

# Computational Mechanics I

(general problem  
mechanical problem)

## mechanical problem

### point description: (lagrangian)

$$y(x,t) = x + u(x,t)$$

$$v(x,t) = \frac{d}{dt} y(x,t) = \dot{y}(x,t) = \dot{u}(x,t)$$

$$a(x,t) = \frac{d^2}{dt^2} y(x,t) = \ddot{y}(x,t) = \ddot{u}(x,t) = \ddot{v}(x,t)$$

$\Omega$ : body ;  $\partial\Omega$ : surface

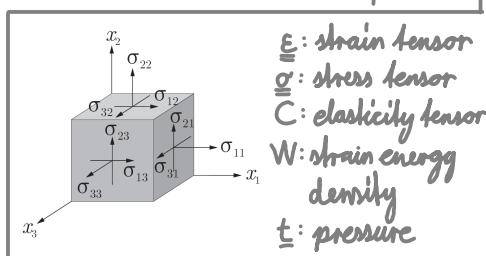
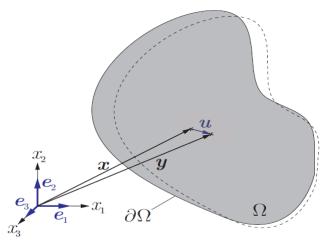
$x$ : undeformed point

$y(x,t)$ : deformed point

$u(x,t)$ : displacement field

$v(x,t)$ : velocity field

$a(x,t)$ : acceleration field



### balance of linear momentum: ( $F=m \cdot a$ )

$$\int_{\text{ext. forces}} \underline{\sigma} \cdot \underline{n} dS + \int_{\text{int. forces}} \underline{g} \cdot \underline{b} dV = \int_{\Omega} \underline{g} \cdot \underline{a} dV \quad \text{m.a}_{\text{ext}}$$

$$\text{example: } \sigma_{ij,j} + g b_i = g a_i \quad \begin{matrix} \text{---} \\ g = \text{const.}, \underline{b} = \underline{0} \end{matrix}$$

from div theorem →

$$\text{div}(\underline{\sigma}) + g \underline{b} = g \underline{a} \Leftrightarrow \sigma_{ij,j} + g b_i = g a_i;$$

(since valid  
for every  $\Omega$ )

$$\begin{aligned} a_i &= \ddot{u}_i, \sigma_{ij} = C_{ijkl} \epsilon_{kl}, \epsilon_{ij} = \frac{1}{2} (u_{ij,i} + u_{ji,i}), \\ & a = \ddot{u}, \sigma = E \epsilon, \epsilon = \frac{1}{2} (u_{xx} + u_{yy}) \end{aligned}$$

$$\underline{g} \ddot{u}_i = C_{ijkl} u_{k,l}$$

$$\underline{g} \ddot{u} = E u_{xx}$$

### boundary/initial conditions

- initial conditions: cond. at  $t=0$  (1 per t deriv.!!)

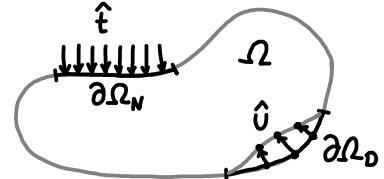
$$u(x, t=0) = u_0(x); \dot{u}(x, t=0) = \dot{u}_0(x)$$

- Dirichlet (essential) cond.: fix cond. at  $\partial\Omega_D$

$$u(x = \partial\Omega_D, t) = \hat{u}(x, t)$$

- Neumann (natural) cond.: deriv. cond. at  $\partial\Omega_N$

$$t(x = \partial\Omega_N, t) = \sigma(x, t) \cdot n(x, t) = \hat{t}(x, t)$$



### strong form: (static)

$$\sigma_{ij,j} + g b_i = 0 \text{ in } \Omega$$

$$u_i = \hat{u}_i \text{ on } \partial\Omega_D$$

$$\sigma_{ij} n_j = \hat{t}_i \text{ on } \partial\Omega_N$$

linear elasticity

$$\sigma_{ij} = C_{ijkl} \epsilon_{kl}$$

$$\epsilon_{kl} = \frac{1}{2} (u_{kl,i} + u_{li,k})$$

$$\hookrightarrow \sigma_{ij} = C_{ijkl} u_{k,l}$$

(b.c.  $C_{ijkl}$  sym.!!)

linear form

$$l[u] = (C_{ijkl} u_{k,l})_j = L = -g b_i$$

$$B_D[u] = u_i \quad = B_D \hat{u}_i$$

$$B_N[u] = (C_{ijkl} u_{k,l}) n_j = B_N \hat{t}_i$$

### weak form: (static)

$$\min I[u] = \underbrace{\int_{\Omega} W(u) dV}_{\text{tot. E.}} - \underbrace{\int_{\Omega} g b_i u_i dV}_{\text{int. E.}} - \underbrace{\int_{\partial\Omega_N} \hat{t}_i u_i dS}_{\text{ext. W.}}$$

$$\text{subj. to: } u_i = \hat{u}_i \text{ on } \partial\Omega_D$$

linear elasticity

$$W(\underline{\epsilon}) = \frac{1}{2} \epsilon_{ij} C_{ijkl} \epsilon_{kl}$$

$$W(u) = \frac{1}{2} u_{ij} C_{ijkl} u_{k,l}$$

$$\min I[u] = \int_{\Omega} \frac{1}{2} u_{ij} C_{ijkl} u_{k,l} dV - \int_{\Omega} g b_i u_i dV - \int_{\partial\Omega_N} \hat{t}_i u_i dS$$

$$\text{subj. to: } u_i = \hat{u}_i \text{ on } \partial\Omega_D$$

$$\text{linear/bilinear form: } I[u] = \frac{1}{2} B[u, u] - l[u]$$

$$\uparrow \quad l[u] = \int g b_i u_i dV + \int_{\partial\Omega_N} \hat{t}_i u_i dS$$

$$B[u, v] = \int_{\Omega} u_{ij} C_{ijkl} v_{k,l} dV$$

variation:

$$0 = \delta I[u] = \underbrace{A[u, \delta u]}_{\text{not bilinear}} - l[\delta u]$$

$$0 = \delta I[u] = \int_{\Omega} \sigma_{ij} u_{ij} \delta u_{ij} dV - \int_{\Omega} g b_i u_i \delta u_i dV - \int_{\partial\Omega_N} \hat{t}_i u_i \delta u_i dS \quad \forall \delta u$$

$$\delta W = \frac{\partial W}{\partial \epsilon_{ij}} \cdot \delta \epsilon_{ij} = \sigma_{ij} \cdot \delta u_{ij} \quad (\frac{\partial W}{\partial \epsilon_{ij}} = \sigma_{ij} = \sigma_{ji}; \delta \epsilon_{ij} = \frac{1}{2} (\delta u_{ij} + \delta u_{ji}))$$

$$\text{subj. to: } u_i = \hat{u}_i \text{ on } \partial\Omega_D \text{ and } \delta u_i = 0 \text{ on } \partial\Omega_D$$

$$0 = \delta I[u] = \underbrace{B[u, \delta u]}_{\text{not bilinear}} - l[\delta u]$$

$$0 = \delta I[u] = \int_{\Omega} u_{ij} C_{ijkl} \delta u_{k,l} dV - \int_{\Omega} g b_i \delta u_i dV - \int_{\partial\Omega_N} \hat{t}_i \delta u_i dS \quad \forall \delta u$$

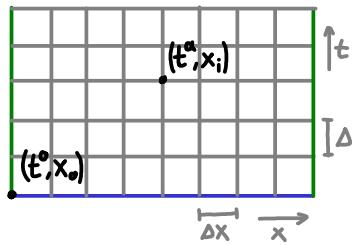
$$\text{subj. to: } u_i = \hat{u}_i \text{ on } \partial\Omega_D \text{ and } \delta u_i = 0 \text{ on } \partial\Omega_D$$

### differences strong/weak form:

( $C^k(\Omega)$ : k-times differentiable in  $\Omega$ ;  $H^k(\Omega)$ : k-times diff. + square integrable)

strong form:  $u \in C^2(\Omega) \neq$  weak form:  $u \in H^1(\Omega) \Leftrightarrow$  not the same set of  $u$  can be used to approx solution!

## Finite Differences method (approx. strong form solution by replacing derivatives with num. approx.)



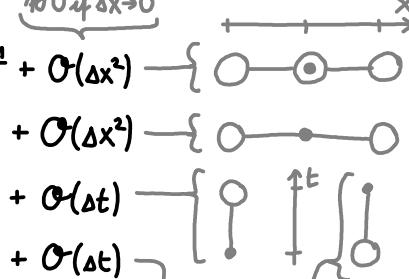
$$\begin{aligned} t^a &= \alpha \cdot \Delta t \\ x_i &= i \cdot \Delta x \\ U_i^a &= U(x_i, t^a) \\ \text{■} &: \text{initial cond.} \\ \text{■} &: \text{boundary cond.} \end{aligned}$$

(all work for t and x derivatives! derived from Taylor expansions: )

$$U(x_{i+1}, t^a) = U_i^a + \frac{\Delta x}{1!} \frac{\partial U}{\partial x} \Big|_{x_i, t^a} + \frac{\Delta x^2}{2!} \frac{\partial^2 U}{\partial x^2} \Big|_{x_i, t^a} + \frac{\Delta x^3}{3!} \frac{\partial^3 U}{\partial t^3} \Big|_{x_i, t^a} + O(\Delta x^4)$$

→ e.g.:  $U_{i+1}^a + U_{i-1}^a = \dots$  = 2nd order central diff.

$\xrightarrow{\text{to } 0 \text{ if } \Delta x \rightarrow 0}$



stencils indicate what data is necessary for using an eq.  
→ explicit: only 1 at newest t  
→ implicit: multiple at newest t

- example: heat equation (2nd centr. space, euler forw. time)

$$\begin{aligned} \dot{T} &= K T_{xx} + S \rightarrow \frac{T_i^{a+1} - T_i^a}{\Delta t} = K \frac{T_{i+1}^a - 2T_i^a + T_{i-1}^a}{\Delta x^2} + S_i^a + O(\Delta t, \Delta x^2) \\ \rightarrow T_i^{a+1} &= T_i^a + \frac{K \Delta t}{\Delta x^2} (T_{i+1}^a - 2T_i^a + T_{i-1}^a) + \Delta t S_i \end{aligned}$$

→ explicit!  
 ↳ solve iteratively

→ implicit!  
↳ solve sys of eq.

