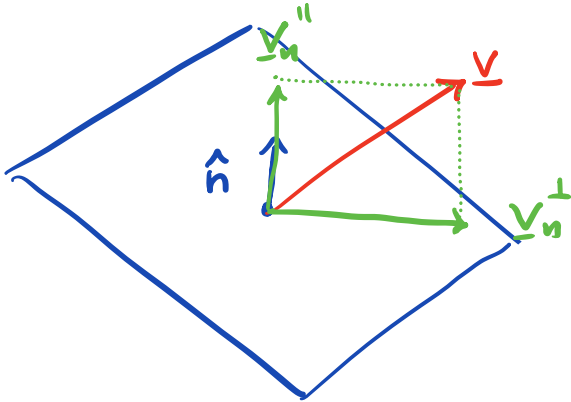


Projection & Reflection tensors

commonly used to partition forces on a surface.



$$\underline{v} = \underline{v}_n^{\parallel} + \underline{v}_n^{\perp}$$

Use dot product:

$$\underline{v}_n^{\parallel} = (\underline{v} \cdot \underline{\hat{n}}) \underline{\hat{n}}$$

$$\underline{v}_n^{\perp} = \underline{v} - \underline{v}_n^{\parallel}$$

Tensors ?

$$\underline{v}_n^{\parallel} = \underline{P}_n^{\parallel} \underline{v}$$
$$\underline{v}_n^{\perp} = \underline{P}_n^{\perp} \underline{v}$$

use dyadic property !

$$\underline{v}_n^{\parallel} = (\underline{v} \cdot \underline{\hat{n}}) \underline{\hat{n}} = (\underline{\hat{n}} \otimes \underline{\hat{n}}) \underline{v} = \underline{P}_n^{\parallel} \underline{v}$$

$$\underline{v}_n^{\perp} = \underline{v} - (\underline{\hat{n}} \otimes \underline{\hat{n}}) \underline{v} = (\underline{I} - \underline{\hat{n}} \otimes \underline{\hat{n}}) \underline{v} = \underline{P}_n^{\perp} \underline{v}$$

Projection tensors:

$$\underline{P}_n^{\parallel} = \underline{\hat{n}} \otimes \underline{\hat{n}}$$

$$\underline{P}_n^{\perp} = \underline{I} - \underline{\hat{n}} \otimes \underline{\hat{n}}$$

Properties:

$$\underline{\underline{P}} = \underline{\underline{P}}^T$$

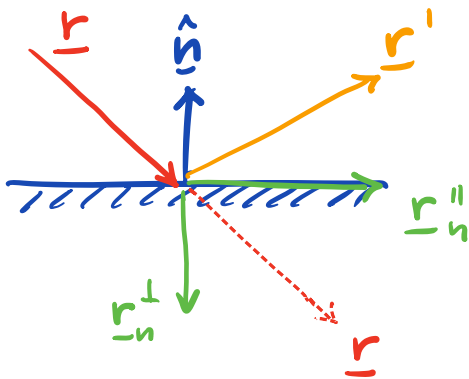
symmetric (HW2)

$$\underline{\underline{P}}^2 = \underline{\underline{P}}$$

$$\underline{\underline{P}}'' + \underline{\underline{P}}^\perp = \underline{\underline{I}}$$

$$\underline{\underline{P}}'' \underline{\underline{P}}^\perp = \underline{\underline{0}}$$

