

# Structural idealization

2020

# Principle

stringers have small cross-section compared with the complete section

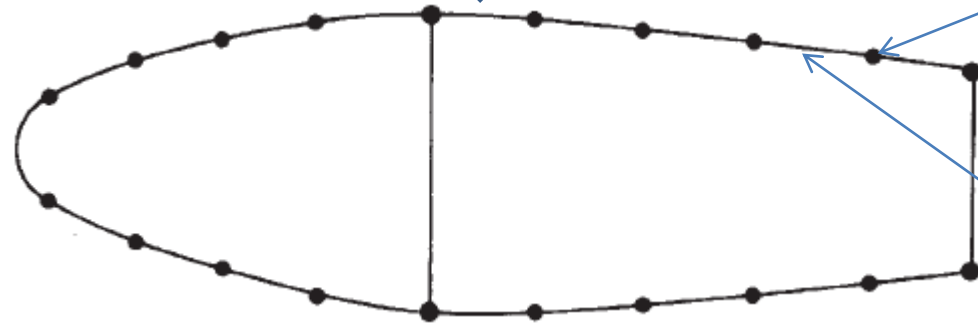
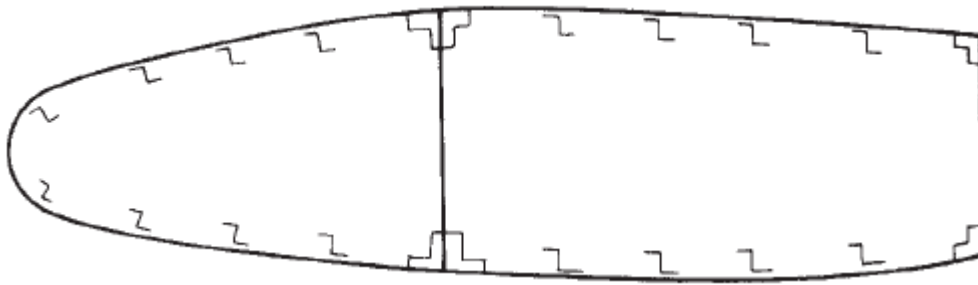


variation of stress over the cross-section of a stringer is small

the difference between the distances of the stringer centroids and the adjacent skin from the wing section axis is small



the  $\sigma$  is constant over the stringer cross-sections



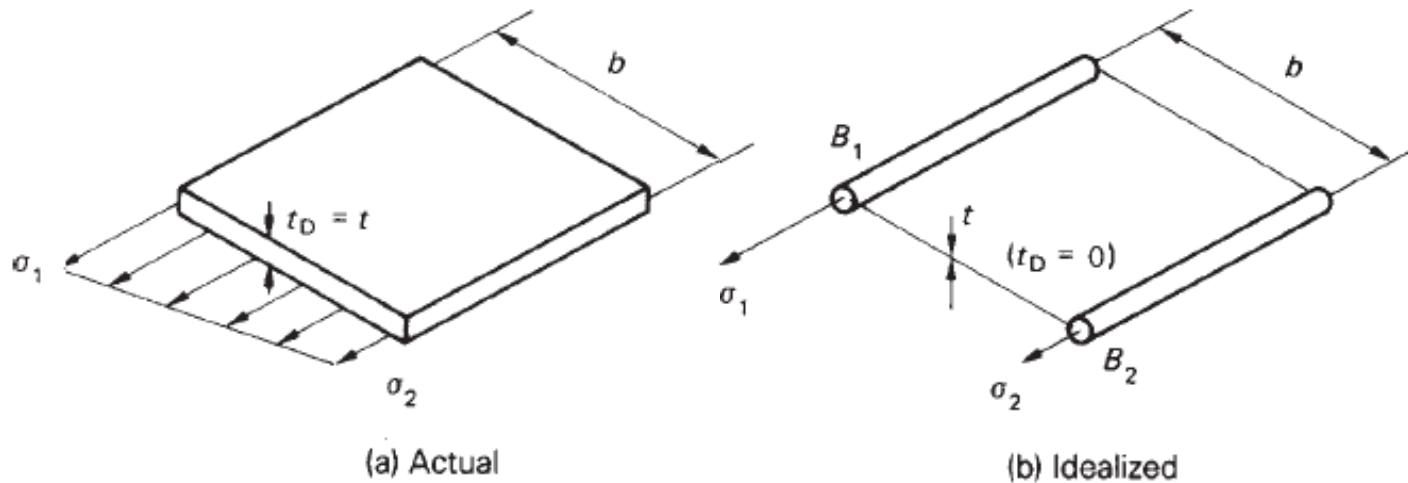
## **Boom:**

- $\sigma = \text{const}$ ;
- located in mid-line of skin;
- carries only direct stresses.

