## Leeture 9: Principal Stresses

Logistics: - HW3 due Th

- HW2 only 4 submissions?

For this class HUs are essential

Last time: Change of basis {e} and {e}?

Representation [v] vs. [v]

[s] vs. [s]

Change in basis tensor: A Aij = e; · e;

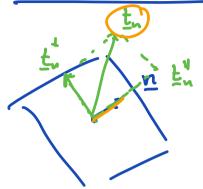
[v] = [A] [v]' [s] = [A] [s] [A]

[v]' = [A] [v] [s]' = [A] [s] [A]

 $I_{1} = \underline{\operatorname{tr}}(\underline{s}) = \lambda_{1} + \lambda_{2} + \lambda_{3}$   $I_{2} = \frac{1}{2} \left( \operatorname{tr}(\underline{s})^{2} + \operatorname{tr}(\underline{s}^{2}) \right) = \lambda_{1} \lambda_{2} + \lambda_{2} \lambda_{3} + \lambda_{1} \lambda_{3}$   $I_{3} = \underline{\operatorname{det}}(\underline{s}) = \lambda_{1} \lambda_{2} \lambda_{3}$ 

Today: Principal stresses

## Nosmal & Shear Stosses



$$\overline{F}^{n} = \overline{\Phi}_{n}$$

$$\frac{1}{2} = \frac{1}{2} \cdot \frac{1}{2} = \frac{1}{2} \cdot \frac{1$$

normal oben: on = n. En = m. En = n; oj nj sher shen: T = m.En = m. En = m; oj nj

## Extremal Stress Values



- I) Max and Hin Normal Stresses
- Q: Given a = at x what are the n's corresponding to to max & uiu normal stress.
  - => Optimization problem for n with constraint that  $|\underline{N}| = 1$

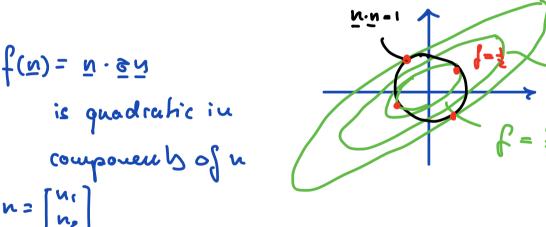
$$\mathcal{L}(\underline{n},\lambda) = \underline{n} \cdot \underline{\varepsilon}\underline{n} - \lambda (\underline{n} \cdot \underline{n} - 1)$$

$$\mathcal{L}(n_i, \lambda) = (n_i \hat{s}_i, n_i) + \lambda (n_i n_i - 1)$$

fuuehicu

coustaint

Lagrange mutiplier



Execuel values ar stationer y polats of L

$$\frac{33}{37} = N!N! - 1 = 0$$

$$\frac{2\mathcal{L}}{2n_k} = \delta_{ij} \frac{2n_k[(n_i \, n_j)]}{2n_k[(n_i \, n_j)]} \frac{2n_k[(n_i \, n_j)]}{2n_k[(n_i \, n_j)]}$$

$$\frac{9\mu^{K}}{5\mu!} = \mu^{i} = 8iK$$

$$\frac{3\mu^{K}}{5\mu!} = 1 \quad \frac{3\mu^{K}}{3\mu!} = 0$$

$$= \chi \left( z_{kj} n_{j} - \lambda n_{k} \right) = 0$$

In dyadie notation:  $(\underline{5} - \lambda \underline{I}) \underline{n} = 0$ In la sange multiplies method leads to eigen value problem

What is eignificance of 
$$\lambda^2$$
  
 $\underline{n} \cdot (\underline{\delta} - \lambda \underline{I})\underline{n} = 0$   
 $\underline{n} \cdot \underline{\delta}\underline{n} - \lambda \underline{n} \cdot \underline{n} = 0$  =>  $\underline{\delta}_n = \lambda$ 

Solu eigenvalue problem to find extrement normal stresses

λ; 's principal normal slæsses ⇒ λ; = ≥; vi's principal directions of @

$$\underline{\underline{s}} = \underline{\sum}_{i=1}^{3} \underline{s}_{i} \underline{v}_{i} \underline{s}_{i}$$
eigen frame

$$[\underline{\underline{o}}] = \begin{bmatrix} 6, & \\ & 6, \end{bmatrix}$$

trachious

$$\overline{f}^{n'} = \overline{g} \, \overline{n}! = \overline{\underline{g}} \, \overline{g}! \, (\overline{n}! \, \underline{g} \, \overline{n}!) \, (\overline{\underline{g}}! \, \underline{n}!)$$

$$\overline{f}^{n'} = \overline{g}! \, \overline{n}! = \overline{\underline{g}}! \, (\overline{n}! \, \underline{g} \, \overline{n}!) \, (\overline{\underline{g}}! \, \underline{n}!) \, (\overline{\underline{g}}! \, \underline{n}!)$$

es no shear stren

II) Max & min shear stresses.