Reflections



incoming: $\underline{r} = \underline{r}_n'' + \underline{r}_n^\perp$

reflected: $\underline{r}' = \underline{r}_n'' - \underline{r}_n^\perp$

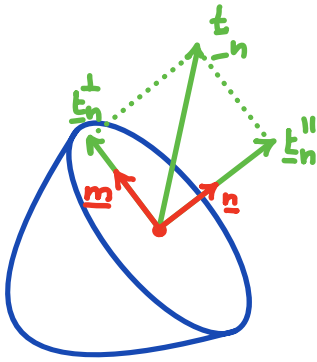
$$\underline{r}' = (\underline{P}_n'' - \underline{P}_n^\perp) \underline{r}$$

$$\underline{r}' = (\underline{I} - 2 \underline{\hat{n}} \otimes \underline{\hat{n}}) \underline{r}$$

$$\underline{r}' = \underline{R}_n \underline{r}$$

Reflection tensor: $\underline{R}_n = \underline{I} - 2 \underline{\hat{n}} \otimes \underline{\hat{n}}$

Normal and Shear Stresses



Consider an arbitrary surface in B

projection matrices:

$$\underline{\underline{P}}'' = \underline{n} \otimes \underline{n}$$

$$\underline{\underline{P}}^\perp = \underline{\underline{I}} - \underline{n} \otimes \underline{n} = \underline{m} \otimes \underline{m}$$

$$\text{normal stress: } \underline{t}_n'' = \underline{\underline{P}}'' \underline{t}_n = (\underline{n} \cdot \underline{t}_n) \underline{n} = \sigma_n \underline{n}$$

$$\text{shear stress: } \underline{t}_n^\perp = \underline{\underline{P}}^\perp \underline{t}_n = (\underline{m} \cdot \underline{t}_n) \underline{m} = \tau \underline{m}$$

The magnitudes of these stresses are:

$$\sigma_n = \underline{n} \cdot \underline{t}_n = \underline{n} \cdot \underline{\sigma} \underline{n} \quad \text{or} \quad \sigma_n = n_i \sigma_{ij} n_j$$

$$\tau = \underline{m} \cdot \underline{t}_n = \underline{m} \cdot \underline{\sigma} \underline{n} \quad \text{or} \quad \tau = m_i \sigma_{ij} n_j$$

normal stresses:

$$\sigma_n > 0 \quad \underline{\text{tensile}}$$

$$\sigma_n < 0 \quad \underline{\text{compressive}}$$

$$\text{From geometry: } \underline{t}_n = \underline{t}_n'' + \underline{t}_n^\perp$$

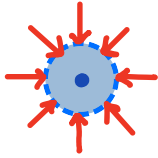
$$|\underline{t}_n|^2 = |\sigma_n \underline{n}|^2 + |\tau \underline{m}|^2 = \sigma_n^2 + \tau^2$$

Simple states of stress

I) Hydrostatic stress

$$\underline{\underline{\sigma}} = -p \underline{\underline{I}} = \begin{bmatrix} -p & 0 & 0 \\ 0 & -p & 0 \\ 0 & 0 & -p \end{bmatrix}$$

$$\Rightarrow \underline{t}_n = \underline{\underline{\sigma}} \underline{n} = -p \underline{n} \quad \text{for all } \underline{n}$$



$$\underline{t}_n'' = \underline{P}_n'' \underline{t} = (\underline{n} \otimes \underline{n})(-p \underline{n}) = -p (\underline{n} \cdot \underline{n}) \underline{n} = -p \underline{n}$$

$$\Rightarrow \underline{t}_n = \underline{t}_n'' \quad \underline{t}_n^\perp = 0$$

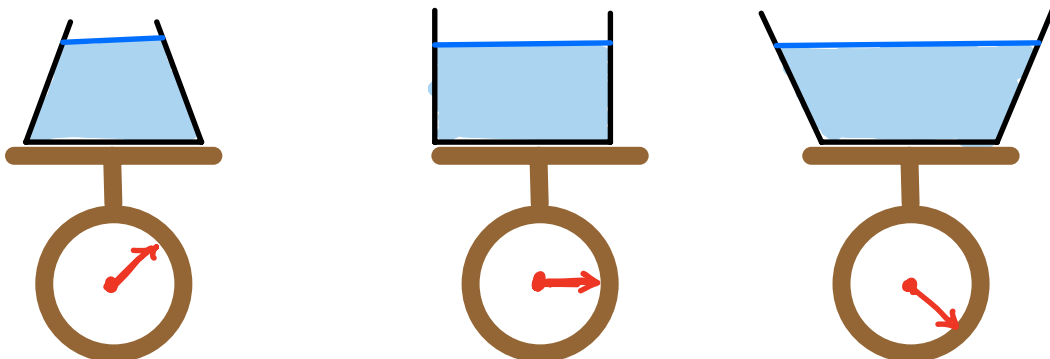
$$\left. \begin{array}{l} \text{normal stress: } \sigma_n = -p \\ \text{shear stress: } \tau = 0 \end{array} \right\} \text{ on all planes}$$