**Skin** is effective only in shear (although can carry some of the direct stresses)

# Idealization of a panel

Idealize the panel into a combination of direct stress carrying booms and shear stress only carrying skin:



Taking moments about the right-hand edge of each panel:

$$\sigma_2 t_D \frac{b^2}{2} + \frac{1}{2}(\sigma_1 - \sigma_2) t_D b \frac{2}{3} b = \sigma_1 B_1 b$$

different loading conditions require different idealizations of the same structure

$$B_1 = B_2 = t_D b / 6 \quad \xleftarrow[\text{bending moment}]{\sigma_1 / \sigma_2 = -1} \left\{ \begin{array}{l} B_1 = \frac{t_D b}{6} \left( 2 + \frac{\sigma_2}{\sigma_1} \right) \\ B_2 = \frac{t_D b}{6} \left( 2 + \frac{\sigma_1}{\sigma_2} \right) \end{array} \right. \xrightarrow[\text{axial load}]{\sigma_1 / \sigma_2 = 1} B_1 = B_2 = t_D b / 2$$

# Idealization of part of a wing section

- Idealize, if
- booms carry direct stresses;
  - panels carry shear stresses;
  - section resists bending moments in vertical plane;
  - booms can be applied at the spar/skin junctions.

**Solution:**

Area of the booms:

$$B_1 = \frac{t_D b}{6} \left( 2 + \frac{\sigma_2}{\sigma_1} \right)$$

$$B_1 = \underbrace{300}_{\text{angle}} + \underbrace{\frac{3 \cdot 400}{6} \left( 2 + \frac{\sigma_6}{\sigma_1} \right)}_{\text{spar 1-6}} + \underbrace{\frac{2 \cdot 600}{6} \left( 2 + \frac{\sigma_2}{\sigma_1} \right)}_{\text{skin 1-2}}$$

$$\frac{\sigma_6}{\sigma_1} = -1$$

$$\frac{\sigma_2}{\sigma_1} = \frac{150}{200}$$

$$B_1 = B_6 = 1050 \text{ mm}^2$$

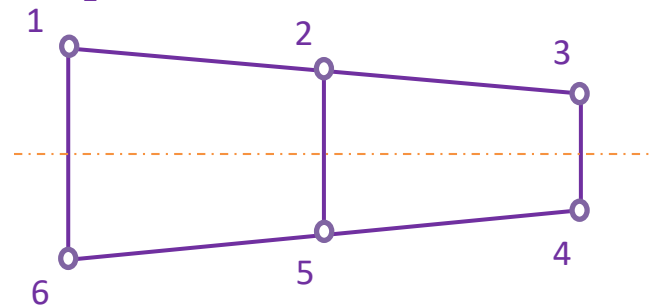
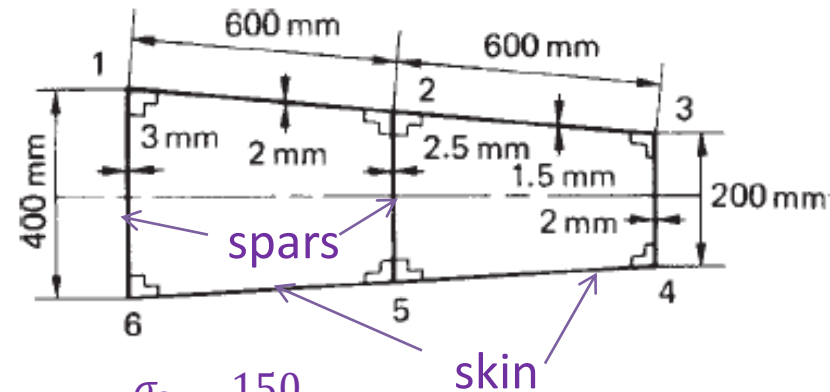
$$B_2 = 2 \cdot 300 + \frac{2 \cdot 600}{6} \left( 2 + \frac{\sigma_1}{\sigma_2} \right) + \frac{2.5 \cdot 300}{6} \left( 2 + \frac{\sigma_5}{\sigma_2} \right)$$

$$B_2 = B_5 = 1791.7 \text{ mm}^2$$

$$B_3 = 300 + \frac{1.5 \cdot 600}{6} \left( 2 + \frac{\sigma_2}{\sigma_3} \right) + \frac{2 \cdot 200}{6} \left( 2 + \frac{\sigma_4}{\sigma_3} \right)$$

