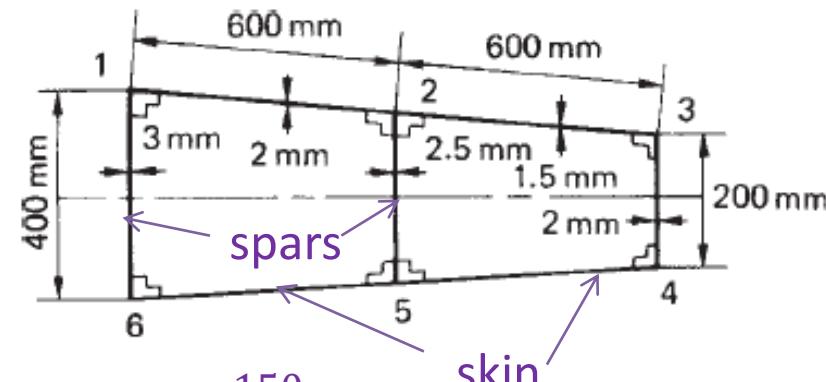
$$B_2 = 2 \cdot 300 + \frac{2 \cdot 600}{6} \left(2 + \frac{\sigma_1}{\sigma_2} \right) + \frac{2.5 \cdot 300}{6} \left(2 + \frac{\sigma_5}{\sigma_2} \right) + \frac{1.5 \cdot 600}{6} \left(2 + \frac{\sigma_3}{\sigma_2} \right) \quad \frac{\sigma_2}{\sigma_1} = \frac{150}{200}$$

$\frac{100}{150}$ $\frac{150}{100}$ $\frac{1}{1}$

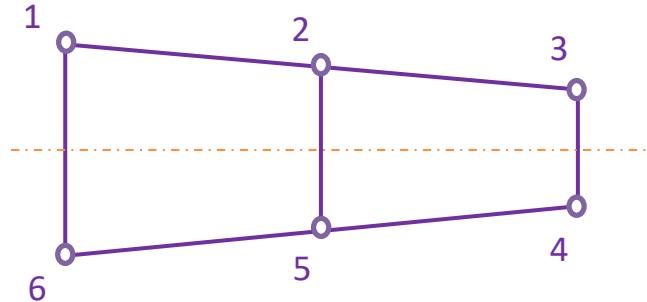
$$B_2 = B_5 = 1791.7 \text{ mm}^2$$

$$B_3 = 300 + \frac{1.5 \cdot 600}{6} \left(2 + \frac{\sigma_2}{\sigma_3} \right) + \frac{2 \cdot 200}{6} \left(2 + \frac{\sigma_4}{\sigma_3} \right) \quad \frac{\sigma_4}{\sigma_3} = \frac{1}{1}$$