- example: heat equation (2nd centr. space, euler bkw. time)

$$\begin{aligned} \dot{T} &= K T_{xx} + S \rightarrow \frac{T_i^a - T_i^{a-1}}{\Delta t} = K \frac{T_{i+1}^a - 2T_i^a + T_{i-1}^a}{\Delta x^2} + S_i^a + O(\Delta t, \Delta x^2) \\ \rightarrow -C T_{i+1}^a + (1+2C) T_i^a - C T_{i-1}^a &\approx \Delta t S_i^a + T_i^{a-1} \quad (C = \frac{K \Delta t}{\Delta x^2}) \end{aligned}$$

$$\begin{bmatrix} \dots & & & \\ -C & (1+2C) & -C & \\ & -C & (1+2C) & -C \\ \dots & & & \end{bmatrix} \cdot \begin{bmatrix} T_1^a \\ \vdots \\ T_n^a \end{bmatrix} = \begin{bmatrix} \Delta t S_1^a + T_1^{a-1} \\ \vdots \\ \Delta t S_n^a + T_n^{a-1} \end{bmatrix}$$

- consistency: method matches solution for  $\Delta x \rightarrow 0, \Delta t \rightarrow 0$   
(true if using central diff, euler, etc.)

- boundary/initial conditions (for implicit formulations)

Dirichlet:  $\begin{bmatrix} 1 & 0 & \dots & 0 & 0 \\ \vdots & & & & \vdots \\ 0 & 0 & \dots & 0 & 1 \end{bmatrix} \cdot \begin{bmatrix} U_0 \\ \vdots \\ U_n \end{bmatrix} = \begin{bmatrix} \hat{U}_0 \\ \vdots \\ \hat{U}_n \end{bmatrix}$

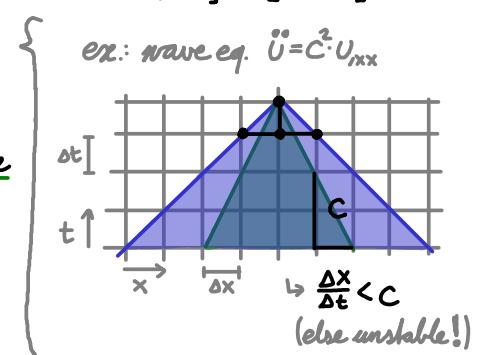
Neumann:  $\begin{bmatrix} -1 & 1 & 0 & \dots & 0 & 0 & 0 \\ 0 & \vdots & \vdots & \ddots & \vdots & \vdots & 0 \\ 0 & 0 & 0 & \dots & 0 & -1 & 1 \end{bmatrix} \cdot \begin{bmatrix} U_{-1} \\ U_0 \\ \vdots \\ U_n \\ U_{n+1} \end{bmatrix} = \begin{bmatrix} \hat{U}_{-1} \cdot \Delta x \\ \vdots \\ \hat{U}_0 \cdot \Delta x \\ I \\ U_n \\ \hat{U}_{n+1} \cdot \Delta x \end{bmatrix}$

ghost nodes

- stability with CFL condition (necessary but not sufficient!)

domain of dependence: points that can affect the value of another point

numerical domain of dependence > mathematical domain of dependence  
(from stencil shape) (encompasses) (from diff. eq.)



- stability with von Neumann analysis (usually only "good guess")

necessary+sufficient if scheme of form:  $U_j^{a+1} = \sum_{k=-?}^{+?} C_k U_{j+k}^a$  ( $C_k$ : const.)

↳ then solutions are additive!

assume error  $\epsilon$  at  $t=0$ :  $\epsilon_j^0 = e^{-ik\omega x_j}$

any error func possible by adding these up as Fourier series

assume  $\epsilon$  propagates with:  $\epsilon_j^a = z^a e^{-ik\omega x_j}$  (only true for above scheme, but generally good approx.)

amplitude = 1 since we only care for amp. changes

$|z| < 1$ : stable (noise vanishes)  
 $|z| = 1$ : stable (noise stays)  
 $|z| > 1$ : unstable (noise amplifies)

$\epsilon$  stays

$\epsilon$  grows

$\epsilon$  decays

$\epsilon$  oscillates

$\epsilon$  disappears

↳ plug in  $\epsilon_j^a$  in scheme → solve for  $z$  →  $|z| < 1$ : stable (noise vanishes) → pick  $\Delta x, \Delta t$  so scheme is stable!

$|z| = 1$ : stable (noise stays)  
 $|z| > 1$ : unstable (noise amplifies)

$\epsilon$  stays

$\epsilon$  grows

$\epsilon$  decays

$\epsilon$  disappears

- example: wave equation (2nd order central in space ; 2nd order central in time)

$$\frac{U_j^{a+1} - 2U_j^a + U_j^{a-1}}{\Delta t^2} = C^2 \frac{U_{j+1}^a - U_j^a + U_{j-1}^a}{\Delta x^2} + O(\Delta x^2, \Delta t^2) \xrightarrow{U_j^a = \epsilon_j^a e^{i\omega a t}} z - 2 + \frac{1}{z} = \left(\frac{C \Delta t}{\Delta x}\right)^2 (\cos(k \omega x) - 1) \rightarrow |z| \leq 1 \text{ stable if: } \left|\frac{\Delta t}{C \Delta x}\right| \leq 1$$

- phase error analysis: like von Neuman, but  $z = e^{i\omega a t}$  (interpret  $z^a$  as a wave in time)

## Weighted Residuals Method (approximate strong form solution with some weighted basis functions)

$$v^h(x) = \sum_{a=1}^n c^a \phi^a(x) \approx v(x) \quad \text{approximate } v(x) \text{ with } v^h(x)$$

↪  $\phi^a$ : n shape functions (e.g.:  $\phi^a(x) = x^a$ ,  $\phi^a(x) = \sin(\pi a x)$ , ...)

↪  $c^a$ : n coefficients to be determined such that  $v^h(x) \approx v(x)$

$v^h$  has to satisfy all Dirichlet BC!

choose  $\phi^a$  with  $\phi^a(x) = 0 \quad \forall a \in \{1, \dots, n\}, x \in \partial\Omega_D$

↪ e.g.:  $v^h(x) = \hat{v}_{\partial\Omega_D}(x) + \sum_{a=1}^n c^a \phi^a(x)$

linear operator notation for PDE:

$$\text{PDE: } L[v] = L$$

$$\text{BC: } B[v] = B$$

$$\begin{pmatrix} \text{linear means:} \\ L[\alpha f + \beta g] = \alpha L[f] + \beta L[g] \\ \forall \alpha, \beta \in \mathbb{R}, \forall f, g \text{ functions} \end{pmatrix}$$

1D PDE	- wave equation ( $\ddot{u} = c^2 u_{xx}$ ):	$L = \frac{\partial^2}{\partial t^2} - c^2 \frac{\partial^2}{\partial x^2}$ ; $L = 0$
1D BC	- heat equation ( $\rho c_v \dot{T} = k T_{xx} + s$ ):	$L = \rho c_v \frac{\partial}{\partial t} - k \frac{\partial^2}{\partial x^2}$ ; $L = s$
	- advection equation ( $\dot{u} + v u_x = 0$ ):	$L = \frac{\partial}{\partial t} + v \frac{\partial}{\partial x}$ ; $L = 0$
	- boundary traction ( $\hat{t} = \sigma n = E u_x$ ):	$B = E \frac{\partial}{\partial x}$ ; $B = \hat{t}$
	- boundary flux ( $\hat{q} = -k T_{xx}$ ):	$B = -k \frac{\partial}{\partial x}$ ; $B = \hat{q}$
	- Dirichlet ( $u = \hat{u}$ ):	$B = 1$ ; $B = \hat{u}$

$$\text{residuals: } r_n = L[v^h] - L \quad (\text{for } x \in \Omega) \quad (\text{error to solution of PDE inside } \Omega)$$

$$(0 \text{ if } v^h = v) \quad r_{\partial\Omega_j} = B_j[v^h] - B_j \quad (\text{for } x \in \partial\Omega_j) \quad (\text{error to Neumann BCs (j=1,..,k) on } \partial\Omega_j)$$

$$\text{total residuals: } R_a = \int_{\Omega} r_n W_a dV + \xi^2 \sum_{j=1}^k \int_{\partial\Omega_j} r_{\partial\Omega_j} w_a dS$$

↪ use  $R_a = 0 \quad \forall a \in \{1, \dots, n\}$  to get n eq. and solve for  $c^a$ !

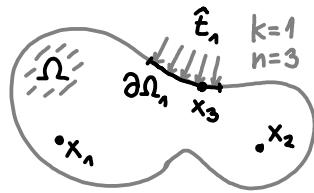
( $a = 1, \dots, n$  weighted sums of residuals)

( $W_a, w_a$ : weighting func.;  $\xi$ : arbitrary const.)

