Example 4.14: Three dimensional frame (p. 290)

Analyze one story three dimensional frame shown in Figure. The height of the columns is 12 ft and the length of the beams is 10 ft. Each beam is subjected to a uniformly distributed load of 2 kip/ft in the downward direction. I-shape sections are used for both columns and beams with the arrangement as shown in the figure. The columns are connected to the foundation through simple connections that do not resist moments. The material is steel with $E = 29000 \, \text{kip/in}^2$ and $G = 11200 \, \text{kip/in}^2$. The section properties are as follows.

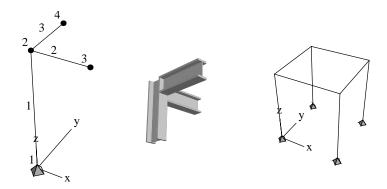
Beams:
$$A = 3.2 \text{ in}^2$$
; $J = 43 \text{ in}^4$; $I_{\text{max}} = I_r = 450 \text{ in}^4$; $I_{\text{min}} = I_s = 32 \text{ in}^2$

Columns:
$$A = 4 \text{ in}^2$$
; $J = 60 \text{ in}^4$; $I_{\text{max}} = I_r = 650 \text{ in}^4$; $I_{\text{min}} = I_s = 54 \text{ in}^2$

Taking advantage of symmetry we model a quarter of the frame using three elements. Because of symmetry, the boundary conditions at nodes 3 and 4 are as follows.

Node 3:
$$u = 0$$
; $\theta_v = 0$; $\theta_z = 0$

Node 4:
$$v = 0$$
; $\theta_x = 0$; $\theta_z = 0$



The distributed load is applied to the elements in their local coordinates. Therefore to assign proper direction and sign to the distributed loads we must carefully establish the local coordinates for the elements as follows.

Element 1: Nodes 1, 2, and 4

 $\implies t$ – axis along global z; s – axis along global x; r – axis along global y

Element 2: Nodes 2, 3, and 4

 $\implies t$ – axis along global x; s – axis along global (-z); r – axis along global y

Distributed load: $q_r = 0$; $q_s = 2/12 \text{ kip/in}$

Element 3: Nodes 2, 4, and 3

 $\implies t$ – axis along global y; s – axis along global z; r – axis along global x

Distributed load: $q_r = 0$; $q_s = -2/12 \text{ kip/in}$

Global equations at start of the element assembly process

Equations for element 1

$$q_s = 0;$$
 $q_r = 0;$ $E = 29000.;$ $G = 11200.$
$$A = 4;$$
 $J = 60;$ $I_r = 650;$ $I_s = 54$

Element nodal coordinates: $\begin{bmatrix} 0 & 0 & 0 \\ 0 & 0 & 144. \\ 0 & 60. & 144. \end{bmatrix}$

Element length, L = 144.

Direction cosines:
$$H = \begin{pmatrix} 0 & 0 & 1. \\ 1. & 0. & 0. \\ 0. & 1. & 0. \end{pmatrix}$$

Element equations in local coordinates

75.7539	0.	0.	0.	5454.28	0.	-75.7539	0.	0.	
0.	6.2934	0.	-453.125	0.	0.	0.	-6.2934	0.	-4
0.	0.	805.556	0.	0.	0.	0.	0.	-805.556	
0.	-453.125	0.	43500.	0.	0.	0.	453.125	0.	217
5454.28	0.	0.	0.	523611.	0.	-5454.28	0.	0.	
0.	0.	0.	0.	0.	4666.67	0.	0.	0.	
-75.7539	0.	0.	0.	-5454.28	0.	75.7539	0.	0.	
0.	-6.2934	0.	453.125	0.	0.	0.	6.2934	0.	4
0.	0.	-805.556	0.	0.	0.	0.	0.	805.556	
0.	-453.125	0.	21750.	0.	0.	0.	453.125	0.	43 5
5454.28	0.	0.	0.	261806.	0.	-5454.28	0.	0.	
0.	0.	0.	0.	0.	-4666.67	0.	0.	0.	

