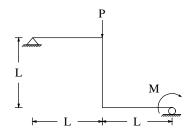
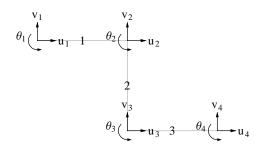
Example 4.10 Three element frame (p. 270)

$$M = 20 \text{ kN} - m$$
; $P = 10 \text{ kN}$; $L = 1 \text{ m}$; $E = 210 \text{ GPa}$; $A = 4 \times 10^{-2} \text{ m}^2$; $I = 4 \times 10^{-4} \text{ m}^4$





Use kN - m units for numerical computations. The computed displacements will be in m and stresses in kN/m^2 .

Specified nodal loads

$$\begin{array}{ccc} \text{Node} & \text{dof} & \text{Value} \\ 2 & v_2 & -10 \\ 4 & \theta_4 & -20 \end{array}$$

Global equations at start of the element assembly process

Equations for element 1

$$E = 2.1 \times 10^8; \hspace{1cm} I = 0.0004; \hspace{1cm} A = 0.04; \hspace{1cm} q = \{0., \, 0.\}$$

Nodal coordinates

Element node	Global node number	X	y
1	1	0	0
2	2	1	0

Length = 1; Direction cosines:
$$\ell_s = 1$$
 $m_s = 0$

Element equations in local coordinates

$$10^{6}. \begin{pmatrix} 8.4 & 0 & 0 & -8.4 & 0 & 0 \\ 0 & 1.008 & 0.504 & 0 & -1.008 & 0.504 \\ 0 & 0.504 & 0.336 & 0 & -0.504 & 0.168 \\ -8.4 & 0 & 0 & 8.4 & 0 & 0 \\ 0 & -1.008 & -0.504 & 0 & 1.008 & -0.504 \\ 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \end{pmatrix} \begin{pmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{pmatrix} = \begin{pmatrix} 0. \\ 0. \\ 0. \\ 0. \\ 0. \\ 0. \end{pmatrix}$$

Global to local transformation,
$$T = \begin{pmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{pmatrix}$$

Element equations in global coordinates

$$10^{6}. \begin{pmatrix} 8.4 & 0 & 0 & -8.4 & 0 & 0 \\ 0 & 1.008 & 0.504 & 0 & -1.008 & 0.504 \\ 0 & 0.504 & 0.336 & 0 & -0.504 & 0.168 \\ -8.4 & 0 & 0 & 8.4 & 0 & 0 \\ 0 & -1.008 & -0.504 & 0 & 1.008 & -0.504 \\ 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \end{pmatrix} \begin{pmatrix} u_1 \\ v_1 \\ \theta_1 \\ u_2 \\ v_2 \\ \theta_2 \end{pmatrix} = \begin{pmatrix} 0. \\ 0. \\ 0. \\ 0. \\ 0. \\ 0. \end{pmatrix}$$

The element contributes to {1, 2, 3, 4, 5, 6} global degrees of freedom.

Adding element equations into appropriate locations we have

Equations for element 2

$$E = 2.1 \times 10^8; \hspace{1cm} I = 0.0004; \hspace{1cm} A = 0.04; \hspace{1cm} q = \{0., \, 0.\}$$

Nodal coordinates

Element node	Global node number	X	y
1	2	1	0
2	3	1	-1

Length = 1; Direction cosines:
$$\ell_s = 0$$
 $m_s = -1$

Element equations in local coordinates

$$10^{6}. \begin{pmatrix} 8.4 & 0 & 0 & -8.4 & 0 & 0 \\ 0 & 1.008 & 0.504 & 0 & -1.008 & 0.504 \\ 0 & 0.504 & 0.336 & 0 & -0.504 & 0.168 \\ -8.4 & 0 & 0 & 8.4 & 0 & 0 \\ 0 & -1.008 & -0.504 & 0 & 1.008 & -0.504 \\ 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \end{pmatrix} \begin{pmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{pmatrix} = \begin{pmatrix} 0. \\ 0. \\ 0. \\ 0. \\ 0. \\ 0. \end{pmatrix}$$

Global to local transformation,
$$T = \begin{pmatrix} 0 & -1 & 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{pmatrix}$$

Element equations in global coordinates

The element contributes to {4, 5, 6, 7, 8, 9} global degrees of freedom.

