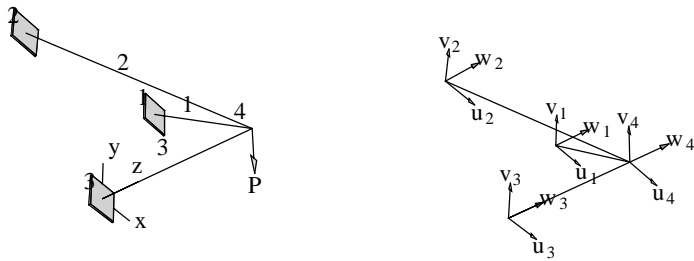


Example 4.2: Three-bar truss (p. 230)

The cross-sectional areas of elements 1 and 2 is 200 mm^2 and that of element 3 is 600 mm^2 . All elements are made of the same material with $E = 200 \text{ GPa}$. The applied load is $P = 20 \text{ kN}$. The nodal coordinates are as follows.

Node	$x(m)$	$y(m)$	$z(m)$
1	0.96	1.92	0
2	-1.44	1.44	0
3	0	0	0
4	0	0	2



The complete computations are as follows. The numerical values are in the $N - \text{mm}$ units. The computed displacements are in mm and the stresses in MPa .

Specified nodal loads

Node	dof	Value
4	u_4	0
	v_4	-20000
	w_4	0

Global equations at start of the element assembly process

$$\begin{pmatrix}
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0
\end{pmatrix}
\begin{pmatrix}
u_1 \\ v_1 \\ w_1 \\ u_2 \\ v_2 \\ w_2 \\ u_3 \\ v_3 \\ w_3 \\ u_4 \\ v_4 \\ w_4
\end{pmatrix}
=
\begin{pmatrix}
0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ -20000 \\ 0
\end{pmatrix}$$

Equations for element 1

$$E = 210000$$

$$A = 200$$

Element node	Global node number	x	y	z
1	1	960.	1920.	0
2	4	0	0	2000.

$$\text{Direction cosines, } \ell_s = -0.327205 \quad m_s = -0.65441 \quad n_s = 0.681677$$

Substituting into the truss element equations we get

$$\begin{pmatrix}
1532.63 & 3065.27 & -3192.99 & -1532.63 & -3065.27 & 3192.99 \\
3065.27 & 6130.53 & -6385.97 & -3065.27 & -6130.53 & 6385.97 \\
-3192.99 & -6385.97 & 6652.06 & 3192.99 & 6385.97 & -6652.06 \\
-1532.63 & -3065.27 & 3192.99 & 1532.63 & 3065.27 & -3192.99 \\
-3065.27 & -6130.53 & 6385.97 & 3065.27 & 6130.53 & -6385.97 \\
3192.99 & 6385.97 & -6652.06 & -3192.99 & -6385.97 & 6652.06
\end{pmatrix}
\begin{pmatrix}
u_1 \\ v_1 \\ w_1 \\ u_4 \\ v_4 \\ w_4
\end{pmatrix}
=
\begin{pmatrix}
0. \\ 0. \\ 0. \\ 0. \\ 0. \\ 0.
\end{pmatrix}$$

The element contributes to {1, 2, 3, 10, 11, 12} global degrees of freedom.