$$B_3 = B_4 = 891.7 \text{ mm}^2$$

Angle sections area = 300 mm<sup>2</sup>



Idealized structure

# Bending

If idealized cross-section comprises

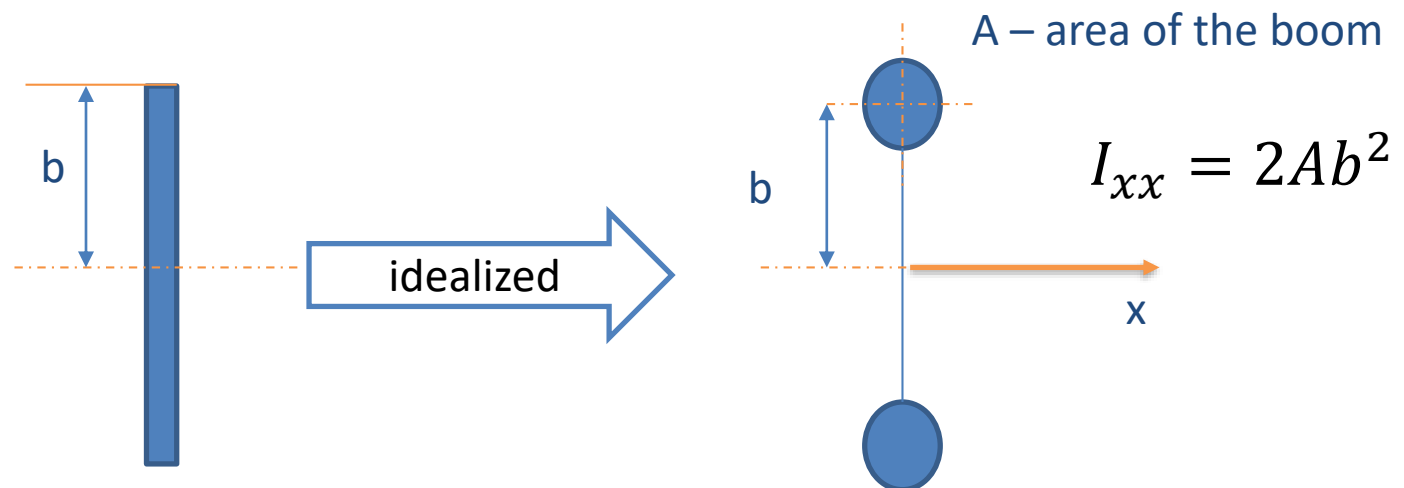
- direct stress carrying booms,
- shear stress only carrying skin ( $t_D = 0$ );

then

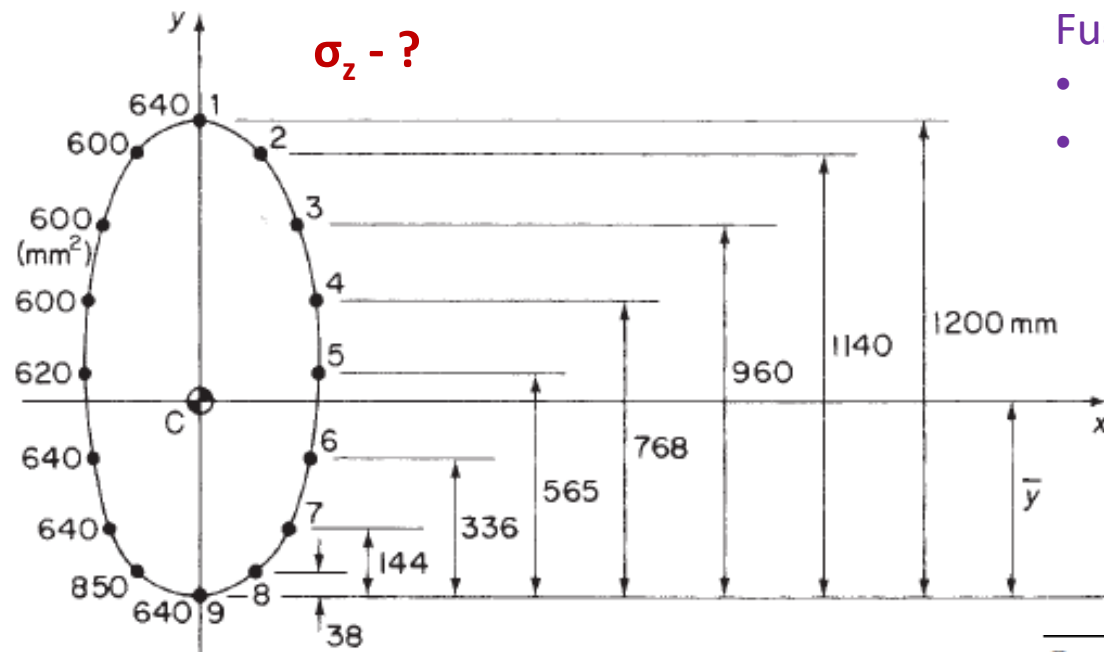
- neutral axis,
- sectional properties ( $I_{xx}$ ,  $I_{yy}$ ,  $I_{xy}$ )

are defined only by area of booms.

