

# Bézier $\bar{B}$ projection

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We demonstrate the use of Bézier projection to alleviate locking phenomena in structural mechanics applications of isogeometric analysis. Based on two different interpretations of well-known  $\bar{B}$  projection, we propose two formulations and name them (symmetric/non-symmetric) Bézier  $\bar{B}$  projection. To demonstrate the utility of the Bézier projection approach for both geometry and material locking phenomena we focus on transverse shear locking in Timoshenko beams and volumetric locking in nearly incompressible linear elasticity although the approach can be applied generally to other types of locking phenomena as well. Bézier projection is a local projection technique with optimal approximation properties, which in many cases produces solutions that are comparable to global  $L^2$  projection. In the context of  $\bar{B}$  methods, the use of Bézier projection produces sparse stiffness matrices with only a slight increase in bandwidth when compared to standard displacement-based methods. Of particular importance is that the approach is applicable to any spline representation that can be written in Bézier form like NURBS, T-splines, LR-splines, etc. We discuss in detail how to integrate this approach into an existing finite element framework with minimal disruption through the use of Bézier extraction operators and a newly introduced dual basis for Bézier projection extraction operator. We then demonstrate the behavior of the two proposed formulations approach through several challenging benchmark problems.

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**Keywords:** Isogeometric analysis, Bézier extraction, Bézier dual basis, Bézier projection,  $\bar{B}$ -projection, locking

## 1 Introduction

Isogeometric analysis (IGA), introduced by Hughes et al. [1], adopts the spline basis, which underlies the CAD geometry, as the basis for analysis. Of particular importance is the positive impact of smoothness on numerical solutions, where, in many application domains, IGA outperforms classical finite elements [2–7]. Initial investigations of IGA focused on non-uniform rational B-splines (NURBS) due to their dominance in commercial CAD packages. However, many advances are being made in analysis-suitable geometry representations that overcome the strict rectangular topological restrictions of NURBS. Examples include T-splines [6, 8] and their analysis-suitable restriction [9, 10], hierarchical B-splines [11–15], and locally refined B-splines [16, 17] among others.

The purpose of this paper is to demonstrate how Bézier projection [18] can be employed as the underlying local projection framework for a  $\bar{B}$  approach to treat locking in isogeometric structural elements. Bézier projection is an element-based local projection methodology for B-splines, NURBS, and T-splines. It relies on the concept of Bézier extraction [19, 20] and an associated operation, spline reconstruction, which enables the use of Bézier projection in standard finite element codes.

Bézier projection exhibits provably optimal convergence and yields projections that are virtually indistinguishable from global  $L^2$  projection. For an isogeometric finite element code that leverages Bézier extraction, Bézier projection can be employed virtually for free. To simplify the implementation of the Bézier  $\bar{B}$  method in existing finite element codes we develop a *dual* element Bézier extraction operator that can be derived directly from the Bézier extraction of a spline representation. It is worth noting that Bézier projection can also be used to develop a unified framework for spline operations including cell subdivision and merging, degree elevation and reduction, basis roughening and smoothing, and spline reparameterization and is applicable to any spline representation that can be written in Bézier form.

Numerical locking in structural finite elements includes geometric locking in thin curved structural members such as membrane and shear locking and also includes volumetric locking in incompressible and nearly incompressible elasticity. There is an immense body of literature on approaches to overcome locking in the finite element community and various approaches have emerged as dominant. These include reduced quadrature [21, 22],

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$\bar{B}$  projection methods [23, 24], and mixed methods based on the Hu-Washizu variational principle [25–27]. It is important to mention that, although ameliorated at high polynomial degrees, smooth splines in the context of IGA still exhibit locking behavior [28, 29].

In IGA, there is a growing literature on the treatment of locking in structural elements. Leveraging higher-order smoothness, transverse shear locking can be eliminated at the theoretical level by employing Kirchhoff-Love [30, 31] and hierarchic Reissner-Mindlin [32–34] shell elements. Reduced quadrature schemes have been explored in [35–37] as a way to alleviate transverse shear locking. The extension of  $\bar{B}$  projection to the isogeometric setting was initiated in [38] for both elastic and plastic problems and was extended in [39] to include local projection techniques [40, 41].

The outline of this paper is as follows: First, we briefly review spline basis functions in Section 2. In Sections 3 and 4, we describe Bézier extraction and projection. We then formulate and use Bézier  $\bar{B}$  projection for the Timoshenko beam (to treat transverse shear locking) and nearly incompressible elasticity (to treat volumetric locking) in Sections 5 and 6, respectively. We provide detailed element level operations in both settings. We also presents numerical tests to show the performance of the proposed strategy.

## 2 Preliminaries and notation

In this section a brief overview of univariate Bernstein, B-spline, and NURBS basis functions is provided. We also describe how these univariate basis functions are extended to higher dimensions.

### 2.1 Univariate Bernstein basis functions

The  $i$ th univariate Bernstein basis function of degree  $p$  is defined by

$$B_{i,p}(\xi) = \binom{p}{i} \xi^i (1 - \xi)^{p-i} \quad (1)$$

where  $\xi \in [0, 1]$  and  $\binom{p}{i} = \frac{p!}{i!(p-i)!}$ ,  $0 \leq i \leq p$ , is a binomial coefficient.

### 2.2 Univariate spline basis functions

A univariate B-spline basis of dimension  $n$  is defined by a polynomial degree  $p$  and a knot vector  $\Xi = \{\xi_0, \xi_1, \dots, \xi_{n+p}\}$ , which is a non-decreasing sequence of real numbers. The  $A$ th B-spline basis function can then be defined using the Cox-de Boor recursion formula:

$$N_{A,0}(\xi) = \begin{cases} 1 & \xi_A \leq \xi \leq \xi_{A+1} \\ 0 & \text{otherwise} \end{cases} \quad (2)$$

$$N_{A,p}(\xi) = \frac{\xi - \xi_A}{\xi_{A+p} - \xi_A} N_{A,p-1}(\xi) + \frac{\xi_{A+p+1} - \xi}{\xi_{A+p+1} - \xi_{A+1}} N_{A+1,p-1}(\xi). \quad (3)$$

For simplicity, we will always use open knot vectors defined over the interval  $[0, 1]$ . An open knot vector satisfies the conditions  $\xi_0 = \xi_1 = \dots = \xi_p = 0$  and  $\xi_n = \xi_{n+1} = \dots = \xi_{n+p} = 1$  and creates interpolatory end conditions. B-spline basis functions can be used to represent piecewise polynomial functions but are not capable of representing conic sections (e.g. circles, ellipses and hyperbolas). NURBS overcome this shortcoming. A NURBS basis function can be written as

$$R_{A,p}(\xi) = \frac{N_{A,p}(\xi)w_A}{W(\xi)} \quad (4)$$

where  $w_A$  is called a weight and

$$W(\xi) = \sum_A N_{A,p}(\xi)w_A \quad (5)$$

is called the weight function. A  $d$ -dimensional rational curve  $\mathbf{S}(\xi) \in \mathbb{R}^d$  can then be defined as

$$\mathbf{S}(\xi) = \sum_A R_{A,p}(\xi)\mathbf{P}_A \quad (6)$$

where  $\mathbf{P}_A = (p_A^1, p_A^2, \dots, p_A^d)^T$  is a  $d$ -dimensional control point. It is often more convenient to represent the  $d$ -dimensional NURBS in a  $(d+1)$ -dimensional homogeneous space by defining  $\mathbf{P}_A^w = (p_A^1 w_A, p_A^2 w_A, \dots, p_A^d w_A, w_A)^T$

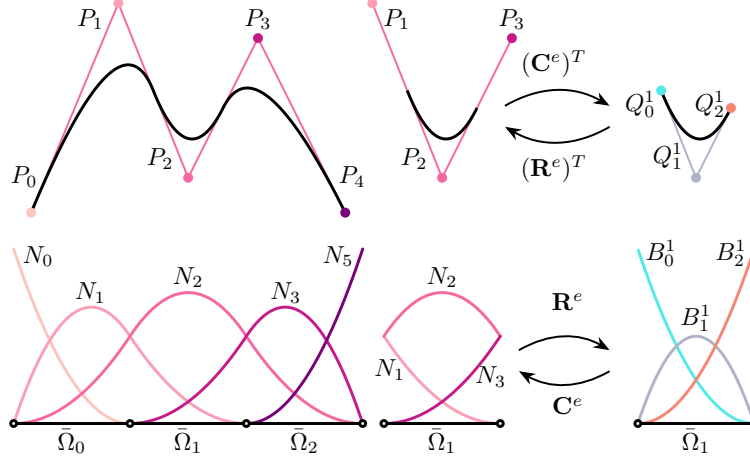


Figure 1: Illustration of Bézier extraction and projection in one dimension for a B-spline of degree 2 and knot vector  $[0, 0, 0, 1/3, 2/3, 1, 1, 1]$  (restricted to the second element for illustrative purposes).

and the corresponding  $(d + 1)$ -dimensional B-spline curve as

$$\mathbf{S}^w(\xi) = \sum_A N_{A,p}(\xi) \mathbf{P}_A^w \quad (7)$$

such that each component of  $\mathbf{S}^w$  can be written as

$$S_i(\xi) = \frac{S_i^w(\xi)}{S_{d+1}^w(\xi)}. \quad (8)$$

In the homogeneous form, NURBS can be manipulated with standard B-spline algorithms.

### 2.3 Multivariate spline basis functions

In higher dimensions, Bernstein, B-spline, and NURBS basis functions are formed by the Kronecker product of univariate basis functions. For example, two-dimensional B-spline basis functions of degree  $\mathbf{p} = (p_\xi, p_\eta)$  are defined by

$$\mathbf{N}^{\mathbf{p}}(\xi, \eta) = \mathbf{N}^{p_\xi}(\xi) \otimes \mathbf{N}^{p_\eta}(\eta) \quad (9)$$

where  $\mathbf{N}^{p_\xi}(\xi)$  and  $\mathbf{N}^{p_\eta}(\eta)$  are vectors of basis functions in the  $\xi$  and  $\eta$  directions, respectively. A particular multivariate basis function can be written as

$$N_{A(i,j)}^{\mathbf{p}}(\xi, \eta) = N_{i,p_\xi}(\xi) N_{j,p_\eta}(\eta) \quad (10)$$

where the index mapping is defined as

$$A(i, j) = n_\eta i + j. \quad (11)$$

The integer  $n_\eta$  is the number of basis functions in  $\eta$  direction.

## 3 Bézier extraction

Given a spline basis  $\mathbf{N}$  there exists a Bernstein basis  $\mathbf{B}$  and a linear operator  $\mathbf{C}$  (see [19]) such that

$$\mathbf{N}(\xi) = \mathbf{C}\mathbf{B}(\xi). \quad (12)$$

The localization of  $\mathbf{C}$  to an element domain produces the element extraction operator  $\mathbf{C}^e$ . Given control points  $\mathbf{P}^e$ , the corresponding Bézier control points  $\mathbf{Q}^e$  can be computed directly as

$$\mathbf{Q}^e = (\mathbf{C}^e)^T \mathbf{P}^e. \quad (13)$$

A graphical depiction of Bézier extraction is shown in Figure 1.

## 4 Bézier projection

Bézier projection can be viewed as the inverse of extraction [18]. Bézier projection uses an element reconstruction operator  $\mathbf{R}^e \equiv (\mathbf{C}^e)^{-1}$  such that the global control point values, corresponding to those basis functions defined over the support of an element  $e$ , can be determined directly from Bézier control values as

$$\mathbf{P}^e = (\mathbf{R}^e)^T \mathbf{Q}^e \quad (14)$$

where  $\mathbf{Q}^e$  is any field in Bézier form. The action of the element reconstruction operator is depicted graphically in Figure 1. For example, given any function  $u \in L^2$ , we can compute  $\mathbf{Q}^e$  as

$$\mathbf{Q}^e = (\mathbf{G}^e)^{-1} \mathbf{F}^e \quad (15)$$

where  $\mathbf{G}^e$  is the Gramian matrix corresponding to the Bernstein basis with components

$$G_{ij}^e = \int_{\Omega^e} B_i^e B_j^e d\Omega = \langle B_i^e, B_j^e \rangle_{\Omega^e} \quad (16)$$

and

$$F_i^e = \int_{\Omega^e} B_i^e u d\Omega = \langle B_i^e, u \rangle_{\Omega^e}. \quad (17)$$

Note that efficiency gains can be had at the expense of accuracy by instead performing the integration in the parametric domain of the element [18].

