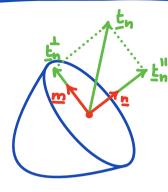
## Normal and Shear Stresses



Consider au arbitrary surface in B

with normal n. Thus we have the

two projection matrices

P"= non and P=I-non=mom

that define the

normal stress:  $\underline{t}_{n}^{\parallel} = \underline{P}^{\parallel} \underline{t}_{n} = (\underline{n} \cdot \underline{t}_{n}) \underline{n} = \underline{s}_{n} \underline{n}$ 

shear stress:  $\underline{t}_{n}^{\perp} = \underline{P}^{\perp}\underline{t}_{n} = (\underline{m} \cdot \underline{t}_{n})\underline{m} = \underline{\tau} \underline{m}$ 

The magnitudes of there stresses are

T = m. tn = m. on T = mioinj

If  $\epsilon_n > 0$  the normal stresses are tensile if  $\epsilon_n < 0$  the normal stresses are compressive.

From geometry:  $\underline{t}_n = \underline{t}_n^{"} + \underline{t}_n^{"}$   $|\underline{t}_n|^2 = |\underline{s}_n^{"}|^2 + |\underline{t}_n^{"}|^2 = \underline{s}_n^2 + \underline{t}_n^2$ 

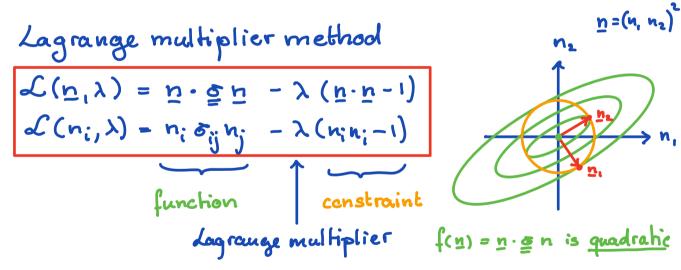
## Extremal Stress Values

I, Haximum and Minimum Normal Stresses

Given a state of stress of at pointx, what are
the unit normals n corresponding to min.

and max. normal stress on.

This is a constrained optimization problem, because we want to find extrema of the function  $\overline{\sigma}_n = \overline{\sigma}_n(\underline{n})$  with the constraint that  $|\underline{n}| = 1$ .



Function  $f(\underline{n}) = \underline{n} \cdot \underline{\sigma}\underline{n}$  is quadratic in components of  $\underline{n}$ . If eigenvalues of  $\underline{\sigma}$  are positive then the level sets of  $f(\underline{n})$  are ellipsoids as shown.

The extremal values are the stationary points of L(m, x)

$$\frac{\partial \mathcal{L}}{\partial \lambda} = n_i n_i - 1 = 0$$

$$\frac{\partial \mathcal{L}}{\partial n_k} = \sigma_{ij} \left( n_{i,k} n_j + n_i n_{j,k} \right) - \lambda \left( 2 n_i n_{i,k} \right) = 0$$
where  $n_{i,k} = S_{i,k}$   $n_{j,k} = S_{j,k}$ 

$$= \sigma_{ij} \left( S_{ik} n_j + S_{jk} n_i \right) - \lambda \left( 2 n_i S_{jk} \right)$$

$$= \sigma_{kj} n_j + \sigma_{ik} n_i - 2\lambda n_k$$

$$= 2 \left( \sigma_{ik} n_k - \lambda n_k \right) = 0$$

In symbolic notation!  $(\underline{\sigma}-\lambda\underline{I})\underline{n}=0$  and  $\underline{I}\underline{n}=1$ . The Lagrange multiplier method leads to an eigen problem, where the Lagrange multiplier,  $\lambda$ , is the eigenvalue and the normal,  $\underline{n}$ , the eigenvalue.

