

CHAPTER EIGHT

Material Nonlinearity

Example 8.3 (p. 427): Four-bar truss

Consider a four bar pin-jointed truss shown in Figure 8.14. Assume $E = 200$ GPa, $\sigma_Y = 150$ MPa, and $H = 10$ GPa. For elements 1 and 2 assume kinematic hardening material with area of cross section $A = 0.0004 \text{ m}^2$. For the rest use an isotropic hardening model with area of cross section $A = 0.0002 \text{ m}^2$. The load $P = 40$ kN and acts at an angle $\alpha = 60^\circ$ at node 2. Determine permanent deflection after the load is removed. The dimensions in meters are shown in the figure. We use N and mm units in calculations. The displacements will be in mm and the stress in MPa.

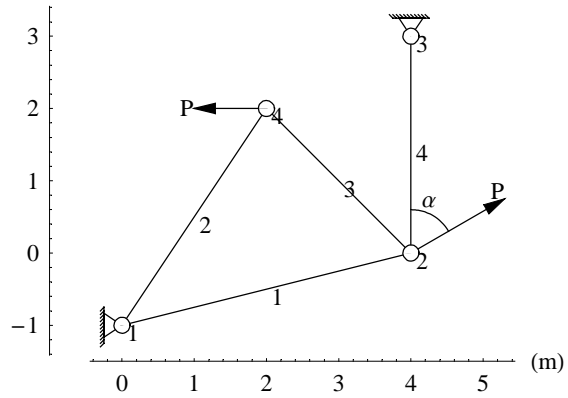


Figure 8.14. Four-bar Truss

AnsysFiles\\Chap08\\FourBarTruss.txt

```

!* Model creation
/PREP7
!* Element type
ET,1,LINK1
!* Real constants
R,1,400, ,
R,2,200,,
!* Material property
!*
*set,e,200000
*set,h,10000
*set,et,e*h/(e+h)
*set, sy, 150
MPTEMP,,,,,,,,
MPTEMP,1,0
MPDATA,EX,1,,e
MPTEMP,,,,,,,,
MPTEMP,1,0
MPDATA,EX,2,,e
TB,BKIN,1,1,2,1

```

```
!*,BISO,2,1,2,
TBTEMP,0
TBDATA,,sy,et,,,,
TB,BISO,2,1,2,
TBTEMP,0
TBDATA,,sy,et,,,,
!* Create nodes
N,1,0,-1000,,,,
N,2,4000,0,,,,
N,3,4000,3000,,,,
N,4,2000,2000,,,,
!* Create elements
!* with appropriate attributes
MAT,1
REAL,1
E,1,2
E,1,4
MAT,2
REAL,2
e,2,4
e,2,3
!*
!* Specify displacement boundary conditions
D,1,,0,,,UX,UY,,,,
D,3,,0,,,UX,UY,,,,
!* Solution module
/SOLU
ARCLen,1,1,0.0001
AUTOTS,-1.0
NCNV,2,0,0,0,0
RESCONTRL,DEFINE,ALL,1,1
OUTRES,ERASE
OUTRES,ALL,-1
AUTOTS,-1.0
!* First load step - no applied load
!* Used for initialization of the arc-length controls
LSWRITE,1,
!* Specify applied forces
F,2,FY,20000
F,2,FX,34641
```

```
!*,2,FY,0
F,4,FX,-40000
LSWRITE,2,
!* Remove applied forces
F,2,FY,0
F,2,FX,0
F,4,FX,0
LSWRITE,3,
LSSOLVE,1,3,1
FINISH
!* Postprocessing
/POST1
SET, LAST
ETABLE, SIGMA, LS, 1
ETABLE, EPSPLAS, LEPL, 1
PRETAB, SIGMA, EPSPLAS
```

Converged results after full application of given loads

```
PRINT U      NODAL SOLUTION PER NODE

***** POST1 NODAL DEGREE OF FREEDOM LISTING *****

LOAD STEP=      2   SUBSTEP=      9
TIME=      2.0000      LOAD CASE=      0

THE FOLLOWING DEGREE OF FREEDOM RESULTS ARE IN GLOBAL COORDINATES

      NODE      UY
      1      0.0000
      2      20.117
      3      0.0000
      4      13.401

MAXIMUM ABSOLUTE VALUES
NODE      2
VALUE      20.117

PRINT ELEMENT TABLE ITEMS PER ELEMENT

***** POST1 ELEMENT TABLE LISTING *****

      STAT      PREVIOUS      PREVIOUS
      ELEM      SIGMA      EPSPLAS
      1      0.14655E-17      0.0000
      2      -0.38538E-17      0.0000
      3      0.0000      0.19723E-02
```