The element-wise projection produces one control value for each element in the support of the function. These values must be combined in order to provide the final control value. A core component of the Bézier projection algorithm is the definition of an appropriate averaging operation. A weighted average of the values is computed using the weighting

$$\omega_a^e = \frac{\int_{\Omega^e} N_a^e d\Omega}{\int_{\Omega^A} N_{A(e,a)} d\Omega} \quad (18)$$

where  $\Omega^e$  corresponds to the physical domain of element  $e$ ,  $A(e, a)$  is a mapping from a local nodal index  $a$  defined over element  $e$  to a corresponding global node index  $A$ , and  $\Omega^A$  corresponds to the physical support of  $N_A$ . The final averaged global control point is then calculated as

$$P_A = \sum_{\Omega^e \in \Omega^A} \omega_{A(e,a)} P_{A(e,a)}. \quad (19)$$

Bézier projection onto NURBS functions can be defined in an analogous manner [18].

The individual steps comprising the Bézier projection algorithm are illustrated in Figure 3 where the curve defined by  $\mathbf{f}(t) = \left(\frac{t}{3}\right)^{3/2} \mathbf{e}_1 + \frac{1}{10} \sin(\pi t) \mathbf{e}_2$ ,  $t \in [0, 3]$  is projected onto the quadratic B-spline basis defined by the knot vector  $[0, 0, 0, 1/3, 2/3, 1, 1, 1]$ . For this example, the algorithm proceeds as follows:

- Step 1: The function  $\mathbf{f}$  is projected onto the Bernstein basis of each element. This results in a set of Bézier coefficients that define an approximation to  $\mathbf{f}$ . The Bézier coefficients are indicated in part (1) of Figure 3 by square markers that have been colored to match the corresponding element. Each Bézier segment is discontinuous.
- Step 2: The element reconstruction operator  $\mathbf{R}^e$  is used to convert the Bézier control points into spline control points associated with the basis function segments over each element. The new control points are marked with inverted triangles and again colored to indicate the element with which the control point is associated. The control points occur in clusters. The clusters of control points represent the contributions from multiple elements to a single spline basis function control point.
- Step 3: Each cluster of control points is averaged to obtain a single control point by weighting each point in the cluster according to the weighting given in (18). The resulting control points are shown as circles with the relative contribution from each element to each control point indicated by the colored fraction of the control point marker. e colors in Figures 2 and 3 are coordinated to illustrate where the averaging weights come from and their values.

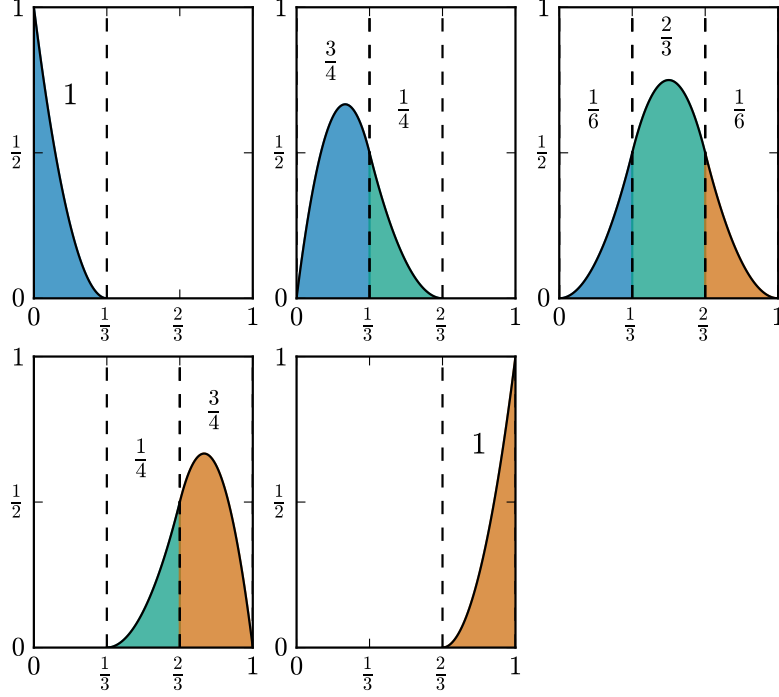


Figure 2: Weights over each knot span associated with the basis function defined by the knot vector  $[0, 0, 0, 1/3, 2/3, 1, 1, 1]$ .

#### 4.1 Dual basis formulation of Bézier projection

To integrate Bézier projection into a standard finite element assembly algorithm, it is convenient to recast Bézier projection in terms of a dual basis. [It plays a key role in developing the non-symmetric Bézier  \$\bar{B}\$  method.](#) A dual basis has the distinguishing property that

$$\int_{\Omega} \hat{N}_A N_B d\Omega = \delta_{AB}. \quad (20)$$

Once a dual basis is defined it can be processed in much the same manner as standard basis functions are processed in a finite element code. A complete exposition on the subject of dual bases and the Bézier projection framework can be found in [18]. We first define the dual element extraction operator

$$\hat{\mathbf{D}}^e = \text{diag}(\boldsymbol{\omega}^e) \mathbf{R}^e (\mathbf{G}^e)^{-1} \quad (21)$$

where  $\mathbf{G}^e$  is the Gramian matrix of the Bernstein basis functions over the element and  $\text{diag}(\boldsymbol{\omega}^e)$  is a diagonal matrix that contains the Bézier projection weights computed by (18). We can then define a dual basis function  $\hat{N}_{A(e,a)}$  restricted to element  $e$  as

$$\hat{N}_a^e = \sum_j \hat{D}_{aj}^e B_j. \quad (22)$$

The biorthogonality of the dual basis can be seen by noting that

$$\int_{\Omega^e} \hat{\mathbf{N}}^e (\mathbf{N}^e)^T d\Omega = \text{diag}(\boldsymbol{\omega}^e) \quad (23)$$

and

$$\mathbf{A}_e \left[ \int_{\Omega^e} \hat{\mathbf{N}}^e (\mathbf{N}^e)^T d\Omega \right] = \mathbf{I} \quad (24)$$

where  $\mathbf{A}$  is the standard finite element assembly operator [42].

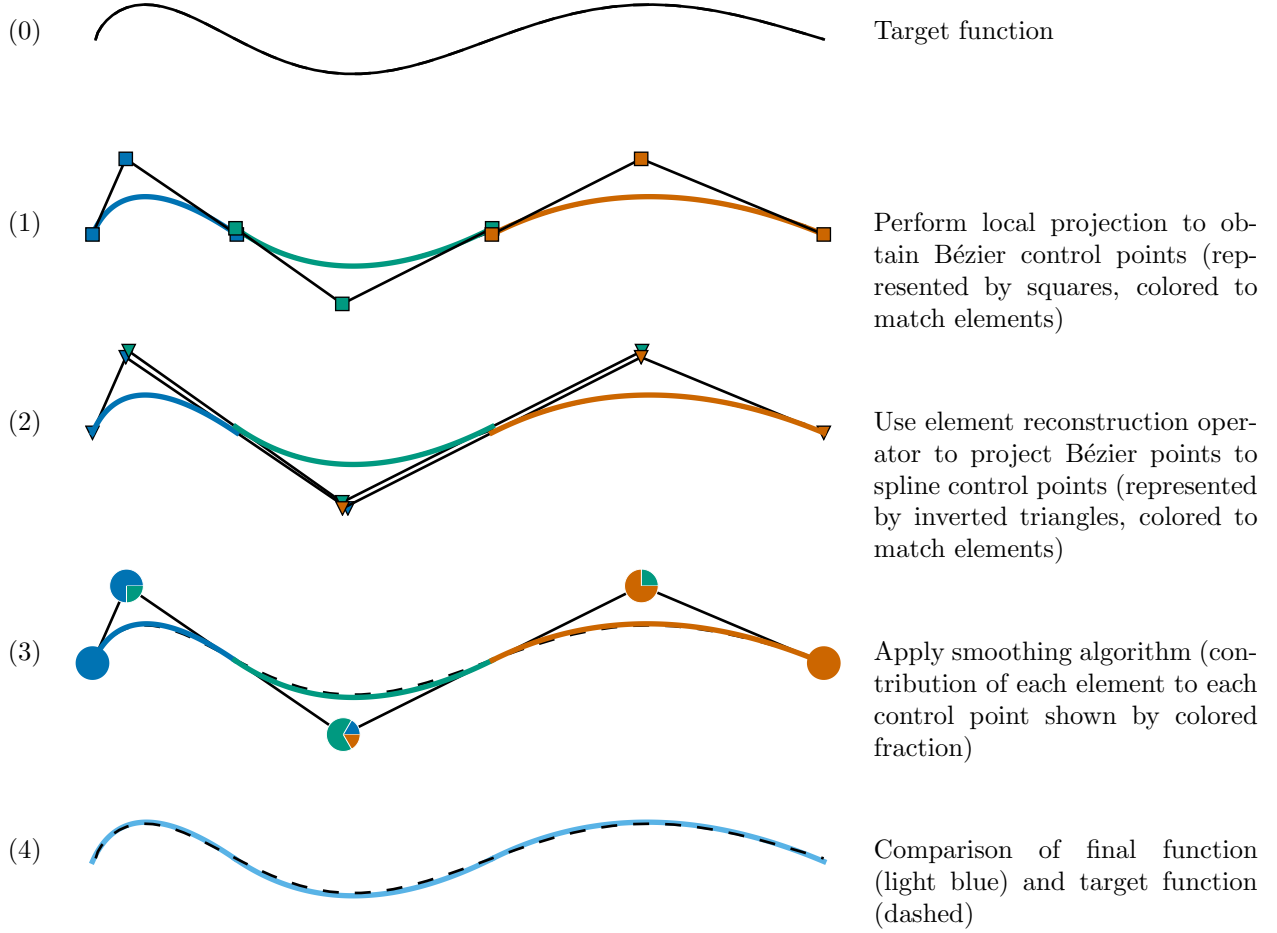


Figure 3: Steps of Bézier projection.

Now, given any function  $u \in L^2$  we can use the dual basis to find its representation in terms of the corresponding spline basis as

$$u = \sum_A P_A N_A \quad (25)$$

where

$$P_A = \int_{\Omega^A} \hat{N}_A u \, d\Omega = \langle \hat{N}_A, u \rangle_{\Omega^A}. \quad (26)$$

A set of dual basis functions corresponding to the quadratic maximally smooth B-spline basis shown in Figure 4a is shown in Figure 4c. Note that these dual functions have compact support and discontinuities which coincide with the underlying knots in the knot vector. The compact support of the dual basis functions will be crucial for maintaining the sparsity of the stiffness matrix for the Bézier  $\bar{B}$  formulations presented in this paper. For comparison, the dual basis corresponding to *global*  $L^2$  projection are shown in Figure 4b. Each of these dual basis functions has global support which explains why the use of global  $\bar{B}$  projections results in dense stiffness matrices.