Adding element equations into appropriate locations we have

$$\begin{pmatrix}
1532.63 & 3065.27 & -3192.99 & 0 & 0 & 0 & 0 & 0 & 0 & -1532.63 & -3065.27 & 3192.99 \\
3065.27 & 6130.53 & -6385.97 & 0 & 0 & 0 & 0 & 0 & 0 & -3065.27 & -6130.53 & 6385.97 \\
-3192.99 & -6385.97 & 6652.06 & 0 & 0 & 0 & 0 & 0 & 0 & 3192.99 & 6385.97 & -6652.06 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
-1532.63 & -3065.27 & 3192.99 & 0 & 0 & 0 & 0 & 0 & 0 & 1532.63 & 3065.27 & -3192.99 \\
-3065.27 & -6130.53 & 6385.97 & 0 & 0 & 0 & 0 & 0 & 0 & 3065.27 & 6130.53 & -6385.97 \\
3192.99 & 6385.97 & -6652.06 & 0 & 0 & 0 & 0 & 0 & 0 & -3192.99 & -6385.97 & 6652.06
\end{pmatrix}
\begin{pmatrix}
u_1 \\
v_1 \\
w_1 \\
u_2 \\
v_2 \\
w_2 \\
u_3 \\
v_3 \\
w_3 \\
u_4 \\
v_4 \\
w_4
\end{pmatrix}
=
\begin{pmatrix}
0 \\
0 \\
0 \\
0 \\
0 \\
0 \\
0 \\
0 \\
0 \\
0 \\
-20000. \\
0
\end{pmatrix}$$

Equations for element 2

$$E = 210000$$

$$A = 200$$

Element node	Global node number	x	y	z
1	2	-1440.	1440.	0
2	4	0	0	2000.

$$\text{Direction cosines, } \ell_s = 0.504497 \quad m_s = -0.504497 \quad n_s = 0.70069$$

Substituting into the truss element equations we get

$$\begin{pmatrix}
3745.09 & -3745.09 & 5201.51 & -3745.09 & 3745.09 & -5201.51 \\
-3745.09 & 3745.09 & -5201.51 & 3745.09 & -3745.09 & 5201.51 \\
5201.51 & -5201.51 & 7224.32 & -5201.51 & 5201.51 & -7224.32 \\
-3745.09 & 3745.09 & -5201.51 & 3745.09 & -3745.09 & 5201.51 \\
3745.09 & -3745.09 & 5201.51 & -3745.09 & 3745.09 & -5201.51 \\
-5201.51 & 5201.51 & -7224.32 & 5201.51 & -5201.51 & 7224.32
\end{pmatrix}
\begin{pmatrix}
u_2 \\
v_2 \\
w_2 \\
u_4 \\
v_4 \\
w_4
\end{pmatrix}
=
\begin{pmatrix}
0. \\
0. \\
0. \\
0. \\
0. \\
0.
\end{pmatrix}$$

The element contributes to {4, 5, 6, 10, 11, 12} global degrees of freedom.

Adding element equations into appropriate locations we have

$$\begin{pmatrix}
 1532.63 & 3065.27 & -3192.99 & 0 & 0 & 0 & 0 & 0 & 0 & -1532.63 & -3065.27 & 319 \\
 3065.27 & 6130.53 & -6385.97 & 0 & 0 & 0 & 0 & 0 & 0 & -3065.27 & -6130.53 & 638 \\
 -3192.99 & -6385.97 & 6652.06 & 0 & 0 & 0 & 0 & 0 & 0 & 3192.99 & 6385.97 & -665 \\
 0 & 0 & 0 & 3745.09 & -3745.09 & 5201.51 & 0 & 0 & 0 & -3745.09 & 3745.09 & -520 \\
 0 & 0 & 0 & -3745.09 & 3745.09 & -5201.51 & 0 & 0 & 0 & 3745.09 & -3745.09 & 520 \\
 0 & 0 & 0 & 5201.51 & -5201.51 & 7224.32 & 0 & 0 & 0 & -5201.51 & 5201.51 & -722 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & \\
 -1532.63 & -3065.27 & 3192.99 & -3745.09 & 3745.09 & -5201.51 & 0 & 0 & 0 & 5277.72 & -679.818 & 200 \\
 -3065.27 & -6130.53 & 6385.97 & 3745.09 & -3745.09 & 5201.51 & 0 & 0 & 0 & -679.818 & 9875.62 & -1158 \\
 3192.99 & 6385.97 & -6652.06 & -5201.51 & 5201.51 & -7224.32 & 0 & 0 & 0 & 2008.52 & -11587.5 & 1387
 \end{pmatrix}$$

Equations for element 3

$$E = 210000 \quad A = 600$$

Element node	Global node number	x	y	z
1	3	0	0	0
2	4	0	0	2000.