For instance



# Bending



$\sigma_z - ?$

Fuselage section:

- subjected to  $M_B = 100 \text{ kN}\cdot\text{m}$ ;
- completely idealized into a combination of
  - direct stress carrying booms,
  - shear stress carrying panels.

$$\sigma_z = \left( \frac{M_y I_{xx} - M_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) x + \left( \frac{M_x I_{yy} - M_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) y \Rightarrow$$

$$\sigma_z = \frac{M_x}{I_{xx}} y$$

Centroid from boom 9:

$$\begin{aligned} & (6 \times 640 + 6 \times 600 + 2 \times 620 + 2 \times 850) \bar{y} \\ &= 640 \times 1200 + 2 \times 600 \times 1140 + 2 \times 600 \times 960 + 2 \times 600 \times 768 \\ &+ 2 \times 620 \times 565 + 2 \times 640 \times 336 + 2 \times 640 \times 144 + 2 \times 850 \times 38 \\ &\Rightarrow \bar{y} = 540 \text{ mm} \end{aligned}$$

$$I_{xx.4} = B y^2 = 600 \cdot (768 - 540)^2 = 31 \cdot 10^6 \text{ mm}^4$$

$$\sigma_{z.4} = 100 \cdot 10^6 \text{ N} \cdot \text{mm} \cdot (768 - 540) \text{ mm} / 1854 \cdot 10^6 \text{ mm}^4$$

$$\sigma_{z.4} = 12.3 \text{ N/mm}^2$$

① Boom	② y (mm)	③ B (mm <sup>2</sup> )	④ $\Delta I_{xx} = B y^2$ (mm <sup>4</sup> )	⑤ $\sigma_z$ (N/mm <sup>2</sup> )
1	+660	640	$278 \times 10^6$	35.6
2	+600	600	$216 \times 10^6$	32.3
3	+420	600	$106 \times 10^6$	22.6
4	+228	600	$31 \times 10^6$	12.3
5	+25	620	$0.4 \times 10^6$	1.3
6	-204	640	$27 \times 10^6$	-11.0
7	-396	640	$100 \times 10^6$	-21.4
8	-502	850	$214 \times 10^6$	-27.0
9	-540	640	$187 \times 10^6$	-29.0

Sum of terms in column 4:  $I_{xx} = 1854 \times 10^6 \text{ mm}^4$

# Shear of open section beams

$$q_s = - \left( \frac{S_x I_{xx} - S_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \int_0^s t_D x \, ds - \left( \frac{S_y I_{yy} - S_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \int_0^s t_D y \, ds$$

skin effective in carrying direct stress

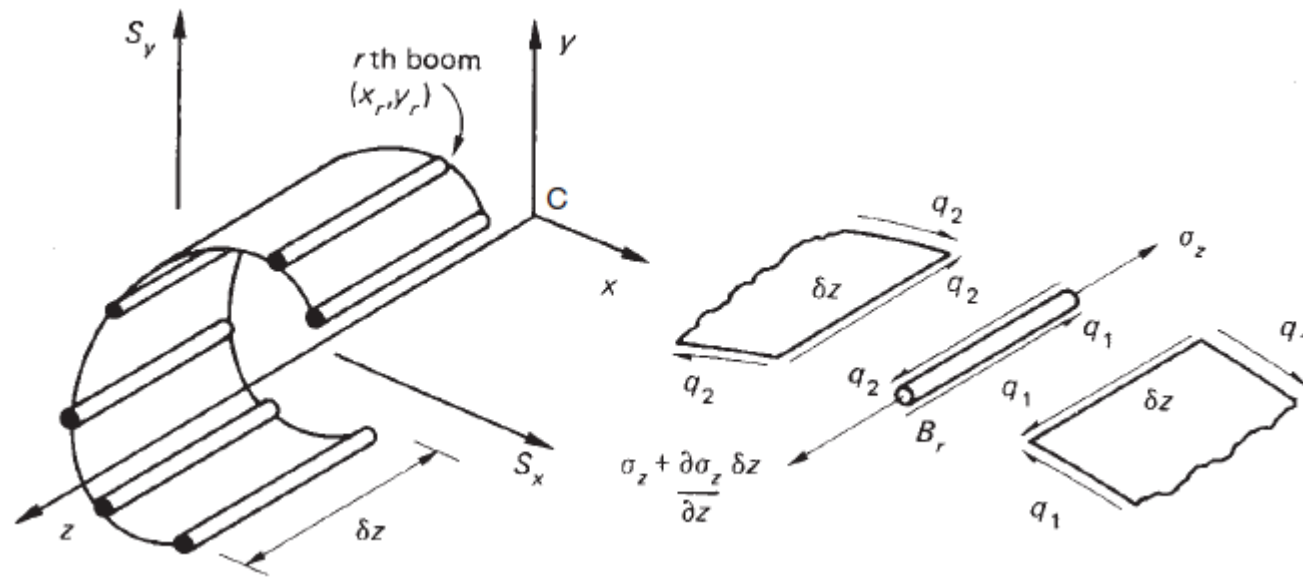
$$t_D = t$$



skin carries only shear stresses

$$t_D = 0$$

no provision for the effects of booms



$$\Sigma F_z = 0 \quad \left( \sigma_z + \frac{\partial \sigma_z}{\partial z} \delta z \right) B_r - \sigma_z B_r + q_2 \delta z - q_1 \delta z = 0 \quad \Rightarrow \quad q_2 - q_1 = - \frac{\partial \sigma_z}{\partial z} B_r$$