4 0.0000 -0.56719E-02

MINIMUM VALUES

ELEM 2 4
VALUE -0.38538E-17 -0.56719E-02

MAXIMUM VALUES

ELEM 1 3
VALUE 0.14655E-17 0.19723E-02

Converged results after the loads are removed

PRINT U NODAL SOLUTION PER NODE

***** POST1 NODAL DEGREE OF FREEDOM LISTING *****

LOAD STEP= 3 SUBSTEP= 12
TIME= 3.0000 LOAD CASE= 0

THE FOLLOWING DEGREE OF FREEDOM RESULTS ARE IN GLOBAL COORDINATES

NODE	UY
1	0.0000
2	17.016
3	0.0000
4	11.664

MAXIMUM ABSOLUTE VALUES

NODE 2
VALUE 17.016

PRINT ELEMENT TABLE ITEMS PER ELEMENT

***** POST1 ELEMENT TABLE LISTING *****

STAT	CURRENT	CURRENT
ELEM	SIGMA	EPSPLAS
1	0.14655E-17	0.0000
2	-0.38538E-17	0.0000
3	0.0000	0.19723E-02
4	0.0000	-0.56719E-02

MINIMUM VALUES

ELEM 2 4
VALUE -0.38538E-17 -0.56719E-02

MAXIMUM VALUES

ELEM 1 3
VALUE 0.14655E-17 0.19723E-02

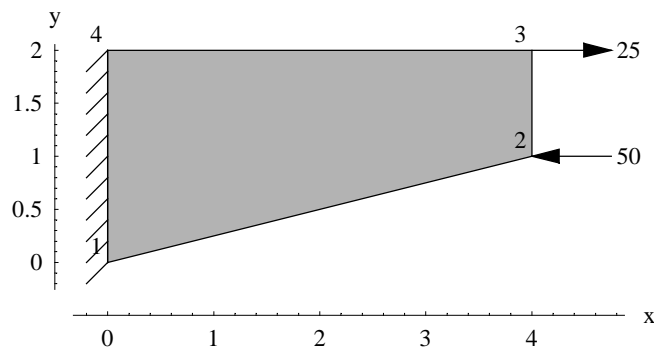
Tension Plate Example

This example is not included in the printed book.

Small displacement analysis of a tension plate with nonlinear material properties.

$$E = 100000, \nu = 0.25, h = 0.1$$

$$\sigma_Y = 380, H = 260$$



■ One element solution with kinematic hardening

AnsyzFiles\\Chap08\\TensionPlateKin1.txt

```
!* Tension plate
!* Small displacement analysis
!* von Mises plasticity
!* with kinematic hardening
/PREP7
!* Element type
ET,1,PLANE42
```

```
!*
!*
KEYOPT,1,1,0
KEYOPT,1,2,1
KEYOPT,1,3,3
KEYOPT,1,5,0
KEYOPT,1,6,0
!*
R,1,0.1
!* Material property
!*
*set,e,100000
*set,h,260
*set,et,e*h/(e+h)
*set, sy,380
MPTEMP,,,,,,,,
MPTEMP,1,0
MPDATA,EX,1,,e
MPDATA,PRXY,1,,0.25
TB,BKIN,1,1,2,1
TBTEMP,0
TBDATA,,sy,et,,,
!*
K,1,0,0
K,2,4,1
K,3,4,2
K,4,0,2
A,1,2,3,4
ESIZE,10
AMESH,1
DL,4, ,ALL
/SOLU
ARCLEN,1,1,0.0001
AUTOTS,-1.0
NCNV,2,0,0,0,0
RESCONTRL,DEFINE,ALL,1,1
ERESX,NO
OUTRES,ERASE
OUTRES,ALL,-1
```

```
AUTOTS,-1.0
!* First load step - no applied load
!* Used for initialization of the arc-length controls
LSWRITE,1,
!* Specify applied forces
FK,2,FX,-50
FK,3,FX,25
LSWRITE,2,
!* Remove applied forces
FK,2,FX,0
FK,3,FX,0
LSWRITE,3,
!* Reverse forces
FK,2,FX,50
FK,3,FX,-25
LSWRITE,4,
!* Remove applied forces
FK,2,FX,0
FK,3,FX,0
LSWRITE,5,
!* Specify applied forces
FK,2,FX,-50
FK,3,FX,25
LSWRITE,6,
!* Remove applied forces
FK,2,FX,0
FK,3,FX,0
LSWRITE,7,
LSSOLVE,1,7,1
FINISH
!* Postprocessing
/POST1
SET,LAST
PRNSOL,UX
```