We can see that  $\lambda$  is the magnitude of the normal stress by taking the dot product of eigenproblem with  $\underline{n}$ .  $\underline{n} \cdot (\underline{\diamond} - \lambda \underline{I})\underline{n} = 0 \implies \underline{n} \cdot \underline{\diamond}\underline{n} = \lambda \underline{n} \cdot \underline{n} \implies \overline{\diamond}_{N} = \lambda$ 

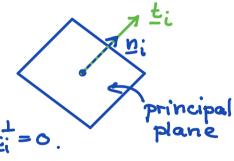
Hence to find the extremal stress values we must find the eigenvalues  $\lambda$ ; and eigenvectors  $\underline{n}$ :.  $\lambda_i$ 's are the principal normal stresses  $\Rightarrow \lambda_i = \overline{s}_i$   $\underline{n}$ 's are the principal elirections of  $\underline{s}$ 

Since  $\underline{\underline{\sigma}} = \underline{\underline{\sigma}}^T$  all  $\lambda_i$  are real and the set  $\{\underline{n}_i\}$  form a mutually orthogonal basis, so that  $\underline{\underline{\sigma}}$  can be represented as  $\underline{\underline{\sigma}} = \underline{\underline{\lambda}}^2 = \underline{\underline{\sigma}}_i$   $\underline{n}_i \otimes \underline{n}_i$ 

The tractions in the principal directions are

Since Lill ni there is no shear

stress on the principal planes, ti=0.



## II. Maximum and minimum shear stresses

Given the principal directions  $n_1$ ,  $n_2$  and  $n_3$  at x what is the unit vector  $s = [s_1 \ s_2 \ s_3]$  that gives the max. and min. values of the shear stresses t > 2

In the frame of the principal directions {n;}

s = s; n; where s; = s.n;

so that the traction vector in direction s is

traction vector associated with 3 is

The magnitudes of normal, on, and shear stress, t, are

$$\delta_{n}^{2} = 5 \cdot \frac{1}{2} = \delta_{1} \cdot 5_{1}^{2} + \delta_{2} \cdot 5_{2}^{2} + \delta_{3} \cdot 5_{3}^{2}$$

$$T = |\xi_{8}|^{2} - \delta_{n}^{2} = \delta_{1}^{2} S_{1}^{2} + \delta_{2}^{2} S_{2}^{2} + \delta_{3}^{2} S_{3}^{2} - (\delta_{1} S_{1}^{2} + \delta_{2} S_{2}^{2} + \delta_{3} S_{3}^{2})^{2}$$

Hence we have the following expression for the shear stress

$$T^{2} = \sum_{i=1}^{3} \delta_{i}^{2} \delta_{i}^{2} - \left(\sum_{j=1}^{3} \delta_{i} \delta_{j}^{2}\right)^{2}$$

we are looking for the extremal values of  $\tau^2$  under the constraint  $|\underline{s}|^2 = 1 \rightarrow s_1^2 + s_2^2 + s_3^2 = 1$   $\Rightarrow$  Solve using Lagrange mult. or direct elimination.

I) Eliminate 
$$S_3^2 = 1 - S_1^2 - S_2^2 \Rightarrow T^2 = T^2(S_1, S_2)$$
.

$$T^2 = S_1^2 S_1^2 + S_2^2 S_2^2 + S_3^2 S_3^2 - (S_1 S_1^2 + S_2 S_2^2 + S_3 S_3^2)^2$$

$$= S_1^2 S_1^2 + S_2^2 S_2^2 + S_3^2 (1 - S_1^2 - S_2^2) - (S_1 S_1^2 + S_2 S_2^2 + S_3 (1 - S_1^2 - S_2^2))$$

S<sub>2</sub> Cou

Constraint 1=1=1 is incorporated

To find extremum

s, postial derivatives mud

to vanish.