$$\text{Direction cosines, } \ell_s = 0 \quad m_s = 0 \quad n_s = 1.$$

Substituting into the truss element equations we get

$$\begin{pmatrix}
 0. & 0. & 0. & 0. & 0. & 0. \\
 0. & 0. & 0. & 0. & 0. & 0. \\
 0. & 0. & 63000. & 0. & 0. & -63000. \\
 0. & 0. & 0. & 0. & 0. & 0. \\
 0. & 0. & 0. & 0. & 0. & 0. \\
 0. & 0. & -63000. & 0. & 0. & 63000.
 \end{pmatrix}
 \begin{pmatrix}
 u_3 \\
 v_3 \\
 w_3 \\
 u_4 \\
 v_4 \\
 w_4
 \end{pmatrix}
 =
 \begin{pmatrix}
 0. \\
 0. \\
 0. \\
 0. \\
 0. \\
 0.
 \end{pmatrix}$$

The element contributes to {7, 8, 9, 10, 11, 12} global degrees of freedom.

Adding element equations into appropriate locations we have

$$\begin{pmatrix}
 1532.63 & 3065.27 & -3192.99 & 0 & 0 & 0 & 0 & 0 & 0 & -1532.63 & -3065.27 \\
 3065.27 & 6130.53 & -6385.97 & 0 & 0 & 0 & 0 & 0 & 0 & -3065.27 & -6130.53 \\
 -3192.99 & -6385.97 & 6652.06 & 0 & 0 & 0 & 0 & 0 & 0 & 3192.99 & 6385.97 \\
 0 & 0 & 0 & 3745.09 & -3745.09 & 5201.51 & 0 & 0 & 0 & -3745.09 & 3745.09 \\
 0 & 0 & 0 & -3745.09 & 3745.09 & -5201.51 & 0 & 0 & 0 & 3745.09 & -3745.09 \\
 0 & 0 & 0 & 5201.51 & -5201.51 & 7224.32 & 0 & 0 & 0 & -5201.51 & 5201.51 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 63000. & 0 & 0 \\
 -1532.63 & -3065.27 & 3192.99 & -3745.09 & 3745.09 & -5201.51 & 0 & 0 & 0 & 5277.72 & -679.818 \\
 -3065.27 & -6130.53 & 6385.97 & 3745.09 & -3745.09 & 5201.51 & 0 & 0 & 0 & -679.818 & 9875.62 \\
 3192.99 & 6385.97 & -6652.06 & -5201.51 & 5201.51 & -7224.32 & 0 & 0 & -63000. & 2008.52 & -11587.5
 \end{pmatrix}$$

Essential boundary conditions

Node	dof	Value
1	u_1	0
	v_1	0
	w_1	0
2	u_2	0
	v_2	0
	w_2	0
3	u_3	0
	v_3	0
	w_3	0

Remove {1, 2, 3, 4, 5, 6, 7, 8, 9} rows and columns.

After adjusting for essential boundary conditions we have

$$\begin{pmatrix}
 5277.72 & -679.818 & 2008.52 \\
 -679.818 & 9875.62 & -11587.5 \\
 2008.52 & -11587.5 & 76876.4
 \end{pmatrix}
 \begin{pmatrix}
 u_4 \\
 v_4 \\
 w_4
 \end{pmatrix}
 =
 \begin{pmatrix}
 0 \\
 -20000. \\
 0
 \end{pmatrix}$$

