Soil tests				sample n										
	Test description	Sample type		g.	0 (- maxy		Specimen	Nor	ninal dime	nsions, o	r maximum gra	ain size, or	fraction
			<2 <2 mm mi		<63 mm	<125 mm	Specimen type	mass	D	L	W	Н	D _{max}	d/D
			kg	kg	kg	kg		kg	mm	mm	mm	mm	mm	min/max
	Split & describe				-		I, D		-		-		1	
	Water Content		1	10	20	60	I, D	0.5-15					-	
	Atteberg limits (4 pt, 1 pt)					20							0.425	-
	Methylene blue	I, D	1	5	10			0.3					2	0/2 0/0.125
Classification & Shrinkage	Linear shrinkage										-		0.425	-
	Particle size distribution by sieving and sedimentation		5	10	60	160		0.2-10					125	0/125
	Particle Density		1	5	10	20		1					37.5 4	0/37.5 0/4
	Bulk density (including SMC)	I			-		I	0.2-4	38-100			(1.0-2.0)D	-	
	Filter paper	I, D	1		-		I, R, r	0.2	70 100		-	10	2	-
	2.5 kg compaction - 1 L mould							2	105			115		
	2.5 kg compaction - CBR mould							5	152	-		127	-	
	4.5 kg compaction - 1 L mould		30	80	100	120		2	105	-		115	20	
	4.5 kg compaction - CBR mould						R, r	_	450			407		-
	Compaction by vibrating hammer							5	152			127		
	Moisture condition value		10	20	40	80		4.5		_				
	Moisture content calibration		20	40	40 80 100	100		1.5	100			200	20	
Compaction & Stabilisation	Chalk crushing value	D	-	20		-	Lumps	1			-			10/20
	California bearing ratio							5	152			127	20	-
	Maximum & minimum density		20	40	80	100	R, r	0.5 2 5	35 70 105 152			2D 2D 115 127	2 4 6.3 37.5	
	Degree of pulverisation							1					-	
	Initial lime content			-				0.5	-			-	0.425	-
	Compressive strength (stabilised material)							0.5-5	50-150			1D	D/5	
Consolidation	Incremental loading consolidation (1D)		1	8				0.2	50 63.5 75			20	H/6	
& Swelling	Consolidation in a hydraulic (Rowe) cell (stress- or strain-controlled, 1D, pwp)	I, D	2	14		-	I, R, r	0.2	70		-	25	•	-
	Constant-rate-of-strain (CRS) consolidation (strain-controlled, 1D, pwp)		2	14				5.2	70			25	H/10	

					mass bas ain size (Tes	t specime	n			
Soil tests	Test description	Sample type		_				Specimen	Non	ninal dime	nsions, o	r maximum gra	in size, or	fraction
			<2 mm	<20 mm	<63 mm	<125 mm	Specimen type	mass	D	L	w	н	\textbf{D}_{max}	d/D
			kg kg kg kg kg mm mm	mm	mm	mm	mm	min/max						
	Triaxial consolidation (isotropic, anisotropic, K ₀ , pwp)		2	14				0.2-3.5	38 50 70 100			2D	D/6	
	Swelling strain index		1	8					50					
	Swelling pressure		1	8				0.2	63.5			20	H/6	
	Collapse or swelling on wetting		1	8				75						
	Hand vane						ı							
	Torvane				-		ı	-	-			-	-	
Undrained shear strength	Lab vane		2	14				2	70 100			1D	H/10	
	Fall cone	I, D	1	4				0.1-0.5	50		_	(0.5-1.0)D	H/10	_
	Thixotropy (fall cone test)	1, D				_	I, R, r	0.1-0.5	30			(0.3-1.0)D	11/10	_
	Unconfined compression		2	14			, ,	0.2-5	38 50 70 100			(1.8-2.5)D	D/6	
	Consolidated undrained direct simple shear							0.2	63.5			0.4D	H/10	
	Consolidated drained direct shear (small shear box)	I, D	1	4	<u>-</u>		I D r							
	Residual shear strength using the small direct shear apparatus		1	4			I, R, r	0.2	60 100	60 100	60 100	20	H/6	
	Interface direct shear (small shear box)		1	4										
Drained shear	Consolidated drained direct shear (large shear box)							>00	200					
strength	Residual shear strength using the large direct shear apparatus		-	120			R, r	≥30	300	300	300	≥150	20	<u>-</u>
	Residual shear strength using the ring shear apparatus				-		13,1							
	Interface ring shear test	D	2					0.5		-		5	H/6	
	ICP ring shear test													
Triaxial	Unconsolidated undrained	1.5	•	4.4			I D -	0.0.5	38 50			00	D/C	-
compression or extension	Consolidated isotropic or anisotropic undrained or drained	I, D	2	14	-	-	I, R, r	0.2-5	70 100		-	2D	D/6	
	Falling head permeability in an oedometer							0.2	50			20		
Permeability	Constant head permeability in a Rowe (hydraulic) cell	I, D					l D r	0.2	70			25	H/6	
i eimeavilly	Constant or falling head in a triaxial cell	I, D			-		I, R, r	0.2-4	100	-		1D		-
	Falling or constant head in a permeameter							0.2-4	100			(1.0-2.0)D	D/6	
	Pinhole	I, D	1				-	0.15	-				2	
Dispersibility	Crumb method	I	-		-		Crumbs of soil	0.5	6-10		-		-	
	Double hydrometer	I, D	1				-	0.2	-				2	