look in {n;} principal shesse

= = s; n; 
s; = s.v.;

> =  $\sum_{i=1}^{3} c_i s_i$  (u; &u;) h; =  $\sum_{i=1}^{3} c_i s_i$ ; (u; &u;) u;

Fs = 5 6; s; n;

ts = 6, 5, mx + 62 52 hz + 63 58 h3

mag. of narmal & shear shear  $t_u = \underline{S} \cdot \underline{t}_{\underline{S}} = \underline{S}_{1} \cdot \underline{S}_{2}^{2} + \underline{S}_{2} \cdot \underline{S}_{3}^{2} + \underline{S}_{3} \cdot \underline{S}_{3}^{2}$   $\underline{t}^{2} = |\underline{t}_{\underline{S}}|^{2} - \underline{S}_{\underline{u}}^{2} - \underline{S}_{\underline{u}^{2}}^{2} - \underline{S}_{\underline{u}}^{2} - \underline{S}_{\underline{u}}^{2} - \underline{S}_{\underline{u}}^{2}$ 

T2 = 6,25, + 6,25, +6,25, - (6,5, +6,5,2+6,5,2)2

lu judex notation

$$T^2 = \sum_{i=1}^{3} \delta_i^2 s_i^2 - \left(\frac{3}{2} \delta_i s_i^2\right)^2$$

function we are optimizing under the constraint  $|S|^2 = 1$   $|S_1^2 + S_2^2 + |S_3^2 = 1$  solu using direct substitution

I) Eliminate  $S_3^2 = 1 - S_1^2 - S_2^2 \Rightarrow T^2 = T(S_1, S_2)$   $T^2 = G_1^2 S_1^2 + G_2^2 S_2^2 + G_3^2 S_3^2 - (G_1 S_1^2 + G_2 S_2^2 + G_3^2 S_3^2)^2$   $= G_1^2 S_1^2 + G_2^2 S_2^2 + G_3^2 (1 - S_1^2 - S_2^2) - (G_1 S_1^2 + G_2 S_2^2 + G_3^2 (1 - S_1^2 - S_2^2))^2$   $\Rightarrow \text{ constant is the cosposation}$ 

We just need to find  $\frac{\partial T^2}{\partial S_1} = \frac{\partial T^2}{\partial S_2} = 0$   $\frac{\partial T^2}{\partial S_1} = 2 \cdot S_1 \cdot (S_1 - S_2) \cdot (S_1 - S_3 - 2[(S_1 - S_3) \cdot S_1^2 + (S_2 - S_3) \cdot S_2^2]) = 0$  $\frac{\partial T^2}{\partial S_2} = 2 \cdot S_2 \cdot (S_2 - S_3) \cdot (S_2 - S_3 - 2[(S_1 - S_3) \cdot S_1^2 + (S_2 - S_3) \cdot S_2^2]) = 0$ 

First solution: 
$$s_1 = s_2 = 0 \Rightarrow s_3 = 1 \quad \underline{s} = \frac{1}{2} n_3$$

$$\overline{c}^2 = \dot{s}_3^2 \cdot 1 - (s_3 \cdot 1)^2 = 0$$

$$\Rightarrow \text{ minimum shear shear in principal plane}$$

Second soluition: s, =0

$$\frac{\partial C^{2}}{\partial S_{2}} = \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} = \frac{\partial C}{\partial S_{2}} = \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} = \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} = \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} = \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} = \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} = \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} = \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} - \frac{\partial C}{\partial S_{2}} - \frac{\partial C}$$

$$\tau^2 = \frac{\sigma_2^2}{2} + \frac{\sigma_3^2}{2} - \left(\frac{\sigma_2}{2} + \frac{\sigma_3}{2}\right)^2$$

Two solution

min 
$$T = 0$$
  $\underline{S} = \pm \underline{N}_3$   
may  $C = \frac{1}{2}(c_2 - c_3)$   $\underline{S} = \pm \frac{\underline{N}_2}{\sqrt{2}} \pm \frac{\underline{N}_3}{\sqrt{2}}$ 

## Playing this two more times

Min. shear slærren

Max shear stresser

$$T_{23} = \frac{1}{2} (\epsilon_2 - \epsilon_3) \quad \text{ou} \quad \underline{S}_{22} = \frac{1}{\sqrt{2}} (\pm \underline{N}_2 \pm \underline{N}_3)$$

$$T_{13} = \frac{1}{2} (\epsilon_1 - \epsilon_3) \quad \text{ou} \quad \underline{S}_{13} = \frac{1}{\sqrt{2}} (\pm \underline{N}_1 \pm \underline{N}_3)$$

$$T_{13} = \frac{1}{2} (\epsilon_1 - \epsilon_2) \quad \text{ou} \quad \underline{S}_{13} = \frac{1}{\sqrt{2}} (\pm \underline{N}_1 \pm \underline{N}_2)$$