

name1 = Truss1  
name2 = K - joint  
loadcase = Fatigue  
location = Support1  
name = JohnSmith

2020-10-20 22:58:43

## Uniplanar SHS/RHS K-Joints with Overlap

### CIDECT DESIGN GUIDE 8

The purpose of this worksheet is to determine the allowable stresses at the K-Joints of the trusses.

The CIDECT 8 design guide is adopted. It can be downloaded at: <https://www.cidect.org/design-guides/>

## INPUT GEOMETRY

Excerpt 1: Notation

### Chord Size

b0 = 350.0 mm (Width of Chord)  
h0 = 350.0 mm (Height of Chord)  
t0 = 20.0 mm (Thickness of Chord)

### Brace size

b1 = 300.0 mm (Width of Brace)  
h1 = 300.0 mm (Height of Brace)  
t1 = 16.0 mm (Thickness of Brace)

### Parameters to determine Brace Angle

$e = -110.000$  mm (Eccentricity)  
chordspacing = 2.000 m (Spacing between Legs for determining brace angle)  
 $L_{chord} = 6.000$  m (Height of legs for determining brace angle)  
div<sub>chord</sub> = 4 (no diagonal braces for determining brace angle)

## INPUT LOADING

Excerpt 2: Load Conditions

$P_{braced} = 100.000$  kN (Axial Force +ve Compression)  
 $P_{braced} = 50.000$  kN (Axial Force +ve Tension)  
 $M_{\theta_{braced}} = 10.000$  kN · m (Moment In-Plane on Chord (Column of Gantry))  
 $M_{\theta_{braced}} = 10.000$  kN · m (Moment Out-of-Plane on Chord (Column of Gantry))  
 $M_{\theta_{braced}} = 10.000$  kN · m (Moment Out-of-Plane on Brace (Between gantry columns))

## CAPACITY

deltaF = deltaF · Pa = 24.300 MPa · Pa = 24.300 MPa (Allowable fatigue stress after SCF applied)

## SCF ASSUMED

CIDECT 8 does not cover out of plane bending of SHS/RHS sections.

Out of plane SCF's are defined manually below.

SCF<sub>h<sub>ov</sub></sub> = 2.0 (Min recommended Sec 5 and Appendix C)

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### Parameters

$\beta = \frac{b1}{b0} = \frac{300.0 \text{ mm}}{350.0 \text{ mm}} = 8.6 \times 10^{-1}$  (Ratio of brace to chord width, where  $0.35 \leq \beta \leq 1.0$ )  
 $\gamma = \frac{b0}{2 \cdot t0} = \frac{350.0 \text{ mm}}{2 \cdot 20.0 \text{ mm}} = 8.8$  (Ratio of chord width to 2\*thickness, where  $10 \leq 2 \cdot \omega \leq 35$ )  
 $\tau = \frac{t1}{t0} = \frac{16.0 \text{ mm}}{20.0 \text{ mm}} = 0.8$  (Ratio of brace to chord thickness, where  $0.25 \leq \tau \leq 1.0$ )  
bracing centreline length between intersection points  
Excerpt 3: Overlap Calculation  
 $h_{brs} = \text{chordspacing} + 2 \cdot e = 2.000 \text{ m} + 2 \cdot -110.000 \text{ mm} = 1.780 \text{ m}$  (Height of truss adjusted by eccentricity.)  
 $L_{truss} = \frac{L_{chord}}{\frac{div_{chord}}{4}} = \frac{6.000 \text{ m}}{4} = 1.500 \text{ m}$  (Length of truss (projection of brace onto the chord))  
 $\theta = \arctan\left(\frac{h_{truss}}{L_{truss}}\right) = \arctan\left(\frac{1.780 \text{ m}}{1.500 \text{ m}}\right) = 8.706 \times 10^{-1}$  (Angle between chord and brace)  
 $p = \frac{b1}{\sin(\theta)} = \frac{300.000 \text{ mm}}{\sin(8.706 \times 10^{-1})} = 392.317 \text{ mm}$  (Projected width of brace)  
 $x = \frac{0.5 \cdot h0 + e}{\tan(\theta)} = \frac{0.5 \cdot 350.000 \text{ mm} + -110.000 \text{ mm}}{\tan(8.706 \times 10^{-1})} = 54.775 \text{ mm}$  (Projection of Intersection)  
 $q = p - 2 \cdot x = 392.317 \text{ mm} - 2 \cdot 54.775 \text{ mm} = 282.766 \text{ mm}$  (Overlap Projection)  
 $Ov = \frac{q}{p} = \frac{282.766 \text{ mm}}{392.317 \text{ mm}} = 7.208 \times 10^{-1}$  (Overlap, where  $50\% \leq Ov \leq 100\%$ )