Soil tests Test description			Field sample mass based on maximum grain size (D _{max})				Test specimen								
	Test description	Sample type						Specimen	Nominal dimensions, or maximum grain size, or fraction						
				<63 mm	<125 mm	Specimen type	mass	D	L	w	Н	D _{max}	d/D		
			kg	kg	kg	kg		kg	mm	mm	mm	mm	mm	min/max	
	Bender elements		4	16	16			1-5	70 100			2D	D/6		
Shear modulus	Resonant column-Torsional shear	I, D				_	I, R, r		70] .		140	D/6	_	
& cyclic stress	Cyclic direct simple shear	., 2	2 4 16					0.5	63.5	-		0.4D	H/10		
	Cyclic triaxial (strain- & stress- controlled)						1-5	70 100			2D	D/6			

I: intact; D: disturbed; R: reconstituted; r: remoulded

The mass of the practical field sample, which is presumably a mixture of fine and coarse grains, depends on the nominal diameter of the coarsest grains.

The practical field sample mass should be sufficiently greater than the test specimen mass, e.g. four times (4X) the standard test specimen mass, to allow for quartering and preparation of test specimens according to the specific standard test method. If uncertain, then collect a sample (kg) that is three times (3X) the diameter of the largest grain(s), e.g. for soil containing 20 mm grains then the recommended sample mass is $20 \times 3 = 60 \text{ kg}$. Individual sample bags should be kept below a manageable mass, e.g. 15 kg.

For disturbed specimens, the test specimen mass $M = \rho \times V$, where ρ the bulk density (Mg/m³, or kg/m³, or g/cm³) and V is the volume of the specimen (m³ or cm³).

Rock tests			Field sample mass based on the maximum grain size (D _{max})						Test specimen							
	Test description	Sample type			J			Specimen type	Specimen mass	Nominal dimensions, or maximum grain size, or fraction						
			<2 mm	<20 mm	<63 mm	<125 mm	<125 mm			D	L	w	н	D _{max}	d/D	
			kg kg	kg	kg	kg		kg	mm	mm	mm	mm	mm	min/max		
Geotechnical description								-								
	Water Content	I, D							1			-				
Index properties	Porosity & density - saturation & callipers	-						Core, irregular pieces	1	05.400			(0.5.0.5)D	-	-	
p. opooo	Porosity & density - saturation & buoyancy											-	(0.5-2.5)D			
	P- and S-wave velocity	- I						Core	-	≥50	≥50	≥50	≥50	D/10		
	Point load index (one or 10 specimens)							Core or rectangular block		30-100	>0.5D	(0.3- 1.0)D	-	-		
	Uniaxial compressive strength	-			-			2000	_	50-100					-	
	Uniaxial compression with Young's modulus & Poisson's ratio	I						0		100			(2.5-3.0)D			
Strength & Deformability	Triaxial compression							Core	-						-	
Deformability	Triaxial compression with Young's modulus & Poisson's ratio	-								54		-	(2.0-3.0)D	D/10		
	Indirect tensile strength							Disk		54			(0.20-0.75)D			
	Shear strength under constant normal load or constant normal stiffness	-						Core with closed or open discontinuities		50-100			2D			
	CERCHAR abrasivity index	I						Core		≥50	≥50	≥50	≥50	-	-	
	LCPC abrasivity & breakability	D			-			Cuttings	5	-	-	-	-	-	4/6.3	
	Schmidt hammer	I						Core	-	50-100		_	2D	-	-	
Hardness &	Los Angeles		-	40	80	120	120	Crushed	10	-	-	-	-	-	10/14	
Durability	Swelling (pressure or axial strain)	-						Core or crushed	0.0502	35-75	-	-	2D	H/6		
	Slaking	I, D			-			Chunks	0.2	-	-	-	-	-	-	
	Slake durability							Lumps (10)	0.5	-	-	-	-	-	1	
	Magnesium sulphate		5	10	40	80	80	Crushed	0.5	-	-	-	-	-	10/14	

Core: right circular cylinder

Geochemical tests		Sample	Field s		iass base in size (D		ximum				t specime			••-	Eng. 40
	Test descriptions	type	<2	<20	<63	<125	<125		Specimen		inal dimer		maximum gra		
			mm		mm	mm	mm	Specimen type	mass	D	L	W	Н	D _{max}	d/D
			kg	kg	kg	kg	kg		kg	mm	mm	mm	mm	mm	min/max
	Electrical Resistivity (four- or two-electrode method)		5-10	10-20	20-40		_	Soil (I, R, r) Rock (core)	1-5	70-100	28 16	8	4 3 (1.0-2.0)D	_	_
Electrochemical	Thermal resistivity		10	20	40			Nock (core)		38-100		-	(1.0-2.0)D		
	Redox potential				-			Core							'
	pH value								0.5						
	Water-soluble sulphate in soil	I, D		5				-							
	Sulphate in groundwater														
	Acid-soluble sulphate in soil														
	Total sulphur														
	Total sulphide (reduced sulphur) in soil														
	Acid-soluble sulphide in soil														
Chamiaal	Organic content		4		40	20	20						-		
Chemical	Loss on ignition		1		10	20	20								
	Chloride in soil & groundwater														
	Magnesium in soil & groundwater														
	Total dissolved solids in groundwater														
	Non-pyritic BRE suite														
	Pyritic BRE suite														
	Non-pyritic BRE brownfield suite														
	Pyritic BRE brownfield suite														
	Petrography	I						1		-	25	50	5	-	-
	SEM/EDS							Lumps	450						'
Mineralogy	Powder XRD	I, D			-			D I	≤50 g				-		
	Clay-fraction content XRD							Powder							

Core: right circular cylinder