Solving the final system of global equations we get

$$\{u_4 = -0.178143, v_4 = -2.46857, w_4 = -0.367431\}$$

Complete table of nodal values

	u	v	w
1	0	0	0
2	0	0	0
3	0	0	0
4	-0.178143	-2.46857	-0.367431

Computation of reactions

Equation numbers of dof with specified values: {1, 2, 3, 4, 5, 6, 7, 8, 9}

Extracting equations {1, 2, 3, 4, 5, 6, 7, 8, 9} from the global system we have

$$\begin{pmatrix}
 1532.63 & 3065.27 & -3192.99 & 0 & 0 & 0 & 0 & 0 & 0 & -1532.63 & -3065.27 & 3192.99 \\
 3065.27 & 6130.53 & -6385.97 & 0 & 0 & 0 & 0 & 0 & 0 & -3065.27 & -6130.53 & 6385.97 \\
 -3192.99 & -6385.97 & 6652.06 & 0 & 0 & 0 & 0 & 0 & 0 & 3192.99 & 6385.97 & -6652.06 \\
 0 & 0 & 0 & 3745.09 & -3745.09 & 5201.51 & 0 & 0 & 0 & -3745.09 & 3745.09 & -5201.51 \\
 0 & 0 & 0 & -3745.09 & 3745.09 & -5201.51 & 0 & 0 & 0 & 3745.09 & -3745.09 & 5201.51 \\
 0 & 0 & 0 & 5201.51 & -5201.51 & 7224.32 & 0 & 0 & 0 & -5201.51 & 5201.51 & -7224.32 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 63000. & 0 & 0 & -63000.
 \end{pmatrix}$$

Substituting the nodal values and re-arranging

$$\begin{pmatrix} R_1 \\ R_2 \\ R_3 \\ R_4 \\ R_5 \\ R_6 \\ R_7 \\ R_8 \\ R_9 \end{pmatrix} = \begin{pmatrix}
 1532.63 & 3065.27 & -3192.99 & 0 & 0 & 0 & 0 & 0 & 0 & -1532.63 & -3065.27 & 3192.99 \\
 3065.27 & 6130.53 & -6385.97 & 0 & 0 & 0 & 0 & 0 & 0 & -3065.27 & -6130.53 & 6385.97 \\
 -3192.99 & -6385.97 & 6652.06 & 0 & 0 & 0 & 0 & 0 & 0 & 3192.99 & 6385.97 & -6652.06 \\
 0 & 0 & 0 & 3745.09 & -3745.09 & 5201.51 & 0 & 0 & 0 & -3745.09 & 3745.09 & -5201.51 \\
 0 & 0 & 0 & -3745.09 & 3745.09 & -5201.51 & 0 & 0 & 0 & 3745.09 & -3745.09 & 5201.51 \\
 0 & 0 & 0 & 5201.51 & -5201.51 & 7224.32 & 0 & 0 & 0 & -5201.51 & 5201.51 & -7224.32 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 63000. & 0 & 0 & -63000.
 \end{pmatrix}$$

Carrying out computations, the reactions are as follows.

Label	dof	Reaction
R ₁	u ₁	6666.67
R ₂	v ₁	13333.3
R ₃	w ₁	-13888.9
R ₄	u ₂	-6666.67
R ₅	v ₂	6666.67
R ₆	w ₂	-9259.26
R ₇	u ₃	0
R ₈	v ₃	0
R ₉	w ₃	23148.1

Sum of Reactions

dof: u	0
dof: v	20000.
dof: w	0

Solution for element 1

Nodal coordinates

Element node	Global node number	x	y	z
1	1	960.	1920.	0
2	4	0	0	2000.

Direction cosines, $\ell_s = -0.327205$ $m_s = -0.65441$ $n_s = 0.681677$

Global to local transformation matrix, $T =$

$$\begin{pmatrix} -0.327205 & -0.65441 & 0.681677 & 0 & 0 & 0 \\ 0 & 0 & 0 & -0.327205 & -0.65441 & 0.681677 \end{pmatrix}$$

$$\text{Element nodal displacements in global coordinates, } \mathbf{d} = \begin{pmatrix} u_1 \\ v_1 \\ w_1 \\ u_4 \\ v_4 \\ w_4 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ 0 \\ -0.178143 \\ -2.46857 \\ -0.367431 \end{pmatrix}$$

$$\text{Element nodal displacements in local coordinates, } \mathbf{d}_\ell = \mathbf{T} \mathbf{d} = \begin{pmatrix} 0. \\ 1.42328 \end{pmatrix}$$