### Check Parameter within Allowable

```
beta:
Check 0.86 <= 1
U111 = 0.86
Check 0.86 >= 0.35
U111 = 0.41
PASS
2*gamma:
Check 17.50 <= 35
U111 = 0.50
Check 17.50 >= 10
U111 = 0.57
PASS
tau:
Check 0.80 <= 1.0
U111 = 0.80
Check 0.80 >= 0.35
U111 = 0.44
PASS
theta:
Check 49.88 <= 60
U111 = 0.83
Check 49.88 >= 30
U111 = 0.60
PASS
Ov:
Check 0.72 <= 1.0
U111 = 0.72
Check 0.72 >= 0.5
U111 = 0.69
PASS
e/h0:
Check -0.31 <= 0.25
Check -0.31 >= -0.55
PASS
```

## Section Properties

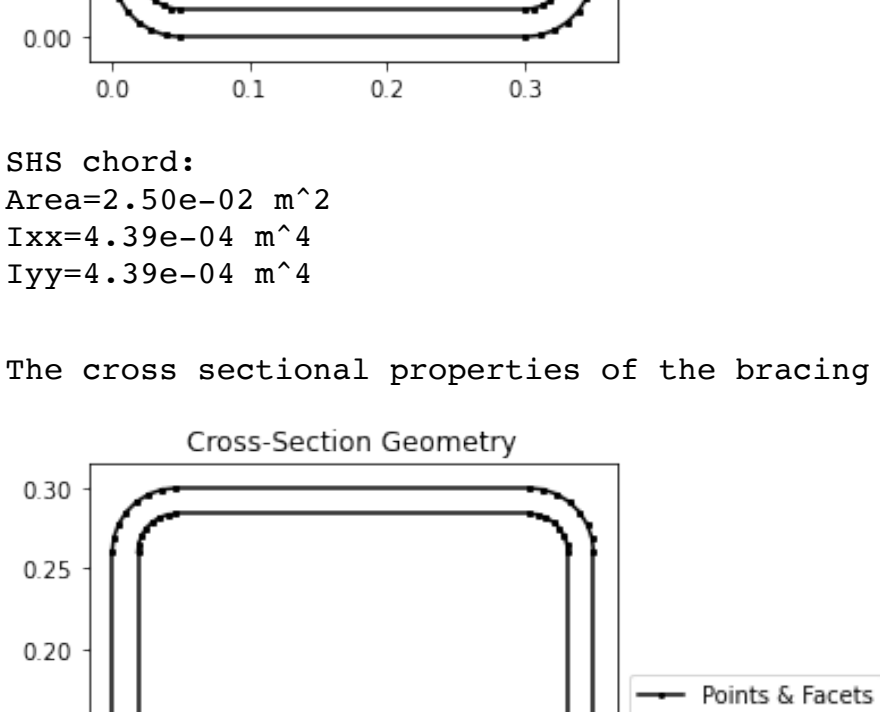
The following section calculates the SHS properties:

- 2nd moment of area in 2 directions.
- area

This is to enable determining the external fibre stresses:

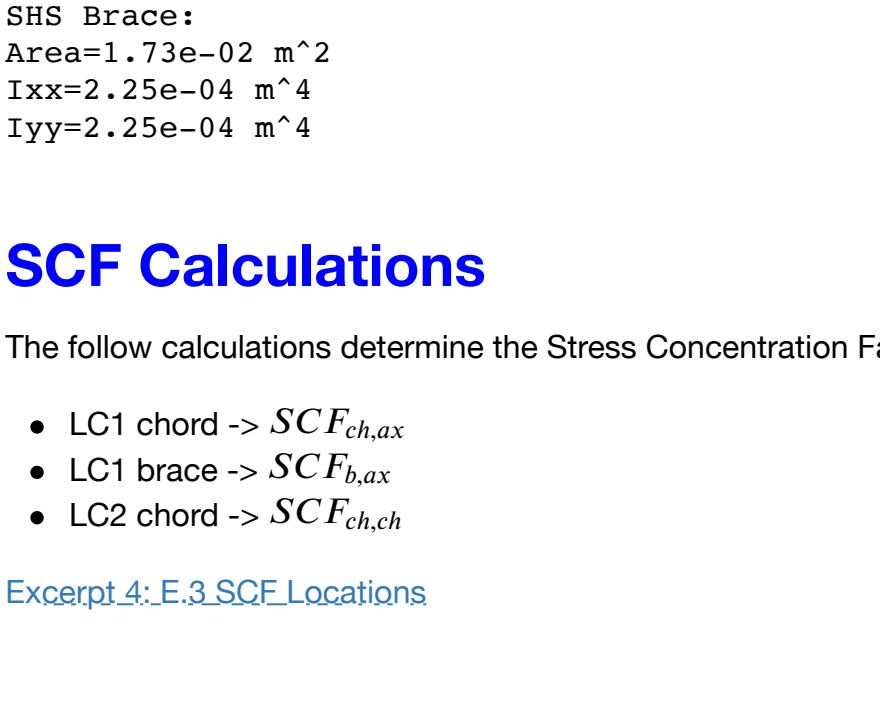
The cross sectional properties of the chord member:

```
/Users/michael11a/ita/opt/miniconda3/envs/mywork1/lib/python3.8/site-packages/sectionproperties/pre/sections.py:304:
MatplotlibDeprecationWarning: Case-insensitive properties were deprecated in 3.3 and support will be removed two min
or releases later.
ax.plot(cp[0], h[1], 'rx', markerSize=5, label='Holes')
/Users/michael11a/ita/opt/miniconda3/envs/mywork1/lib/python3.8/site-packages/sectionproperties/pre/sections.py:311:
MatplotlibDeprecationWarning: Case-insensitive properties were deprecated in 3.3 and support will be removed two min
or releases later.
ax.plot(cp[0], cp[1], 'bo', markerSize=5,
```



SHS chord:  
Area=2.50e-02 m<sup>2</sup>  
Ixx=4.39e-04 m<sup>4</sup>  
Iyy=4.39e-04 m<sup>4</sup>

The cross sectional properties of the bracing member:



SHS Brace:  
Area=1.73e-02 m<sup>2</sup>  
Ixx=2.25e-04 m<sup>4</sup>  
Iyy=2.25e-04 m<sup>4</sup>

## SCF Calculations

The follow calculations determine the Stress Concentration Factors (SCF) for each:

- LC1 chord -> SCF<sub>chax</sub>
- LC1 brace -> SCF<sub>bax</sub>
- LC2 chord -> SCF<sub>chh</sub>

Excerpt 4: E.3 SCF Locations

### SCF<sub>chax</sub> Calculation

$SCF_{chax1} = (0.5 + 2.38 \cdot \beta - 2.87 \cdot (\beta)^2 + 2.18 \cdot \beta \cdot Ov + 0.39 \cdot Ov - 1.43 \cdot \sin(\theta))$   
 $= (0.5 + 2.38 \cdot 8.571 \times 10^{-1} - 2.87 \cdot (8.571 \times 10^{-1})^2 + 2.18 \cdot 8.571 \times 10^{-1} \cdot 7.208 \times 10^{-1} + 0.39 \cdot 7.208 \times 10^{-1} - 1.43 \cdot \sin(8.706 \times 10^{-1}))$   
 $= 9.658 \times 10^{-1}$