Angle sections area = 300 mm²



$$\frac{\sigma_2}{\sigma_1} = \frac{150}{200}$$



Idealized structure

$$B_3 = B_4 = 891.7 \text{ mm}^2$$

Bending

If idealized cross-section comprises

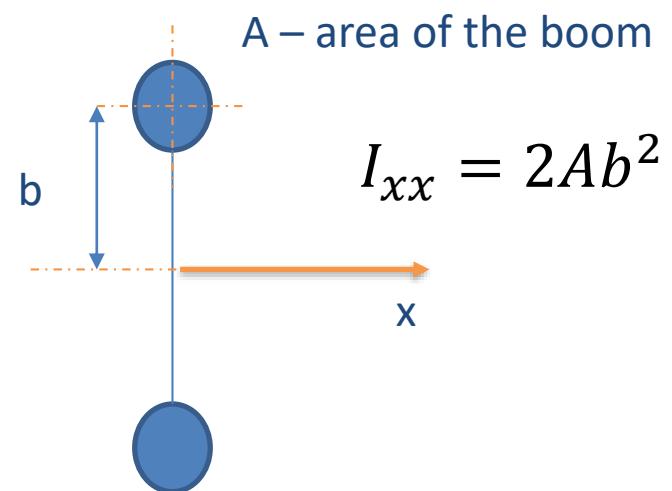
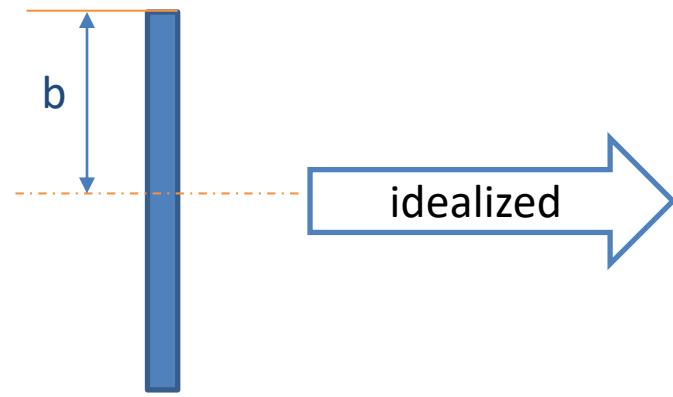
- direct stress carrying booms,
- shear stress only carrying skin ($t_D = 0$);

then

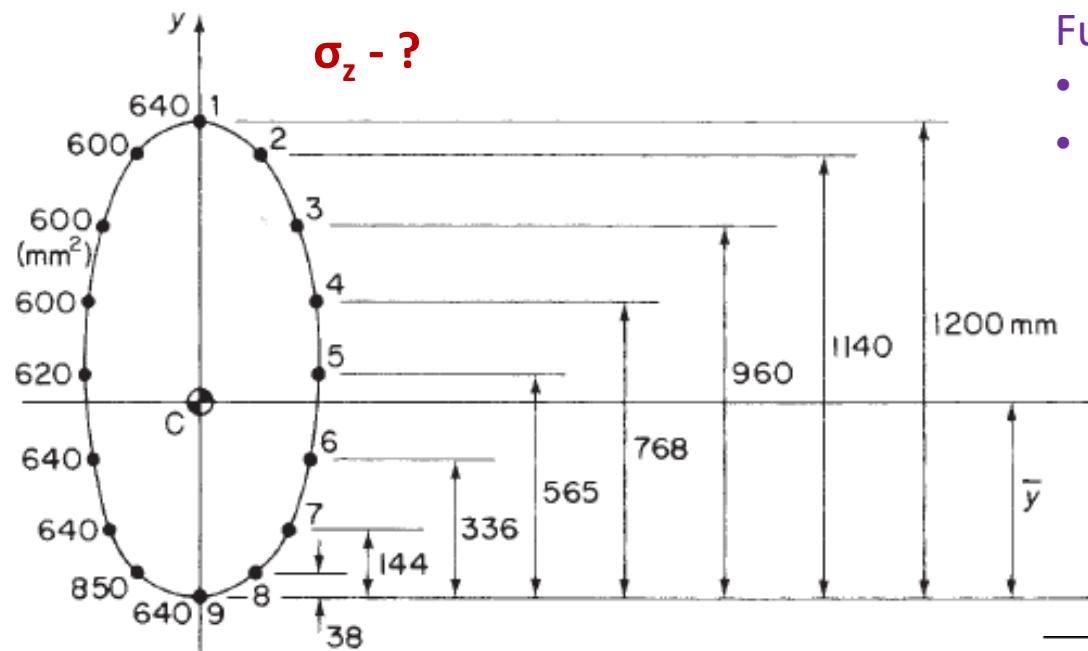
- neutral axis,
- sectional properties (I_{xx} , I_{yy} , I_{xy})

are defined only by area of booms.