#### 4.1.1 Rational dual basis functions

If rational basis functions are used, the construction of the dual basis must be modified slightly. A rational dual basis must satisfy the biorthogonality requirement

$$\int_{\Omega} \bar{R}_A R_B \, d\Omega = \delta_{AB}. \quad (27)$$

A simple way to achieve biorthogonality is to define

$$\bar{R}_A = W \bar{N}_A \quad (28)$$

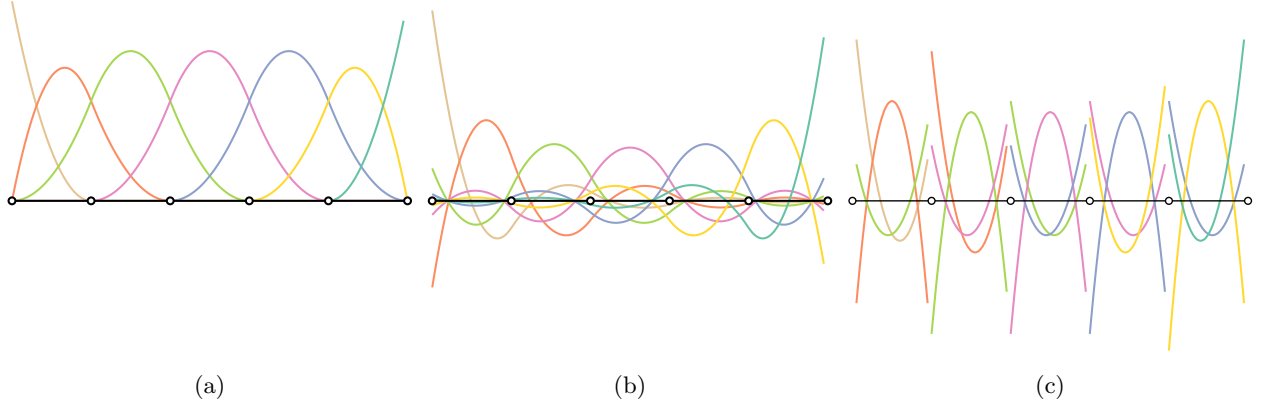


Figure 4: A set of quadratic B-spline basis functions (a), and corresponding dual basis functions computed by a global  $L^2$  projection (b) and the dual basis functions computed using Bézier projection (c). Note that the support of the dual basis functions in (b) is not compact. The dual basis functions shown in (c) have compact support but are not continuous.

where  $W$  is the rational weight given in (5). Now

$$\int_{\Omega} \bar{R}_A R_B d\Omega = \int_{\Omega} \bar{N}_A N_B d\Omega = \delta_{AB}. \quad (29)$$

## 5 Geometric locking: Timoshenko beams

To illustrate the use of Bézier  $\bar{B}$  projection to overcome geometric locking effects we study transverse shear locking in Timoshenko beams. The Timoshenko beam problem provides a simple one dimensional setting in which to describe Bézier  $\bar{B}$  projection. Note however, that the approach can be directly generalized to more complex settings like spatial beams and shells and other geometric locking mechanisms like membrane locking. We consider a planar cantilevered Timoshenko beam as shown in Figure 5. The strong form for this problem can be stated as

$$\left. \begin{aligned} -sGA\gamma' &= f(x) \\ -EI\kappa' - sGA\gamma &= 0 \\ \kappa &= \phi' \\ \gamma &= \omega' - \phi \end{aligned} \right\} \text{ in } \Omega \quad (30)$$

$$\left. \begin{aligned} \omega &= 0 \\ \phi &= 0 \end{aligned} \right\} \text{ at } x = 0 \quad (31)$$

$$\left. \begin{aligned} sAG\gamma &= Q \\ -EI\kappa &= M \end{aligned} \right\} \text{ at } x = L \quad (32)$$

where  $\gamma$  is the shear strain,  $\kappa$  is the bending strain,  $\omega$  is the vertical displacement,  $\phi$  is the angle of rotation of the normal to the mid-plane of the beam,  $f$  is the distributed transverse load,  $Q$  is a point load,  $M$  is the moment,  $E$  is the Young's modulus,  $G$  is the shear modulus,  $A$  is the cross-sectional area,  $I$  is the second moment of inertia of the beam cross-section,  $s$  is the shear correction factor, normally set to 5/6 for rectangular cross-sections, and  $\Omega = (0, L)$ . When  $\omega$  and  $\phi$  are interpolated by basis functions of the same order the finite element solution to this problem exhibits shear locking as the beam becomes slender.

### 5.1 Symmetric Bézier $\bar{B}$ projection

For the Timoshenko beam, the source of locking comes from the transverse shear. Hence, under the framework of  $\bar{B}$  projection, the shear strain  $\gamma$  is projected onto a lower order function space. The projected shear strain  $\bar{\gamma}$  is used in solving the strain energy minimization problem. Here we use the Bézier projection operator to obtain  $\bar{\gamma}$ , and refer to it as Symmetric Bézier  $\bar{B}$  projection, as the resulting stiffness matrix is symmetric.

#### 5.1.1 The weak form

Given the function spaces  $\mathcal{S}(\Omega) = \{\mathbf{u} \mid \mathbf{u} \in H^1(\Omega) \times H^1(\Omega), \mathbf{u}|_{\Gamma_g} = \mathbf{g}\}$  and  $\mathcal{V}(\Omega) = \{\mathbf{w} \mid \mathbf{w} \in H^1(\Omega) \times H^1(\Omega), \mathbf{w}|_{\Gamma_g} = \mathbf{0}\}$  where  $\mathbf{u} = \{\omega, \phi\}^T$ ,  $\mathbf{w} = \{\delta\omega, \delta\phi\}^T$ ,  $\mathbf{g}$  is the prescribed Dirichlet

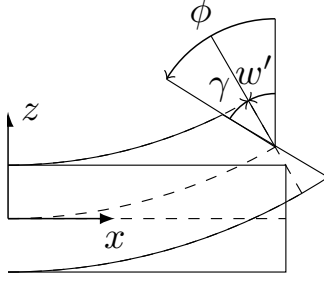


Figure 5: Deformation of a Timoshenko beam. The normal rotates by the angle  $\phi$ , which is not equal to  $w'$ , due to shear deformation.

boundary condition, and  $\Gamma_g$  is the Dirichlet boundary at  $x = 0$ , the weak form of the problem can be stated as: find  $\mathbf{u} \in \mathcal{S}(\Omega)$  such that for all  $\mathbf{w} \in \mathcal{V}(\Omega)$

$$\int_0^L \kappa(\mathbf{w}) EI \kappa(\mathbf{u}) + \bar{\gamma}(\mathbf{w}) sGA \bar{\gamma}(\mathbf{u}) dx = \int_0^L \delta \omega f dx + \delta \omega(L) Q + \delta \phi(L) M. \quad (33)$$

and  $\bar{\gamma}$  is the projected shear strain.

### 5.1.2 Discretization

We discretize  $\omega$  and  $\phi$  as ~~We discretize  $\mathbf{u}$  and  $\mathbf{w}$  as~~

$$\omega = \sum_A \omega_A N_A \mathbf{u} = \sum_A \mathbf{U}_A N_A \quad (34)$$

$$\phi = \sum_A \phi_A N_A \mathbf{u} = \sum_A \mathbf{U}_A N_A \quad (35)$$

where  $N_A$  is a degree  $p$  spline basis function,  $\omega_A$  and  $\phi_A$  are the corresponding control point values.  ~~$\mathbf{U}_A = \{\omega_A, \phi_A\}^T$  and  $\mathbf{W}_A = \{\delta \omega_A, \delta \phi_A\}^T$  and  $N_A$  is a degree  $p$  spline basis function.~~ The shear strain and bending strain can then be expressed as

$$\gamma = \sum_A [N'_A \quad -N_A] [\omega_A \quad \phi_A]^T \quad (36)$$

$$\kappa = \sum_A [0 \quad N'_A] [\omega_A \quad \phi_A]^T \quad (37)$$

The shear strain  $\bar{\gamma}$  is constructed by Bézier projection of the true shear strain  $\gamma$  onto a lower degree space. In other words, we project from a  $p^{th}$  degree spline space with  $n$  basis functions  $\mathbf{N}$  defined by the knot vector

$$\Xi_p = \underbrace{\{0, 0, \dots, 0\}}_{p+1 \text{ copies}}, \underbrace{\{\Xi_{int}, 1, 1, \dots, 1\}}_{p+1 \text{ copies}}, \quad (38)$$

onto a  $p-1^{th}$  degree spline space with  $\bar{n}$  basis functions  $\bar{\mathbf{N}}$  defined by the knot vector

$$\bar{\Xi}_{p-1} = \underbrace{\{0, 0, \dots, 0\}}_{p \text{ copies}}, \underbrace{\{\Xi_{int}, 1, 1, \dots, 1\}}_{p \text{ copies}} \quad (39)$$

where the internal knots, denoted by  $\Xi_{int}$ , are the same for both spaces. The projected shear strain  $\bar{\gamma}$  can then be written as

$$\bar{\gamma} = \sum_A \bar{\gamma}_A \bar{N}_A. \quad (40)$$

The control variables  $\bar{\gamma}_A$  are simply

$$\bar{\gamma}_A = \int_{\Omega^A} \hat{N}_A \gamma d\Omega = \langle \hat{N}_A, \gamma \rangle_{\Omega^A} \quad (41)$$

where  $\hat{N}_A$  is a dual basis function for the spline space of degree  $p-1$  computed from (22).



Localizing to the Bézier element we define the strain-displacement arrays in terms of element Bernstein basis functions of degree  $p$  and  $p - 1$  as

$$\mathbf{B}_e^\kappa = [0 \quad -B_{0,p}^e{}' \quad \cdots \quad 0 \quad -B_{p,p}^e{}'], \quad (42)$$

$$\mathbf{B}_e^\gamma = [B_{0,p}^e{}' \quad -B_{0,p}^e \quad \cdots \quad B_{p,p}^e{}' \quad -B_{p,p}^e], \quad (43)$$

$$\bar{\mathbf{B}}_e = [\bar{B}_{0,p-1}^e \quad \cdots \quad \bar{B}_{p-1,p-1}^e], \quad (44)$$

where  $B_{i,p}^e$  is the  $i^{th}$  Bernstein basis function of order  $p$ . We can then compute the element arrays as

$$\mathbf{K}_e^b = EIC^e \langle \mathbf{B}_e^{\kappa T}, \mathbf{B}_e^\kappa \rangle (\mathbf{C}^e)^T, \quad (45)$$

$$\bar{\mathbf{M}}_e = sGA\bar{\mathbf{C}}^e \langle \bar{\mathbf{B}}_e^T, \bar{\mathbf{B}}_e \rangle (\bar{\mathbf{C}}^e)^T, \quad (46)$$

$$\hat{\mathbf{P}}_e \mathbf{P}_{\bar{e}} = \langle (\hat{\mathbf{N}}^e)^T, \mathbf{B}_e^\gamma \rangle (\mathbf{C}^e)^T, \quad (47)$$

where  $\mathbf{C}^e$  is the element extraction operator for the degree  $p$  spline space,  $\bar{\mathbf{C}}^e$  is the element extraction operator for the degree  $p - 1$  spline space, and  $\hat{\mathbf{N}}^e$  are the dual basis functions restricted to the element for the degree  $p - 1$  spline space. The global stiffness matrix can then be written as

$$\mathbf{K} = \mathbf{K}^b + \bar{\mathbf{K}}_S^s \quad (48)$$

where

$$\mathbf{K}^b = \mathbf{A}_e \mathbf{K}_e^b, \quad (49)$$

$$\bar{\mathbf{K}}^s = \hat{\mathbf{P}} \mathbf{P}^T \bar{\mathbf{M}} \mathbf{P} \mathbf{P} \quad (50)$$

$$\hat{\mathbf{P}} \mathbf{P} = \mathbf{A}_e \hat{\mathbf{P}}_e \mathbf{P}_{\bar{e}} \quad (51)$$

$$\bar{\mathbf{M}} = \mathbf{A}_e \bar{\mathbf{M}}_e \quad (52)$$

and  $\mathbf{A}$  is the standard finite element assembly operator [42]. We note that the assembly of  $\bar{\mathbf{K}}^s$  requires the assembly of two intermediate matrices,  $\bar{\mathbf{M}}$  and  $\hat{\mathbf{P}} \mathbf{P}$ . The computation of these matrices is needed because the product of two integrals over the entire domain can not be localized to the element level.

## 5.2 Non-symmetric Bézier $\bar{B}$ projection

The relationship of  $\bar{B}$ -formulation with mixed formulations has been studied and proved to be equivalent by Simo and Hughes [27]. However, the symmetric Bézier  $\bar{B}$  projection lacks the connection to the mixed formulation. For this reason, instead of considering  $\bar{B}$  formulation as a strain projection method, we regard it as a mixed formulation with the auxiliary variable (the shear stress for the case of Timoshenko beam) being eliminated. Taking the advantage of the dual basis formulation of Bézier projection operator, we can incorporate Bézier projection into the mixed formulation by using the Bézier dual basis functions as the test functions of the auxiliary variable. Since the resulting stiffness matrix is not symmetric, this formulation is referred to as non-symmetric Bézier  $\bar{B}$  projection.