# Shear of open section beams

$$q_2 - q_1 = -\frac{\partial \sigma_z}{\partial z} B_r$$

$$\frac{\partial \sigma_z}{\partial z} = \frac{[(\partial M_y / \partial z) I_{xx} - (\partial M_x / \partial z) I_{xy}]_x}{I_{xx} I_{yy} - I_{xy}^2} + \frac{[(\partial M_x / \partial z) I_{yy} - (\partial M_y / \partial z) I_{xy}]_y}{I_{xx} I_{yy} - I_{xy}^2}$$

*the same procedure of derivation as in lecture for shear*

$$q_2 - q_1 = - \left[ \frac{(\partial M_y / \partial z) I_{xx} - (\partial M_x / \partial z) I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right] B_r x_r - \left[ \frac{(\partial M_x / \partial z) I_{yy} - (\partial M_y / \partial z) I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right] B_r y_r$$

$$\downarrow \partial M_y / \partial z = S_x$$

Coordinates of a boom under consideration

$$q_2 - q_1 = - \left( \frac{S_x I_{xx} - S_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) B_r x_r - \left( \frac{S_y I_{yy} - S_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) B_r y_r$$

the change in shear flow induced by a boom

At any distance  $s$  around the profile of the section,  $n$  booms have been passed, the shear flow at the point is given by

$$q_s = - \left( \frac{S_x I_{xx} - S_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left( \int_0^s t_D x \, ds + \sum_{r=1}^n B_r x_r \right) - \left( \frac{S_y I_{yy} - S_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left( \int_0^s t_D y \, ds + \sum_{r=1}^n B_r y_r \right)$$

The terms are taken into consideration only in case when **skin effective in carrying direct stress**



# Shear of open section beams

**Channel section** with area of each boom  $300 \text{ mm}^2$ :

- shear load acts through SC;
- assume that
  - booms carry all the direct stresses,
  - walls are effective only in resisting shear stresses.

**Solution:**

$$q_s = - \left( \frac{S_x I_{xx} - S_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left( \int_0^s t_D x \, ds + \sum_{r=1}^n B_r x_r \right) - \left( \frac{S_y I_{yy} - S_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left( \int_0^s t_D y \, ds + \sum_{r=1}^n B_r y_r \right)$$

↓  $I_{xy} = 0 \quad S_x = 0 \quad t_D = 0$

$$q_s = - \frac{S_y}{I_{xx}} \sum_{r=1}^n B_r y_r \quad I_{xx} = 4 \cdot 300 \cdot 200^2 = 4.8 \cdot 10^7 \text{ mm}^4$$

$$q_s = - \frac{4.8 \cdot 10^3 \text{ N}}{4.8 \cdot 10^7 \text{ mm}^4} \sum_{r=1}^n B_r y_r = -0.1 \cdot 10^{-3} \sum_{r=1}^n B_r y_r$$

Before the boom 1 (outside the section):  $q_s = 0$

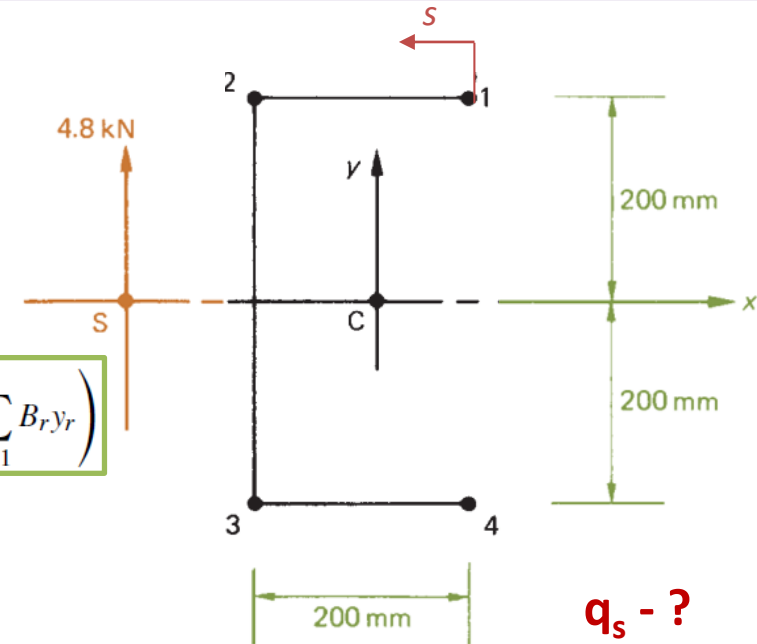
After the boom 1:  $\Delta q_1 = -0.1 \cdot 10^{-3} \cdot 300 \cdot 200 = -6 \text{ N/mm}$