Element equations in global coordinates

75.7539	0	0	0	5454.28	0	-75.7539	0	0	
0	6.2934	0	-453.125	0	0	0	-6.2934	0	-4
0	0	805.556	0	0	0	0	0	-805.556	
0	-453.125	0	43500.	0	0	0	453.125	0	217
5454.28	0	0	0	523611.	0	-5454.28	0	0	
0	0	0	0	0	4666.67	0	0	0	
-75.7539	0	0	0	-5454.28	0	75.7539	0	0	
0	-6.2934	0	453.125	0	0	0	6.2934	0	4
0	0	-805.556	0	0	0	0	0	805.556	
0	-453.125	0	21750.	0	0	0	453.125	0	435
5454.28	0	0	0	261806.	0	-5454.28	0	0	
0	0	0	0	0	-4666.67	0	0	0	

The element contributes to $\{1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12\}$ global degrees of freedom.

Adding element equations into appropriate locations we have

1	75.7539	0	0	0	5454.28	0	-75.7539	0	0	
l	0	6.2934	0	-453.125	0	0	0	-6.2934	0	-4
	0	0	805.556	0	0	0	0	0	-805.556	
	0	-453.125	0	43500.	0	0	0	453.125	0	217
	5454.28	0	0	0	523611.	0	-5454.28	0	0	
l	0	0	0	0	0	4666.67	0	0	0	
	-75.7539	0	0	0	-5454.28	0	75.7539	0	0	
	0	-6.2934	0	453.125	0	0	0	6.2934	0	4
l	0	0	-805.556	0	0	0	0	0	805.556	
	0	-453.125	0	21750.	0	0	0	453.125	0	435
	5454.28	0	0	0	261806.	0	-5454.28	0	0	
	0	0	0	0	0	-4666.67	0	0	0	
l	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	
	0	0	0	0	0	0	0	0	0	

Equations for element 2

$$\begin{split} q_s &= 0.166667; & q_r &= 0; & E &= 29000.; & G &= 11200. \\ A &= 3.2; & J &= 43; & I_r &= 450; & I_s &= 32 \end{split}$$
 Element nodal coordinates:
$$\begin{pmatrix} 0 & 0 & 144. \\ 60. & 0 & 144. \\ 0 & 60. & 144. \end{pmatrix}$$

Element length, L = 60.

Direction cosines:
$$H = \begin{pmatrix} 1 & 0 & 0 \\ 0 & 0 & -1 \\ 0 & 1 & 0 \end{pmatrix}$$

Element equations in local coordinates

0

0 0

0

1. 0

0. 0 0

Element equations in global coordinates

1	1546.67	0	0	0	0	0	-1546.67	0	0	0
	0	51.5556	0	0	0	1546.67	0	-51.5556	0	0
	0	0	725.	0	-21750.	0	0	0	-725.	0
	0	0	0	8026.67	0	0	0	0	0	-8026.67
	0	0	-21750.	0	870000.	0	0	0	21750.	0
	0	1546.67	0	0	0	61866.7	0	-1546.67	0	0
	-1546.67	0	0	0	0	0	1546.67	0	0	0
	0	-51.5556	0	0	0	-1546.67	0	51.5556	0	0
	0	0	-725.	0	21750.	0	0	0	725.	0
	0	0	0	-8026.67	0	0	0	0	0	8026.67
	0	0	-21750.	0	435000.	0	0	0	21750.	0
	0	1546.67	0	0	0	30933.3	0	-1546.67	0	0

The element contributes to {7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18} global degrees of freedom.

Adding element equations into appropriate locations we have

(75.7539	0	0	0	5454.28	0	-75.7539	0	0
0			-453.125		0	0	-6.2934	0
0	0	805.556	0	0	0	0	0	-805.556
0	-453.125	0	43500.	0	0	0	453.125	0
5454.28	0	0	0	523611.	0	-5454.28	0	0
0	0	0	0	0	4666.67	0	0	0
-75.7539	0	0	0	-5454.28	0	1622.42	0	0
0	-6.2934	0	453.125	0	0	0	57.849	0
0	0	-805.556	0	0	0	0	0	1530.56
0	-453.125	0	21750.	0	0	0	453.125	0
5454.28	0	0	0	261806.	0	-5454.28	0	-21750.
0	0	0	0	0	-4666.67	0	1546.67	0
0	0	0	0	0	0	-1546.67	0	0
0	0	0	0	0	0	0	-51.5556	0
0	0	0	0	0	0	0	0	-725.
0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	-21750.
0	0	0	0	0	0	0	1546.67	0
0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0