### • collocation method:

$$x_a = \begin{cases} a \leq n-k: \text{some } x \in \Omega \\ \text{else: some } x \in \partial\Omega_{a-(n-k)} \end{cases} \rightarrow W_a = \begin{cases} a \leq n-k: \delta(x-x_a) \\ \text{else: 0} \end{cases}; w_a = \begin{cases} a \leq n-k: 0 \\ \text{else: } \delta(x-x_a) \end{cases}$$

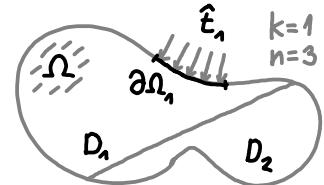
$$\hookrightarrow r_n(x_i) = 0 \quad (\text{for } i = 1, \dots, n-k); \quad r_{\partial\Omega_{a-(n-k)}}(x_i) = 0 \quad (\text{for } i = n-k+1, \dots, n) \rightarrow \text{solve for } c^a$$



### • subdomain method:

$$D_a = \begin{cases} a \leq n-k: \text{some } D \subset \Omega \\ \text{else: } \partial\Omega_{a-(n-k)} \end{cases} \rightarrow W_a = \begin{cases} a \leq n-k: 1 \text{ on } x \in D_a \\ \text{else: 0} \end{cases}; w_a = \begin{cases} a \leq n-k: 0 \\ \text{else: } 1 \text{ on } x \in D_a \end{cases}$$

$$\hookrightarrow \int_{\Omega} r_n dV = 0 \quad (\text{for } i = 1, \dots, n-k); \quad \int_{\partial\Omega_j} r_{\partial\Omega_j} dS = 0 \quad (\text{for } j = 1, \dots, k) \rightarrow \text{solve for } c^a$$



### • least square method:

$$E = \int_{\Omega} r_n^2 dV + \xi^2 \sum_{j=1}^k \int_{\partial\Omega_j} r_{\partial\Omega_j}^2 dS \quad (\text{square error}) \rightarrow c^a = \arg \min E \quad (\text{least square coeff.})$$

$$\hookrightarrow \frac{\partial E}{\partial c^a} = \underbrace{\int_{\Omega} r_n \frac{\partial r_n}{\partial c^a} dV}_{= L[\phi^a] (= W_a)} + \xi^2 \sum_{j=1}^k \underbrace{\int_{\partial\Omega_j} r_{\partial\Omega_j} \frac{\partial r_{\partial\Omega_j}}{\partial c^a} dS}_{= B_j[\phi^a] (= w_a)} = 0 = R_a$$

$$\rightarrow \sum_{b=1}^n \left( \underbrace{\int_{\Omega} L[\phi^a] L[\phi^b] dV}_{\sum_{b=1}^n K^{ab}} + \xi^2 \sum_{j=1}^k \underbrace{\int_{\partial\Omega_j} B_j[\phi^a] B_j[\phi^b] dS}_{\cdot C^b} \right) \cdot C^b = \underbrace{\int_{\Omega} L \cdot L[\phi^a] dV}_{F^a} + \xi^2 \sum_{j=1}^k \underbrace{\int_{\partial\Omega_j} B_j \cdot B_j[\phi^a] dS}_{\underline{K} \subseteq F} \quad (\text{for } a = 1, \dots, n)$$

$$\hookrightarrow \underline{K} \subseteq F \rightarrow \underline{C} = \underline{K}^{-1} F \quad (\text{for } a = 1, \dots, n)$$

### • Galerkin's method (for linear momentum eq.):

$$\text{principle of virtual work: } \int_{\Omega} \sigma_{ij} v_i v_j dV = \int_{\Omega} g b_i v_i dV + \int_{\partial\Omega} \hat{t}_i v_i dS \quad (v(x): \text{any admissible virtual displacement})$$

$$\hookrightarrow \int_{\Omega} (\sigma_{ij,j} + g b_i) v_i dV + \int_{\partial\Omega} (\hat{t}_i - \sigma_{ij,j} n_j) v_i dS = 0 \quad (\text{for this to be true for all allowable } v(x): \sigma_{ij,j} = g b_i \text{ and } \hat{t}_i = \sigma_{ij,j} n_j)$$

• higher dimensions:  $v \rightarrow \underline{v}$  e.g.  $(u_x, u_y, u_z)$   $x \rightarrow \underline{x}$  e.g.  $(x, y, z, t)$   $c^a \phi^a \rightarrow \underline{c}^a \underline{\phi}^a$

• example: static bar under centripetal accel.  $\omega^2 x$

$$EQ: \alpha_{ij,j} + g b_i = g a_i \quad 10, b = \omega^2 x, a = 0 \rightarrow \alpha_{xx} + g \omega^2 x = 0 \rightarrow \alpha = E \varepsilon = E u_{xx} \rightarrow u_{xx} = -g \omega^2 / E \cdot x \rightarrow L = E \frac{\partial^2}{\partial x^2}; L = -g \omega^2 x$$

$$BC: u(0) = 0 \text{ and } F(L) = AE u_{xx}(L) = m \omega^2 L \rightarrow B_1 = AE \frac{\partial}{\partial x}; B_2 = m \omega^2 x$$

approx func.:  $u(x) \approx u^h(x) = \sum_{a=0}^3 c^a x^a = c^1 x + c^2 x^2 + c^3 x^3$  (e.g. polynom 3rd order) ( $c^0 = 0$  for BC  $u(0) = 0$ )

$$\hookrightarrow \text{residuals: } r_n = L[u^h] - L = E(2c^2 + 6c^3 x) + g \omega^2 x$$

$$r_{\alpha\alpha_1} = B[u^h] - B = AE(c^1 + 2c^2 x + 3c^3 x^2) - m \omega^2 x$$

- collocation method:  $x_1 = L/3, x_2 = 2L/3, x_3 = L$

$$r_n(x_1) = 0, r_n(x_2) = 0, r_{\alpha\alpha_1}(x_3) = 0$$

$$\begin{bmatrix} 0 & 2E & 2EL \\ 0 & 2E & 2EL \\ EA & 2EAL & 3EAL^2 \end{bmatrix} \cdot \begin{bmatrix} c^1 \\ c^2 \\ c^3 \end{bmatrix} = \begin{bmatrix} -g \omega^2 L/3 \\ -2g \omega^2 L/3 \\ m \omega^2 L \end{bmatrix}$$

$\rightarrow$  solve for  $[c^1, c^2, c^3]^T$  ....

- least square method: ( $\xi^2 = 1/L$  so units match)

$$\underline{K} = \begin{bmatrix} AE^2/L & 2AE^2 & 3AE^2L \\ 2AE^2 & 8AE^2L & 12AE^2L^2 \\ 3AE^2L & 12AE^2L^2 & 21AE^2L^3 \end{bmatrix} \quad \underline{F} = \begin{bmatrix} Em \omega^2 \\ EL \omega^2(2m - AL) \\ EL^2 \omega^2(3m - 2AL) \end{bmatrix}$$

$\rightarrow$  solve  $\underline{K} \cdot (c^1, c^2, c^3)^T = \underline{F}$  for  $(c^1, c^2, c^3)^T$

Variational Method (approx. by finding function with minimal "energy" = weak form)

energy functional:  $I[u]: \mathcal{U} \rightarrow \mathbb{R}$  (assigning "energy"  $\in \mathbb{R}$  to all functions  $u \in \mathcal{U}$ )

$$\left( \begin{array}{l} \text{1D example: } I[u] = \int_0^1 u_{xx}^2 dx; \mathcal{U} = H^0(0,1) \\ \text{u has to be square integrable in } (0,1) \end{array} \right) \quad \left( \begin{array}{l} \text{1D Sobolev space: } H^k(\Omega) = \{ u(x): \Omega \rightarrow \mathbb{R} \text{ such that } \|u(x)\|_{H^k(\Omega)} < \infty \} \\ \text{1D Sobolev norm: } \|u(x)\|_{H^k(\Omega)} = \sqrt{\int_{\Omega} u^2 dx + \int_{\Omega} (du/dx)^2 dx + \dots + \int_{\Omega} (d^k u / dx^k)^2 dx} \end{array} \right)$$

variation:  $\delta u \in \mathcal{U}_0 = \{ u \in \mathcal{U} \text{ with } u=0 \text{ on } \partial\Omega_{\text{Dirichlet}} \}$  (arbitrary perturbation)

first variation:  $\delta I[u] = \lim_{\varepsilon \rightarrow 0} \frac{I[u + \varepsilon \delta u] - I[u]}{\varepsilon} = \frac{d}{d\varepsilon} I[u + \varepsilon \delta u]|_{\varepsilon=0}$  (analog to first deriv.)

usefull rules:  $\delta(\alpha_1 I_1 + \alpha_2 I_2) = \alpha_1 \delta I_1 + \alpha_2 \delta I_2$ ;  $\delta(I_1 I_2) = \delta I_1 \cdot I_2 + I_1 \delta I_2$ ;  $\delta \frac{du}{dx} = \frac{d}{dx} (\delta u)$

$$\delta \int_{\Omega} u dx = \int_{\Omega} \delta u dx; \quad \delta I[u, v, \dots] = \frac{d}{d\varepsilon} I[u + \varepsilon \delta u, v + \varepsilon \delta v, \dots]|_{\varepsilon=0}; \quad I[u + \delta u] = \sum_{i=0}^{\infty} \frac{1}{i!} \delta^i I[u]$$

$$\text{common form: } I[u] = \int_{\Omega} f(u, \nabla u, \nabla^2 u, \dots) dV \rightarrow \delta I[u] = \int_{\Omega} \left( \frac{\partial f}{\partial u} \delta u + \frac{\partial f}{\partial \nabla u} \delta \nabla u + \frac{\partial f}{\partial \nabla^2 u} \delta \nabla^2 u + \dots \right) dV$$

$$= \int_{\Omega} f(u_i, u_{i,j}, u_{i,jk}, \dots) dV \rightarrow = \int_{\Omega} \left( \frac{\partial f}{\partial u_i} \delta u_i + \frac{\partial f}{\partial u_{i,j}} \delta u_{i,j} + \frac{\partial f}{\partial u_{i,jk}} \delta u_{i,jk} + \dots \right) dV$$

general receipt: find  $I[u] \rightarrow$  calculate  $\delta I[u], \delta^2 I[u] \rightarrow$  find all  $u_s$  with  $\delta I[u] = 0 \forall \delta u$

$\rightarrow$  keep only  $u_s$  with  $u_s \in \mathcal{U} \rightarrow$  keep only  $u_s$  with  $\delta^2 I[u_s] > 0 \forall \delta u$

true  $u$  can be any of  $u_s$  or some  $u$  on border of  $\mathcal{U}$ !

bilinear + linear form: if  $I[u]$  can be written as  $I[u] = \frac{1}{2} \mathcal{B}[u, u] + \mathcal{L}[u]$  then  $\delta I[u] = \mathcal{B}[u, \delta u] - \mathcal{L}[\delta u]$