### 5.2.1 The weak form

In the mixed formulation, the shear stress  $\tau = sGA\gamma$  is taken as a new independent variable. The weak form of the mixed formulation is then of the following form: find  $\mathbf{u} \in \mathcal{S}(\Omega)$  and  $\tau \in L^2(\Omega)$  such that for all  $\mathbf{w} \in \mathcal{V}(\Omega)$  and  $\delta\tau \in L^2(\Omega)$

$$\int_0^L \kappa(\mathbf{w}) EI \kappa(\mathbf{u}) + \gamma(\mathbf{w}) \tau \, dx = l \langle \mathbf{w} \rangle \quad (53)$$

$$\int_0^L \delta\tau (sGA\gamma(\mathbf{u}) - \tau) \, dx = 0 \quad (54)$$

### 5.2.2 Discretization

In the finite element formulation of the mixed form, the discretization of  $\mathbf{u}$  and  $\mathbf{w}$  remain the same. Since the shear strain and its variation are in  $L^2(\Omega)$ , their finite element approximation can be made from functions with poor regularity e.g. discontinuous polynomials. For consistency, if the field  $\mathbf{u}$  is discretized by  $p^{th}$  degree spline basis functions defined by the knot vector (38), the shear stress  $\tau$  is then discretized by  $p - 1^{th}$  degree

spline basis functions defined by the knot vector (39) and its variation  $\delta\tau$  is discretized by the corresponding dual basis functions, as

$$\tau = \sum_A \tau_A \tilde{N}_A \quad (55)$$

$$\delta\tau = \sum_A \delta\tau_A \hat{\tilde{N}}_A \quad (56)$$

The stiffness matrix for the mixed form can be written as

$$\mathbf{K}_{mix} = \begin{bmatrix} \mathbf{K}^b & \mathbf{P}^T \\ sG\mathbf{A}\hat{\mathbf{P}} & -\mathbf{I} \end{bmatrix} \quad (57)$$

where

$$\mathbf{P} = \mathbf{A} \mathbf{P}_e, \quad (58)$$

with

$$\mathbf{P}_e = \bar{\mathbf{C}}^e \langle \bar{\mathbf{B}}_e^T, \mathbf{B}_e^\gamma \rangle (\mathbf{C}^e)^T. \quad (59)$$

The control variable of shear stress in (57) can be eliminated, which leads to a pure displacement formulation as

$$\mathbf{K} = \mathbf{K}^b + \bar{\mathbf{K}}_{NS}^s, \quad (60)$$

where

$$\bar{\mathbf{K}}_{NS}^s = sG\mathbf{A}\mathbf{P}^T\hat{\mathbf{P}}. \quad (61)$$

Compared to (45), the formulation of stiffness matrix contributed by shear strain requires one less matrix operation in (61). One can note that, due to the different choices of function spaces of shear stress and its variation, the global stiffness matrix is no longer symmetric.

### 5.3 Bandwidth of the stiffness matrix

A global  $\bar{B}$  method that utilizes a global  $L^2$  projection results in a dense stiffness matrix. The Bézier  $\bar{B}$  method, on the other hand, produces sparse stiffness matrices. However, the coupling of the local dual basis functions does increase the bandwidth slightly. This is illustrated in Figure 6, which shows the structure of the stiffness matrix for the Timoshenko beam problem using the second order basis functions of maximal smoothness for a displacement-based method (Figure 6a), global  $\bar{B}$  method (Figure 6b), symmetric Bézier  $\bar{B}$  method (Figure 6c) and non-symmetric Bézier  $\bar{B}$  method (Figure 6d). The blank cells indicate zero terms in the matrix while colored cells show the location of nonzero terms. The increased bandwidth of the

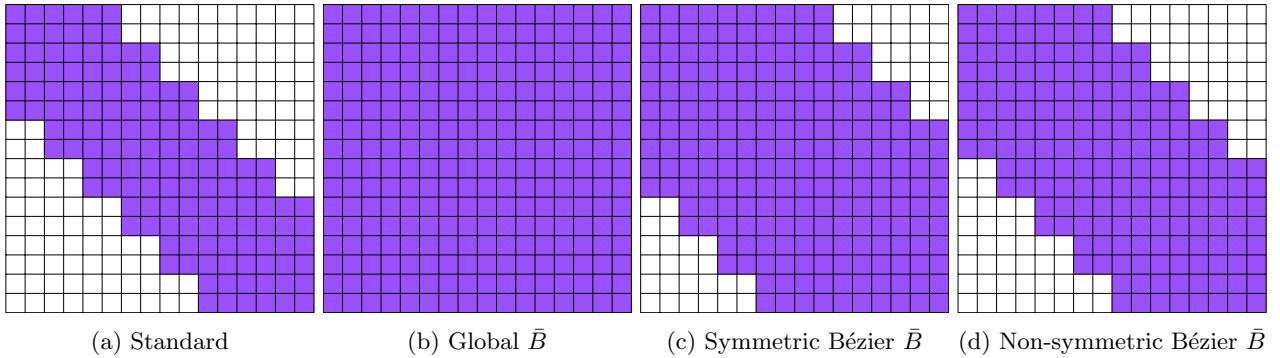


Figure 6: Illustrations of the structure of  $2^{nd}$  order Timoshenko beam stiffness matrices for (a) a standard displacement method, (b) a global  $\bar{B}$  method, (c) a symmetric Bézier  $\bar{B}$  method and (d) a non-symmetric Bézier  $\bar{B}$  method.

symmetric Bézier  $\bar{B}$  method when compared to a displacement-based method can be explained by looking at the product of the integrals in (50). For example, if we consider the basis functions  $N_1$  and  $N_5$  in Figure 7 we see that  $\text{supp}(N_1) \cap \text{supp}(N_5) = \emptyset$ , which means that the inner product of these two functions will be zero and the corresponding coefficient in the stiffness matrix will be zero in the displacement-based method. For the Bézier  $\bar{B}$  method, however, the form of (50) leads to a coupling between  $N_1$  and  $N_5$ . This can be seen by considering  $\Omega_2$ . Over this element, the shear stiffness can be represented as

$$\bar{\mathbf{K}}_2^s = \sum_{i=1}^3 \sum_{j=1}^3 \mathbf{P}_i^T \bar{\mathbf{M}}_2 \mathbf{P}_j \quad (62)$$

and the term of this summation that results in the coupling between  $N_1$  and  $N_5$  is  $\mathbf{P}_1^T \bar{\mathbf{M}}_2 \mathbf{P}_3$ , where  $\mathbf{P}_1$  is the inner product of  $N_1$  and  $\hat{N}_2$ ,  $\mathbf{P}_3$  is the inner product of  $N_5$  and  $\hat{N}_3$ , and  $\bar{\mathbf{M}}_2$  is the inner product of  $\bar{N}_2$  and  $\bar{N}_3$ . We can see from Figure 7 that  $\text{supp}(N_1) \cap \text{supp}(\hat{N}_2) = \Omega_1$ ,  $\text{supp}(N_5) \cap \text{supp}(\hat{N}_3) = \Omega_3$  and  $\text{supp}(\bar{N}_2) \cap \text{supp}(\bar{N}_3) = \Omega_2$ , so that  $\mathbf{P}_1^T \bar{\mathbf{M}}_2 \mathbf{P}_3$  is not zero. Thus we have increased the number of nonzero coefficients in the shear stiffness matrix. However, the same exercise can be used to show that there is no coupling between  $N_0$  and  $N_6$  for this set of basis functions so matrix is not dense. [With the absence of the gramian matrix in the formulation of shear stiffness matrix, the bandwidth of the non-symmetric Bézier  \$\bar{B}\$  method is even more reduced.](#) In fact, from the formulation of the element stiffness matrix, we can show that the bandwidth of the stiffness matrix of the symmetric Bézier  $\bar{B}$  and non-symmetric Bézier  $\bar{B}$  methods for the Timoshenko beam are  $6p - 3$  and  $4p - 1$ , respectively.

**Remark** In [39] a local  $\bar{B}$  method for shells was proposed that was based on the local least squares method presented in [41]. This approach has a similar structure to the [symmetric Bézier  \$\bar{B}\$  method](#) presented here. However, it was shown in [18] that choosing (18) as the weighting provides a significant increase in the accuracy of the approximation.

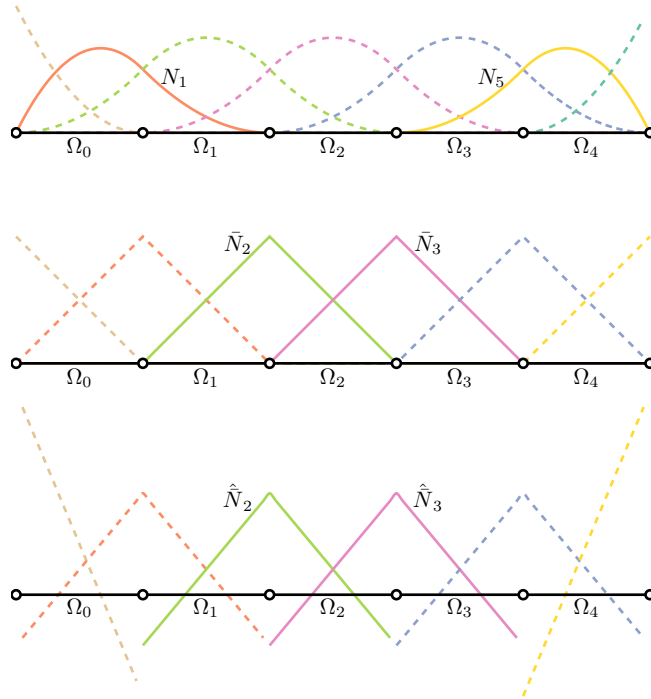


Figure 7: Quadratic maximally smooth B-spline basis functions (top), associated linear basis functions (middle), and dual basis functions (bottom) for the Bézier  $\bar{B}$  formulation.

## 5.4 Numerical results

In our study, a straight planar cantilever beam is clamped on the left end and a sinusoidal distributed load  $f(x) = \sin(\pi \frac{x}{l})$  is applied, as depicted in Figure 8. The analytical solution for vertical displacement  $w$ , rotation  $\phi$ , bending moment  $M$ , and transverse shear force  $Q$  are given by

$$\begin{aligned}
 w(x) &= \frac{EI (6\pi^2 l^2 \sin(\frac{\pi x}{l}) + 6\pi^3 l x) + sGA (6l^4 \sin(\frac{\pi x}{l}) - 6\pi l^3 x + 3\pi^3 l^2 x^2 - \pi^3 l x^3)}{6\pi^4 s E I G A} \\
 \phi(x) &= \frac{2l^3 \cos(\frac{\pi x}{l}) - 2l^3 + 2\pi^2 l^2 x - \pi^2 l x^2}{2\pi^3 EI} \\
 M(x) &= \frac{l^2 \sin(\frac{\pi x}{l}) - \pi l^2 + \pi l x}{\pi^2} \\
 Q(x) &= \frac{-l \cos(\frac{\pi x}{l}) - l}{\pi}.
 \end{aligned} \tag{63}$$

The beam has a rectangular cross-section and we use the following non-dimensional sectional and material parameters: length  $l = 10$ , width  $b = 1$ , thickness  $t = 0.01$ , Young's modulus  $E = 10^9$ , Poisson's ratio  $\nu = 0.3$ ,

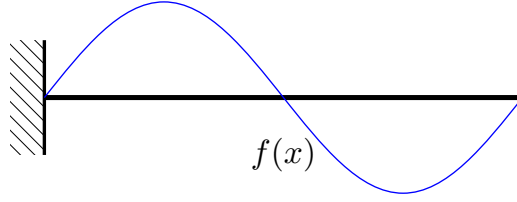


Figure 8: Straight planar cantilevered Timoshenko beam clamped at the left and loaded by a distributed load  $f(x)$ .