Adding element equations into appropriate locations we have

Equations for element 3

$$E = 2.1 \times 10^8$$
; $I = 0.0004$; $A = 0.04$; $q = \{0., 0.\}$

Nodal coordinates

Element node Global node number		X	y
1	3	1	-1
2	4	2	-1

Length = 1; Direction cosines:
$$\ell_s = 1$$
 $m_s = 0$

Element equations in local coordinates

$$10^{6} \begin{pmatrix} 8.4 & 0 & 0 & -8.4 & 0 & 0 \\ 0 & 1.008 & 0.504 & 0 & -1.008 & 0.504 \\ 0 & 0.504 & 0.336 & 0 & -0.504 & 0.168 \\ -8.4 & 0 & 0 & 8.4 & 0 & 0 \\ 0 & -1.008 & -0.504 & 0 & 1.008 & -0.504 \\ 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \end{pmatrix} \begin{pmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{pmatrix} = \begin{pmatrix} 0. \\ 0. \\ 0. \\ 0. \\ 0. \\ 0. \end{pmatrix}$$

Global to local transformation,
$$T = \begin{pmatrix} 1 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{pmatrix}$$

Element equations in global coordinates

$$10^{6} \begin{pmatrix} 8.4 & 0 & 0 & -8.4 & 0 & 0 \\ 0 & 1.008 & 0.504 & 0 & -1.008 & 0.504 \\ 0 & 0.504 & 0.336 & 0 & -0.504 & 0.168 \\ -8.4 & 0 & 0 & 8.4 & 0 & 0 \\ 0 & -1.008 & -0.504 & 0 & 1.008 & -0.504 \\ 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \end{pmatrix} \begin{pmatrix} u_3 \\ v_3 \\ \theta_3 \\ u_4 \\ v_4 \\ \theta_4 \end{pmatrix} = \begin{pmatrix} 0. \\ 0. \\ 0. \\ 0. \\ 0. \\ 0. \end{pmatrix}$$

The element contributes to {7, 8, 9, 10, 11, 12} global degrees of freedom.

Adding element equations into appropriate locations we have

$$10^{6} \begin{pmatrix} 8.4 & 0 & 0 & -8.4 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1.008 & 0.504 & 0 & -1.008 & 0.504 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0.504 & 0.336 & 0 & -0.504 & 0.168 & 0 & 0 & 0 & 0 & 0 & 0 \\ -8.4 & 0 & 0 & 9.408 & 0 & 0.504 & -1.008 & 0 & 0.504 & 0 & 0 & 0 \\ 0 & -1.008 & -0.504 & 0 & 9.408 & -0.504 & 0 & -8.4 & 0 & 0 & 0 & 0 \\ 0 & 0.504 & 0.168 & 0.504 & -0.504 & 0.672 & -0.504 & 0 & 0.168 & 0 & 0 & 0 \\ 0 & 0 & 0 & -1.008 & 0 & -0.504 & 9.408 & 0 & -0.504 & -8.4 & 0 & 0 \\ 0 & 0 & 0 & 0 & -8.4 & 0 & 0 & 9.408 & 0.504 & 0 & -1.008 & 0.504 \\ 0 & 0 & 0 & 0.504 & 0 & 0.168 & -0.504 & 0.504 & 0.672 & 0 & -0.504 & 0.168 \\ 0 & 0 & 0 & 0 & 0 & 0 & -8.4 & 0 & 0 & 8.4 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & -8.4 & 0 & 0 & 8.4 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & -1.008 & -0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & -0.504 & 0.336 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 &$$

$$\begin{pmatrix} u_1 \\ v_1 \\ \theta_1 \\ u_2 \\ v_2 \\ \theta_2 \\ u_3 \\ v_3 \\ \theta_3 \\ u_4 \\ v_4 \\ \theta_4 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ 0 \\ -10. \\ 0 \\ 0 \\ 0 \\ 0 \\ -20. \end{pmatrix}$$

Essential boundary conditions

Node	dof	Value
1	$\mathbf{u_1}$	0
1	\mathbf{v}_1	0
4	V_4	0

Remove {1, 2, 11} rows and columns.