$\hookrightarrow$  no need to check  $\delta^2 I[u] > 0$   $\left( \begin{array}{l} \text{• linear form } \mathcal{L}[a]: \mathcal{L}[\alpha_1 a_1 + \alpha_2 a_2] = \alpha_1 \mathcal{L}[a_1] + \alpha_2 \mathcal{L}[a_2] \quad \forall \alpha_1, \alpha_2 \in \mathbb{R} \quad \forall a_1, a_2 \in \text{func.} \\ \text{• bilinear form } \mathcal{B}[a, b]: \text{ linear in } a, b \text{ and } \mathcal{B}[a+c, b] = \mathcal{B}[a, b] + \mathcal{B}[c, b] \end{array} \right)$

$$\bullet \text{ func. discretization: } (u^a, v^a: \text{const.}; N^a(x): \text{shape func.}) \rightarrow \text{solve } \delta I[u^h] = \mathcal{B}[u^h, v^h] - \mathcal{L}[u^h] = 0 \quad \forall v^h$$

$$\left. \begin{array}{l} u^h(x) = \sum_{a=1}^n u^a N^a(x) \approx u(x) \quad (\text{subj. to: } u^h = \hat{u} \text{ on } \partial\Omega_D) \\ v^h(x) = \sum_{a=1}^n v^a N^a(x) \approx \delta u(x) \quad (\text{subj. to: } v^h = 0 \text{ on } \partial\Omega_D) \end{array} \right\} \rightarrow \sum_{a=1}^n u^a \mathcal{B}[N^a, N^b] = \mathcal{L}[N^b] \quad \forall b = 1, \dots, n$$

$$\rightarrow \underline{K} \cdot \underline{u} = \underline{F} \text{ with } K^{ab} = \mathcal{B}[N^a, N^b], F^a = \mathcal{L}[N^a]$$

• mech. prob: static 2D/3D linear elasticity (function discretization)

3D Voigt notation: (write  $\underline{\sigma}, \underline{\varepsilon}$  as vectors  $\underline{\underline{\sigma}}, \underline{\underline{\varepsilon}}$ )

$$\underline{\sigma}_{ij} = \underline{C}_{ijkl} \cdot \underline{\varepsilon}_{kl} \leftrightarrow \underline{\underline{\sigma}} = \underline{\underline{E}} \cdot \underline{\underline{\varepsilon}}$$

$$\begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{yx} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{zx} & \sigma_{zy} & \sigma_{zz} \end{bmatrix} = \underline{\underline{C}} \cdot \begin{bmatrix} \varepsilon_{xx} & \varepsilon_{xy} & \varepsilon_{xz} \\ \varepsilon_{yx} & \varepsilon_{yy} & \varepsilon_{yz} \\ \varepsilon_{zx} & \varepsilon_{zy} & \varepsilon_{zz} \end{bmatrix}$$

$$\begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ 2\sigma_{yz} \\ 2\sigma_{xz} \\ 2\sigma_{xy} \end{bmatrix} = \underline{\underline{E}} \cdot \begin{bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \varepsilon_{zz} \\ 2\varepsilon_{yz} \\ 2\varepsilon_{xz} \\ 2\varepsilon_{xy} \end{bmatrix}$$

from:  $\varepsilon_{xx} = \frac{1}{E} \cdot (\sigma_{xx} - \nu(\sigma_{yy} + \sigma_{zz}))$ , ...,  $\varepsilon_{yz} = \frac{1+\nu}{E} \sigma_{xy}$ , ...

with:  $\underline{\underline{E}} = \frac{E}{(1+\nu)(1-2\nu)} \cdot \begin{bmatrix} 1-\nu & \nu & \nu & 0 & 0 & 0 \\ \nu & 1-\nu & \nu & 0 & 0 & 0 \\ \nu & \nu & 1-\nu & 0 & 0 & 0 \\ 0 & 0 & 0 & \frac{1}{2}-\nu & 0 & 0 \\ 0 & 0 & 0 & 0 & \frac{1}{2}-\nu & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{1}{2}-\nu \end{bmatrix}$

( $E$ : Young's modulus)  
 $\nu$ : Poisson's ratio

$$\varepsilon_{ij} = \frac{1}{2} (u_{i,j} + u_{j,i}) \approx \frac{1}{2} (u_i^h + u_j^h) \leftrightarrow \underline{\underline{\varepsilon}} \approx \underline{\underline{B}} \cdot \underline{\underline{U}}^h$$

$u_i^h$  approx of  $u_i$ ; with:  
 $u_i \approx u_i^h = \sum_{a=1}^n u_i^a N^a$   
 $\uparrow u_i^a$ : constants  
 $\downarrow N^a$ : basis functions

$$\begin{bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \varepsilon_{zz} \\ 2\varepsilon_{yz} \\ 2\varepsilon_{xz} \\ 2\varepsilon_{xy} \end{bmatrix} \approx \underline{\underline{B}} \cdot \begin{bmatrix} U_x^h \\ U_y^h \\ U_z^h \\ U_x^h \\ U_y^h \\ U_z^h \end{bmatrix}$$

with:  $\underline{\underline{B}} = \begin{bmatrix} N_{,x}^1 & \dots & N_{,x}^n & \dots & N_{,x}^n & \dots \\ \vdots & \vdots & \vdots & \vdots & \vdots & \vdots \\ N_{,y}^1 & \dots & N_{,y}^n & \dots & N_{,y}^n & \dots \\ N_{,z}^1 & \dots & N_{,z}^n & \dots & N_{,z}^n & \dots \\ \vdots & \vdots & \vdots & \vdots & \vdots & \vdots \\ N_{,y}^1 & N_{,x}^1 & \dots & N_{,y}^n & N_{,x}^n & \dots \\ N_{,y}^2 & N_{,x}^2 & \dots & N_{,y}^n & N_{,x}^n & \dots \\ \vdots & \vdots & \vdots & \vdots & \vdots & \vdots \\ N_{,y}^n & N_{,x}^n & \dots & N_{,y}^n & N_{,x}^n & \dots \end{bmatrix}$

$$W = \frac{1}{2} \varepsilon_{ij} \underline{C}_{ijkl} \varepsilon_{kl} \leftrightarrow W = \frac{1}{2} \underline{\underline{\varepsilon}}^T \underline{\underline{E}} \underline{\underline{\varepsilon}} \approx \frac{1}{2} \underline{\underline{U}}^h{}^T \underline{\underline{B}}^T \underline{\underline{E}} \underline{\underline{B}} \underline{\underline{U}}^h$$

$$u_i \approx u_i^h \leftrightarrow \underline{u} \approx \underline{U}^h = \underline{\underline{C}} \underline{\underline{U}}^h \quad \text{with: } \underline{\underline{C}} = \begin{bmatrix} N^1 & N^1 & \dots & N^n & N^n & \dots \\ \vdots & \vdots & \vdots & \vdots & \vdots & \vdots \\ N^1 & N^1 & \dots & N^n & N^n & \dots \end{bmatrix}$$

2D Voigt notation: (analog to 3D)

$$\underline{\underline{\varepsilon}} = [\varepsilon_{xx}, \varepsilon_{yy}, 2\varepsilon_{xy}]^T ; \quad \underline{\underline{\sigma}} = [\sigma_{xx}, \sigma_{yy}, \sigma_{xy}]^T$$

$$\underline{U}^h = [U_x^h, U_y^h, U_x^h, U_y^h, \dots, U_x^h, U_y^h]^T$$

$$\underline{\underline{B}} = \begin{bmatrix} N_{,x}^1 & \dots & N_{,x}^n & \dots & N_{,x}^n & \dots \\ \vdots & \vdots & \vdots & \vdots & \vdots & \vdots \\ N_{,y}^1 & \dots & N_{,y}^n & \dots & N_{,y}^n & \dots \\ N_{,y}^2 & \dots & N_{,y}^n & \dots & N_{,y}^n & \dots \\ \vdots & \vdots & \vdots & \vdots & \vdots & \vdots \\ N_{,y}^n & \dots & N_{,y}^n & \dots & N_{,y}^n & \dots \end{bmatrix}$$

$$\hookrightarrow W \approx \frac{1}{2} \underline{U}^h{}^T \underline{\underline{B}}^T \underline{\underline{E}} \underline{\underline{B}} \underline{U}^h ; \quad \underline{u} \approx \underline{\underline{C}} \underline{U}^h$$

• plane strain: ( $\varepsilon_{zz}=0, \sigma_{zz}\neq 0$ ) ( $\varepsilon_{yz}=\varepsilon_{xz}=\sigma_{yz}=\sigma_{xz}=0$ )

$$\underline{\underline{E}} = \frac{E}{(1+\nu)(1-2\nu)} \cdot \begin{bmatrix} 1-\nu & \nu & 0 \\ \nu & 1-\nu & 0 \\ 0 & 0 & \frac{1}{2}-\nu \end{bmatrix} \quad (\text{from 3D } \underline{\underline{E}} \text{ and above cond.})$$

$$\hookrightarrow \text{recover } \sigma_{zz} \text{ from } \underline{\underline{\varepsilon}}: \sigma_{zz} = \frac{E}{(1+\nu)(1-2\nu)} \cdot (v\varepsilon_{xx} + v\varepsilon_{yy})$$

• plane stress: ( $\varepsilon_{zz}\neq 0, \sigma_{zz}=0$ ) ( $\varepsilon_{yz}=\varepsilon_{xz}=\sigma_{yz}=\sigma_{xz}=0$ )

$$\underline{\underline{E}} = \frac{E}{1-\nu^2} \cdot \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \quad (\text{from 3D } \underline{\underline{E}} \text{ and above cond. and substitute: } \varepsilon_{zz} = \dots \leftarrow \sigma_{zz} = E_s \underline{\underline{\varepsilon}} = 0)$$

$$\hookrightarrow \text{recover } \varepsilon_{zz} \text{ from } \underline{\underline{\varepsilon}}: \varepsilon_{zz} = -\frac{\nu}{\nu-1} \varepsilon_{xx} - \frac{\nu}{\nu-1} \varepsilon_{yy}$$

approx.:  $\underline{u} \approx \underline{u}^h(\underline{x}) = \sum_{a=1}^n \underline{u}^a N^a(\underline{x}) = \underline{\underline{C}} \underline{U}^h$  (subj. to:  $\underline{u}^h = \hat{\underline{u}}$  on  $\partial\Omega_D$ )  $\left( \underline{u}^a, \underline{v}^a : \text{constants} \right)$   
 $\delta \underline{u} \approx \underline{v}^h(\underline{x}) = \sum_{a=1}^n \underline{v}^a N^a(\underline{x}) = \underline{\underline{C}} \underline{V}^h$  (subj. to:  $\underline{v}^h = 0$  on  $\partial\Omega_D$ )  $\left( N^a : \text{shape functions} \right)$

$$\min I[\underline{u}^h] = \int_{\Omega} W(\underline{u}^h) dV - \int_{\Omega} \underline{S} \underline{u}^h{}^T \underline{b} dV - \int_{\partial\Omega_N} \underline{u}^h{}^T \underline{\underline{\varepsilon}} \underline{\underline{t}} dS = \frac{1}{2} \underline{U}^h{}^T \int_{\Omega} \underline{\underline{B}}^T \underline{\underline{E}} \underline{\underline{B}} dV \underline{U}^h - \underline{U}^h{}^T \left( \int_{\Omega} \underline{S} \underline{\underline{C}}^T \underline{b} dV + \int_{\partial\Omega_N} \underline{\underline{C}}^T \underline{\underline{t}} dS \right)$$

$$0 = \delta I[\underline{u}^h] = \underbrace{\dots}_{\text{wavy line}} = \underline{V}^h{}^T \underbrace{\int_{\Omega} \underline{\underline{B}}^T \underline{\underline{E}} \underline{\underline{B}} dV \underline{U}^h}_{F_{\text{int}}} - \underline{V}^h{}^T \underbrace{\left( \int_{\Omega} \underline{S} \underline{\underline{C}}^T \underline{b} dV + \int_{\partial\Omega_N} \underline{\underline{C}}^T \underline{\underline{t}} dS \right)}_{F_{\text{ext}}} \quad \forall \underline{V}^h$$

solve  $\underline{\underline{K}} \underline{U}^h = \underline{F}$   $\leftarrow \begin{array}{c} \underline{\underline{K}} \\ F_{\text{int}} \end{array} \quad \begin{array}{c} \underline{F} \\ F_{\text{ext}} \end{array}$