Pascal's law:

The pressure in a fluid at rest is independent of the direction of a surface. Pressure is a scalar!

Hydrostatic paradox: (Blaise Pascal)

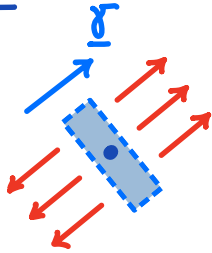
Weight different but the force on base is same $f = pA$



II) Uniaxial stress

$$\underline{\underline{\sigma}} = \sigma \underline{\underline{\gamma}} \otimes \underline{\underline{\gamma}}$$

($\underline{\gamma}$ is unit vector)



$$\Rightarrow \underline{t}_n = \underline{\underline{\sigma}} \underline{n} = \sigma (\underline{\gamma} \cdot \underline{n}) \underline{\gamma}$$

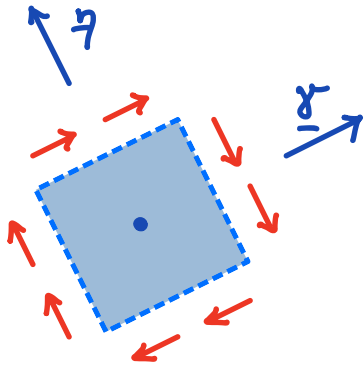
Traction is always parallel to $\underline{\gamma}$ and vanished on surfaces with normal perpendicular to $\underline{\gamma}$.

$\sigma > 0$: pure tension

$\sigma < 0$: pure compression

III, Pure shear stress $\underline{\gamma} \cdot \underline{\eta} = 0$

$$\underline{\underline{\underline{\sigma}}} = \tau (\underline{\underline{\underline{\gamma}}} \otimes \underline{\underline{\underline{\eta}}} + \underline{\underline{\underline{\eta}}} \otimes \underline{\underline{\underline{\gamma}}}) \Rightarrow \underline{\underline{\underline{\tau}}}_n = \underline{\underline{\underline{\sigma}}} \underline{\underline{\underline{n}}} = \tau (\underline{\underline{\underline{\eta}}} \cdot \underline{\underline{\underline{n}}}) \underline{\underline{\underline{\gamma}}} + \tau (\underline{\underline{\underline{\gamma}}} \cdot \underline{\underline{\underline{n}}}) \underline{\underline{\underline{\eta}}}$$



$$\underline{\underline{\underline{n}}} = \underline{\underline{\underline{\eta}}}: \underline{\underline{\underline{\tau}}}_n = \tau \underline{\underline{\underline{\gamma}}}$$

$$\underline{\underline{\underline{n}}} = \underline{\underline{\underline{\gamma}}}: \underline{\underline{\underline{\tau}}}_n = \tau \underline{\underline{\underline{\eta}}}$$

IV, Plane stress

If there exists a pair of orthogonal vectors $\underline{\underline{\gamma}}$ and $\underline{\underline{\eta}}$ such that the matrix representation of $\underline{\underline{\underline{\sigma}}}$ in the frame $\{\underline{\underline{\gamma}}, \underline{\underline{\eta}}, \underline{\underline{\gamma}} \times \underline{\underline{\eta}}\}$ is of the form

$$[\underline{\underline{\underline{\sigma}}}] = \begin{pmatrix} \sigma_{11} & \sigma_{12} & 0 \\ \sigma_{21} & \sigma_{22} & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

then a state of plane stress exists.

