

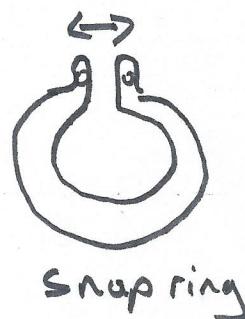
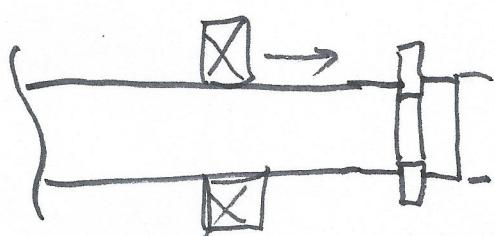
Deformation and StiffnessChap 4

Design for high rigidity

- minimize misalignment
- avoid interference w/ other components
- reduce noise
- reduce wear rates
- reduce stress

Design for flexibility

- energy storage and absorption
 - springs
- elastic deformations for change in dimensions



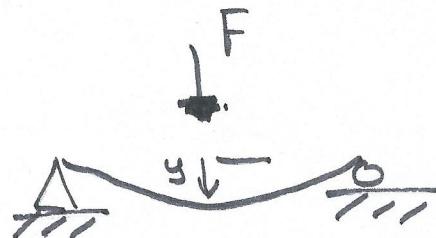
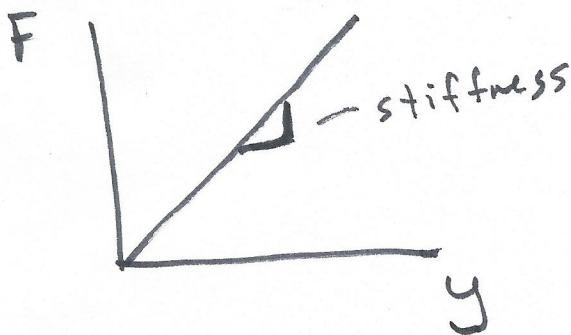
snap ring

Rigidity deflection per load $\frac{\Delta y}{F}$

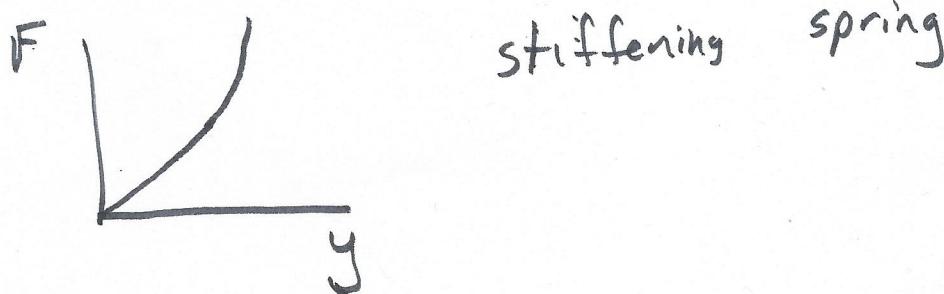
- Mod of Elasticity gives a good indication of rigidity
- geometry of the structural element is essential to characterize rigidity
- inverse of rigidity is "stiffness"