Thus  $q_{12} = -6 \text{ N/mm}$

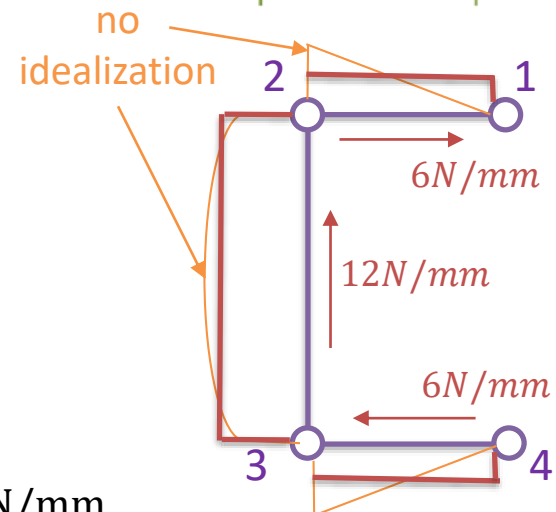
$q_{23} = -6 - 0.1 \cdot 10^{-3} \cdot 300 \cdot 200 = -12 \text{ N/mm}$

$q_{34} = -12 - 0.1 \cdot 10^{-3} \cdot 300 \cdot (-200) = -6 \text{ N/mm}$

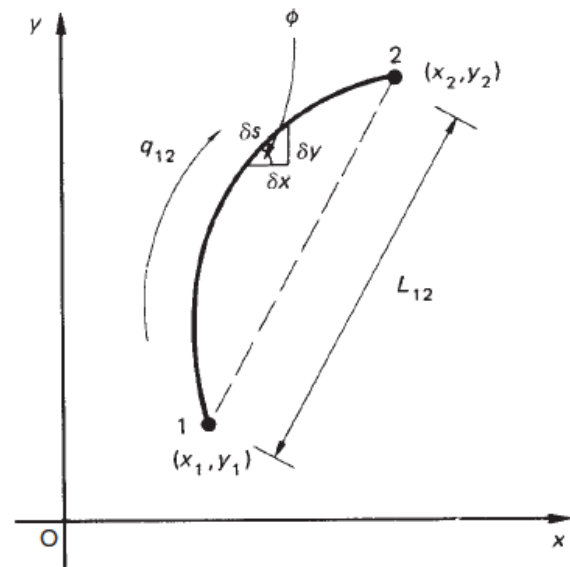
Outside the boom 4  $\Delta q_4 = -6 - 0.1 \cdot 10^{-3} \cdot 300 \cdot (-200) = 0 \text{ N/mm}$



$q_s - ?$



# Resultant shear load of curved web



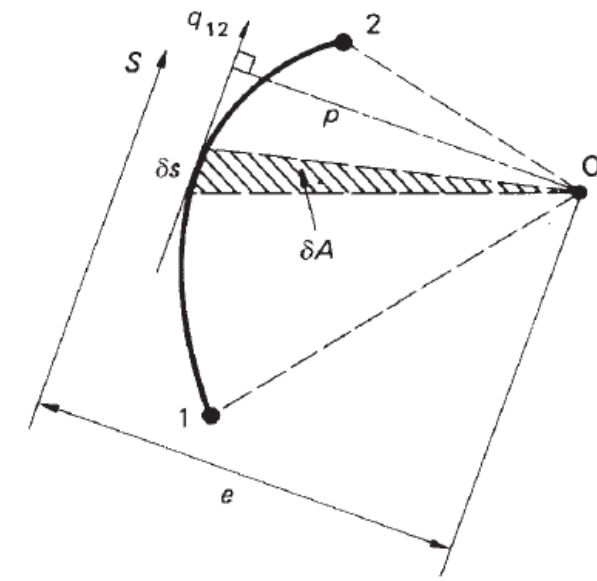
$$S_x = \int_1^2 q_{12} \cos \phi \, ds \xrightarrow{q_{12} = \text{const}} S_x = q_{12} \int_1^2 \cos \phi \, ds$$

$$S_x = q_{12} \int_1^2 dx = q_{12}(x_2 - x_1)$$

$$S_y = q_{12}(y_2 - y_1)$$

$$S = \sqrt{S_x^2 + S_y^2} = q_{12} \sqrt{(x_2 - x_1)^2 + (y_2 - y_1)^2}$$

$$S = q_{12} L_{12}$$



$$M_q = \int_1^2 q_{12} p \, ds = q_{12} \int_1^2 2 \, dA \xrightarrow{\quad} M_q = 2Aq_{12}$$

$$Se = 2Aq_{12} \xrightarrow{\quad} e = \frac{2A}{S} q_{12} \xrightarrow{\quad} e = \frac{2A}{L_{12}}$$