For instance



Bending



Fuselage section:

- subjected to $M_B = 100 \text{ kN}\cdot\text{m}$;
- completely idealized into a combination of
 - direct stress carrying booms,
 - shear stress carrying panels.

$$\sigma_z = \left(\frac{M_y I_{xx} - M_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) x + \left(\frac{M_x I_{yy} - M_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) y \quad \Rightarrow$$

$$\sigma_z = \frac{M_x}{I_{xx}} y$$

Centroid from boom 9:

$$(6 \times 640 + 6 \times 600 + 2 \times 620 + 2 \times 850) \bar{y} \\ = 640 \times 1200 + 2 \times 600 \times 1140 + 2 \times 600 \times 960 + 2 \times 600 \times 768 \\ + 2 \times 620 \times 565 + 2 \times 640 \times 336 + 2 \times 640 \times 144 + 2 \times 850 \times 38$$

$$\Rightarrow \bar{y} = 540 \text{ mm}$$

$$I_{xx.4} = B y^2 = 600 \cdot (768 - 540)^2 = 31 \cdot 10^6 \text{ mm}^4$$

$$\sigma_{z.4} = 100 \cdot 10^6 \text{ N} \cdot \text{mm} \cdot (768 - 540) \text{ mm} / 1854 \cdot 10^6 \text{ mm}^4$$

$$\sigma_{z.4} = 12.3 \text{ N/mm}^2$$

① Boom	② y (mm)	③ B (mm^2)	④ $\Delta I_{xx} = B y^2$ (mm^4)	⑤ σ_z (N/mm^2)
1	+660	640	278×10^6	35.6
2	+600	600	216×10^6	32.3
3	+420	600	106×10^6	22.6
4	+228	600	31×10^6	12.3
5	+25	620	0.4×10^6	1.3
6	-204	640	27×10^6	-11.0
7	-396	640	100×10^6	-21.4
8	-502	850	214×10^6	-27.0
9	-540	640	187×10^6	-29.0

Sum of terms in column 4: $I_{xx} = 1854 \times 10^6 \text{ mm}^4$

Shear of open section beams

$$q_s = - \left(\frac{S_x I_{xx} - S_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \int_0^s t_D x \, ds - \left(\frac{S_y I_{yy} - S_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \int_0^s t_D y \, ds$$