Q: Is uniaxial stress a plane stress?

$$\underline{\underline{\sigma}} = \sigma \underline{a} \otimes \underline{a}$$

Pick a frame $\{\underline{e}_i\}$ and evaluate $[\underline{\underline{\sigma}}]$.

What frame $\underline{e}_1 = \underline{a}$ know $\underline{e}_2 \cdot \underline{a} = \underline{e}_3 \cdot \underline{a} = 0$

$$\sigma_{ij} = \underline{e}_i \cdot \underline{\underline{\sigma}} \underline{e}_j$$

substitute with $\underline{a} = \underline{e}_1$

$$\begin{aligned}\sigma_{ij} &= \underline{e}_i \cdot (\sigma \underline{e}_1 \otimes \underline{e}_1) \underline{e}_j \\ &= \sigma \underline{e}_i \cdot (\underline{e}_1 \otimes \underline{e}_1) \underline{e}_j = \sigma (\underline{e}_i \cdot \underline{e}_1) (\underline{e}_j \cdot \underline{e}_1)\end{aligned}$$

$$\sigma_{11} = \sigma (\underline{e}_1 \cdot \underline{e}_1) (\underline{e}_1 \cdot \underline{e}_1) = \sigma$$

$$\sigma_{12} = \sigma (\underline{e}_1 \cdot \underline{e}_1) (\underline{e}_1 \cdot \underline{e}_2) = 0$$

$$\sigma_{22} = \sigma (\underline{e}_2 \cdot \underline{e}_1) (\underline{e}_1 \cdot \underline{e}_2) = 0$$

...

$$\Rightarrow [\underline{\underline{\sigma}}] = \begin{bmatrix} \sigma & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} \quad \checkmark \text{ plane stress}$$

Spherical and deviatoric stress tensors

The Cauchy stress tensor can be decomposed

as
$$\underline{\underline{\sigma}} = \underline{\underline{\sigma}}_S + \underline{\underline{\sigma}}_D$$

spherical stress tensor:
$$\underline{\underline{\sigma}}_S = -p \underline{\underline{I}} \quad p = -\frac{1}{3} \text{tr}(\underline{\underline{\sigma}})$$

deviatoric stress tensor:
$$\underline{\underline{\sigma}}_D = \underline{\underline{\sigma}} + p \underline{\underline{I}}$$

The pressure
$$p = -\frac{1}{3} \text{tr}(\underline{\underline{\sigma}}) = -\frac{1}{3} (\sigma_1 + \sigma_2 + \sigma_3)$$