# Shear of closed section beams

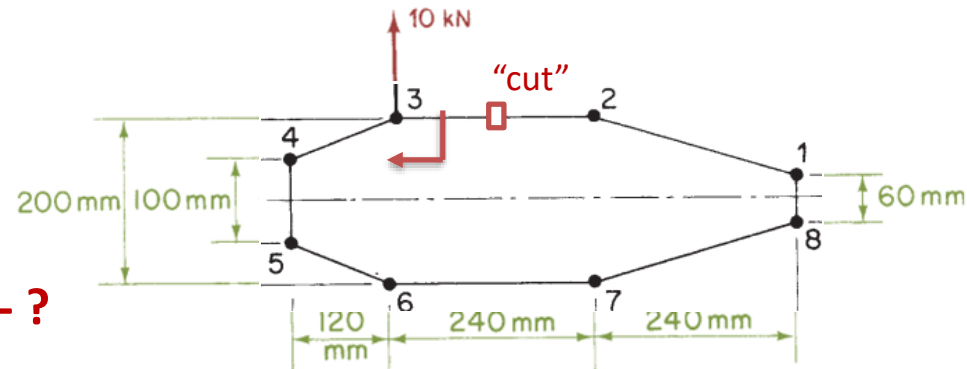
## Idealization

- booms carry all the direct stresses,
- walls are effective only in resisting shear stresses.

Booms area:  $B_1 = B_8 = 200 \text{ mm}^2$ ,  $B_2 = B_7 = 250 \text{ mm}^2$ ,  
 $B_3 = B_6 = 400 \text{ mm}^2$ ,  $B_4 = B_5 = 100 \text{ mm}^2$

## Solution:

$q_s - ?$

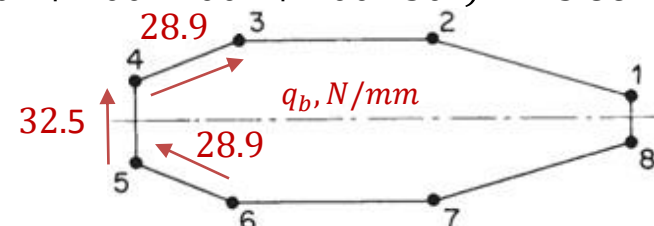


$$q_s = - \left( \frac{S_x I_{xx} - S_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left( \int_0^s t_D x \, ds + \sum_{r=1}^n B_r x_r \right) - \left( \frac{S_y I_{yy} - S_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2} \right) \left( \int_0^s t_D y \, ds + \sum_{r=1}^n B_r y_r \right)$$

$\Rightarrow I_{xy} = 0 \quad S_x = 0 \quad t_D = 0$

$$q_s = - \frac{S_y}{I_{xx}} \sum_{r=1}^n B_r y_r + q_{s,0} \quad I_{xx} = 2(200 \cdot 30^2 + 250 \cdot 100^2 + 400 \cdot 100^2 + 100 \cdot 50^2) = 13.86 \cdot 10^6 \text{ mm}^4$$

$$q_s = -7.22 \cdot 10^{-4} \sum_{r=1}^n B_r y_r + q_{s,0}$$



Web 2-3:  $q_{b,23} = 0 \text{ N/mm}$

Web 3-4:  $y_3 = 100 \quad q_{b,34} = -7.22 \cdot 10^{-4}(400 \cdot 100) + 0 = -28.9 \text{ N/mm}$

Web 4-5:  $y_4 = 50 \quad q_{b,34} = -7.22 \cdot 10^{-4}(100 \cdot 50) - 28.9 = -32.5 \text{ N/mm}$

Web 5-6:  $q_{b,56} = q_{b,34} = -28.9 \text{ N/mm}$

Web 6-7:  $q_{b,67} = q_{b,23} = 0 \text{ N/mm}$

} due to symmetry

# Shear of closed section beams

Booms area:  $B_1 = B_8 = 200 \text{ mm}^2$ ,  $B_2 = B_7 = 250 \text{ mm}^2$ ,  
 $B_3 = B_6 = 400 \text{ mm}^2$ ,  $B_4 = B_5 = 100 \text{ mm}^2$

$$q_s = -7.22 \cdot 10^{-4} \sum_{r=1}^n B_r y_r + q_{s,0}$$

Web 2-1:  $y_3 = 100$

$$q_{b,21} = -7.22 \cdot 10^{-4}(250 \cdot 100) + 0 = -18.1 \text{ N/mm}$$

Web 1-8:  $y_1 = 30$

$$q_{b,18} = -7.22 \cdot 10^{-4}(200 \cdot 30) - 18.1 = -22.4 \text{ N/mm}$$

Web 8-7:  $q_{b,87} = q_{b,21} = -18.1 \text{ N/mm}$  (symmetry)