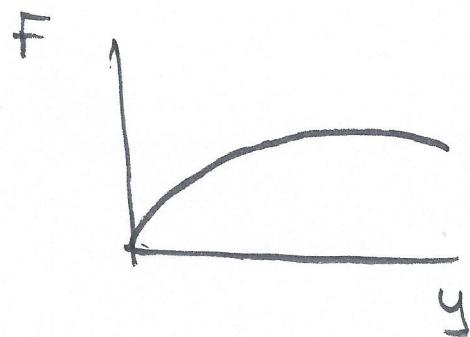
$$k = \frac{F}{\Delta y}$$

Linear Springs

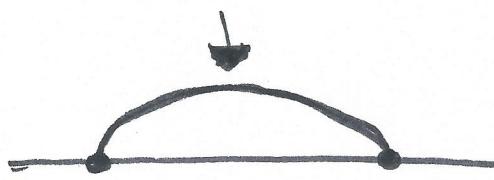


Nonlinear Springs





Softening Spring



Stiffness (Spring constant) for linear springs

Axial

$$S = \frac{F/l}{AE}$$

↓ stiffness

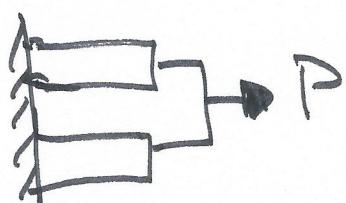
$$K = \frac{AE}{l}$$

Torsional

$$\Theta = \frac{T/l}{GJ}$$

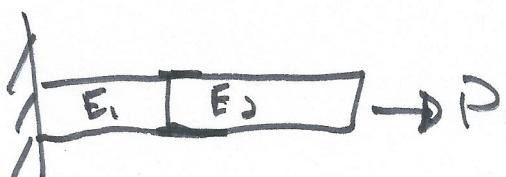
$$K = \frac{GJ}{l} \quad \text{torsional rigidity}$$

Parallel springs



$$K = \sum_{i=1}^N K_i$$

Series



$$K = \frac{1}{\sum_{i=1}^N \frac{1}{k_i}}$$

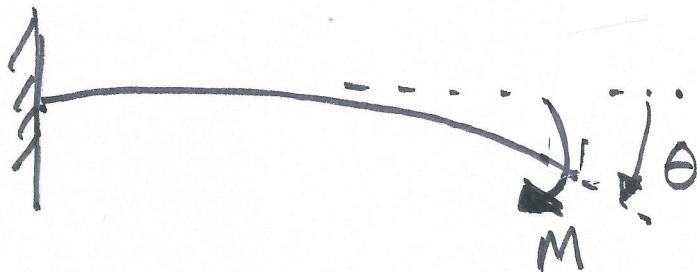
Bending (Angular stiffness)

stiffness due to

Moment that cause angular def.

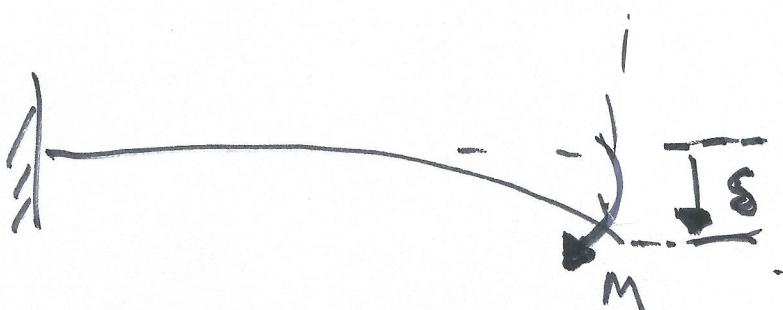
flexural rigidity def.

$$k = \frac{EI}{L}$$



$$\theta = \frac{ML}{EI}$$

Bending (Linear stiffness)



$$\delta = \frac{ML^2}{2EI} \quad k = \frac{2EI}{L^2}$$

Bending (Linear stiffness wrt Force)



$$\delta = \frac{PL^3}{3EI} \quad k = \frac{3EI}{L^3}$$

Relative Stiffness

① 1" OD round steel bar, 10" w/ tensile load

$$K = \frac{AE}{l} \Rightarrow K = 23.6 \times 10^5 \frac{\text{lb}}{\text{in}}$$

② same element w/ bending

$$K = \frac{3EI}{l^3} \Rightarrow K = 4.4 \times 10^3 \frac{\text{lb}}{\text{in}}$$

③ cantilever w/ shear load

$$K = \frac{9AG}{10l} \Rightarrow K = 81.3 \times 10^4 \frac{\text{lb}}{\text{in}}$$

④ torsion (Solid bar)

$$K_t = \frac{JG}{l} \Rightarrow K_t = 1.13 \times 10^5 \frac{\text{lb}\cdot\text{in}}{\text{rad}}$$