can be interpreted as the mean normal

traction. The spherical stress is the part

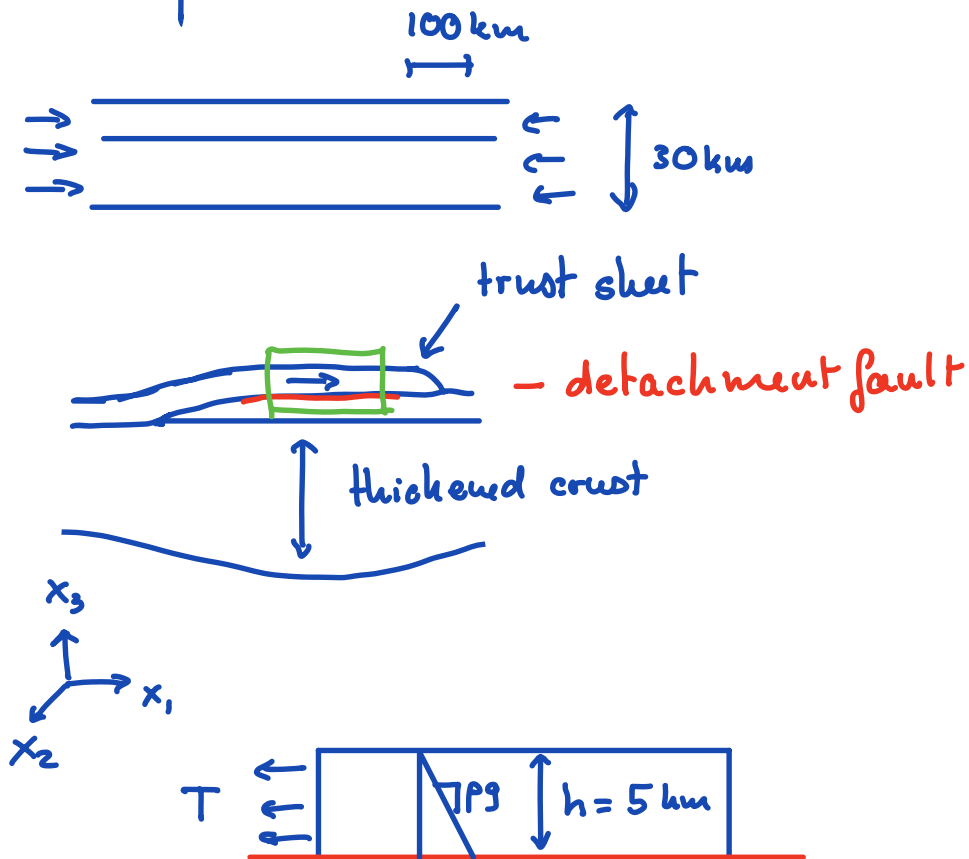
of $\underline{\underline{\sigma}}$ that changes the volume of the body.

Note that $p > 0$ corresponds to compression.

The deviatoric stress is the part of $\underline{\underline{\sigma}}$ that changes the shape of a body without changing its volume. By definition $\text{tr} \underline{\underline{\sigma}}_D = 0$.

$$\Gamma_3[\partial B] = - \int_B \rho g \underline{e}_3 dV = - \rho g \underline{e}_3 \underbrace{\int_B dV}_{V_B} = - \rho g V_B \underline{e}_3 \quad \checkmark$$

Example: Fault block on detachment



Normal stresses:

Vertical stress: $\sigma_{33} = \rho g h$

Horizontal stress (x_1 -dir): $\sigma_{11} = \kappa \sigma_{33} - T$

Horizontal stress (x_2 -dir): $\sigma_{22} = \kappa \sigma_{33}$

In fluid $\kappa=1$, but in rock $\kappa < 1$ due to finite strength.

T is tensile tectonic stress

Assume only shear stress is in 1-3 coord. plane

$$\sigma_{13} = \sigma_{31} = \mu (pgh) \quad \mu = \text{coefficient of friction}$$

$$\sigma_{21} = \sigma_{12} = 0 \quad \sigma_{23} = \sigma_{32} = 0$$

This results in following stress tensor:

$$\underline{\underline{\sigma}} = \begin{bmatrix} \kappa pgh - T & 0 & \mu pgh \\ 0 & \kappa pgh & 0 \\ \mu pgh & 0 & pgh \end{bmatrix}$$

Traction on basal plane:

$$\underline{t}(\underline{e}_3) = \underline{\underline{\sigma}} \underline{e}_3 = \begin{bmatrix} \mu \\ 0 \\ 1 \end{bmatrix} pgh$$

Normal stress on fault: $\underline{t}(\underline{e}_3) \cdot \underline{e}_3 = pgh$

Shear stress on fault: $\underline{t}(\underline{e}_3) \cdot \underline{e}_1 = \mu pgh$