We just und to find 
$$\frac{\partial \tau^2}{\partial s_1} = \frac{\partial \tau^2}{\partial s_2} = 0$$
.  
 $\frac{\partial \tau^2}{\partial s_1} = 2 s_1(\delta_1 - \delta_3) \{\delta_1 - \delta_3 - 2[(\delta_1 - \delta_3) s_1^2 + (\delta_2 - \delta_3) s_2^2]\} = 0$   
 $\frac{\partial \tau^2}{\partial s_2} = 2 s_2(\delta_2 - \delta_3) \{\delta_2 - \delta_3 - 2[(\delta_1 - \delta_3) s_1^2 + (\delta_2 - \delta_3) s_2^2]\} = 0$ 

First solution: 
$$S_1 = S_2 = 0 \implies S_3 = 1 \implies S = \pm n_3$$

$$T^2 = \delta_3^2 \cdot 1 - (\delta_3 \cdot 1)^2 = 0$$

> minimum in the shear stress

which vanishes on principal plane

Second solution: 3, = 0

$$\frac{\partial z^{2}}{\partial n_{z}} = \delta_{2} - \delta_{3} - 2\left[(\delta_{2} - \delta_{3})s_{2}^{2}\right] = 0$$

$$(\delta_{2} - \delta_{3})\left(1 - 2s_{2}^{2}\right) = 0 \implies s_{2} = \pm \frac{1}{\sqrt{2}}$$

$$\text{from } s_{2}^{2} + s_{3}^{2} = 1 \implies s_{3} = \pm \frac{1}{\sqrt{2}}$$

$$\Rightarrow s = \pm \frac{1}{\sqrt{2}} \underbrace{n_{2}}_{2} \pm \frac{1}{\sqrt{2}} \underbrace{n_{3}}_{3}$$

$$z = \frac{\delta_{2}^{2}}{2} + \frac{\delta_{3}^{2}}{2} - \left(\frac{\delta_{2}}{4} + 2\frac{\delta_{3}}{2}\right)^{2}$$

$$= \frac{\delta_{2}^{2}}{2} + \frac{\delta_{3}^{2}}{2} - \left(\frac{\delta_{2}^{2}}{4} + 2\frac{\delta_{3}^{2}}{2}\right)^{2}$$

$$\mathcal{T}^2 = \left(\frac{\delta_2}{2}\right)^2 - 2\frac{\delta_2}{2}\frac{\delta_3}{2} + \left(\frac{\delta_3}{2}\right)^2 = \left(\frac{\delta_2 - \delta_3}{2}\right)^2$$

We have the following two solutions:

min. 
$$T=0$$
 for  $\underline{s}=\pm \underline{n}_3$   
max.  $T=\frac{1}{2}(\delta_2-\delta_3)$  for  $\underline{s}=\pm \frac{\underline{n}_2}{\sqrt{2}}\pm \frac{\underline{n}_3}{\sqrt{2}}$ 

Two additional pairs of solutions can be obtained by eliminating n, or no from to and folling similar steps. So that we have

Minimum shear stresses:

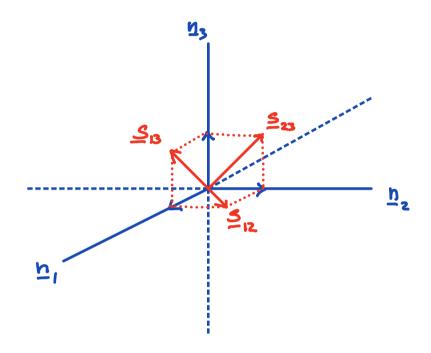
Maximum shear stresses:

$$T_{13} = \frac{1}{2} (\delta_{2} - \delta_{3}) \quad \text{on} \quad \underline{S}_{23} = \frac{1}{\sqrt{2}} (\pm \underline{n}_{2} \pm \underline{n}_{3})$$

$$T_{13} = \frac{1}{2} (\delta_{1} - \delta_{3}) \quad \text{on} \quad \underline{S}_{13} = \frac{1}{\sqrt{2}} (\pm \underline{n}_{1} \pm \underline{n}_{3})$$

$$T_{12} = \frac{1}{2} (\delta_{1} - \delta_{2}) \quad \text{on} \quad \underline{S}_{12} = \frac{1}{\sqrt{2}} (\pm \underline{n}_{1} \pm \underline{n}_{2})$$

where we assume of 2 of 2 of



Note: G&S do this with Lagrange multipliers but it leads to odd expressions in judex notation, such as

where 'i'seems to be a dummy on the 1.h.s. but en free index on the ohs.

>> we did it the pedestrian way.