Equations for element 3

$$\begin{split} q_s &= -0.166667; & q_r = 0; & E = 29000.; & G = 11200. \\ A &= 3.2; & J = 43; & I_r = 450; & I_s = 32 \end{split}$$
 Element nodal coordinates:
$$\begin{pmatrix} 0 & 0 & 144. \\ 0 & 60. & 144. \\ 60. & 0 & 144. \end{pmatrix}$$

Element length, L = 60.

Direction cosines:
$$\mathbf{H} = \begin{pmatrix} 0 & 1. & 0. \\ 0. & 0. & 1. \\ 1. & 0. & 0. \end{pmatrix}$$

Element equations in local coordinates

(51.5556	0.	0.	0.	0.	-1546.67	-51.5556	0.	0.	0.
0.	1546.67	0.	0.	0.	0.	0.	-1546.67	0.	0.
0.	0.	725.	21750.	0.	0.	0.	0.	-725.	21750.
0.	0.	21750.	870000.	0.	0.	0.	0.	-21750.	435000.
0.	0.	0.	0.	8026.67	0.	0.	0.	0.	0.
-1546.67	0.	0.	0.	0.	61866.7	1546.67	0.	0.	0.
-51.5556	0.	0.	0.	0.	1546.67	51.5556	0.	0.	0.
0.	-1546.67	0.	0.	0.	0.	0.	1546.67	0.	0.
0.	0.	-725.	-21750.	0.	0.	0.	0.	725.	-21750.
0.	0.	21750.	435000.	0.	0.	0.	0.	-21750.	870000.
0.	0.	0.	0.	-8026.67	0.	0.	0.	0.	0.
-1546.67	0.	0.	0.	0.	30933.3	1546.67	0.	0.	0.

Element equations in global coordinates

1	51.5556	0	0	0	0	-1546.67	-51.5556	0	0	0
	0	1546.67	0	0	0	0	0	-1546.67	0	0
	0	0	725.	21750.	0	0	0	0	-725.	21750.
	0	0	21750.	870000.	0	0	0	0	-21750.	435000.
	0	0	0	0	8026.67	0	0	0	0	0
	-1546.67	0	0	0	0	61866.7	1546.67	0	0	0
	-51.5556	0	0	0	0	1546.67	51.5556	0	0	0
	0	-1546.67	0	0	0	0	0	1546.67	0	0
	0	0	-725.	-21750.	0	0	0	0	725.	-21750.
	0	0	21750.	435000.	0	0	0	0	-21750.	870000.
	0	0	0	0	-8026.67	0	0	0	0	0
	-1546.67	0	0	0	0	30933.3	1546.67	0	0	0

The element contributes to {7, 8, 9, 10, 11, 12, 19, 20, 21, 22, 23, 24} global degrees of freedom.

Adding element equations into appropriate locations we have

(75.7539	0	0	0	5454.28	0	-75.7539	0	0
0	6.2934	0	-453.125	0	0	0	-6.2934	0
0	0	805.556	0	0	0	0	0	-805.55€
0	-453.125	0	43500.	0	0	0	453.125	0
5454.28	0	0	0	523611.	0	-5454.28	0	0
0	0	0	0	0	4666.67	0	0	0
-75.7539	0	0	0	-5454.28	0	1673.98	0	0
0	-6.2934	0	453.125	0	0	0	1604.52	0
0	0	-805.556	0	0	0	0	0	2255.56
0	-453.125	0	21750.	0	0	0	453.125	21750.
5454.28	0	0	0	261806.	0	-5454.28	0	-21750.
0	0	0	0	0	-4666.67	-1546.67	1546.67	0
0	0	0	0	0	0	-1546.67	0	0
0	0	0	0	0	0	0	-51.5556	0
0	0	0	0	0	0	0	0	-725.
0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	-21750.
0	0	0	0	0	0	0	1546.67	0
0	0	0	0	0	0	-51.5556	0	0
0	0	0	0	0	0	0	-1546.67	0
0	0	0	0	0	0	0	0	-725.
0	0	0	0	0	0	0	0	21750.
0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	-1546.67	0	0