$SCF_{chax2} = ((2 \cdot \gamma)^{0.29} \cdot (\tau)^{0.7} \cdot (Ov)^{(0.73-5.53 \cdot \sin(\theta)+2)} \cdot (\sin(\theta))^{((-0.4)-0.08 \cdot Ov)})$   
 $= ((2 \cdot 8.75)^{0.29} \cdot (0.8)^{0.7} \cdot (7.208 \times 10^{-1})^{(0.73-5.53 \cdot \sin(8.706 \times 10^{-1})+2)} \cdot (\sin(8.706 \times 10^{-1}))^{((-0.4)-0.08 \cdot 7.208 \times 10^{-1})})$   
 $= 5.035$

$SCF_{chax} = SCF_{chax1} \cdot SCF_{chax2}$   
 $= 9.658 \times 10^{-1} \cdot 5.035$   
 $= 4.863$  (Balanced Loading condition Chord Forces)

### SCF<sub>bax</sub> Calculation

$SCF_{bax1} = 0.15 + 1.1 \cdot \beta - 0.48 \cdot (\beta)^2 - \frac{0.14}{Ov}$   
 $= 0.15 + 1.1 \cdot 8.571 \times 10^{-1} - 0.48 \cdot (8.571 \times 10^{-1})^2 - \frac{0.14}{7.208 \times 10^{-1}}$   
 $= 0.546$

$SCF_{bax2} = (2 \cdot \gamma)^{0.55} \cdot (\tau)^{(-0.3)} \cdot (Ov)^{((-2.57)+1.62 \cdot \beta+2)} \cdot (\sin(\theta))^{0.31}$   
 $= (2 \cdot 8.75)^{0.55} \cdot (0.8)^{(-0.3)} \cdot (7.208 \times 10^{-1})^{((-2.57)+1.62 \cdot 8.571 \times 10^{-1}+2)} \cdot (\sin(8.706 \times 10^{-1}))^{0.31}$   
 $= 7.462$

$SCF_{bax} = SCF_{bax1} \cdot SCF_{bax2}$   
 $= 0.546 \cdot 7.462$   
 $= 4.074$  (Balanced Loading condition Brace Forces)

### SCF<sub>chh</sub> Calculation

$SCF_{chh} = 1.2 + 1.46 \cdot \beta - 0.028 \cdot (\beta)^2$   
 $= 1.2 + 1.46 \cdot 8.571 \times 10^{-1} - 0.028 \cdot (8.571 \times 10^{-1})^2$   
 $= 2.431$

## 3.3 Nominal Stress Ranges

Nominal stresses are obtained by getting the principal and outer fiber bending stresses of each element defined in Sec 3.3.

### σ<sub>chord</sub>

#### Balanced LC

$\sigma_{chord1P} = SCF_{chax} \cdot \frac{P_{braced} \cdot \cos(\theta)}{\text{chord.area} \cdot (\text{m})^2}$   
 $= 4.863 \cdot \frac{50.000 \text{ kN} \cdot \cos(8.706 \times 10^{-1})}{\text{chord.area} \cdot (\text{m})^2}$   
 $= 6.271 \text{ MPa}$

#### Unbalanced LC

$\sigma_{chord2P} = SCF_{chh} \cdot \frac{P_{braced} \cdot \cos(\theta)}{\text{chord.area} \cdot (\text{m})^2}$   
 $= 2.431 \cdot \frac{100.000 \text{ kN} \cdot \cos(8.706 \times 10^{-1})}{\text{chord.area} \cdot (\text{m})^2}$   
 $= 6.595 \text{ MPa}$

### Bending Moment

$y_{P_{braced}} = \frac{h0}{2} = \frac{350.000 \text{ mm}}{2} = 175.000 \text{ mm}$  (Dist from NA to top/bottom of brace)

$y_{P_{braced}} = \frac{b0 - b1}{2} = \frac{350.000 \text{ mm} - 300.000 \text{ mm}}{2} = 25.000 \text{ mm}$  (Dist from NA to left/right of brace)