Results after three full load cycles (load step 6)

```
PRINT U      NODAL SOLUTION PER NODE
```

```
***** POST1 NODAL DEGREE OF FREEDOM LISTING *****
```

LOAD STEP= 6 SUBSTEP= 6
TIME= 6.0000 LOAD CASE= 0

THE FOLLOWING DEGREE OF FREEDOM RESULTS ARE IN GLOBAL COORDINATES

NODE	UX
1	0.0000
2	-0.24793E-01
3	0.79347E-02
4	0.0000

MAXIMUM ABSOLUTE VALUES

NODE 2
VALUE -0.24793E-01

PRINT S ELEMENT SOLUTION PER ELEMENT

***** POST1 ELEMENT NODAL STRESS LISTING *****

LOAD STEP= 6 SUBSTEP= 6
TIME= 6.0000 LOAD CASE= 0

THE FOLLOWING X,Y,Z VALUES ARE IN GLOBAL COORDINATES

ELEMENT=	1	PLANE42				
NODE	SX	SY	SZ	SXY	SYZ	
SXZ						
1	-378.10	-102.04	0.0000	-100.01	0.0000	
0.0000						
2	-301.68	-58.105	0.0000	151.77	0.0000	
0.0000						
3	9.9876	107.82	0.0000	211.93	0.0000	
0.0000						
4	16.510	45.894	0.0000	-146.29	0.0000	
0.0000						

PRINT EPPL ELEMENT SOLUTION PER ELEMENT

***** POST1 ELEMENT NODAL PLASTIC STRAIN LISTING *****

LOAD STEP= 6 SUBSTEP= 6
TIME= 6.0000 LOAD CASE= 0

THE FOLLOWING X,Y,Z VALUES ARE IN GLOBAL COORDINATES

ELEMENT=	1	PLANE42				
NODE	EPPLX	EPPLY	EPPLZ	EPPLXY	EPPLYZ	
EPPLXZ						

```
      1 -0.17056E-02 0.49277E-03 0.12128E-02-0.17275E-02 0.0000
0.0000
      2 -0.57990E-02 0.21285E-02 0.36705E-02 0.90483E-02 0.0000
0.0000
      3 -0.70142E-03 0.12484E-02-0.54700E-03 0.83867E-02 0.0000
0.0000
      4 0.0000      0.0000      0.0000      0.0000      0.0000
0.0000
```

Results after load is removed (load step 7)

```
PRINT U      NODAL SOLUTION PER NODE
```

```
***** POST1 NODAL DEGREE OF FREEDOM LISTING *****
```

```
LOAD STEP=      7  SUBSTEP=     46
TIME=      7.0000      LOAD CASE=    0
```

```
THE FOLLOWING DEGREE OF FREEDOM RESULTS ARE IN GLOBAL COORDINATES
```

```
NODE      UX
  1      0.0000
  2 -0.13312E-01
  3 0.49024E-02
  4      0.0000
```

```
MAXIMUM ABSOLUTE VALUES
```

```
NODE      2
VALUE -0.13312E-01
```