skin effective in carrying direct stress

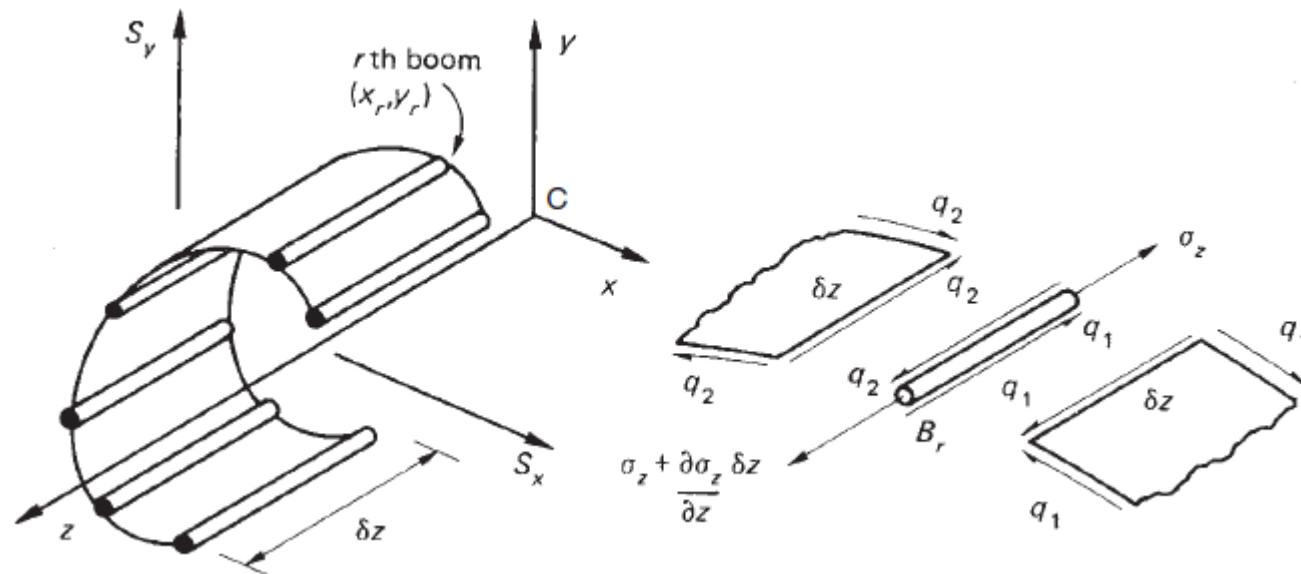
$$t_D = t$$



skin carries only shear stresses

$$t_D = 0$$

no provision for the effects of booms



$$\Sigma F_z = 0 \quad \left(\sigma_z + \frac{\partial \sigma_z}{\partial z} \delta z \right) B_r - \sigma_z B_r + q_2 \delta z - q_1 \delta z = 0 \quad \rightarrow \quad q_2 - q_1 = - \frac{\partial \sigma_z}{\partial z} B_r$$

Shear of open section beams

$$q_2 - q_1 = -\frac{\partial \sigma_z}{\partial z} B_r \quad \frac{\frac{\partial \sigma_z}{\partial z} = \frac{[(\partial M_y / \partial z) I_{xx} - (\partial M_x / \partial z) I_{xy}]}{I_{xx} I_{yy} - I_{xy}^2} x + \frac{[(\partial M_x / \partial z) I_{yy} - (\partial M_y / \partial z) I_{xy}]}{I_{xx} I_{yy} - I_{xy}^2} y}{\text{the same procedure of derivation as in lecture for shear}}$$

$$q_2 - q_1 = - \left[\frac{(\partial M_y / \partial z) I_{xx} - (\partial M_x / \partial z) I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right] B_r x_r - \left[\frac{(\partial M_x / \partial z) I_{yy} - (\partial M_y / \partial z) I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right] B_r y_r$$

$\downarrow \partial M_y / \partial z = S_x$

Coordinates of a boom under consideration

$$q_2 - q_1 = - \left(\frac{S_x I_{xx} - S_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) B_r x_r - \left(\frac{S_y I_{yy} - S_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) B_r y_r$$

the change in shear flow induced by a boom

At any distance s around the profile of the section, n booms have been passed, the shear flow at the point is given by

$$q_s = - \left(\frac{S_x I_{xx} - S_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left(\int_0^s t_D x \, ds + \sum_{r=1}^n B_r x_r \right) - \left(\frac{S_y I_{yy} - S_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left(\int_0^s t_D y \, ds + \sum_{r=1}^n B_r y_r \right)$$

The terms are taken into consideration only in case when skin effective in carrying direct stress

Shear of open section beams

Channel section with area of each boom 300 mm²:

- shear load acts through SC;
- assume that
 - booms carry all the direct stresses,
 - walls are effective only in resisting shear stresses.

Solution:

$$q_s = - \left(\frac{S_x I_{xx} - S_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left(\int_0^s t_D x \, ds + \sum_{r=1}^n B_r x_r \right) - \left(\frac{S_y I_{yy} - S_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left(\int_0^s t_D y \, ds + \sum_{r=1}^n B_r y_r \right)$$

$$\downarrow \quad I_{xy} = 0 \quad S_x = 0 \quad t_D = 0$$

$$q_s = - \frac{S_y}{I_{xx}} \sum_{r=1}^n B_r y_r \quad I_{xx} = 4 \cdot 300 \cdot 200^2 = 4.8 \cdot 10^7 \text{ mm}^4$$

$$q_s = - \frac{4.8 \cdot 10^3 \text{ N}}{4.8 \cdot 10^7 \text{ mm}^4} \sum_{r=1}^n B_r y_r = -0.1 \cdot 10^{-3} \sum_{r=1}^n B_r y_r$$