Essential boundary conditions

Node	dof	Value
1	$\mathbf{u_1}$	0
1	\mathbf{v}_1	0
1	\mathbf{w}_1	0
3	\mathbf{u}_3	0
3	$ heta \mathbf{y}_3$	0
3	$\theta \mathbf{z}_3$	0
4	\mathbf{v}_4	0
4	$\theta \mathbf{x_4}$	0
4	$ heta \mathbf{z}_4$	0

Remove {1, 2, 3, 13, 17, 18, 20, 22, 24} rows and columns.

After adjusting for essential boundary conditions we have

1	43500.	0	0	0	453.125	0	21750.	0	
	0	523611.	0	-5454.28	0	0	0	261806.	
	0	0	4666.67	0	0	0	0	0	
	0	-5454.28	0	1673.98	0	0	0	-5454.28	
	453.125	0	0	0	1604.52	0	453.125	0	
I	0	0	0	0	0	2255.56	21750.	-21750.	
	21750.	0	0	0	453.125	21750.	921527.	0	
	0	261806.	0	-5454.28	0	-21750.	0	$1.40164\!\times\!10^{6}$	
İ	0	0	-4666.67	-1546.67	1546.67	0	0	0	1
	0	0	0	0	-51.5556	0	0	0	
	0	0	0	0	0	-725.	0	21750.	
	0	0	0	0	0	0	-8026.67	0	
	0	0	0	-51.5556	0	0	0	0	
	0	0	0	0	0	-725.	-21750.	0	
	0	0	0	0	0	0	0	-8026.67	

Solving the final system of global equations we get

```
\begin{split} \{\theta x_1 &= 0.000398634, \ \theta y_1 = -0.00015917, \ \theta z_1 = 0, \ u_2 = 0.000575402, \ v_2 = 0.000117025, \\ w_2 &= -0.0248276, \ \theta x_2 = -0.000799706, \ \theta y_2 = 0.000330328, \ \theta z_2 = 0, \ v_3 = 0.000117025, \\ w_3 &= -0.041634, \ \theta x_3 = -0.000799706, \ u_4 = 0.000575402, \ w_4 = -0.0557153, \ \theta y_4 = 0.000330328 \} \end{split}
```

Complete table of nodal values

	u	v	W	$\theta \mathbf{x}$	$ heta \mathbf{y}$	$\theta \mathbf{z}$
1	0	0	0	0.000398634	-0.00015917	0
2	0.000575402	0.000117025	-0.0248276	-0.000799706	0.000330328	0
3	0	0.000117025	-0.041634	-0.000799706	0	0
4	0.000575402	0	-0.0557153	0	0.000330328	0

Computation of reactions

Equation numbers of dof with specified values: {1, 2, 3, 13, 17, 18, 20, 22, 24}

Extracting equations {1, 2, 3, 13, 17, 18, 20, 22, 24} from the global system we have

75.7539	0	0	0	5454.28	0	-75.7539	0	0	0	5
0	6.2934	0	-453.125	0	0	0	-6.2934	0	-453.125	
0	0	805.556	0	0	0	0	0	-805.556	0	
0	0	0	0	0	0	-1546.67	0	0	0	
0	0	0	0	0	0	0	0	-21750.	0	435
0	0	0	0	0	0	0	1546.67	0	0	
0	0	0	0	0	0	0	-1546.67	0	0	
0	0	0	0	0	0	0	0	21750.	435000.	
0	0	0	0	0	0	-1546.67	0	0	0	

Substituting the nodal values and re-arranging

R_1)	75.7539	0	0	0	5454.28	0	-75.7539	0	0	0
R_2		0	6.2934	0	-453.125	0	0	0	-6.2934	0	-453. 1
R_3		0	0	805.556	0	0	0	0	0	-805.556	0
R ₄		0	0	0	0	0	0	-1546.67	0	0	0
R_5	=	0	0	0	0	0	0	0	0	-21750.	0
R ₆		0	0	0	0	0	0	0	1546.67	0	0
R ₇	l	0	0	0	0	0	0	0	-1546.67	0	0
R ₈		0	0	0	0	0	0	0	0	21750.	435000.
R_9)	(0	0	0	0	0	0	-1546.67	0	0	0

Carrying out computations, the reactions are as follows.