⑤ tube w/ circular cross section same area as the bar in ①

$$K_t = \frac{JG}{l} \Rightarrow K_t = 55.3 \times 10^5 \frac{\text{lb}\cdot\text{in}}{\text{rad}}$$

Same bar is most stiff axially, then in shear, and then in bending

tube with same amount of material is 55X stiffer

Deflection of Beams

theory of elasticity

Curvature: $\frac{1}{\rho} = \frac{M}{EI}$

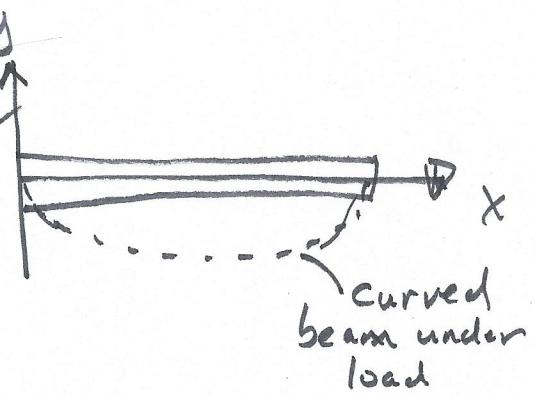
From math: $\frac{1}{\rho} = \frac{d^2y/dx^2}{[1 + (dy/dx)^2]^{3/2}}$

$$\frac{1}{\rho} = \frac{d^2y/dx^2}{[1 + (dy/dx)^2]^{3/2}}$$



$$\frac{1}{\rho} = \frac{d^2y}{dx^2}$$

$$\boxed{\frac{M}{EI} = \frac{d^2y}{dx^2}}$$



$\frac{dy}{dx}$ is small

$$\frac{dy}{dx} \approx 0$$

$$\frac{M}{EI} = \frac{d^2y}{dx^2}$$

$$\frac{V}{EI} = \frac{d^3y}{dx^3}$$

$$\frac{q}{EI} = \frac{u}{dx^4}$$

Given $q(x)$

$$V = \left\{ q(x) dx + C_V \right\}$$
$$M = \left\{ V(x) dx + C_M \right\}$$

C_V and C_M
obtained
from static
equilibrium conditions

$$EI\Theta = \left\{ M(x) dx + C_1 \right\}$$
$$y = \left\{ \Theta(x) dx + C_2 \right\}$$

$C_1 + C_2$
obtained from
boundary conditions

Multiple ways to solve these

- method of section S
- Superposition *
- moment area method
- singularity functions *
- numerically integrate

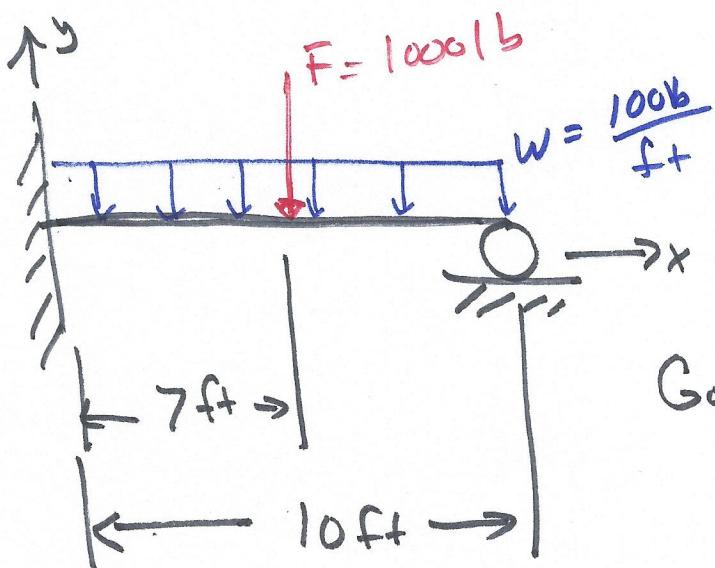
Superposition

- Many simple beam cases have been pre-solved and the results can be algebraically combined
- Table A-9 \Rightarrow look for small set of beams
- Roark's Formulas for Stress and Strain

Rules

- each effect must be linearly related to the load that produced it, e.g. $\sigma = \frac{PL}{AE}$
- a load does not create a condition that affect the result of another load
- deformation from any specific load is not large enough to appreciably alter the geometric relation of the structural system

Example: Superposition



$$E = 30 \text{ G psi}$$

$$I = 5 \text{ in}^4$$

Goal: What is
 y_{\max} ?