Before the boom 1 (outside the section): $q_s = 0$

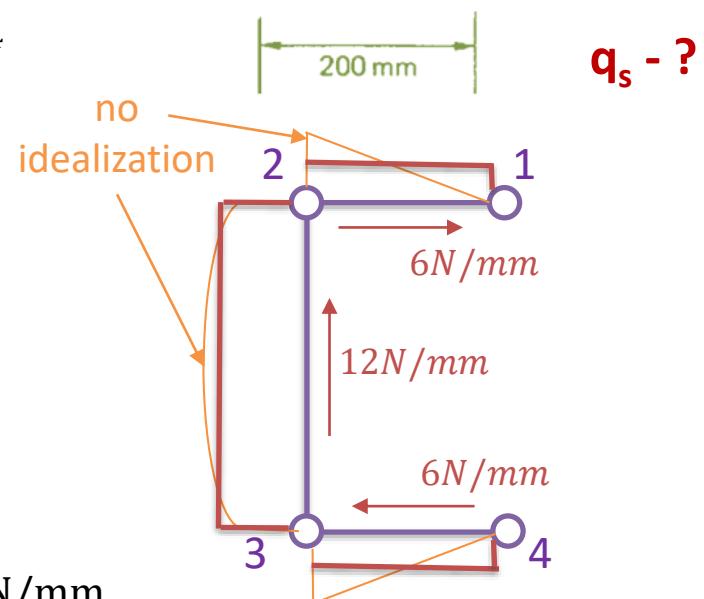
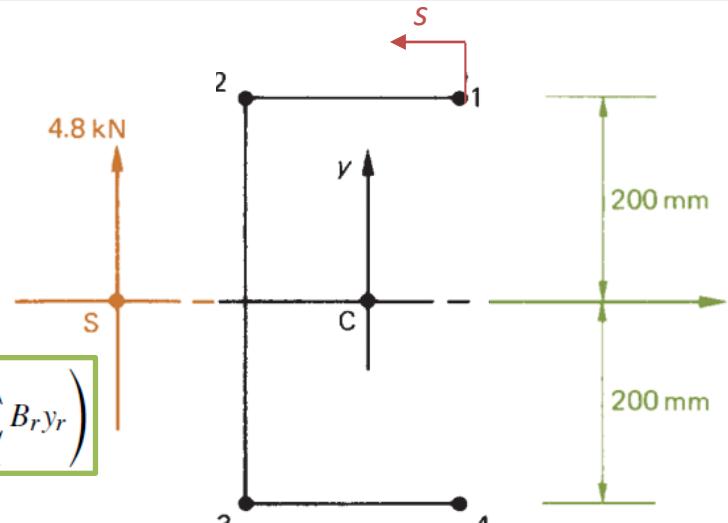
After the boom 1: $\Delta q_1 = -0.1 \cdot 10^{-3} \cdot 300 \cdot 200 = -6 \text{ N/mm}$

Thus $q_{12} = -6 \text{ N/mm}$

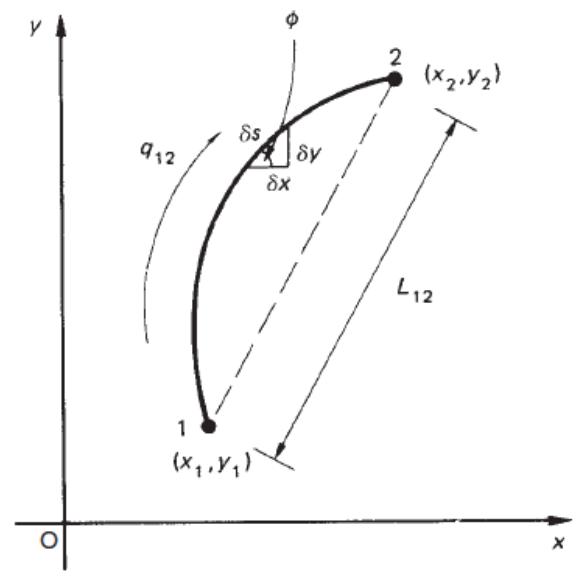
$$q_{23} = -6 - 0.1 \cdot 10^{-3} \cdot 300 \cdot 200 = -12 \text{ N/mm}$$