Label	dof	Reaction
R_1	$\mathbf{u_1}$	0.889955
R_2	\mathbf{v}_1	0.180999
R_3	\mathbf{w}_1	20.
R_4	\mathbf{u}_3	-0.889955
R_5	$ heta \mathbf{y}_3$	-171.846
R_6	θz_3	0
R_7	$\mathbf{v_4}$	-0.180999
R_8	θx_4	273.936
R_9	$ heta\mathbf{z_4}$	0

Sum of Reactions

dof: u
0

dof: v
0

dof: w
20.

dof:
$$\theta x$$
273.936

dof: θy
-171.846

dof: θz
0

Solution for element 1

Nodal values in global coordinates, $d^{T} = \{0, 0, 0, 0.000398634,$

-0.00015917, 0, 0.000575402, 0.000117025, -0.0248276, -0.000799706, 0.000330328, 0

Nodal values in local coordinates, $\boldsymbol{d}_{\ell}^{\mathrm{T}} = \boldsymbol{T}\boldsymbol{d} = \{0., 0., 0., 0., 0.000398634,$

 $-0.00015917,\ -0.0248276,\ 0.000575402,\ 0.000117025,\ 0.,\ -0.000799706,\ 0.000330328\}$

Axial effects:

Interpolation functions, $N_u^T = \{1. -0.00694444t, 0.00694444t\}$

Axial displacement,
$$u(t) = \textit{N}_{u}^{T} \begin{pmatrix} d_{1} \\ d_{7} \end{pmatrix} = -0.000172414 \, t$$

Axial force, EA du(t)/dt = -20.

Torsional effects:

Twist angle,
$$\psi(t) = N_u^T \begin{pmatrix} d_4 \\ d_{10} \end{pmatrix} = 0$$

Twisting moment, GJ $d\psi(t)/dt = 0$.

Bending about r-axis:

 $\begin{aligned} \boldsymbol{N}_{v}^{T} &= \left\{6.69796 \times 10^{-7} \ t^{3} - 0.000144676 \ t^{2} + 1, \ 0.0000482253 \ t^{3} - 0.0138889 \ t^{2} + t, \right. \\ &0.000144676 \ t^{2} - 6.69796 \times 10^{-7} \ t^{3}, \ 0.0000482253 \ t^{3} - 0.00694444 \ t^{2} \right\} \end{aligned}$

$$\mathbf{v}(t) = \mathbf{N}_{\mathbf{v}}^{T} \begin{pmatrix} \mathbf{d}_{2} \\ \mathbf{d}_{6} \\ \mathbf{d}_{8} \\ \mathbf{d}_{12} \end{pmatrix} = 7.86875 \times 10^{-9} \, t^{3} - 0.00015917 \, t$$

Bending moment, $M_r = E I_r d^2 v(t)/dt^2 = 0.889955 t$

Shear force, $V_s = dM_r/dt = 0.889955$

Bending about s-axis:

 $\begin{aligned} \textbf{\textit{N}}_{w}^{T} &= \left\{6.69796 \times 10^{-7} \ t^{3} - 0.000144676 \ t^{2} + 1, \ -0.0000482253 \ t^{3} + 0.0138889 \ t^{2} - t, \right. \\ &\left. 0.000144676 \ t^{2} - 6.69796 \times 10^{-7} \ t^{3}, \ 0.00694444 \ t^{2} - 0.0000482253 \ t^{3} \right\} \end{aligned}$

$$w(t) = \textbf{\textit{N}}_{w}^{T} \begin{pmatrix} d_{3} \\ d_{5} \\ d_{9} \\ d_{11} \end{pmatrix} = -1.94202 \times 10^{-8} \, t^{3} + 3.38615 \times 10^{-8} \, t^{2} + 0.000398634 \, t$$