• mech. prob: static 2D/3D non-linear elasticity (function discretization)

same approx  $\underline{u}^h(\underline{x}), \underline{v}^h(\underline{x})$ .  $\underline{\underline{B}}, \underline{\underline{C}}$  of Voigt notation also valid for non-linear!

weak form:  $\min I[\underline{u}^h] = \int_{\Omega} W(\underline{u}^h) dV - \int_{\Omega} \underline{S} \underline{u}^h{}^T \underline{b} dV - \int_{\partial\Omega_N} \underline{u}^h{}^T \underline{\underline{\varepsilon}} \underline{\underline{t}} dS$

variational:  $0 = \delta I[\underline{u}^h] = \int_{\Omega} \delta \underline{S} \underline{v}^h(\underline{u}^h) \cdot \underline{\underline{\sigma}}(\underline{u}^h) dV - \int_{\Omega} \underline{S} \delta \underline{u}^h{}^T \underline{b} dV - \int_{\partial\Omega_N} \delta \underline{u}^h{}^T \underline{\underline{\varepsilon}} \underline{\underline{t}} dS \quad \forall \underline{V}^h \quad (\delta W = \delta \underline{\underline{\varepsilon}} \cdot \partial W / \partial \underline{\underline{\varepsilon}} = \delta \underline{\underline{\varepsilon}} \cdot \underline{\underline{\sigma}})$

$$0 = \underline{V}^h{}^T \underbrace{\int_{\Omega} \underline{\underline{B}}^T \underline{\underline{\sigma}}(\underline{u}^h) dV}_{F_{\text{int}}(\underline{u}^h)} - \underline{V}^h{}^T \underbrace{\left( \int_{\Omega} \underline{S} \underline{\underline{C}}^T \underline{b} dV + \int_{\partial\Omega_N} \underline{\underline{C}}^T \underline{\underline{t}} dS \right)}_{F_{\text{ext}}} \quad \forall \underline{V}^h \quad (\delta \underline{\underline{\varepsilon}} \cdot \underline{\underline{\sigma}} = \delta \underline{\underline{\varepsilon}} \cdot \underline{\underline{\sigma}} = \underline{V}^h \underline{\underline{B}}^T \underline{\underline{\sigma}}) \quad (\delta \underline{u} = \underline{\underline{C}} \underline{V}^h)$$

$\hookrightarrow$  solve  $\underline{f}(\underline{U}^h) = F_{\text{int}}(\underline{U}^h = \underline{\underline{C}} \underline{U}^h) - F_{\text{ext}} = 0$   
using numerical solvers!

$\underline{\underline{I}}: \text{incremental stiffness matrix}$        $\underline{\underline{\tilde{E}}}(\underline{\underline{\varepsilon}}) \text{ is non-linear equivalent of } \underline{\underline{E}}$

$$\underline{\underline{I}}(\underline{U}^h) = \partial \underline{f} / \partial \underline{U}^h(\underline{U}^h) = \int_{\Omega} \underline{\underline{B}}^T \partial \underline{\underline{\sigma}} / \partial \underline{\underline{\varepsilon}} \cdot \partial \underline{\underline{\varepsilon}} / \partial \underline{U}^h dV = \int_{\Omega} \underline{\underline{B}}^T \underline{\underline{\tilde{E}}}(\underline{\underline{\varepsilon}} = \underline{\underline{B}} \underline{U}^h) \underline{B} dV$$

# Finite Element Method FEM (= variational method with piecewise-polynomial shape functions)

- 1) split a body into elements with some space filling shape (can be distorted)
- 2) choose a polynomial interpolation order  $q$  and # of shape functions  $n$
- 2) pick  $n$  node points  $\underline{x}^a$  per element on vertices, edges, surfaces or inside element  
(nodes not inside element can be shared with adjacent elements to enforce continuity)
- 3) pick  $n$  polynomial shape functions  $N_e^a(\underline{x})$  of order  $p$  on every element, such that:

$$N_e^a(x^b) = \begin{cases} 1 & \text{if } a=b \\ 0 & \text{if } a \neq b \end{cases} \quad \forall a,b \in \{1,..,n\} \rightarrow U^h(x^a) = U^a: \text{coeff. for } N^a = \text{value at } x^a !$$

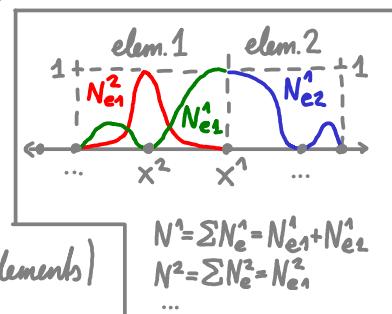
1D: line segment
2D: squares, triangle
3D: cubes, tetrahedra,..
1D: $n \geq q+1$
2D: $n \geq (q+1)(q+2)/2$
3D: $n \geq (q+1)(q+2)(q+3)/6$

4) write down variational method formula  $K_e U = F_e$  for every element

5) combine element formulas into global formula  $K = \sum K_e, F = \sum F_e$

(b.c.: actual shape func. of shared node  $N^a = \sum N_e^a$  union of shape funcs. from sharing elements)

6) enforce Dirichlet BC by fixing some  $U^a$  in  $U$       7) solve  $KU=F$



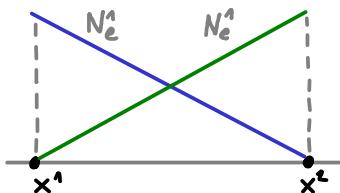
## • polynomial basis functions: $1D (n=q+1) \quad 2D (n=(q+1)(q+2)/2) \quad 3D (n=(q+1)(q+2)(q+3)/6) \quad (n: \# \text{ of basis func.})$

completeness	$q=0$ : $\{1\}$	$q=1$ : $\{1, x\}$	$q=2$ : $\{1, x, x^2\}$
order	$q=0$ : $\{1\}$	$q=1$ : $\{1, x, y\}$	$q=2$ : $\{1, x, y, x^2, y^2\}$
	$q=0$ : $\{1\}$	$q=1$ : $\{1, x, y, z\}$	$q=2$ : $\{1, x, y, z, xy, xz, yz, x^2, y^2, z^2\}$

sufficient completeness:  $q \geq p$  ( $p$ : order of highest derivative in equation)  $\rightarrow$  mech prob. weak form:  $p=1 \rightarrow q \geq 1$

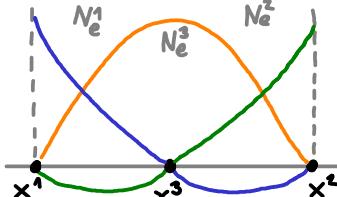
## • 1D polynomial shape functions:

$$n=2, q=1, N_e^a \in \{1, x, x^2\}$$



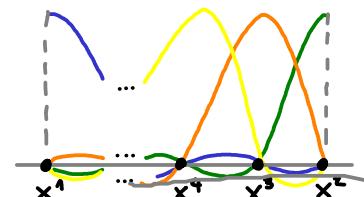
$$N_e^1 = 1 - \frac{x-x^1}{\Delta x}; \quad N_e^2 = \frac{x-x^1}{\Delta x}$$

$$n=3, q=2, N_e^a \in \{1, x, x^2, x^3\}$$



$$N_e^1(x) = ?; \quad N_e^2(x) = ?; \quad N_e^3(x) = ?$$

$$q=2, n=q+1, N_e^a \in \{1, x, x^2, x^3\}$$



$$N_e^a(x) = \frac{(x-x^1) \dots (x-x^{a-1})(x-x^{a+1}) \dots (x-x^n)}{(x^a-x^1) \dots (x^a-x^{a-1})(x^a-x^{a+1}) \dots (x^a-x^n)}$$

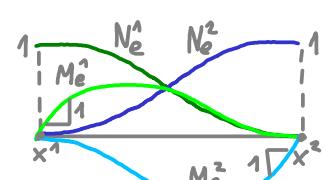
Hermite polynomials: (places nodes on derivatives of  $U$ ) e.g.  $q=3, n=4$ :

$$U^h = U^1 N_e^1 + U^2 N_e^2 + \theta^1 M_e^1 + \theta^2 M_e^2$$

$$\hookrightarrow U^h(x^1) = U^1; \quad U^h(x^2) = U^2; \quad U_{,x}^h(x^1) = \theta^1; \quad U_{,x}^h(x^2) = \theta^2$$

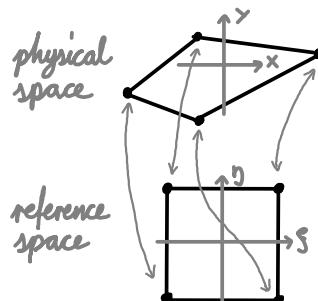
$$N_e^1 = 1 - 3\left(\frac{x}{L}\right)^2 + 2\left(\frac{x}{L}\right)^3; \quad N_e^2 = 3\left(\frac{x}{L}\right)^2 - 2\left(\frac{x}{L}\right)^3$$

$$M_e^1 = x\left(1 - \frac{x}{L}\right)^2; \quad M_e^2 = \frac{x^2}{L}\left(\frac{x}{L} - 1\right)$$