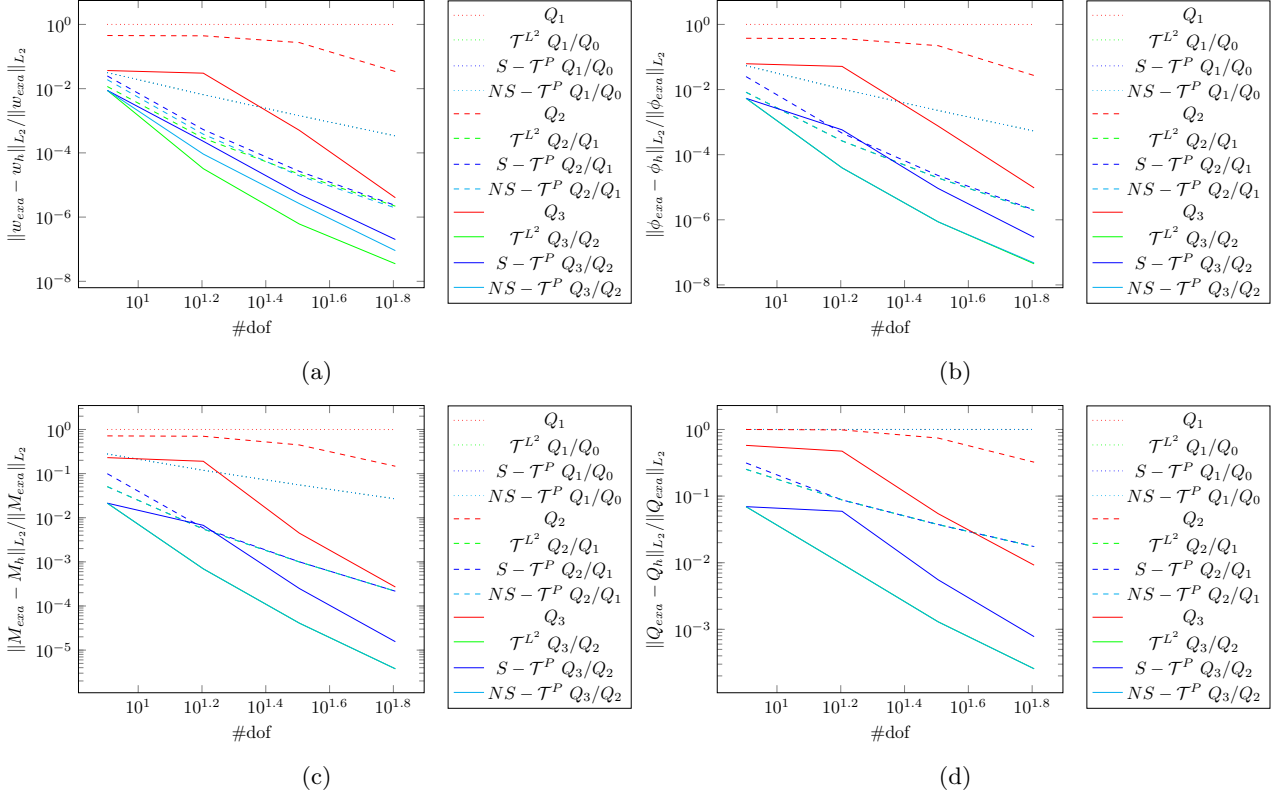


Figure 9: Convergence studies for slenderness factor  $l/t = 10^{-3}$ . Error in the  $L^2$ -norm for (a) displacement  $w$ , (b) rotation  $\phi$ , (c) bending moment  $M$ , and (d) shear force  $Q$ .

and a shear correction factor of  $s = 5/6$ . A comparison of the normalized error in the  $L^2$  norm for  $w$ ,  $\phi$ ,  $M$  and  $Q$  versus the number of degrees of freedom for polynomial degrees  $p = 1, 2, 3$  is shown in Figure 9. Results computed using standard finite elements are labeled  $Q_1$ ,  $Q_2$ , and  $Q_3$ . Results computed using a global  $\bar{B}$  method are labeled  $\mathcal{T}^{L^2}$ , those computed with the symmetric B  zier  $\bar{B}$  method and the non-symmetric B  zier  $\bar{B}$  method are labeled  $S - \mathcal{T}^P$  and  $NS - \mathcal{T}^P$ , respectively. the B  zier  $\bar{B}$  method are labeled  $\mathcal{T}^P$ . As expected, the  $Q_1$  results lock and the error remains virtually unchanged as the mesh is refined. Increasing the polynomial degree does reduce the locking effect, although the reduction is minor for the  $Q_2$  results.  $\mathcal{T}^{L^2}$ ,  $S - \mathcal{T}^P$  and  $NS - \mathcal{T}^P$  Both  $\mathcal{T}^{L^2}$  and  $\mathcal{T}^P$  are essentially locking free for all polynomial orders. The convergence rates for the  $\bar{B}$  methods are at least  $p + 1$  for  $w$ ,  $p$  for  $\phi$ ,  $p - 1$  for  $M$ , and  $p - 2$  for  $Q$ . These rates agree with those reported in [43] and are optimal. To reiterate, B  zier  $\bar{B}$  methods produces the same convergence rates as the global  $\bar{B}$  method and the error plots of  $NS - \mathcal{T}^P$  for  $\phi$ ,  $M$  and  $Q$  are identical to those of  $\mathcal{T}^{L^2}$ .

We have also studied the relationship between shear locking and decreasing slenderness ratios for  $p = 2$ . The results are shown in Figure 10. For all three methods, the number of degrees of freedom are fixed, and the sectional and material parameters are the same as in the previous study. The slenderness ratio varies from 10 to  $5 \times 10^3$ .  $Q_2$  locks severely. The  $\bar{B}$  methods, on the other hand, are locking free.

## 6 Volumetric locking: Nearly incompressible linear elasticity

To demonstrate the use of B  zier  $\bar{B}$  methods to alleviate volumetric locking effects we study the nearly incompressible elasticity problem in two dimensions. We start with the small strain tensor  $\varepsilon$ , which is

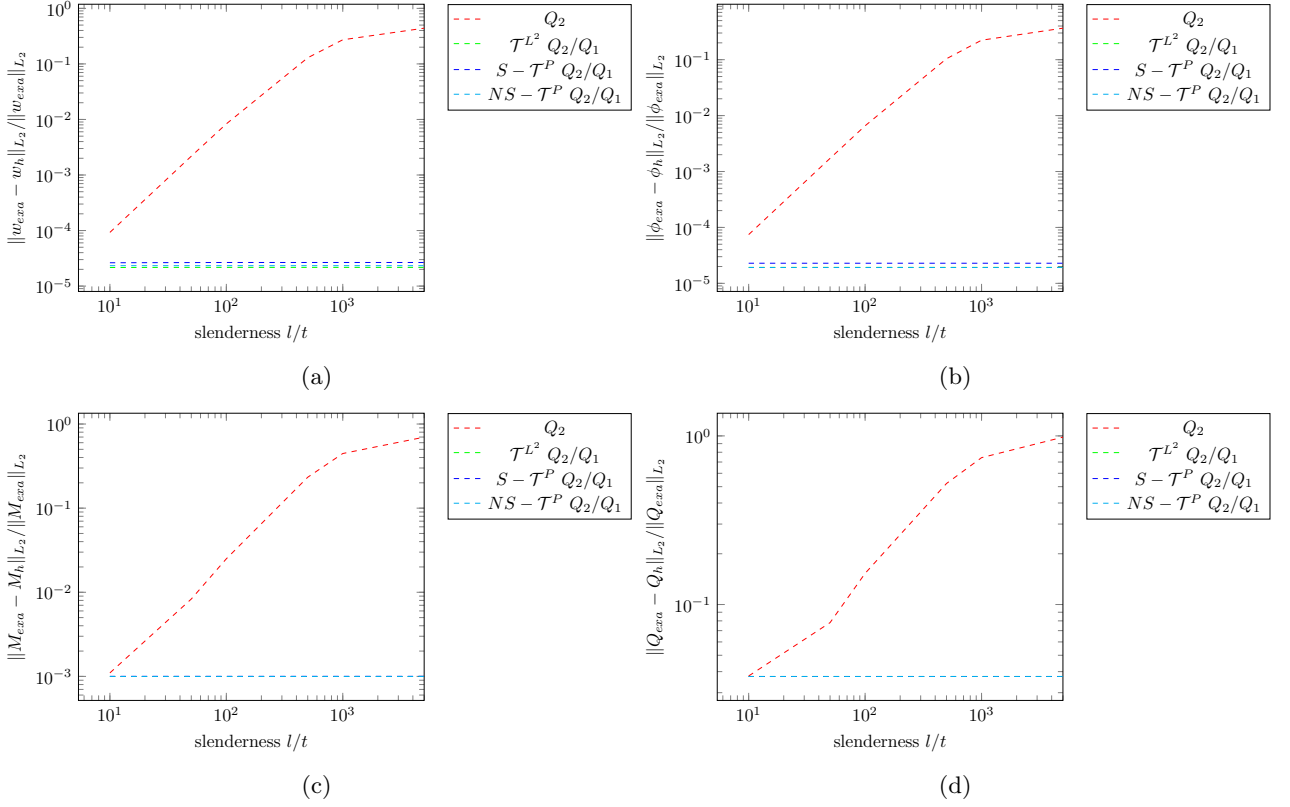


Figure 10: Convergence study for increasing slenderness,  $p = 2$ , and  $\#dof = 32$ . Error in the  $L^2$ -norm for (a) displacement  $w$ , (b) rotation  $\phi$ , (c) bending moment  $M$ , and (d) shear force  $Q$ .

defined as the symmetric part of the displacement gradient, i.e.,

$$\varepsilon_{ij} = \frac{u_{i,j} + u_{j,i}}{2}. \quad (64)$$

The stress tensor is related to the strain tensor through the generalized Hooke's law

$$\sigma_{ij} = c_{ijkl} \varepsilon_{kl} \quad (65)$$

where, for isotropic elasticity, the elastic coefficients and stress tensor can be expressed in terms of the Lamé parameters  $\lambda$  and  $\mu$  as

$$c_{ijkl} = \mu(\delta_{ik}\delta_{jl} + \delta_{il}\delta_{jk}) + \lambda\delta_{ij}\delta_{kl} \quad (66)$$

$$\sigma_{ij} = \lambda\varepsilon_{kk}\delta_{ij} + 2\mu\varepsilon_{ij}. \quad (67)$$

The Lamé parameters  $\lambda$  and  $\mu$  are defined in terms of Young's modulus,  $E$ , and Poisson's ratio,  $\nu$ , as

$$\lambda = \frac{\nu E}{(1 + \nu)(1 - 2\nu)} \quad (68)$$

$$\mu = \frac{\nu E}{2(1 + \nu)}. \quad (69)$$

we can write the strong form of linear elasticity as

$$\sigma_{ij,j} + f_i = 0 \text{ in } \Omega \quad (70)$$

$$u_i = g_i \text{ on } \Gamma_{g_i} \quad (71)$$

$$\sigma_{ij}n_j = h_i \text{ on } \Gamma_{h_i} \quad (72)$$

where Dirichlet boundary conditions are applied on  $\Gamma_{g_i}$ , Neumann boundary conditions are applied on  $\Gamma_{h_i}$ , and the closure of the domain  $\Omega$  is  $\bar{\Omega} = \Omega \cup \Gamma_{g_i} \cup \Gamma_{h_i}$ . [To demonstrate the source of volumetric locking, we introduce the pressure term](#)

$$p = -(\lambda + 2\mu/3)\epsilon_{ii}. \quad (73)$$

If  $\nu \rightarrow \frac{1}{2}$  then  $\lambda$  becomes very large and [an additional constraint is applied to the volumetric strain as standard finite element methods exhibit volumetric locking.](#)

$$\epsilon_{ii} = 0. \quad (74)$$

## 6.1 Symmetric Bézier $\bar{B}$ projection

### 6.1.1 The weak form

The  $\bar{B}$  approach for nearly incompressible linear elasticity splits the strain tensor  $\varepsilon$  into volumetric and deviatoric strains and then replaces the volumetric strain with a projected strain. We begin with

$$\varepsilon(\mathbf{u}) = \varepsilon^{vol}(\mathbf{u}) + \varepsilon^{dev}(\mathbf{u}) \quad (75)$$

where  $\varepsilon^{vol} = \frac{1}{3}\text{tr}(\varepsilon)\mathbf{I}$  is the volumetric strain and  $\varepsilon^{dev} = \varepsilon - \frac{1}{3}\text{tr}(\varepsilon)\mathbf{I}$  is the deviatoric strain. The volumetric strain is then replaced by a projected volumetric strain  $\bar{\varepsilon}^{vol}$  and the new total strain becomes

$$\bar{\varepsilon} = \bar{\varepsilon}^{vol} + \varepsilon^{dev}. \quad (76)$$

The weak form can then be written as: find  $\mathbf{u} \in \mathcal{S}(\Omega)$  such that for all  $\mathbf{w} \in \mathcal{V}(\Omega)$

$$\bar{a}\langle \mathbf{w}, \mathbf{u} \rangle = \bar{l}\langle \mathbf{u} \rangle \quad (77)$$

where

$$\bar{a}\langle \mathbf{w}, \mathbf{u} \rangle = \int_{\Omega} \bar{\varepsilon}_{ij}(\mathbf{w}) c_{ijkl} \bar{\varepsilon}_{kl}(\mathbf{u}) d\Omega, \quad (78)$$

$$\bar{l}\langle \mathbf{u} \rangle = \int_{\Omega} \mathbf{u} \cdot \mathbf{f} d\Omega + \int_{\Gamma_h} \mathbf{u} \cdot \mathbf{h} d\Gamma. \quad (79)$$