From Table A-9 #12

$$y_{AB} = \frac{F b x^2}{12 E I l^3} \left[3l(b^2 - l^2) + x(3l^2 - b^2) \right]$$

$$y_{BC} = y_{AB} - \frac{F(x-a)^3}{6 E I}$$

$$\#13 \quad y = \frac{w x^2}{48 E I} (l-x)(2x-3l)$$

$$y_{\text{tot AB}} = y_{AB} + y \quad \left. \right\} \text{super position}$$

$$y_{\text{tot BC}} = y_{BC} + y$$

$$y_{\text{tot AB}} = c_1 x^4 + c_2 x^3 + c_3 x^2 + c_4 x^1 + c_5$$

$$\frac{dy_{\text{tot AB}}}{dx} = 0 \quad \text{Solve for } x \text{ where } \theta = 0$$

► cubic polynomial

See the Jupyter Notebook with code to solve for the roots of this polynomial.

* Note: determine if max deflection is between $0 \leq x \leq 7$ or $7 \leq x \leq 10$ *

$$\theta_{AB}(x) = -9.26 \times 10^{-9} x^3 + 3.54 \times 10^{-6} x^2 - 2.1 \times 10^{-4} x$$

$$\theta_{BC}(x) = -9.26 \times 10^{-9} x^3 + 2.05 \times 10^{-7} x^2 + 3.5 \times 10^{-4} x - 0.024$$

$\theta_{BC} = 0 \Rightarrow$ no solutions between 7 ft and 10 ft

$x = (0, \underline{73.11}, 309.03)$ in for $\theta_{AB}(x) = 0$

↓
only valid
answer

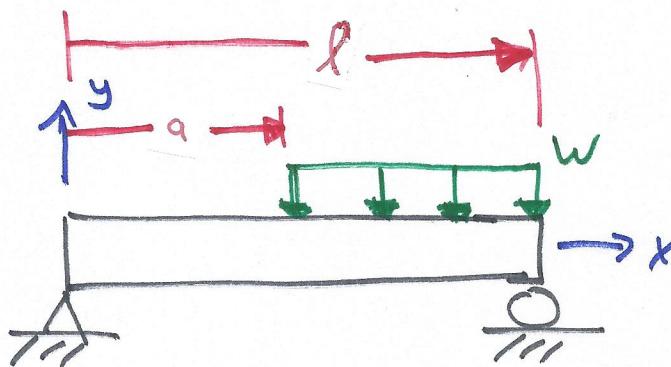
$$y_{AB}(73.11) = \boxed{y_{\max} = -0.102 \text{ "}}$$

Deflection of Beam : Singularity Functions

$$EI\theta = \int M(x) dx + C_1 \Rightarrow \theta = \frac{1}{EI} \int M(x) dx + C_1$$