$\sigma_{chordM_{\theta}} = SCF_{chh} \cdot \frac{M_{\theta_{braced}} \cdot y_{P_{braced}}}{\text{chord.ixx} \cdot (\text{m})^4}$   
 $= 2.431 \cdot \frac{10.000 \text{ kN} \cdot \text{m} \cdot 175.000 \text{ mm}}{\text{chord.ixx} \cdot (\text{m})^4}$   
 $= 9.690 \text{ MPa}$

$\sigma_{chordM_{\theta}} = SCF_{chh} \cdot \frac{M_{\theta_{braced}} \cdot y_{P_{braced}}}{\text{chord.iyy} \cdot (\text{m})^4}$   
 $= 2.0 \cdot \frac{10.000 \text{ kN} \cdot \text{m} \cdot 25.000 \text{ mm}}{\text{chord.iyy} \cdot (\text{m})^4}$   
 $= 1.139 \text{ MPa}$

### Chord Total

$\sigma_{chord} = \sigma_{chord1P} + \sigma_{chord2P} + \sigma_{chordM_{\theta}} + \sigma_{chordM_{\theta}}$   
 $= 6.3 \text{ MPa} + 6.6 \text{ MPa} + 9.7 \text{ MPa} + 1.1 \text{ MPa}$   
 $= 23.7 \text{ MPa}$

Sigma\_chord capacity:  
Check 23694948.19 <= 243000000.0  
U111 = 0.98  
PASS

## Brace Stresses

### σ<sub>brace</sub>

#### Balanced LC

$\sigma_{brace1P} = SCF_{bax} \cdot \frac{P_{braced}}{\text{brace.area} \cdot (\text{m})^2}$   
 $= 4.074 \cdot \frac{50.000 \text{ kN}}{\text{brace.area} \cdot (\text{m})^2}$   
 $= 11.795 \text{ MPa}$

### Bending Moment

$z_{P_{braced}} = \frac{b1}{2} = \frac{300.000 \text{ mm}}{2} = 150.000 \text{ mm}$

$\sigma_{braceM_{\theta}} = SCF_{bax} \cdot \frac{M_{\theta_{braced}} \cdot z_{P_{braced}}}{\text{brace.iyy} \cdot (\text{m})^4} = 2.0 \cdot \frac{10.000 \text{ kN} \cdot \text{m} \cdot 150.000 \text{ mm}}{\text{brace.iyy} \cdot (\text{m})^4} = 13.312 \text{ MPa}$

### Brace Total

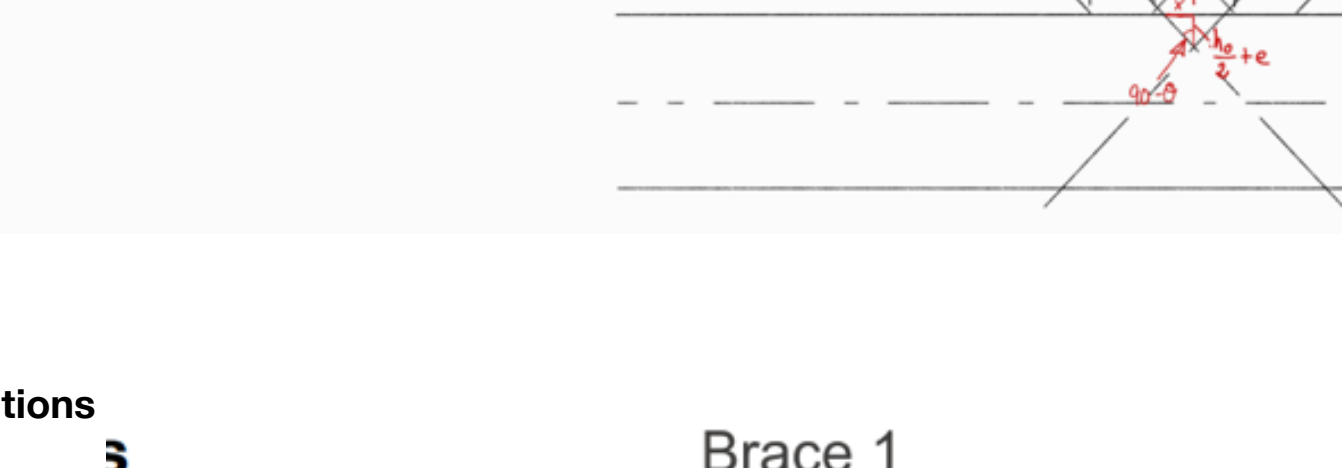
$\sigma_{brace} = \sigma_{brace1P} + \sigma_{braceM_{\theta}}$   
 $= 11.795 \text{ MPa} + 13.312 \text{ MPa}$   
 $= 25.107 \text{ MPa}$