## • 2D polynomial shape functions:

irregular element shapes  $\rightarrow$  isoparametric mapping:



point:  $\underline{x} = (x, y)$

node:  $\underline{x}^a = (x_a^x, x_a^y)$  ✓

shape func:  $N_e^a(\underline{x})$  ?

point:  $\underline{\xi} = (\xi, \eta)$

node:  $\underline{\xi}^a = (\xi_a^x, \xi_a^y)$  ✓

shape func:  $N_e^a(\underline{\xi})$  ✓

• define an "easy" reference element in ref. space  $\underline{\xi}$  with easy node pos  $\underline{\xi}^a$  and easy shape func  $N_e^a(\underline{\xi})$

• write a mapping  $\underline{x} = \phi(\underline{\xi})$  such that the known node points  $\underline{\xi}^a \leftrightarrow \underline{x}^a$  match:  $\underline{x} = \phi(\underline{\xi}) = \sum_{a=1}^n \underline{\xi}^a N_e^a(\underline{\xi})$

$\hookrightarrow$  approx. solution of phys. elem.:  $U^h(\underline{x}) = \sum_{a=1}^n U^a N_e^a(\underline{x})$  becomes:

$$U^h(\underline{x})|_{\underline{x}=\phi(\underline{\xi})} = \sum_{a=1}^n U^a N_e^a(\underline{x})|_{\underline{x}=\phi(\underline{\xi})} = \sum_{a=1}^n U^a N_e^a(\underline{\xi}) = U^h(\underline{\xi})$$

$\hookrightarrow$  same  $U^a$  in both elements  $\rightarrow$  solve for ref. elem., since easier!

when variational method asks for derivatives of  $N_{e,i}^a(\underline{\xi})$  (e.g.:  $N_{e,x}^a(\underline{\xi}) = ?$ ):

$$\begin{bmatrix} N_{e,S}^a(\underline{\xi}) \\ N_{e,J}^a(\underline{\xi}) \end{bmatrix} = \begin{bmatrix} N_{e,x}^a(\underline{\xi}) \cdot x_{x,S}(\underline{\xi}) + N_{e,y}^a(\underline{\xi}) \cdot x_{y,S}(\underline{\xi}) \\ N_{e,x}^a(\underline{\xi}) \cdot x_{x,J}(\underline{\xi}) + N_{e,y}^a(\underline{\xi}) \cdot x_{y,J}(\underline{\xi}) \end{bmatrix} = \underbrace{\begin{bmatrix} x_{x,S}(\underline{\xi}) & x_{y,S}(\underline{\xi}) \\ x_{x,J}(\underline{\xi}) & x_{y,J}(\underline{\xi}) \end{bmatrix}}_{J} \cdot \begin{bmatrix} N_{e,x}^a(\underline{\xi}) \\ N_{e,y}^a(\underline{\xi}) \end{bmatrix} = J \cdot \begin{bmatrix} N_{e,x}^a(\underline{\xi}) \\ N_{e,y}^a(\underline{\xi}) \end{bmatrix} \rightarrow \begin{bmatrix} N_{e,x}^a(\underline{\xi}) \\ N_{e,y}^a(\underline{\xi}) \end{bmatrix} = J^{-1} \cdot \begin{bmatrix} N_{e,S}^a(\underline{\xi}) \\ N_{e,J}^a(\underline{\xi}) \end{bmatrix}$$

coeff from  $x = \phi(\underline{\xi})$

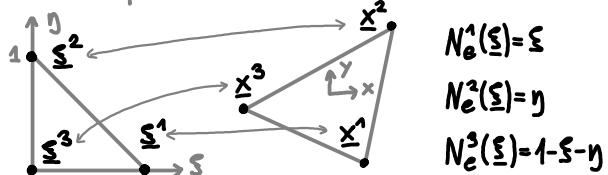
when variational method asks for an integral  $dA$ :

$$dA = \|dx \times dy\| = \left\| \left( \frac{\partial x}{\partial S} \cdot dS \cdot e_x + \frac{\partial x}{\partial \eta} \cdot d\eta \cdot e_y \right) \times \left( \frac{\partial y}{\partial S} \cdot dS \cdot e_x + \frac{\partial y}{\partial \eta} \cdot d\eta \cdot e_y \right) \right\| \Rightarrow dA = \det(J) \cdot dS \cdot d\eta \cdot \|e_x \times e_y\|$$

basis vols. of phys. frame

### • reference constant strain triangle (T3):

$$n=3, q=1, N_e^a \in \{1, \xi, \eta\}$$

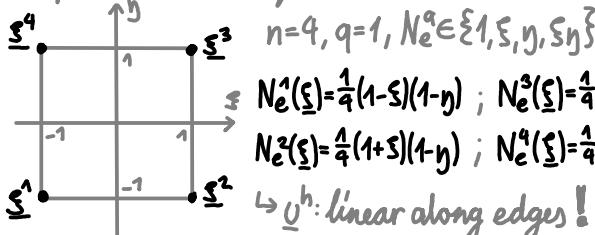


isoparametric mapping:  $\underline{x} = \phi(\underline{\xi}) = \underline{x}^1 \xi + \underline{x}^2 \eta + \underline{x}^3 (1 - \xi - \eta)$

$$\underline{J} = \begin{bmatrix} \underline{x}_x^1 - \underline{x}_x^3 & \underline{x}_y^1 - \underline{x}_y^3 \\ \underline{x}_x^2 - \underline{x}_x^3 & \underline{x}_y^2 - \underline{x}_y^3 \end{bmatrix} \quad \underline{J}^{-1} = \frac{1}{2A_e} \begin{bmatrix} \underline{x}_y^2 - \underline{x}_y^3 & \underline{x}_y^3 - \underline{x}_y^1 \\ \underline{x}_x^3 - \underline{x}_x^2 & \underline{x}_x^1 - \underline{x}_x^3 \end{bmatrix}$$

$$\det(\underline{J}) = (\underline{x}_x^1 - \underline{x}_x^3)(\underline{x}_y^2 - \underline{x}_y^3) - (\underline{x}_x^2 - \underline{x}_x^3)(\underline{x}_y^1 - \underline{x}_y^3) = 2A_e$$

### • reference bilinear quadrilateral element (Q4)



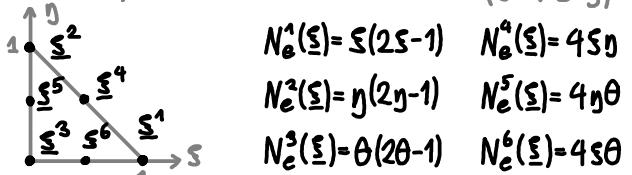
$$N_e^1(\underline{\xi}) = \frac{1}{4}(1-\xi)(1-\eta); N_e^2(\underline{\xi}) = \frac{1}{4}(1+\xi)(1+\eta)$$

$$N_e^3(\underline{\xi}) = \frac{1}{4}(1+\xi)(1-\eta); N_e^4(\underline{\xi}) = \frac{1}{4}(1-\xi)(1+\eta)$$

$\hookrightarrow$  linear along edges!

### • reference linear strain triangle (T6)

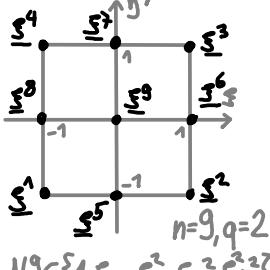
$$n=6, q=2, N_e^a \in \{1, \xi, \eta, \xi\eta, \xi^2, \eta^2\} \quad (\theta = 1 - \xi - \eta)$$



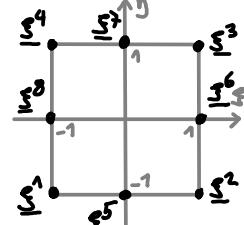
isoparametric mapping:  $\underline{x} = \phi(\underline{\xi}) = \sum_{a=1}^n x^a N_e^a(\underline{\xi})$

$\underline{J}(\underline{\xi}) = \dots$  (here  $\underline{J}$  func of  $\underline{\xi}$ !),  $\underline{J}^{-1}(\underline{\xi}) = \dots$ ,  $\det(\underline{J})(\underline{\xi}) = \dots$

### • reference quadratic quadrilateral element (Q9 or Q8)



$$N_e^a \in \{1, \xi, \eta, \xi\eta, \xi^2, \eta^2, \xi^2\eta^2\}$$



$$N_e^a \in \{1, \xi, \eta, \xi\eta, \xi^2, \eta^2, \xi^2\eta^2\}$$

### • 3D polynomial shape functions:

irregular element shapes  $\rightarrow$  isoparametric mapping

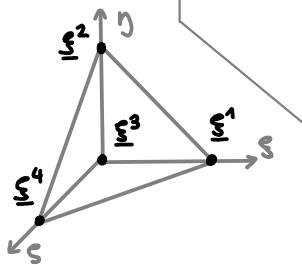
same as 2D case, just now with:

$$\underline{J} = \begin{bmatrix} x_{,S}(\underline{\xi}) & y_{,S}(\underline{\xi}) & z_{,S}(\underline{\xi}) \\ x_{,y}(\underline{\xi}) & y_{,y}(\underline{\xi}) & z_{,y}(\underline{\xi}) \\ x_{,\zeta}(\underline{\xi}) & y_{,\zeta}(\underline{\xi}) & z_{,\zeta}(\underline{\xi}) \end{bmatrix}, \quad dV = \det(\underline{J}) d\xi d\eta d\zeta$$

### • constant strain tetrahedron (T4)

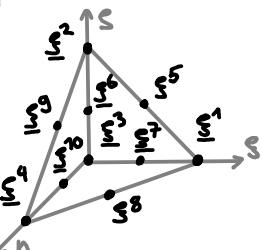
$$n=4, q=1, N_e^a \in \{1, \xi, \eta, \zeta\}$$

$$\begin{aligned} N_e^1(\underline{\xi}) &= \xi \\ N_e^2(\underline{\xi}) &= \eta \\ N_e^3(\underline{\xi}) &= \zeta \\ N_e^4(\underline{\xi}) &= 1 - \xi - \eta - \zeta \end{aligned}$$