Axial displacements at element ends, $d_1 = 0.$ $d_2 = 1.42328$

$E = 210000$ $A = 200$

Axial strain, $\epsilon = (d_2 - d_1)/L = 0.000485109$

$$\text{Axial stress, } \sigma = E\epsilon = 101.873$$

$$\text{Axial force} = \sigma A = 20374.6$$

Solution for element 2

Nodal coordinates

Element node	Global node number	x	y	z
1	2	-1440.	1440.	0
2	4	0	0	2000.

$$\text{Direction cosines, } \ell_s = 0.504497 \quad m_s = -0.504497 \quad n_s = 0.70069$$

Global to local transformation matrix, $T =$

$$\begin{pmatrix} 0.504497 & -0.504497 & 0.70069 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0.504497 & -0.504497 & 0.70069 \end{pmatrix}$$

$$\text{Element nodal displacements in global coordinates, } \mathbf{d} = \begin{pmatrix} u_2 \\ v_2 \\ w_2 \\ u_4 \\ v_4 \\ w_4 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ 0 \\ -0.178143 \\ -2.46857 \\ -0.367431 \end{pmatrix}$$

$$\text{Element nodal displacements in local coordinates, } \mathbf{d}_l = \mathbf{T} \mathbf{d} = \begin{pmatrix} 0. \\ 0.89806 \end{pmatrix}$$

$$\text{Axial displacements at element ends, } d_1 = 0. \quad d_2 = 0.89806$$

$$E = 210000 \quad A = 200$$

$$\text{Axial strain, } \epsilon = (d_2 - d_1)/L = 0.000314631$$

$$\text{Axial stress, } \sigma = E\epsilon = 66.0725$$

$$\text{Axial force} = \sigma A = 13214.5$$

Solution for element 3

Nodal coordinates

Element node	Global node number	x	y	z
1	3	0	0	0
2	4	0	0	2000.

$$\text{Direction cosines, } \ell_s = 0 \quad m_s = 0 \quad n_s = 1.$$

$$\text{Global to local transformation matrix, } \mathbf{T} = \begin{pmatrix} 0 & 0 & 1. & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1. \end{pmatrix}$$

$$\text{Element nodal displacements in global coordinates, } \mathbf{d} = \begin{pmatrix} u_3 \\ v_3 \\ w_3 \\ u_4 \\ v_4 \\ w_4 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ 0 \\ -0.178143 \\ -2.46857 \\ -0.367431 \end{pmatrix}$$

$$\text{Element nodal displacements in local coordinates, } \mathbf{d}_\ell = \mathbf{T} \mathbf{d} = \begin{pmatrix} 0. \\ -0.367431 \end{pmatrix}$$

$$\text{Axial displacements at element ends, } d_1 = 0. \quad d_2 = -0.367431$$

$$E = 210000 \quad A = 600$$

$$\text{Axial strain, } \epsilon = (d_2 - d_1)/L = -0.000183715$$

$$\text{Axial stress, } \sigma = E\epsilon = -38.5802 \quad \text{Axial force} = \sigma A = -23148.1$$

Solution summary

Nodal solution

	x-coord	y-coord	z-coord	u	v	w
1	960.	1920.	0	0	0	0
2	-1440.	1440.	0	0	0	0
3	0	0	0	0	0	0
4	0	0	2000.	-0.178143	-2.46857	-0.367431

Element solution

	Stress	Axial force
1	101.873	20374.6
2	66.0725	13214.5
3	-38.5802	-23148.1

Support reactions

Node	dof	Reaction
1	u_1	6666.67
1	v_1	13333.3
1	w_1	-13888.9
2	u_2	-6666.67
2	v_2	6666.67
2	w_2	-9259.26
3	u_3	0.
3	v_3	0.
3	w_3	23148.1

Sum of applied loads $\rightarrow (0 \quad -20000. \quad 0)$

Sum of support reactions $\rightarrow (0 \quad 20000. \quad 0)$