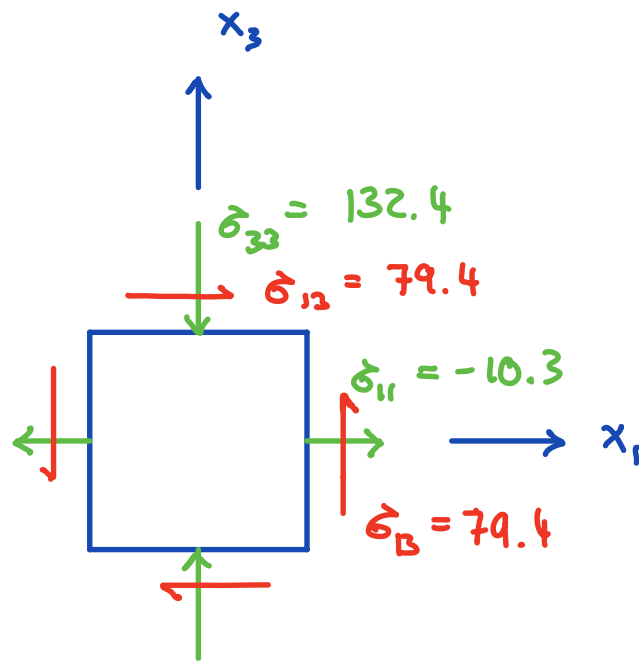
Assume following numbers:

$$\rho = 2700 \text{ kg/m}^3 \quad h = 5000 \text{ m}$$

$$g = 9.8 \text{ m/s}^2 \quad T = 50 \text{ MPa}$$

$$\kappa = 0.3 \quad \mu = 0.6$$

$$\Rightarrow \underline{\underline{\sigma}} = \begin{bmatrix} -10.3 & 0 & 79.4 \\ 0 & 39.7 & 0 \\ 79.4 & 0 & 132.4 \end{bmatrix} \text{ MPa}$$



$$\underline{t}_1 = \underline{t}(\underline{e}_1) = \underline{\underline{\sigma}} \underline{e}_1 = \begin{bmatrix} -10.3 \\ 0 \\ 79.4 \end{bmatrix} \text{ MPa}$$

$$\underline{t}_3 = \underline{t}(\underline{e}_3) = \underline{\underline{\sigma}} \underline{e}_3 = \begin{bmatrix} 79.4 \\ 0 \\ 132.4 \end{bmatrix} \text{ MPa}$$

Normal & shear stress on x_1 -coord. plane:

$$\underline{t}_1'' = \underline{P}_1'' \underline{t}_1 = (\underline{e}_1 \otimes \underline{e}_1) \underline{t}_1 = \underbrace{(\underline{e}_1 \cdot \underline{t}_1)}_{\sigma_n = \sigma_{11}} \underline{e}_1 = \begin{bmatrix} -10.3 \\ 0 \\ 0 \end{bmatrix} \text{ MPa}$$

$$\underline{t}_1^\perp = \underline{P}_1^\perp \underline{t}_1 = \underline{t}_1 - \underline{t}_1'' = \begin{bmatrix} -10.3 \\ 0 \\ 79.4 \end{bmatrix} - \begin{bmatrix} -10.3 \\ 0 \\ 0 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 79.4 \end{bmatrix} \text{ MPa}$$

Normal & shear stress on x_3 -coordinate plane:

$$\underline{t}_3'' = \underline{P}_3'' \underline{t}_3 = (\underline{e}_3 \otimes \underline{e}_3) \underline{t}_3 = (\underline{t}_3 \cdot \underline{e}_3) \underline{e}_3 = \begin{bmatrix} 0 \\ 0 \\ 132.4 \end{bmatrix} \text{ MPa}$$

$$\underline{t}_3^\perp = \underline{t}_3 - \underline{t}_3'' = \begin{bmatrix} 79.4 \\ 0 \\ 132.4 \end{bmatrix} - \begin{bmatrix} 0 \\ 0 \\ 132.4 \end{bmatrix} = \begin{bmatrix} 79.4 \\ 0 \\ 0 \end{bmatrix} \text{ MPa}$$