■ One element solution with isotropic hardening

AnsysFiles\\Chap08\\TensionPlateIso1.txt

```
!* Tension plate
!* Small displacement analysis
!* von Mises plasticity
!* with isotropic hardening
/PREP7
!* Element type
ET,1,PLANE42
!*
!*
```

```
KEYOPT,1,1,0
KEYOPT,1,2,1
KEYOPT,1,3,3
KEYOPT,1,5,0
KEYOPT,1,6,0
!*
R,1,0.1
!* Material property
!*
*set,e,100000
*set,h,260
*set,et,e*h/(e+h)
*set, sy,380
MPTEMP,,,,,,,,
MPTEMP,1,0
MPDATA,EX,1,,e
MPDATA,PRXY,1,,0.25
TB,BISO,1,1,2,
TBTEMP,0
TBDATA,,sy,et,,,
!*
K,1,0,0
K,2,4,1
K,3,4,2
K,4,0,2
A,1,2,3,4
ESIZE,10
AMESH,1
DL,4, ,ALL
/SOLU
ARCLen,1,1,0.0001
AUTOTS,-1.0
NCNV,2,0,0,0,0
RESCONTRL,DEFINE,ALL,1,1
ERESX,NO
OUTRES,ERASE
OUTRES,ALL,-1
AUTOTS,-1.0
!* First load step - no applied load
```

```
!* Used for initialization of the arc-length controls
LSWRITE,1,
!* Specify applied forces
FK,2,FX,-50
FK,3,FX,25
LSWRITE,2,
!* Remove applied forces
FK,2,FX,0
FK,3,FX,0
LSWRITE,3,
!* Reverse forces
FK,2,FX,50
FK,3,FX,-25
LSWRITE,4,
!* Remove applied forces
FK,2,FX,0
FK,3,FX,0
LSWRITE,5,
!* Specify applied forces
FK,2,FX,-50
FK,3,FX,25
LSWRITE,6,
!* Remove applied forces
FK,2,FX,0
FK,3,FX,0
LSWRITE,7,
LSSOLVE,1,7,1
FINISH
!* Postprocessing
/POST1
SET,LAST
PRNSOL,UX
```

Results after three full load cycles (load step 6)

```
PRINT U      NODAL SOLUTION PER NODE

***** POST1 NODAL DEGREE OF FREEDOM LISTING *****

LOAD STEP=      6  SUBSTEP=      5
```

TIME= 6.0000 LOAD CASE= 0

THE FOLLOWING DEGREE OF FREEDOM RESULTS ARE IN GLOBAL COORDINATES

NODE	UX
1	0.0000
2	-0.21067E-01
3	0.65911E-02
4	0.0000

MAXIMUM ABSOLUTE VALUES

NODE	2
VALUE	-0.21067E-01

PRINT S ELEMENT SOLUTION PER ELEMENT

***** POST1 ELEMENT NODAL STRESS LISTING *****

LOAD STEP= 6 SUBSTEP= 5
TIME= 6.0000 LOAD CASE= 0

THE FOLLOWING X,Y,Z VALUES ARE IN GLOBAL COORDINATES

ELEMENT=	1	PLANE42				
NODE	SX	SY	SZ	SXY	SYZ	
SXZ						
0.0000	1	-360.49	-84.457	0.0000	-114.35	0.0000
0.0000	2	-308.16	-56.531	0.0000	152.97	0.0000
0.0000	3	-0.73189	99.387	0.0000	214.79	0.0000
0.0000	4	10.549	36.619	0.0000	-134.70	0.0000

PRINT EPPL ELEMENT SOLUTION PER ELEMENT

***** POST1 ELEMENT NODAL PLASTIC STRAIN LISTING *****

LOAD STEP= 6 SUBSTEP= 5
TIME= 6.0000 LOAD CASE= 0

THE FOLLOWING X,Y,Z VALUES ARE IN GLOBAL COORDINATES

ELEMENT=	1	PLANE42				
NODE	EPPLX	EPPLY	EPPLZ	EPPLXY	EPPLYZ	
EPPLXZ						
	1	-0.10561E-02	0.28317E-03	0.77291E-03	-0.97283E-03	0.0000

```
0.0000
      2 -0.44158E-02 0.16676E-02 0.27482E-02 0.67819E-02 0.0000
0.0000
      3 -0.50900E-03 0.87697E-03-0.36797E-03 0.59218E-02 0.0000
0.0000
      4 0.0000      0.0000      0.0000      0.0000      0.0000
0.0000
```

Results after load is removed (load step 7)

```
PRINT U      NODAL SOLUTION PER NODE
```

```
***** POST1 NODAL DEGREE OF FREEDOM LISTING *****
```

```
LOAD STEP=      7  SUBSTEP=     23
TIME=      7.0000      LOAD CASE=    0
```

```
THE FOLLOWING DEGREE OF FREEDOM RESULTS ARE IN GLOBAL COORDINATES
```

```
NODE      UX
  1      0.0000
  2 -0.95864E-02
  3  0.35588E-02
  4      0.0000
```

```
MAXIMUM ABSOLUTE VALUES
```

```
NODE      2
VALUE -0.95864E-02
```