After adjusting for essential boundary conditions we have

$$\begin{pmatrix} 0.336 & 0 & -0.504 & 0.168 & 0 & 0 & 0 & 0 & 0 \\ 0 & 9.408 & 0 & 0.504 & -1.008 & 0 & 0.504 & 0 & 0 \\ -0.504 & 0 & 9.408 & -0.504 & 0 & -8.4 & 0 & 0 & 0 \\ 0.168 & 0.504 & -0.504 & 0.672 & -0.504 & 0 & 0.168 & 0 & 0 \\ 0 & -1.008 & 0 & -0.504 & 9.408 & 0 & -0.504 & -8.4 & 0 \\ 0 & 0 & -8.4 & 0 & 0 & 9.408 & 0.504 & 0 & 0.504 \\ 0 & 0 & 0.504 & 0 & 0.168 & -0.504 & 0.504 & 0.672 & 0 & 0.168 \\ 0 & 0 & 0 & 0 & -8.4 & 0 & 0 & 8.4 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0.504 & 0.168 & 0 & 0.336 \end{pmatrix} \begin{pmatrix} \theta_1 \\ u_2 \\ v_2 \\ \theta_2 \\ u_3 \\ v_3 \\ \theta_3 \\ u_4 \\ \theta_4 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ -10. \\ 0 \\ 0 \\ 0 \\ 0 \\ -20. \end{pmatrix}$$

Solving the final system of global equations we get

$$\{\theta_1=0.0000784722,\ u_2=0,\ v_2=0.0000685516,\ \theta_2=0.0000487103,\ u_3=0.0000189484,\ v_3=0.0000703373,\ \theta_3=-0.0000108135,\ u_4=0.0000189484,\ \theta_4=-0.000159623\}$$

Complete table of nodal values

	u	V	θ
1	0	0	0.0000784722
2	0	0.0000685516	0.0000487103
3	0.0000189484	0.0000703373	-0.0000108135
4	0.0000189484	0	-0.000159623

Computation of reactions

Equation numbers of dof with specified values: {1, 2, 11}

Extracting equations {1, 2, 11} from the global system we have

Substituting the nodal values and re-arranging

$$\begin{pmatrix} R_1 \\ R_2 \\ R_3 \end{pmatrix} = 10^6$$

Carrying out computations, the reactions are as follows.

Sum of Reactions

$$\begin{array}{ll} \text{dof: u} & 0 \\ \text{dof: v} & 10. \\ \text{dof: } \theta & 0 \end{array}$$

Solution for element 1

$$E = 2.1 \times 10^8; \hspace{1.5cm} I = 0.0004; \hspace{1.5cm} A = 0.04; \hspace{1.5cm} q = \{0., \ 0.\}$$

Length = 1; Direction cosines: $\ell_s = 1$ $m_s = 0$

Nodal values in global coordinates, $d^{T} = (0 \ 0 \ 0.0000784722 \ 0 \ 0.0000685516 \ 0.0000487103)$

Global to local transformation,
$$T = \begin{pmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{pmatrix}$$

Nodal values in local coordinates, $\mathbf{d}_{\ell}^{T} = T\mathbf{d} = (0 \ 0 \ 0.0000784722 \ 0 \ 0.0000685516 \ 0.0000487103)$

Axial displacement interpolation functions, $N_u^T = \{1 - s, s\}$

Axial displacement,
$$\mathbf{u}(s) = \mathbf{N}_{\mathbf{u}}^{T} \begin{pmatrix} d_1 \\ d_4 \end{pmatrix} = \mathbf{0}$$

Axial force, EA du(s)/ds = 0

Beam bending interpolation functions, $N_{v}^{T} = \left\{2\,s^{3} - 3\,s^{2} + 1,\,s^{3} - 2\,s^{2} + s,\,3\,s^{2} - 2\,s^{3},\,s^{3} - s^{2}\right\}$

$$Transverse \ displacement, \ v(s) = \textbf{\textit{N}}_{v}^{T} \begin{pmatrix} d_{2} \\ d_{3} \\ d_{5} \\ d_{6} \end{pmatrix} = 0.0000784722 \, s - 9.92063 \times 10^{-6} \, s^{3}$$

Fixed-end displacement solution, = $0.(1-s)^2 s^2$

Total transverse displacement, $v(s) = 0.0000784722 s - 9.92063 \times 10^{-6} s^3$

Bending moment, $M = EI d^2v(s)/ds^2 = -5. s$

Shear force, V(s) = dM/ds = -5.