### 6.1.2 Discretization

Following the same approach as was described for Timoshenko beams in Section 5 we define element level strain-displacement matrices in terms of the Bernstein basis

$$\mathbf{B}_e^{dev} = \begin{bmatrix} \frac{2}{3} \frac{\partial B_{0,p}^e}{\partial x} & -\frac{1}{3} \frac{\partial B_{0,p}^e}{\partial y} & -\frac{1}{3} \frac{\partial B_{0,p}^e}{\partial z} & \dots & \frac{2}{3} \frac{\partial B_{p,p}^e}{\partial x} & -\frac{1}{3} \frac{\partial B_{p,p}^e}{\partial y} & -\frac{1}{3} \frac{\partial B_{p,p}^e}{\partial z} \\ \frac{1}{3} \frac{\partial B_{0,p}^e}{\partial x} & \frac{2}{3} \frac{\partial B_{0,p}^e}{\partial y} & -\frac{1}{3} \frac{\partial B_{0,p}^e}{\partial z} & \dots & \frac{1}{3} \frac{\partial B_{p,p}^e}{\partial x} & \frac{2}{3} \frac{\partial B_{p,p}^e}{\partial y} & -\frac{1}{3} \frac{\partial B_{p,p}^e}{\partial z} \\ -\frac{3}{1} \frac{\partial x}{\partial B_{0,p}^e} & \frac{3}{1} \frac{\partial y}{\partial B_{0,p}^e} & \frac{3}{2} \frac{\partial z}{\partial B_{0,p}^e} & \dots & -\frac{3}{1} \frac{\partial x}{\partial B_{p,p}^e} & \frac{3}{1} \frac{\partial y}{\partial B_{p,p}^e} & \frac{3}{2} \frac{\partial z}{\partial B_{p,p}^e} \\ -\frac{3}{1} \frac{\partial x}{\partial B_{0,p}^e} & -\frac{3}{1} \frac{\partial y}{\partial B_{0,p}^e} & \frac{3}{2} \frac{\partial z}{\partial B_{0,p}^e} & \dots & -\frac{3}{1} \frac{\partial x}{\partial B_{p,p}^e} & -\frac{3}{1} \frac{\partial y}{\partial B_{p,p}^e} & \frac{3}{2} \frac{\partial z}{\partial B_{p,p}^e} \\ 0 & \frac{\partial B_{0,p}^e}{\partial z} & \frac{\partial B_{0,p}^e}{\partial y} & \dots & 0 & \frac{\partial B_{p,p}^e}{\partial z} & \frac{\partial B_{p,p}^e}{\partial y} \\ \frac{\partial B_{0,p}^e}{\partial z} & 0 & \frac{\partial B_{0,p}^e}{\partial x} & \dots & \frac{\partial B_{p,p}^e}{\partial z} & 0 & \frac{\partial B_{p,p}^e}{\partial x} \\ \frac{\partial B_{0,p}^e}{\partial y} & \frac{\partial B_{0,p}^e}{\partial x} & 0 & \dots & \frac{\partial B_{p,p}^e}{\partial y} & \frac{\partial B_{p,p}^e}{\partial x} & 0 \end{bmatrix}, \quad (80)$$

$$\mathbf{B}_e^{vol} = \begin{bmatrix} \frac{\partial B_{0,p}^e}{\partial x} & \frac{\partial B_{0,p}^e}{\partial y} & \frac{\partial B_{0,p}^e}{\partial z} & \dots & \frac{\partial B_{p,p}^e}{\partial x} & \frac{\partial B_{p,p}^e}{\partial y} & \frac{\partial B_{p,p}^e}{\partial z} \end{bmatrix} \quad (81)$$

The deviatoric part of the element stiffness matrix can then be computed from the corresponding strain-displacement matrices as

$$\mathbf{K}_e^{dev} = \mathbf{C}^e \langle \mathbf{B}_e^{devT} \mathbf{D} \mathbf{B}_e^{dev} \rangle (\mathbf{C}^e)^T. \quad (82)$$

where  $\mathbf{C}^e$  is the element extraction operator for the degree  $p$  spline space. The volumetric part of the stiffness matrix is computed using Bézier projection. The intermediate element matrices are

$$\bar{\mathbf{M}}_e^{vol} = \bar{\mathbf{C}}^e \langle \bar{\mathbf{B}}_e^T, \frac{1}{3}(3\lambda + 2\mu) \bar{\mathbf{B}}_e \rangle (\bar{\mathbf{C}}^e)^T \quad (83)$$

$$\hat{\mathbf{P}}_e^{vol} = \langle (\hat{\mathbf{N}}^e)^T, \mathbf{B}_e^{vol} \rangle (\mathbf{C}^e)^T \quad (84)$$

where  $\bar{\mathbf{C}}^e$  is the element extraction operator for the degree  $p-1$  spline space,  $\hat{\mathbf{N}}^e$  are the dual basis functions restricted to the element, and

$$\bar{\mathbf{B}}_e = [B_{0,p-1}^e \quad \dots \quad B_{p-1,p-1}^e]. \quad (85)$$

The global stiffness matrix can then be assembled as

$$\mathbf{K} = \mathbf{K}^{dev} + \bar{\mathbf{K}}_S^{vol} \quad (86)$$

where

$$\mathbf{K}^{dev} = \mathbf{A}_e \mathbf{K}_e^{dev}, \quad (87)$$

$$\bar{\mathbf{K}}_S^{vol} = \hat{\mathbf{P}}^T \bar{\mathbf{M}} \hat{\mathbf{P}} \quad (88)$$

$$\hat{\mathbf{P}} = \mathbf{A}_e \hat{\mathbf{P}}_e^{vol} \quad (89)$$

$$\bar{\mathbf{M}} = \mathbf{A}_e \bar{\mathbf{M}}_e^{vol}. \quad (90)$$

## 6.2 Non-symmetric Bézier $\bar{B}$ projection

### 6.2.1 The weak form

In the context of nearly incompressible elasticity, the pressure  $p$  can be viewed as an independent unknown, leading to a mixed formulation, as: find  $\mathbf{u} \in \mathcal{S}(\Omega)$  and  $p \in L^2(\Omega)$  such that for all  $\mathbf{w} \in \mathcal{V}(\Omega)$  and  $\delta p \in L^2(\Omega)$

$$\hat{a}\langle \mathbf{w}, \mathbf{u} \rangle - b\langle \mathbf{w}, p \rangle = l\langle \mathbf{w} \rangle, \quad (91)$$

$$-b\langle \mathbf{u}, \delta p \rangle - \frac{1}{(\lambda + 2\mu/3)} \int_{\Omega} \delta p p d\Omega = 0, \quad (92)$$

where

$$\hat{a}\langle \mathbf{w}, \mathbf{u} \rangle = \int_{\Omega} \epsilon_{ij}(\mathbf{w}) \hat{c}_{ijkl} \epsilon_{kl}(\mathbf{u}) d\Omega, \quad (93)$$

$$\hat{c}_{ijkl} = \mu (\delta_{ik} \delta_{jl} + \delta_{il} \delta_{jk} - 2/3 \delta_{ij} \delta_{kl}), \quad (94)$$

$$b\langle \mathbf{w}, p \rangle = \int_{\Omega} \epsilon_{ii}(\mathbf{w}) p d\Omega. \quad (95)$$

### 6.2.2 Discretization

Following the same pattern of non-symmetric Bézier  $\bar{B}$  in Timoshenko beam problem, we keep the discretization of  $\mathbf{u}$  and  $\mathbf{w}$  untouched, while use lower order spline basis functions and corresponding dual basis functions for the discretization of pressure  $p$  and its variation  $\delta p$ , respectively. The discretized stiffness matrix in mixed form can be written as

$$\mathbf{K}_{mix} = \begin{bmatrix} \mathbf{K}^{dev} & -\mathbf{P}^T \\ -\hat{\mathbf{P}} & -\frac{1}{(\lambda + 2\mu/3)} \mathbf{I} \end{bmatrix}, \quad (96)$$

where

$$\mathbf{P} = \mathbf{A}_e \mathbf{P}_e^{vol}, \quad (97)$$

with

$$\mathbf{P}_e^{vol} = \bar{\mathbf{C}}^e \langle \bar{\mathbf{B}}_e^T, \mathbf{B}_e^{vol} \rangle (\mathbf{C}^e)^T. \quad (98)$$

By eliminating the pressure control points, we obtain the pure displacement formulation as

$$\mathbf{K} = \mathbf{K}^{dev} + \bar{\mathbf{K}}_{NS}^{vol}, \quad (99)$$

where

$$\bar{\mathbf{K}}_{NS}^{vol} = (\lambda + 2\mu/3) \hat{\mathbf{P}}^T \mathbf{P}. \quad (100)$$

## 6.3 Numerical results

We investigate the performance of the Bézier  $\bar{B}$  method for two nearly incompressible linear elasticity problems under plane strain conditions. [Previous to the examples, we numerically evaluate the infsup constant for global  \$\bar{B}\$  and non-symmetric Bézier  \$\bar{B}\$  methods.](#) For numerical examples, we first study the Cook's membrane problem, which is discretized with B-spline basis functions, and in the second problem we model the infinite plate with a circular hole problem using NURBS. Results computed using standard finite elements are labeled  $Q_1$ ,  $Q_2$ ,  $Q_3$ , and  $Q_4$ . Results computed using a global  $\bar{B}$  method are labeled  $\mathcal{T}^{L^2}$  and those computed with the [symmetric Bézier  \$\bar{B}\$  method](#) and the [non-symmetric Bézier  \$\bar{B}\$  method](#) are labeled  $S - \mathcal{T}^P$  and  $NS - \mathcal{T}^P$ , respectively.

Table 1: Weights for the plate with a circular hole

i	$w_{i,1}$	$w_{i,2}$	$w_{i,3}$
1	1	$1/\sqrt{2}$	1
2	1	$1/\sqrt{2}$	1
3	1	$1/\sqrt{2}$	1

Table 2: Control points for the plate with a circular hole

i	$B_{i,1}$	$B_{i,2}$	$B_{i,3}$
1	(0,1)	(1,1)	(1,0)
2	(0,2.5)	(2.5,2.5)	(2.5,0)
3	(0,4)	(4,4)	(4,0)

### 6.3.1 Numerical evaluation of the inf-sup condition

The inf-sup condition is also referred to as the Ladyzhenskaya-Babuska-Brezzi condition (or simply LBB). It is a crucial condition to ensure the solvability, stability and optimality of a mixed problem. For the nearly incompressible elasticity problem, the inf-sup condition is, for  $\delta p \neq 0$  and  $\mathbf{u} \neq 0$

$$\inf_{\delta p \in L^2(\Omega)} \sup_{\mathbf{u} \in \mathcal{S}(\Omega)} \frac{|b(\mathbf{u}, \delta p)|}{\|\delta p\|_{L^2(\Omega)} \|\mathbf{u}\|_{H^1(\Omega)}} \geq \beta > 0. \quad (101)$$

In a discretized problem, the inf-sup condition requires the variable  $\beta$  to be a constant that is independent of the mesh size.

Here, we consider the inf-sup condition of a uniformly refined quarter annulus. The geometry and boundary conditions are demonstrated in Figure 11. The geometry of the quarter annulus can be exactly represented using a biquadratic NURBS basis. The knot vector for the coarsest discretization is given by

$$\Xi_\xi \times \Xi_\eta = \{0, 0, 0, 1, 1, 1\} \times \{0, 0, 0, 1, 1, 1\} \quad (102)$$

and the corresponding weights and control points associated with each basis function are given in Table 1 and 2. For higher-order elements and finer discretizations the weights and corresponding control points are identified by an order elevation and knot insertion algorithm, respectively. The Bézier mesh representation for the discretizations are shown in Figure 12.

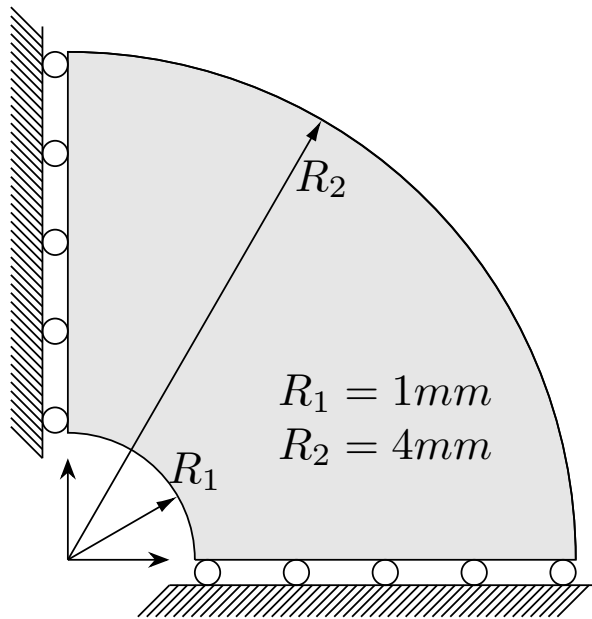


Figure 11: Geometry and boundary conditions for the inf-sup test.