$$q_{34} = -12 - 0.1 \cdot 10^{-3} \cdot 300 \cdot (-200) = -6 \text{ N/mm}$$

$$\text{Outside the boom 4 } \Delta q_4 = -6 - 0.1 \cdot 10^{-3} \cdot 300 \cdot (-200) = 0 \text{ N/mm}$$



Resultant shear load of curved web



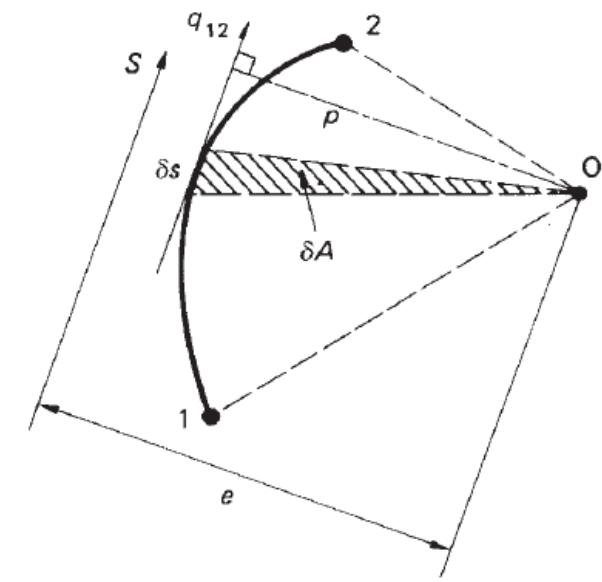
$$S_x = \int_1^2 q_{12} \cos \phi \, ds \xrightarrow{q_{12} = \text{const}} S_x = q_{12} \int_1^2 \cos \phi \, ds$$

$$S_x = q_{12} \int_1^2 dx = q_{12}(x_2 - x_1)$$

$$S_y = q_{12}(y_2 - y_1)$$

$$S = \sqrt{S_x^2 + S_y^2} = q_{12} \sqrt{(x_2 - x_1)^2 + (y_2 - y_1)^2}$$

$$S = q_{12} L_{12}$$



$$M_q = \int_1^2 q_{12} p \, ds = q_{12} \int_1^2 2 \, dA \xrightarrow{} M_q = 2A q_{12}$$

$$Se = 2A q_{12} \xrightarrow{} e = \frac{2A}{S} q_{12} \xrightarrow{} e = \frac{2A}{L_{12}}$$

$$e = \frac{2A}{L_{12}}$$

Shear of closed section beams

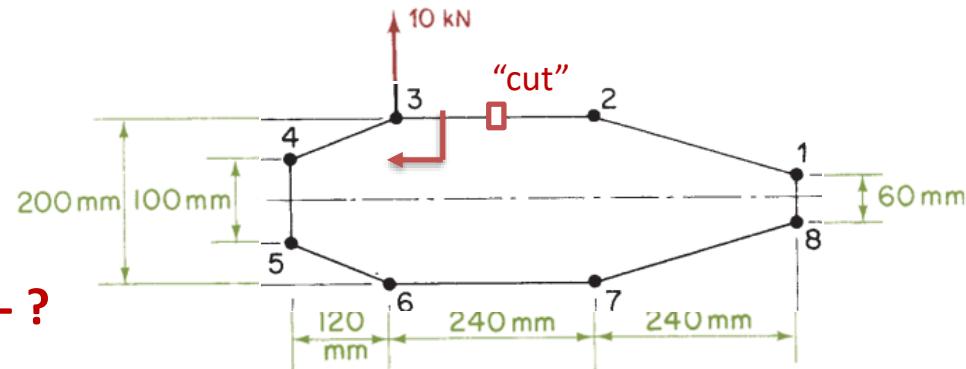
Idealization

- booms carry all the direct stresses,
- walls are effective only in resisting shear stresses.