Solution for element 2

$$E = 2.1 \times 10^8; \hspace{1cm} I = 0.0004; \hspace{1cm} A = 0.04; \hspace{1cm} q = \{0., \ 0.\}$$

Length = 1; Direction cosines:
$$\ell_s = 0$$
 $m_s = -1$

Nodal values in global coordinates, $d^{T} =$

 $(0\ 0.0000685516\ 0.0000487103\ 0.0000189484\ 0.0000703373\ -0.0000108135)$

Global to local transformation,
$$T = \begin{pmatrix} 0 & -1 & 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{pmatrix}$$

Nodal values in local coordinates, $d_{\ell}^{T} = Td =$

 $(-0.0000685516\ 0\ 0.0000487103\ -0.0000703373\ 0.0000189484\ -0.0000108135)$

Axial displacement interpolation functions, $N_{u}^{T} = \{1 - s, s\}$

Axial displacement,
$$u(s) = N_u^T \begin{pmatrix} d_1 \\ d_4 \end{pmatrix} = -1.78571 \times 10^{-6} \text{ s} - 0.0000685516$$

Axial force, EA du(s)/ds = -15.

Beam bending interpolation functions, $\textit{N}_{v}^{T} = \left\{2\,s^{3}\,-3\,s^{2}\,+\,1,\,s^{3}\,-\,2\,s^{2}\,+\,s,\,3\,s^{2}\,-\,2\,s^{3},\,s^{3}\,-\,s^{2}\right\}$

$$\text{Transverse displacement, } \mathbf{v}(s) = \textbf{\textit{N}}_{v}^{T} \begin{pmatrix} d_{2} \\ d_{3} \\ d_{5} \\ d_{6} \end{pmatrix} = 0.0000487103\,s - 0.0000297619\,s^{2}$$

Fixed-end displacement solution, = $0.(1-s)^2 s^2$

Total transverse displacement, $v(s) = 0.0000487103 s - 0.0000297619 s^2$

Bending moment, $M = EI d^2v(s)/ds^2 = -5$.

Shear force, V(s) = dM/ds = 0

Solution for element 3

$$E = 2.1 \times 10^8$$
; $I = 0.0004$;

$$A = 0.04;$$
 $q = \{0., 0.\}$

Length
$$= 1$$
;

Direction cosines: $\ell_s = 1$ $m_s = 0$

Nodal values in global coordinates, d^{T} =

 $(0.0000189484 \ 0.0000703373 \ -0.0000108135 \ 0.0000189484 \ 0 \ -0.000159623)$

Global to local transformation, $T = \begin{bmatrix} 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \end{bmatrix}$

Nodal values in local coordinates, $\mathbf{d}_{\ell}^{\mathrm{T}} = T\mathbf{d} =$

 $(\ 0.0000189484 \ \ 0.0000703373 \ \ -0.0000108135 \ \ 0.0000189484 \ \ 0 \ \ -0.000159623 \,)$

Axial displacement interpolation functions, $N_{u}^{T} = \{1 - s, s\}$

Axial displacement, $\mathbf{u}(s) = \mathbf{N}_{\mathbf{u}}^{T} \begin{pmatrix} \mathbf{d}_{1} \\ \mathbf{d}_{4} \end{pmatrix} = 0.0000189484$

Axial force, EA du(s)/ds = 0

Beam bending interpolation functions, $N_v^T = \{2 s^3 - 3 s^2 + 1, s^3 - 2 s^2 + s, 3 s^2 - 2 s^3, s^3 - s^2\}$

Transverse displacement,
$$\mathbf{v}(s) = \textbf{N}_{\mathbf{v}}^T \begin{pmatrix} d_2 \\ d_3 \\ d_5 \\ d_6 \end{pmatrix} =$$

 $-0.0000297619\,{s}^{3}-0.0000297619\,{s}^{2}-0.0000108135\,{s}+0.0000703373$

Fixed-end displacement solution, = $0.(1-s)^2 s^2$

Bending moment, $M = EI d^2v(s)/ds^2 = -15. s - 5.$

Shear force, V(s) = dM/ds = -15.

Forces at element ends

	X	y	Axial force	Bending moment	Shear force
1	0	0	0	0	-5.
1	1	0	0	-5.	-5.
2	1	0	-15 .	-5.	0
۷	1	-1	-15 .	-5.	0
9	1	-1	0	-5.	-15.
3	2	-1	0	-20.	-15.