$$y = \int \theta(x) dx + C_2 = \frac{1}{EI} \int \int M(x) dx + C_3$$

Example



Find

1) expressions for slope and deflection

and also 2) max deflection with following parameters

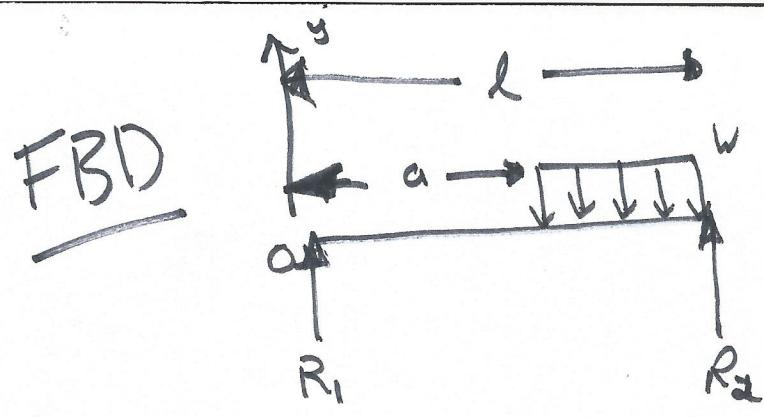
$$l = 10"$$

$$a = 4"$$

$$w = 100 \frac{lb}{in}$$

$$E = 30 \text{ MPsi}$$

$$I = .163 \text{ in}^4$$



$$q(x) = R_1 \langle x \rangle^0 - w \langle x-a \rangle^0 + w \langle x-l \rangle^0 + R_2 \langle x-l \rangle^0$$

$$V(x) = R_1 \langle x \rangle^0 - w \langle x-a \rangle^1 + w \langle x-l \rangle^1 + R_2 \langle x-l \rangle^0$$

$$M(x) = R_1 \langle x \rangle^1 - \frac{w}{2} \langle x-a \rangle^2 + \frac{w}{2} \langle x-l \rangle^2 + R_2 \langle x-l \rangle^1$$

$$V(l^+) = 0 = R_1 - w(l-a) + w(l-l) + R_2$$

$$M(l^+) = 0 = R_1 l - \frac{w}{2} (l-a)^2 + \frac{w}{2} (l-l)^2 + R_2 (l-l)$$

$$\underline{R_1 = \frac{w}{2l} (l-a)^2}$$

$$0 = \frac{w}{2l} (l-a)^2 - w(l-a) + R_2$$

$$\underline{R_2 = w(l-a) - \frac{w}{2l} (l-a)^2}$$

$$R_1 = 180 \text{ lb} \quad R_2 = 420 \text{ lb}$$

* drop discontinuities @ end of beam *

$$EI\theta = \frac{R_1}{2} \langle x \rangle^2 - \frac{W}{6} \langle x-a \rangle^3 + C_1$$

$$\theta = \frac{1}{EI} \underbrace{\left(\frac{R_1}{2} \langle x \rangle^2 - \frac{W}{6} \langle x-a \rangle^3 \right)}_{=}$$

$$EIy = \frac{R_1}{6} \langle x \rangle^3 - \frac{W}{24} \langle x-a \rangle^4 + C_1x + C_2$$

$$\underline{\underline{y(0)=0}}$$

$$\underline{\underline{y(l)=0}}$$

no deflection
@ simply supported ends

boundary conditions

$$@ x=0: 0 = C_2$$

$$@ x=l: 0 = \frac{R_1 l^3}{6} - \frac{W}{24} (l-a)^4 + C_1 l$$

Solve for C_1 and C_2

$$C_1 = \frac{W}{24l} (l-a)^4 - \frac{R_1 l^2}{6}$$

$$C_2 = 0$$

i) Expressions for θ and y :

$$\theta = \frac{1}{EI} \left[\frac{R_1}{2} \langle x \rangle^2 - \frac{W}{6} \langle x-a \rangle^3 + \frac{W}{24l} (l-a)^4 - \frac{R_1 l^2}{6} \right]$$

$$y = \frac{1}{EI} \left[\frac{R_1}{6} \langle x \rangle^3 - \frac{W}{24} \langle x-a \rangle^4 + \frac{Wx}{24l} (l-a)^4 - \frac{R_1 l^2 x}{6} \right]$$

The max deflection will be where $a < x < l$, so:

$$\Theta(x) = \frac{1}{EI} \left[\frac{R_1}{2} x^2 - \frac{W}{6} (x-a)^3 + \frac{W}{24l} (l-a)^4 - \frac{R_1 l^2}{6} \right]$$

Where is $\Theta(x) = 0$? Solve cubic polynomial for x .

$$-3.4E-6 x^3 + 5.93E-5 x^2 - 1.64E-4 x - 2.85E-4 = 0$$

$$x = (1.19, \underline{5.26}, 13.3)$$

2) max deflection $4'' < 5.26'' < 10''$ only valid answer

$$y(5.26) = -0.00176''$$