Bending moment, $M_s = -EI_s d^2w(t)/dt^2 = -0.180999 t$

Shear force, $V_r = -dM_s/dt = 0.180999$

Solution for element 2

Nodal values in global coordinates, $\mathbf{d}^{T} = \{0.000575402, 0.000117025, -0.0248276, -0.000799706, 0.000330328, 0, 0, 0.000117025, -0.041634, -0.000799706, 0, 0\}$

Nodal values in local coordinates, $\boldsymbol{d}_{\ell}^{\mathrm{T}} = \boldsymbol{T}\boldsymbol{d} = \{0.000575402, 0.0248276, 0.000117025, -0.000799706, 0., 0.000330328, 0., 0.041634, 0.000117025, -0.000799706, 0., 0.\}$

Axial effects:

Interpolation functions, $N_u^T = \{1. -0.0166667 t, 0.0166667 t\}$

Axial displacement,
$$u(t) = \textbf{\textit{N}}_u^T\!\!\left(\frac{d_1}{d_7}\right) = 0.000575402 - 9.59003 \times 10^{-6}~t$$

Axial force, EA du(t)/dt = -0.889955

Torsional effects:

Twist angle,
$$\psi(t) = N_u^T \begin{pmatrix} d_4 \\ d_{10} \end{pmatrix} = -0.000799706$$

Twisting moment, GJ $d\psi(t)/dt = 8.70253 \times 10^{-16}$

Bending about r-axis:

$$\mathbf{N}_{v}^{T} = \left\{9.25926 \times 10^{-6} \text{ t}^{3} - 0.000833333 \text{ t}^{2} + 1,\right.$$

$$0.000277778\,t^3 - 0.0333333\,t^2 + t,\ 0.000833333\,t^2 - 9.25926 \times 10^{-6}\,t^3,\ 0.000277778\,t^3 - 0.0166667\,t^2\}$$

$$v(t) = \textbf{\textit{N}}_v^T \left(\begin{array}{c} d_2 \\ d_6 \\ d_8 \\ d_{12} \end{array} \right) = -6.3857 \times 10^{-8} \ t^3 + 2.99439 \times 10^{-6} \ t^2 + 0.000330328 \ t + 0.0248276$$

Fixed-end displacement solution, = $5.32141 \times 10^{-10} (60. - t)^2 t^2$

Transverse displacement, v(t) =

$$5.32141 \times 10^{-10} t^4 - 1.27714 \times 10^{-7} t^3 + 4.9101 \times 10^{-6} t^2 + 0.000330328t + 0.0248276$$

Bending moment, $M_r = E I_r d^2 v(t)/d$

$$t^2 = 1.305 \times 10^7 (6.3857 \times 10^{-9} t^2 - 7.66284 \times 10^{-7} t + 9.8202 \times 10^{-6})$$

Shear force, $V_s = dM_r/dt = 0.166667 t - 10$.

Bending about s-axis:

$$\textbf{\textit{N}}_{w}^{T} = \left\{9.25926 \times 10^{-6} \ t^{3} - 0.000833333 \ t^{2} + 1, \ -0.000277778 \ t^{3} + 0.0333333 \ t^{2} - t, \right. \\ 0.000833333 \ t^{2} - 9.25926 \times 10^{-6} \ t^{3}, \ 0.0166667 \ t^{2} - 0.000277778 \ t^{3} \right\}$$

$$w(t) = \vec{N}_{w}^{T} \begin{pmatrix} d_{3} \\ d_{5} \\ d_{9} \\ d_{11} \end{pmatrix} = 0.000117025$$

Bending moment, $M_s = -EI_s d^2w(t)/dt^2 = 0$

Shear force,
$$V_r = -dM_s/dt = 0$$

Solution for element 3

Nodal values in global coordinates, $\boldsymbol{d}^T = \{0.000575402, 0.000117025, -0.0248276, -0.000799706, 0.000330328, 0, 0.000575402, 0, -0.0557153, 0, 0.000330328, 0\}$