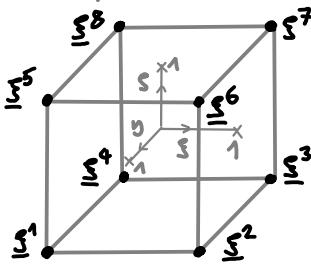
### • linear strain tetrahedron (T10)

$$n=10, q=2, N_e^a \in \{\dots\}$$

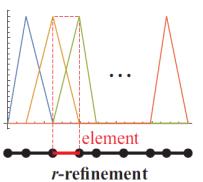
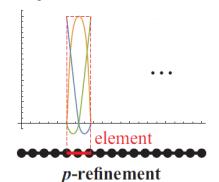
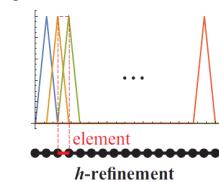
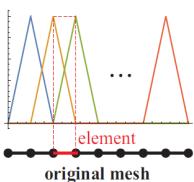


### • brick element

$$n=8, q=1, N_e^a \in \{1, \xi, \eta, \zeta, \xi\eta, \xi\zeta, \eta\zeta, \xi\eta\zeta\}$$



### • refinements (ways to increase accuracy of FEM solution)



h-refinement: decrease element sizes

p-refinement: increase poly. interp. order

r-refinement: move nodes more where func. is not smooth

## FEM bar example (1D bar under axial load) (static + lin.elastic)

• 2-node bar element in 1D:

$$U^h(x) = N^1(x)U^1 + N^2(x)U^2$$

$$N^1(x) = 1 - \frac{x}{L}; N^2(x) = \frac{x}{L}$$

$$\hookrightarrow N_{,x}^1 = -\frac{1}{L}; \quad \hookrightarrow N_{,x}^2 = \frac{1}{L}$$

$$K^{ab} = B[N^a, N^b] = \int_0^L EN_x^a N_x^b A dx = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$F^a = \underline{L}[N^a] = ?$$

$$\hookrightarrow \underline{K} \cdot \underline{U} = \underline{F} \rightarrow \underline{U} = \begin{pmatrix} U^1 \\ U^2 \end{pmatrix} = ?$$

• 2-node bar element in 2D:

$$R(\psi) = \begin{pmatrix} \cos \psi & \sin \psi & 0 & 0 \\ 0 & 0 & \cos \psi & \sin \psi \end{pmatrix} \quad (\text{projection on bar, so 1D formula can be used})$$

$$\hookrightarrow \underline{\underline{U}} = (\underline{\underline{U}}^1, \underline{\underline{U}}^2)^T = R \cdot (U_x^1, U_y^1, U_x^2, U_y^2)^T = R \underline{U}$$

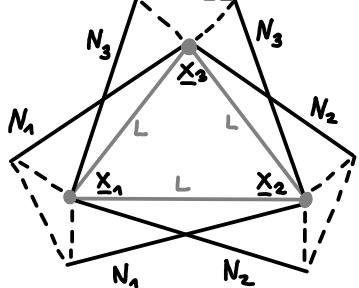
$$\hookrightarrow \underline{\underline{E}} = (\underline{\underline{F}}^1, \underline{\underline{F}}^2)^T = R \cdot (F_x^1, F_y^1, F_x^2, F_y^2)^T = R \underline{F}$$

$$\hookrightarrow \underline{\underline{K}} = \frac{EA}{\Delta x} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \quad (\text{like 1D})$$

• 2-node bar element in 3D:

$$R = \begin{pmatrix} \tilde{x} \cdot e^x & \tilde{x} \cdot e^y & \tilde{x} \cdot e^z & 0 & 0 & 0 \\ 0 & 0 & 0 & \tilde{x} \cdot e^x & \tilde{x} \cdot e^y & \tilde{x} \cdot e^z \end{pmatrix} \quad \left( \begin{array}{l} \tilde{x}: \text{unit vector along bar} \\ e^x, e^y, e^z: \text{cartesian basis} \end{array} \right) \quad \text{rest same as 2D!}$$

## multiple bar elements in 2D:



bar 1 → 2:  $\frac{EA}{L} \cdot \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} \cdot \begin{bmatrix} U_x^1 \\ U_y^1 \\ U_x^2 \\ U_y^2 \end{bmatrix} \rightarrow \frac{EA}{L} \cdot \begin{bmatrix} 1 & 0 & -1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix} \cdot \begin{bmatrix} U_x^1 \\ U_y^1 \\ U_x^2 \\ U_y^2 \\ U_x^3 \\ U_y^3 \end{bmatrix}$

bar 2 → 3:  $K_{23}^{\text{loc}} = R^T(\frac{2\pi}{3}) \tilde{K} R(\frac{2\pi}{3}) \rightarrow K_{23}^{\text{glob}}$

bar 3 → 1:  $K_{31}^{\text{loc}} = R^T(\frac{4\pi}{3}) \tilde{K} R(\frac{4\pi}{3}) \rightarrow K_{31}^{\text{glob}}$

$K_{12}^{\text{glob}}$  analog to  $K_{23}^{\text{glob}}$

$$\rightarrow \underline{\underline{K}} = \underline{\underline{K}}_{12}^{\text{glob}} + \underline{\underline{K}}_{23}^{\text{glob}} + \underline{\underline{K}}_{31}^{\text{glob}} \rightarrow$$

(combine eq. of shared nodes)

$E$  can be interpreted as forces on nodes!  
 $\underline{F} = [F_x^1 \ F_y^1 \ F_x^2 \ F_y^2 \ F_x^3 \ F_y^3]^T$

- Dirichlet BC: write force in  $F_i^a$   
- Neumann BC: remove row of  $\underline{\underline{K}} \underline{U} = \underline{F}$   
with  $F_i^a \neq 0$  it reads  $U_i^a = \hat{U}_i^a$

## FEM Euler beam example (1D beam under transversal load) (static + lin.elastic)

strong form:  $EI_y \frac{\partial^4 w}{\partial x^4}(x) = q(x)$  BCs:  $F_z^1 = EI_y \frac{\partial^3 w}{\partial x^3}(0); F_z^2 = -EI_y \frac{\partial^3 w}{\partial x^3}(L); M_y^1 = EI_y \frac{\partial^2 w}{\partial x^2}(0); M_y^2 = -EI_y \frac{\partial^2 w}{\partial x^2}(L)$

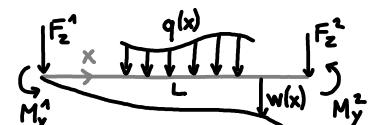
weak form:  $\min I[w] = \int_0^L EI_y w_{xx}^2 - qw dx - F_z^1 w(0) - F_z^2 w(L) + M_y^1 w_{,x}(0) + M_y^2 w_{,x}(L)$

$I[w] = B[w, w] - L[w]: B[w, v] = \int_0^L EI_y w_{xx} v_{xx} dx; L[v] = \int_0^L qv dx + F_z^1 v(0) + F_z^2 v(L) - M_y^1 v_{,x}(0) - M_y^2 v_{,x}(L)$   
 $\rightarrow 0 = \delta I[w] = B[w, \delta w] - L[\delta w] \quad \forall \delta w \in \mathcal{U}_0$

poly. order:  $q=3 \rightarrow n=4$  so that we have 4 nodes (same as #of Neumann BC)

nodes: pick nodes on  $\bullet \bullet \bullet$  so one can replace Neumann BC with Dirichlet BC  
 $\bullet \bullet$  fix points  $w_e^1 = w_e^h(0), w_e^2 = w_e^h(L); \bullet \bullet$  fix angles  $\theta_e^1 = w_{e,x}^h(0), \theta_e^2 = w_{e,x}^h(L)$

shape func:  $w_e^h = w_e^1 N_e^1 + w_e^2 N_e^2 + \theta_e^1 M_e^1 + \theta_e^2 M_e^2$  (Hermite polynomials)  $\begin{cases} N_e^1 = 1 - 3\left(\frac{x}{L}\right)^2 + 2\left(\frac{x}{L}\right)^3; N_e^2 = 3\left(\frac{x}{L}\right)^2 - 2\left(\frac{x}{L}\right)^3 \\ M_e^1 = x(1 - \frac{x}{L})^2; M_e^2 = \frac{x^2}{L}(1 - \frac{x}{L}) \end{cases}$



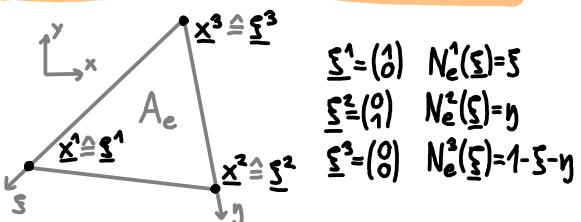
var.method:  $\underline{U} = \begin{bmatrix} w_e^1 \\ \theta_e^1 \\ w_e^2 \\ \theta_e^2 \end{bmatrix} \quad \underline{\underline{K}} = \frac{2EI_y}{L^3} \begin{bmatrix} 6 & 3L & -6 & 3L \\ 3L & 2L^2 & -3L & L^2 \\ -6 & -3L & 6 & -3L \\ 3L & L^2 & -3L & 2L^2 \end{bmatrix} \quad \underline{\underline{F}} = \begin{bmatrix} F_e^1 \\ M_e^1 \\ F_e^2 \\ M_e^2 \end{bmatrix} \quad (\underline{\underline{F}} = \text{forces and moments on nodes})$

## FEM beam example (1D euler beam AND 1D bar)

• 2-node beam element in 1D:  $\underline{U} = \begin{bmatrix} U_e^1 \\ W_e^1 \\ \theta_e^1 \\ U_e^2 \\ W_e^2 \\ \theta_e^2 \end{bmatrix}$ ;  $\underline{\underline{K}} = \frac{2EIy}{L^3} \cdot \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 6 & 3L & 0 & -6 & 3L \\ 0 & 3L & 2L^2 & 0 & -3L & L^2 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & -6 & -3L & 0 & 6 & -3L \\ 0 & 3L & L^2 & 0 & -3L & 2L^2 \end{bmatrix} + \frac{EA}{L} \begin{bmatrix} 1 & 0 & 0 & -1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ -1 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$ ;  $\underline{F} = \begin{bmatrix} F_{\text{axial}}^1 \\ F_{\text{trans}}^1 \\ M^1 \\ F_{\text{axial}}^2 \\ F_{\text{trans}}^2 \\ M^2 \end{bmatrix}$