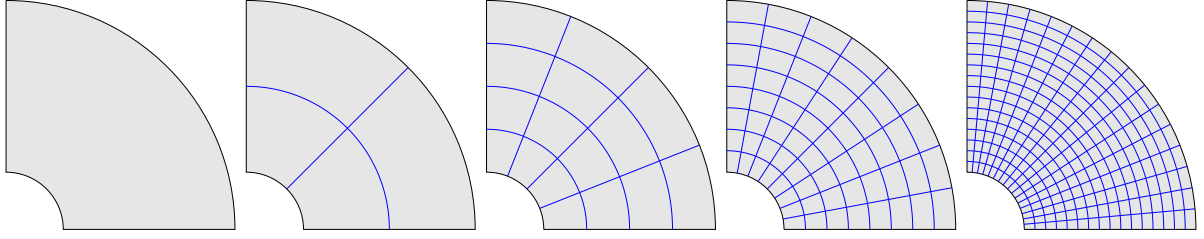


Figure 12: Sequence of meshes for inf-sup test.

Only the global  $\bar{B}$  method and the non-symmetric Bézier  $\bar{B}$  method are considered here, as the symmetric Bézier  $\bar{B}$  method lacks connection to the mixed formulation. As a counter example, the well-known pair  $Q_p/Q_p$  of global  $\bar{B}$  that violates the inf-sup condition is also tested here. Our tests follow the procedure proposed by Chapelle and Bathe in [44].

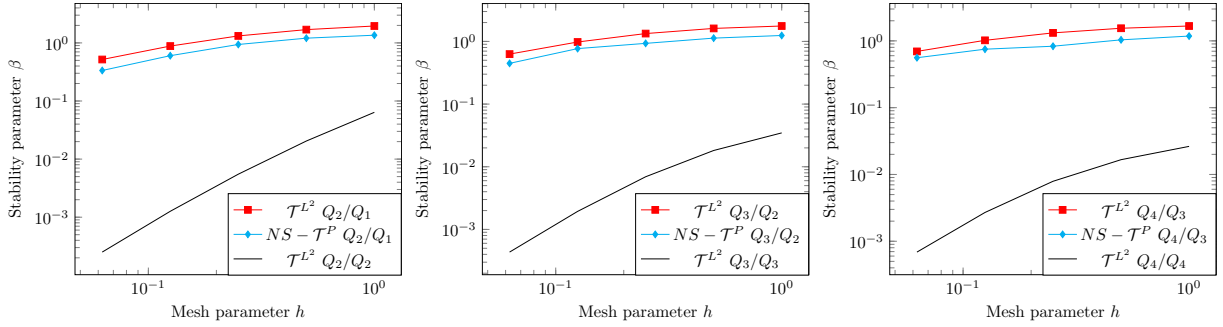


Figure 13: Inf-sup test results for nearly incompressible elasticity. The stability parameter  $\beta$  with respect to mesh size.

Figure 13 shows the numerical results. As can be seen, the global  $\bar{B}$  method and the non-symmetric Bézier  $\bar{B}$  method are not strictly satisfying the LBB condition, which is consistent with the statement that the global  $\bar{B}$  method does not reduce the constraints sufficiently to satisfy LBB condition in [38]. But compared to the  $Q_p/Q_p$  pair, both methods reduce constraints to a favorable level. Comparing the global  $\bar{B}$  method with non-symmetric Bézier  $\bar{B}$  method, their stability parameter  $\beta$  decrease in the same rate and the stability parameter for the non-symmetric Bézier  $\bar{B}$  method is slightly lower than that for the global  $\bar{B}$  method, which indicate a similar optimality in convergence for both methods and a slightly higher error for the non-symmetric Bézier  $\bar{B}$  method.

### 6.3.2 Cook's membrane problem

This benchmark problem is a standard test for combined bending and shearing response. The geometry, boundary conditions, and material properties are shown in Figure 14. The left boundary of the tapered panel is clamped, the top and bottom edges are free with zero traction boundary conditions, and the right boundary is subjected to a uniformly distributed traction load in the  $y$ -direction as shown. The meshes used are shown in Figure 15.

A comparison of the displacement of the top right corner with respect to the number of elements per side is shown in Figure 16.  $Q_1$  locks and mesh refinement has little impact. Locking is somewhat reduced for the higher-order elements  $Q_p$ ,  $p > 1$ . The  $\bar{B}$  methods perform very well for all degrees.

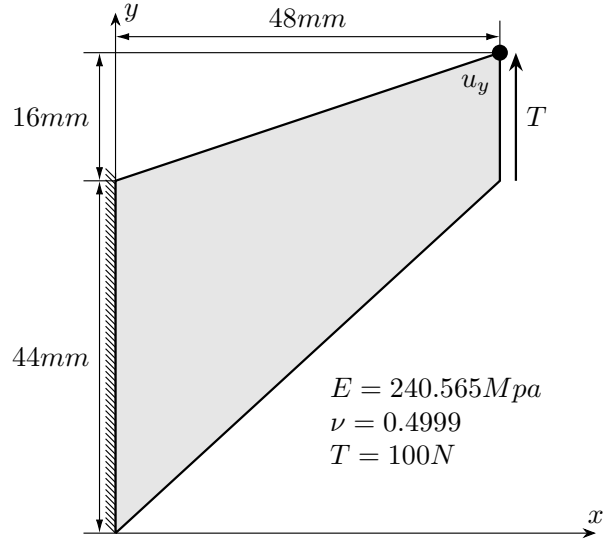


Figure 14: Geometry, boundary conditions, and material properties for the Cook's membrane problem.

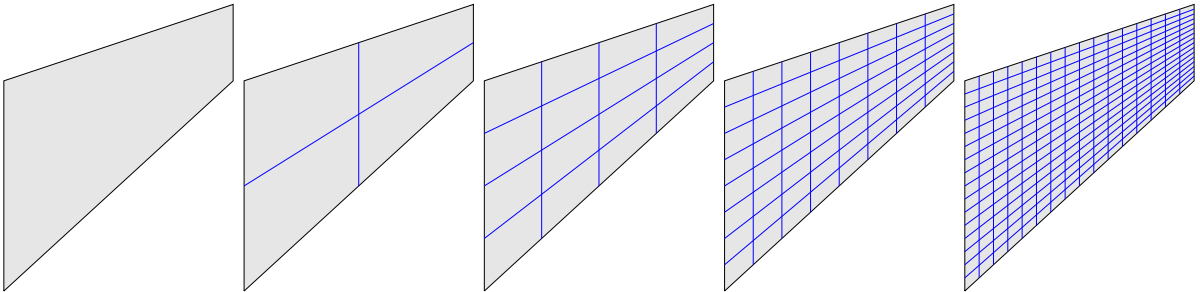


Figure 15: Sequence of meshes for Cook's membrane problem.

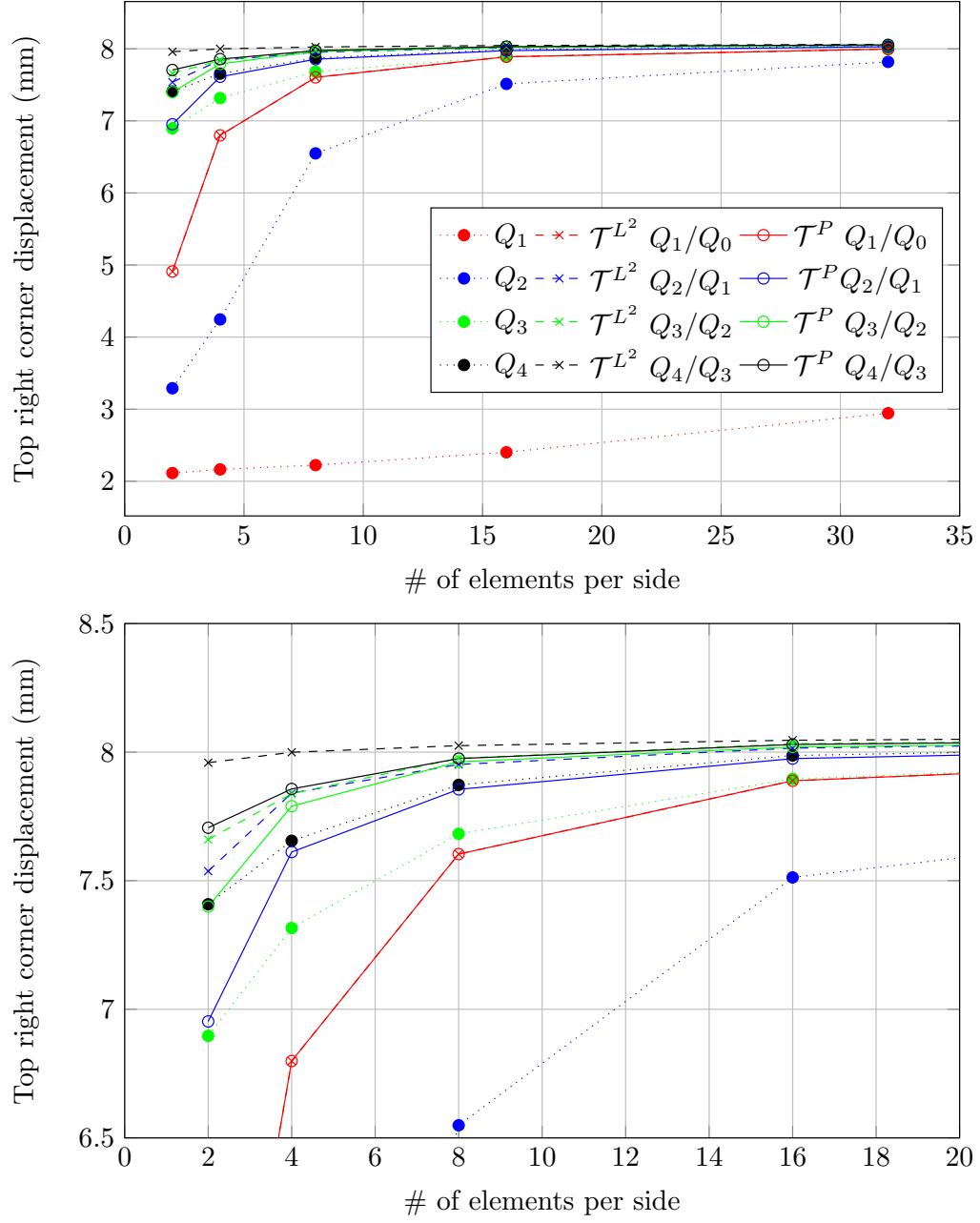
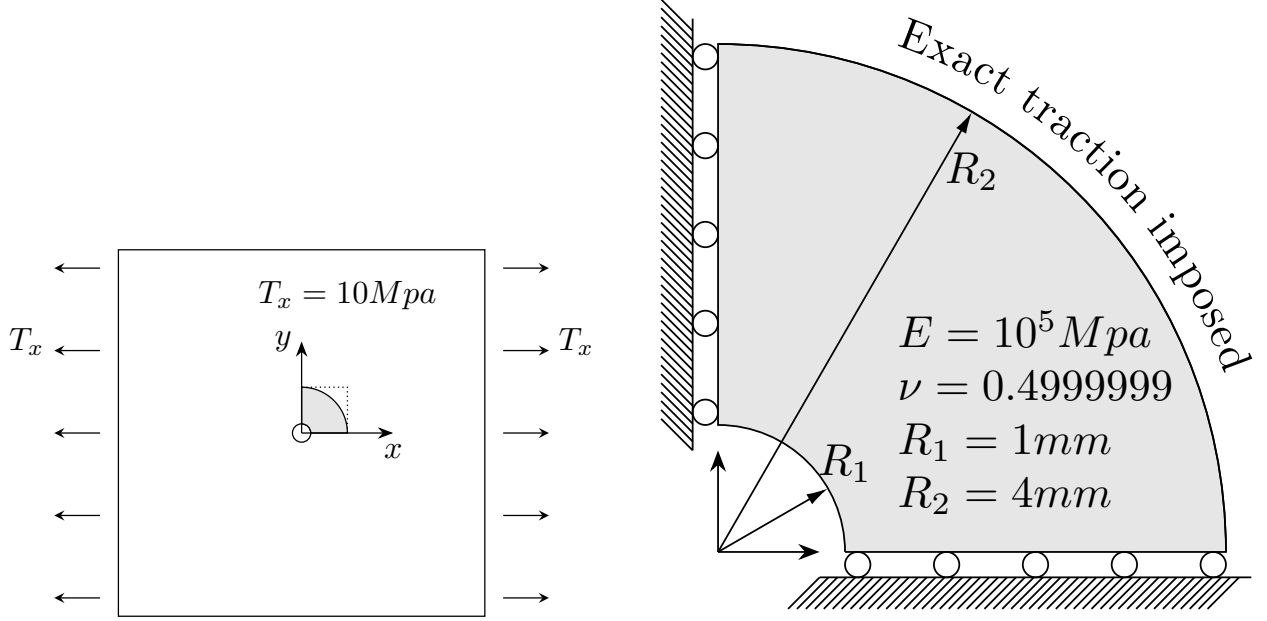


Figure 16: Cook's membrane: comparison of the vertical displacement at the top right corner for the different methods and degrees.