Booms area: $B_1 = B_8 = 200 \text{ mm}^2$, $B_2 = B_7 = 250 \text{ mm}^2$,

$B_3 = B_6 = 400 \text{ mm}^2$, $B_4 = B_5 = 100 \text{ mm}^2$

$q_s - ?$



Solution:

$$q_s = - \left(\frac{S_x I_{xx} - S_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left(\int_0^s t_D x \, ds + \sum_{r=1}^n B_r x_r \right) - \left(\frac{S_y I_{yy} - S_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left(\int_0^s t_D y \, ds + \sum_{r=1}^n B_r y_r \right)$$

↓ $I_{xy} = 0 \quad S_x = 0 \quad t_D = 0$

$$q_s = - \frac{S_y}{I_{xx}} \sum_{r=1}^n B_r y_r + q_{s,0} \quad I_{xx} = 2(200 \cdot 30^2 + 250 \cdot 100^2 + 400 \cdot 100^2 + 100 \cdot 50^2) = 13.86 \cdot 10^6 \text{ mm}^4$$

$$q_s = -7.22 \cdot 10^{-4} \sum_{r=1}^n B_r y_r + q_{s,0}$$

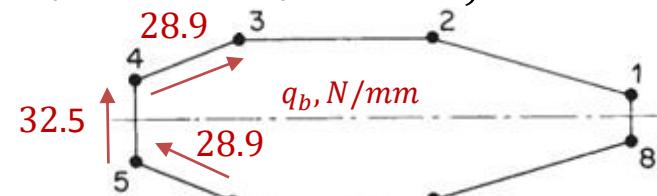
Web 2-3: $q_{b,23} = 0 \text{ N/mm}$

Web 3-4: $y_3 = 100 \quad q_{b,34} = -7.22 \cdot 10^{-4} (400 \cdot 100) + 0 = -28.9 \text{ N/mm}$

Web 4-5: $y_4 = 50 \quad q_{b,34} = -7.22 \cdot 10^{-4} (100 \cdot 50) - 28.9 = -32.5 \text{ N/mm}$

Web 5-6: $q_{b,56} = q_{b,34} = -28.9 \text{ N/mm}$

Web 6-7: $q_{b,67} = q_{b,23} = 0 \text{ N/mm}$



} due to symmetry

Shear of closed section beams

Booms area: $B_1 = B_8 = 200 \text{ mm}^2$, $B_2 = B_7 = 250 \text{ mm}^2$,
 $B_3 = B_6 = 400 \text{ mm}^2$, $B_4 = B_5 = 100 \text{ mm}^2$

$$q_s = -7.22 \cdot 10^{-4} \sum_{r=1}^n B_r y_r + q_{s,0}$$

Web 2-1: $y_3 = 100$

$$q_{b,21} = -7.22 \cdot 10^{-4} (250 \cdot 100) + 0 = -18.1 \text{ N/mm}$$

Web 1-8: $y_1 = 30$

$$q_{b,18} = -7.22 \cdot 10^{-4} (200 \cdot 30) - 18.1 = -22.4 \text{ N/mm}$$

Web 8-7: $q_{b,87} = q_{b,21} = -18.1 \text{ N/mm}$ (symmetry)

To determine $q_{s,0}$: $0 = \oint p q_b ds + 2A q_{s,0}$ $\Rightarrow q_{s,0} = -\frac{\oint p q_b ds}{2A}$