Sigma\_brace capacity:  
Check 25107040.62 <= 243000000.0  
U111 = 1.03  
FAIL

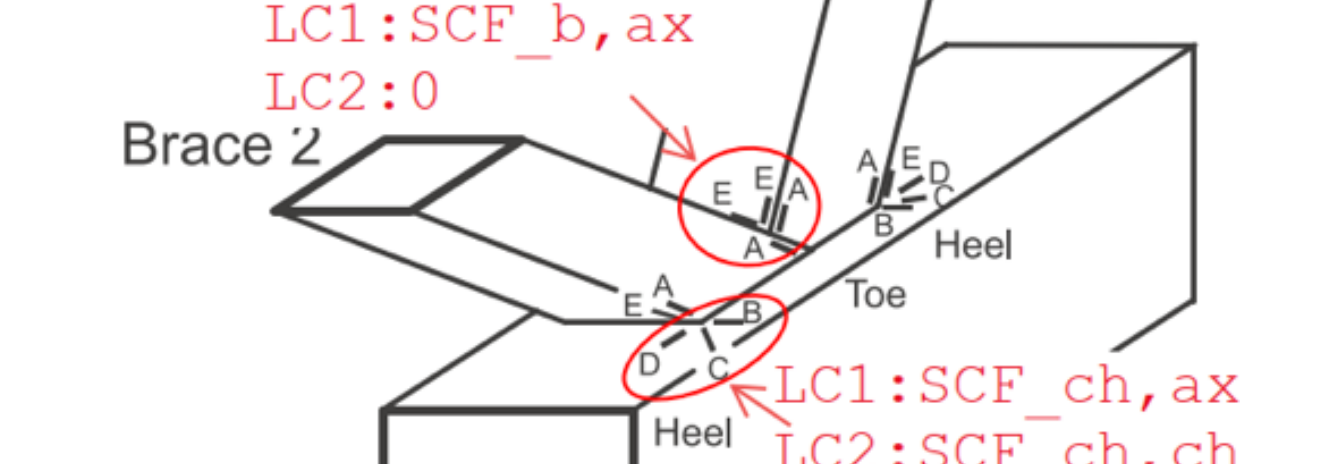
## RELEVANT CLAUSES

### Excerpt 1: Notation

Definition of geometrical parameters



### Excerpt 2: Load Conditions



### Excerpt 3: Overlap Calculation



### Excerpt 4: E.3 SCF Locations



## SUMMARY

### Geometry Check Summary

```
beta:
Check 0.86 <= 1
U111 = 0.86
Check 0.86 >= 0.35
U111 = 0.41
PASS
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Check 17.50 <= 35
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Check 17.50 >= 10
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U111 = 0.44
PASS
theta:
Check 49.88 <= 60
U111 = 0.83
Check 49.88 >= 30
U111 = 0.60
PASS
Ov:
Check 0.72 <= 1.0
U111 = 0.72
Check 0.72 >= 0.5
U111 = 0.69
PASS
e/h0:
Check -0.31 <= 0.25
Check -0.31 >= -0.55
PASS
```

### Stress Check Summary

Sigma\_chord capacity:  
Check 23694948.19 <= 243000000.0  
U111 = 0.98  
PASS

Sigma\_brace capacity:  
Check 25107040.62 <= 243000000.0  
U111 = 1.03  
FAIL