### 6.3.3 Infinite plate with a circular hole

The setup for the infinite plate with a circular hole problem is shown in Figure 17 and the discretizations are shown in Figure 12. The traction along the outer edge is evaluated from the exact solution which is given by

$$\begin{aligned}\sigma_{rr}(r, \theta) &= \frac{T_x}{2} \left(1 - \frac{R_1^2}{r^2}\right) + \frac{T_x}{2} \left(1 - 4\frac{R_1^2}{r^2} + 3\frac{R_1^4}{r^4}\right) \cos(2\theta) \\ \sigma_{\theta\theta}(r, \theta) &= \frac{T_x}{2} \left(1 + \frac{R_1^2}{r^2}\right) - \frac{T_x}{2} \left(1 + 3\frac{R_1^4}{r^4}\right) \cos(2\theta) \\ \sigma_{r\theta}(r, \theta) &= -\frac{T_x}{2} \left(1 + 2\frac{R_1^2}{r^2} - 3\frac{R_1^4}{r^4}\right) \sin(2\theta).\end{aligned}\tag{103}$$



(a) Infinite plate with a hole subjected to uniaxial tension at  $x = \pm\infty$ .

(b) A representation of the computational model.

Figure 17: Geometry, boundary conditions, and material properties for the infinite plate with a hole.

Convergence plots for the relative error of the displacement and energy in the  $L^2$  norm are shown in Figure 18. As can be seen, the standard  $Q_p$  approximations suffer from severe volumetric locking for all orders while, on the other hand, the  $\bar{B}$  methods remedy locking for all cases. For the symmetric Bézier  $\bar{B}$  method, the optimality in all three measures has been obtained for biquadratic elements; the optimal energy convergence rate has been achieved for bicubic elements; however, the degradation of convergence rates in all three measures has been observed for biquartic elements. This can be attributed to the fact that the derivation of the symmetric Bézier  $\bar{B}$  method is purely based on the engineering analogy between the  $L^2$  projection and Bézier projection. On the other hand, the non-symmetric Bézier  $\bar{B}$  method, derived from the mixed formulation, is optimal for displacement, stress and energy for all elements with slightly higher errors than that of global  $\bar{B}$  method, which agrees with the inf-sup test. For biquadratic elements, the Bézier  $\bar{B}$  method obtains optimal convergence rates for both the displacement and energy error, and the difference in the energy error between the global  $\bar{B}$  and the Bézier  $\bar{B}$  methods is indistinguishable. For bicubic and biquartic elements, the convergence rates of the Bézier  $\bar{B}$  method for the energy error are optimal. The convergence rates of Bézier  $\bar{B}$  method for the displacement error, however, are not optimal. This degradation in the rates can likely be attributed to the conditioning of the Bernstein basis and the element extraction operators.

The contour plots of  $\sigma_{xx}$  for bicubic elements of the third mesh are shown in Figure 19,  $Q_3$  elements are severely locked while all  $\bar{B}$  methods converge to the reference solution.

## 7 Conclusions

We have presented the Bézier  $\bar{B}$  method as an approach to overcome locking phenomena in structural mechanics applications of isogeometric analysis. The approach utilizes Bézier extraction and projection which

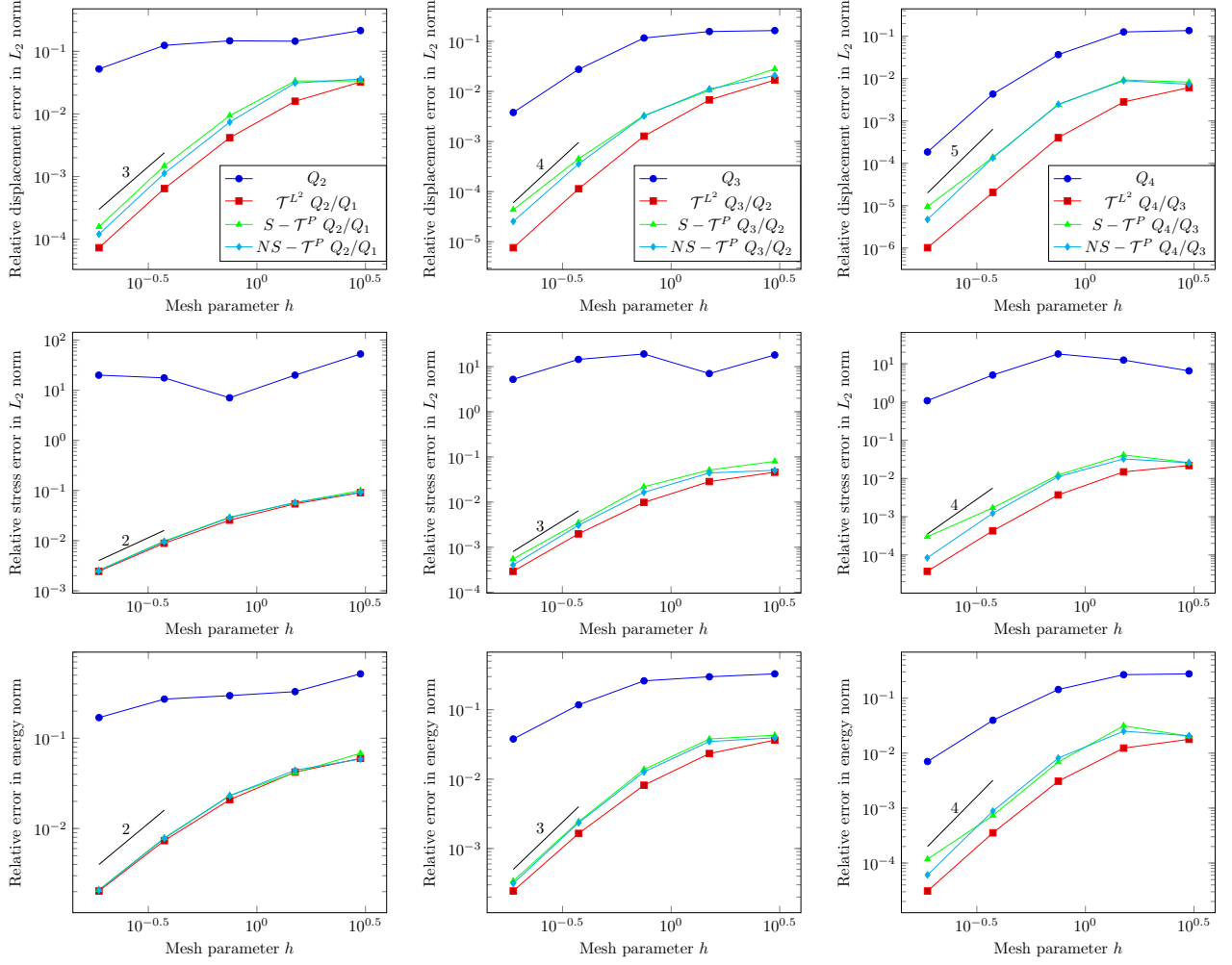


Figure 18: Convergence study of the plate with a circular hole. The relative  $L^2$  error of displacement, stress and the relative error in energy norm with respect to mesh refinement.

makes it simple to implement in an existing finite element framework and makes it applicable to any spline representation which can be written in Bézier form. In contrast to global  $\bar{B}$  methods, which produce dense stiffness matrices, the Bézier  $\bar{B}$  approach results in a sparse stiffness matrix while still benefiting from higher-order convergence rates.

We have demonstrated the performance of the approach in the context of shear deformable beams (to alleviate transverse shear locking) and nearly incompressible elasticity problems (to alleviate volumetric locking). The proposed method reduces locking errors and achieves (nearly) optimal convergence rates. The cases where optimal rates were not achieved warrant further study.

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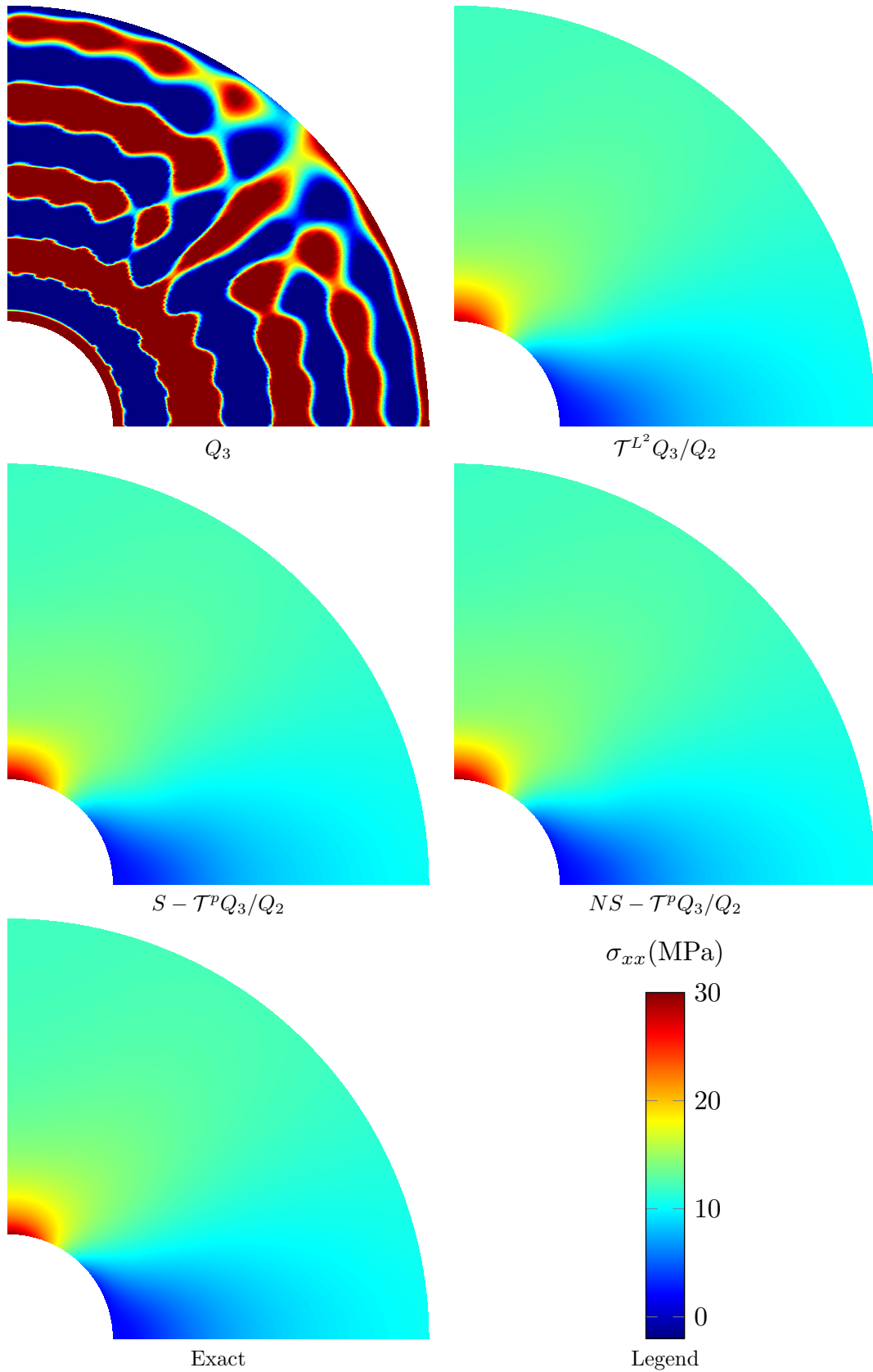


Figure 19: Contour plots of  $\sigma_{xx}$  for the plate with a circular hole ( $p = 3$ , the  $3^{rd}$  mesh is used).

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