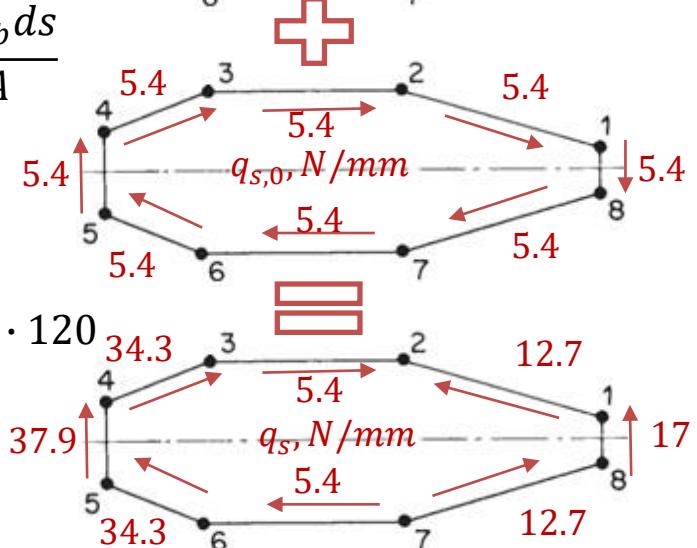
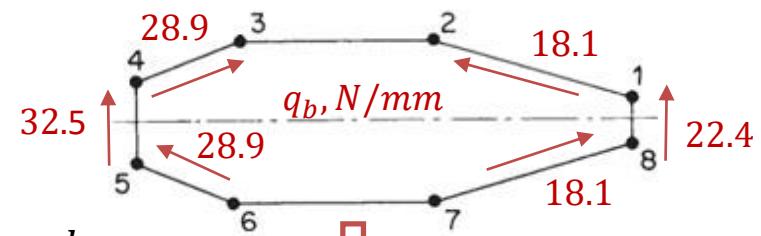
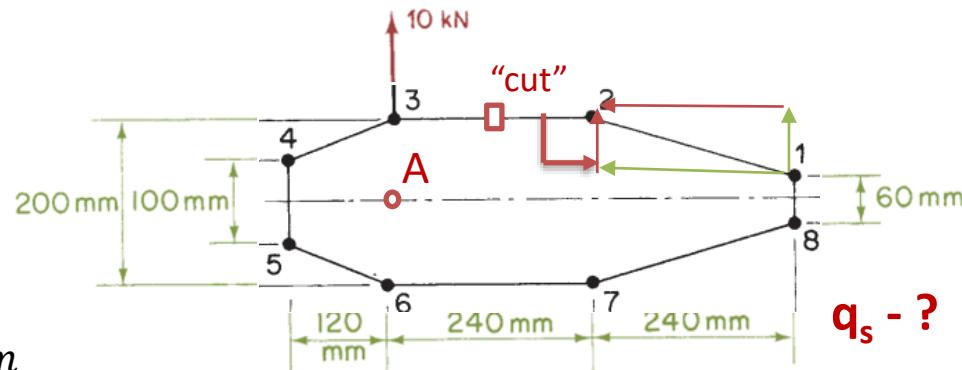
Taking moment about p. A (clockwise is negative):

$$\oint p q_b ds = q_{b,81} \cdot 60 \cdot 480 + 2q_{b,12} \cdot \left\{ \begin{array}{l} 240 \cdot 100 + 70 \cdot 240 \\ 240 \cdot 30 + 70 \cdot 480 \end{array} \right\} + q_{b,23} \cdot 240 \cdot 100 - 2q_{b,34} \cdot \left\{ \begin{array}{l} 120 \cdot 50 + 50 \cdot 120 \\ 120 \cdot 100 + 50 \cdot 0 \end{array} \right\} - q_{b,45} \cdot 100 \cdot 120$$

$$A = 97200 \text{ mm}^2$$

$$q_{s,0} = -5.4 \text{ N/mm}$$

Distribution of shear flow q_s :



Obrigado!