Nodal values in local coordinates, $\boldsymbol{d}_{\ell}^{\mathrm{T}} = \boldsymbol{T}\boldsymbol{d} = \{0.000117025, -0.0248276, 0.000575402, 0.000330328, 0., -0.000799706, 0., -0.0557153, 0.000575402, 0.000330328, 0., 0.\}$

Axial effects:

Interpolation functions, $N_u^T = \{1. -0.0166667 t, 0.0166667 t\}$

Axial displacement,
$$u(t) = \textbf{\textit{N}}_u^T\!\!\left(\frac{d_1}{d_7}\right) = 0.000117025 - 1.95042 \times 10^{-6}~t$$

Axial force, EA du(t)/dt = -0.180999

Torsional effects:

Twist angle,
$$\psi(t) = N_{u}^{T} \begin{pmatrix} d_{4} \\ d_{10} \end{pmatrix} = 0.000330328$$

Twisting moment, GJ $d\psi(t)/dt = 0$.

Bending about r-axis:

$$\begin{aligned} \textbf{\textit{N}}_{v}^{T} &= \left\{9.25926 \times 10^{-6} \ t^{3} - 0.000833333 \ t^{2} + 1, \right. \\ &0.000277778 \ t^{3} - 0.0333333 \ t^{2} + t, \ 0.000833333 \ t^{2} - 9.25926 \times 10^{-6} \ t^{3}, \ 0.000277778 \ t^{3} - 0.0166667 \ t^{2} \right\} \end{aligned}$$

$$v(t) = N_v^T \begin{pmatrix} d_2 \\ d_6 \\ d_8 \\ d_{12} \end{pmatrix} = 6.3857 \times 10^{-8} t^3 + 9.17092 \times 10^{-7} t^2 - 0.000799706 t - 0.0248276$$

Fixed-end displacement solution, = $-5.32141 \times 10^{-10} (60. - t)^2 t^2$

Transverse displacement, v(t) =

$$-5.32141\times 10^{-10}\,t^4 + 1.27714\times 10^{-7}\,t^3 - 9.98617\times 10^{-7}\,t^2 - 0.000799706\,t - 0.0248276$$

Bending moment,
$$M_r = E I_r d^2 v(t)/d$$

$$t^2 = 1.305 \times 10^7 (-6.3857 \times 10^{-9} t^2 + 7.66284 \times 10^{-7} t - 1.99723 \times 10^{-6})$$

Shear force,
$$V_s = dM_r/dt = 10. - 0.166667 t$$

Bending about s-axis:

$$\begin{aligned} \textbf{\textit{N}}_{w}^{T} &= \left\{9.25926 \times 10^{-6}~t^{3} - 0.000833333~t^{2} + 1,~ -0.000277778~t^{3} + 0.0333333~t^{2} - t, \right. \\ &\left. 0.000833333~t^{2} - 9.25926 \times 10^{-6}~t^{3},~ 0.0166667~t^{2} - 0.000277778~t^{3} \right\} \end{aligned}$$

$$w(t) = \textbf{\textit{N}}_{w}^{T} \left(\begin{array}{c} d_{3} \\ d_{5} \\ d_{9} \\ d_{11} \end{array} \right) = 0.000575402$$

Bending moment, $M_s = -EI_s d^2w(t)/dt^2 = 0$

Shear force, $V_r = -dM_s/dt = 0$

Forces & Moments at element ends

	X	y	\mathbf{z}	Axial force	V_s	$V_{\rm r}$	$M_{\rm r}$	M_s	M_t
1	0	0	0	-20.	0.889955	0.180999	0	0	0
	0	0	144.	-20.	0.889955	0.180999	128.154	-26.0639	0
2	0	0	144.	-0.889955	-10 .	0	128.154	0	0
	60.	0	144.	-0.889955	0	0	-171.846	0	0
3	0	0	144.	-0.180999	10.	0	-26.0639	0	0
	0	60.	144.	-0.180999	0	0	273.936	0	0