To determine  $q_{s,0}$ :  $0 = \oint p q_b ds + 2A q_{s,0} \Rightarrow q_{s,0} = -\frac{\oint p q_b ds}{2A}$

Taking moment about p. A (clockwise is negative):

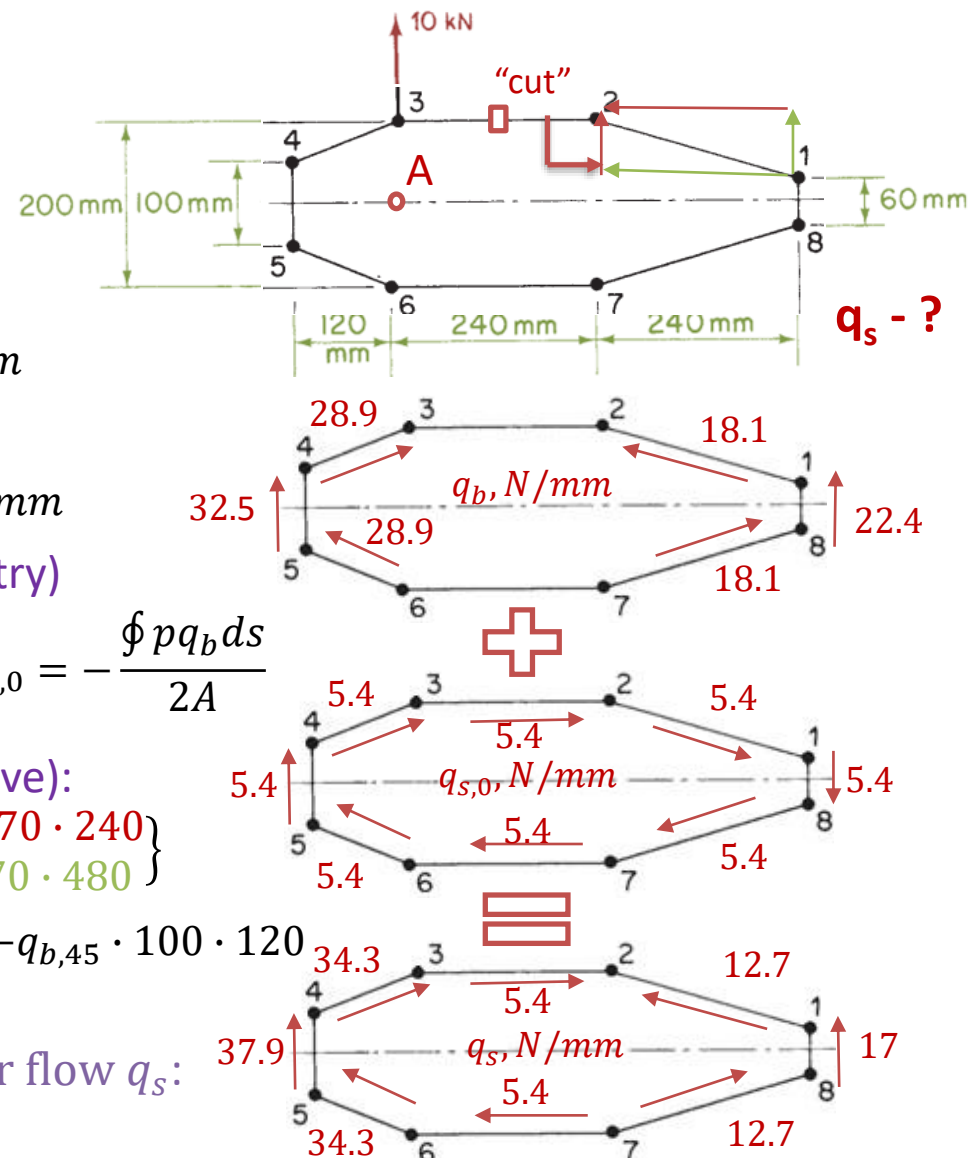
$$\oint p q_b ds = q_{b,81} \cdot 60 \cdot 480 + 2q_{b,12} \cdot \left\{ \begin{matrix} 240 \cdot 100 + 70 \cdot 240 \\ 240 \cdot 30 + 70 \cdot 480 \end{matrix} \right\}$$

$$+ q_{b,23} \cdot 240 \cdot 100 - 2q_{b,34} \cdot \left\{ \begin{matrix} 120 \cdot 50 + 50 \cdot 120 \\ 120 \cdot 100 + 50 \cdot 0 \end{matrix} \right\} - q_{b,45} \cdot 100 \cdot 120$$

$$A = 97200 \text{ mm}^2$$

$$q_{s,0} = -5.4 \text{ N/mm}$$

Distribution of shear flow  $q_s$ :



Obrigado!