• 2-node beam element in 2D:  $\underline{U} = \begin{bmatrix} U_x^1 \\ U_y^1 \\ \theta^1 \\ U_x^2 \\ U_y^2 \\ \theta^2 \end{bmatrix}$ ;  $R = \begin{bmatrix} \cos\varphi & \sin\varphi & 0 & 0 & 0 & 0 \\ -\sin\varphi & \cos\varphi & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & \cos\varphi & \sin\varphi & 0 \\ 0 & 0 & 0 & -\sin\varphi & \cos\varphi & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \rightarrow \underline{\underline{K}} = R^T \underline{\underline{K}} R$ ;  $\underline{F} = \begin{bmatrix} F_x^1 \\ F_y^1 \\ M^1 \\ F_x^2 \\ F_y^2 \\ M^2 \end{bmatrix}$   
(rest analog to FEM bar example)

## FEM plane stress/strain on T3 element (triangle element of fixed depth t) (static + lin.elastic)



from variational method example:  $\underline{\underline{K}} \cdot \underline{U}^h = \underline{F}$

$$\underline{U}^h = [U_x^1, U_y^1, U_x^2, U_y^2, U_x^3, U_y^3]^T$$

$$\underline{K}_e = \int_{\Omega_e} B^T E B dV = A_e \cdot t \cdot B^T E B \quad (\text{since } B, E \text{ const.})$$

$$\underline{F}_e = \int_{\Omega_e} \underline{\underline{\sigma}} \underline{\underline{C}}^T \underline{\underline{b}} dV + \int_{\partial \Omega_e} \underline{\underline{C}}^T \underline{\underline{\epsilon}} dS = \dots$$

assemble problem  $\underline{\underline{K}} = \sum_e \underline{\underline{K}}_e$ ;  $\underline{F} = \sum_e \underline{F}_e$

↳ solve  $\underline{\underline{K}} \underline{U} = \underline{F}$  for  $\underline{U}$

( $\underline{F}$  = forces on nodes =  $[F_x^1, F_y^1, F_x^2, F_y^2, \dots]^T$ )

$$N_{e,x}^1(\underline{\xi}) = \frac{1}{2A_e} \cdot X_y^2 - X_y^3; N_{e,x}^2(\underline{\xi}) = \frac{1}{2A_e} \cdot X_y^3 - X_y^1; N_{e,x}^3(\underline{\xi}) = \frac{1}{2A_e} \cdot X_y^1 - X_y^2$$

$$N_{e,y}^1(\underline{\xi}) = \frac{1}{2A_e} \cdot X_x^3 - X_x^2; N_{e,y}^2(\underline{\xi}) = \frac{1}{2A_e} \cdot X_x^1 - X_x^3; N_{e,y}^3(\underline{\xi}) = \frac{1}{2A_e} \cdot X_x^2 - X_x^1$$

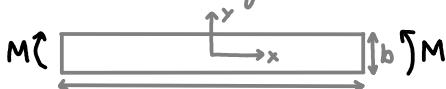
$$\underline{\underline{B}} = \left( \text{see Voigt notation} \right) = \frac{1}{2A_e} \begin{bmatrix} X_y^2 - X_y^3 & 0 & X_y^3 - X_y^1 & 0 & X_y^1 - X_y^2 & 0 \\ 0 & X_x^3 - X_x^2 & 0 & X_x^1 - X_x^3 & 0 & X_x^2 - X_x^1 \\ X_x^3 - X_x^2 & X_y^2 - X_y^3 & X_x^1 - X_x^3 & X_y^3 - X_y^1 & X_x^1 - X_x^2 & X_y^1 - X_y^2 \end{bmatrix}$$

$\underline{\underline{E}}$  = (see Voigt notation) = pick the one for plane stress or plane strain!

$$dV = t \cdot dA = t \cdot \det(J) d\underline{\xi} dy = 2A_e t d\underline{\xi} dy \quad (t: \text{element thickness})$$

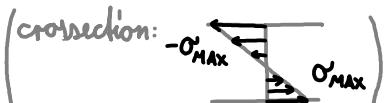
## Element deflcs: euler beam as Q4 element (static + lin.elastic)

real beam bending:

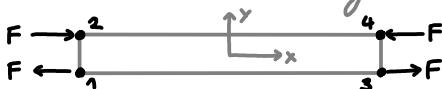


$$\sigma_{xx} = -\frac{2y}{b} \cdot \sigma_{\max}; \sigma_{yy} = 0; \sigma_{xy} = 0$$

$$\epsilon_{xx} = \sigma_{xx}/E; \epsilon_{yy} = -\nu \sigma_{xx}/E; \epsilon_{xy} = 0$$



Q4 element beam bending:



$$\underline{\underline{K}} \underline{U} = \underline{F} \rightarrow \underline{U} = \dots \text{ (solve)}$$

$$\rightarrow U_y = U_y^1 = U_y^2 = U_y^3 = U_y^4 = 0$$

$$U_x = -U_x^1 = U_x^2 = -U_x^3 = U_x^4 = 2 \frac{\sigma_{\max}}{E} \cdot \frac{(1-\nu)(1-2\nu)}{2(1-\nu)+(a/b)^2(1-2\nu)}$$

$$\tilde{\underline{\underline{\epsilon}}} = \underline{\underline{B}} \underline{U} \rightarrow \epsilon_{xx} = -U_x \cdot \frac{2y}{ab}; \epsilon_{yy} = 0; \epsilon_{xy} = -U_x \cdot \frac{2x}{ab}$$

DON'T MATCH!

$\underline{F}$ : forces on nodes from real stresses at ends

$$F_x^1 = \int_{-b/2}^{b/2} \left( -\frac{2y}{b} \sigma_{\max} \right) (1-2y/b) dy = \frac{1}{3} b \sigma_{\max}$$

$$\sigma_{xx} \text{ real } N^1(-a/2, y)$$

$$F = F_x^1 - F_y^1 = F_z^1 = -F^1 = \frac{1}{3} b \sigma_{\max} \quad (\text{analog})$$

$$\frac{\theta_{Q4}}{\theta_{\text{real}}} = \frac{1-\nu^2}{1+\frac{1-\nu}{2} \frac{(a/b)^2}{(1-2\nu)}}$$

→ if  $a/b$  is large (slender beam) then Q4 predicts much lower displacements than real beam → "locking"

## shape function requirements

$$v(x) \approx v^h(x) = \sum_{a=1}^n v^a N^a(x) \quad (v^a: \text{constants}; N^a: \text{shape functions}) \quad (v \in U; v^h \in U^h: \text{function spaces})$$

requirements:

- 1)  $\forall x \in \Omega \exists a \text{ s.t. } N^a(x) \neq 0 \quad (N^a \text{ covers } \Omega)$
- 2) simple way to enforce Dirichlet BC
- 3)  $\sum_{a=1}^n v^a N^a = 0 \Leftrightarrow v^a = 0 \forall a \quad (\text{linearly indep.})$
- 4) satisfy requirements of equation (e.g.  $N^a \subset C^2(\Omega)$ )
- 5)  $v^h \rightarrow v$  for  $n \rightarrow \infty$  and any  $v$  (completeness)

support:  $\text{supp } N^a = \{x \in \Omega : N^a(x) \neq 0\}$   
 global basis func: if  $\text{supp } N^a = \Omega \forall a$   
 local basis func: if  $\text{supp } N^a \subset \Omega \forall a$

## Numerical quadrature = numerical integration

usefull if e.g.  $K = \int \dots d\xi$  or  $F = \int \dots d\xi$  is hard to compute analytically!

### • Gauss-Legendre quadrature (good for line/square/cube element and polynomial func.)

$$1D: \int_{-1}^1 f(\xi) d\xi \approx \sum_{i=0}^{n_Q-1} W_i f(\xi_i) \quad \left( \begin{array}{l} f(\xi): \text{function to be integrated} \quad \xi_i: \text{integration points} \\ n_Q: \# \text{of integration points} \quad W_i: \text{integration weights} \end{array} \right)$$

↪ with the correct  $W_i, \xi_i$ , this method produces exact integration for  $f(\xi) = \text{polynomial of order } < q_Q = 2n_Q - 1$  !  
 solve:  $\int_{-1}^1 \xi^k d\xi = \sum_{i=0}^{n_Q-1} W_i \xi_i^k \quad \forall k = 0, \dots, q_Q$

$$(\text{proof: } \int_{-1}^1 f(\xi) d\xi = \int_{-1}^1 \sum_{j=0}^{q_Q} a_j \xi^j d\xi = \sum_{j=0}^{q_Q} a_j \int_{-1}^1 \xi^j d\xi = \sum_{j=0}^{q_Q} a_j \sum_{i=0}^{n_Q-1} W_i \xi_i^j = \sum_{i=0}^{n_Q-1} W_i \sum_{j=0}^{q_Q} a_j \xi_i^j = \sum_{i=0}^{n_Q-1} W_i f(\xi_i) \quad \blacksquare)$$

$$2D: \int_{-1}^1 \int_{-1}^1 f(\xi) d\xi dy \approx \sum_{k=0}^{n_Q-1} W_k f(\xi_k): \text{ exact integration for } f(\xi) = \text{polynomial of order } < q_Q = 2\sqrt{n_Q} - 1 !$$

$$\hookrightarrow W_k = W_i \cdot W_j; \quad \xi_k = (\xi_i, \xi_j) \quad (i, j: \text{all comb. of above 1D solutions}) \quad \left( \begin{array}{l} \uparrow \quad \uparrow \\ \text{1D solution values} \end{array} \right) \quad \left( \begin{array}{l} \text{e.g.: } q_Q = 1, n_Q = 1 \rightarrow W_0 = 1, \xi_0 = (0, 0) \\ \text{e.g.: } q_Q = 3, n_Q = 4 \rightarrow W_{0-3} = 1, \xi_{0-3} = (\pm \frac{1}{\sqrt{3}}, \pm \frac{1}{\sqrt{3}}) \end{array} \right)$$

$$(\text{proof: } \int_{-1}^1 \int_{-1}^1 f(\xi, \eta) d\xi dy = \int_{-1}^1 \sum_{i=0}^{N-1} W_i f(\xi_i, \eta) dy = \sum_{i=0}^{N-1} W_i \sum_{j=0}^{N-1} W_j f(\xi_i, \eta_j) = \sum_{j=0}^{N-1} \sum_{i=0}^{N-1} W_i W_j f(\xi_i, \eta_j) \quad \blacksquare \quad (N = \sqrt{n_Q}))$$

$$3D: \int_{-1}^1 \int_{-1}^1 \int_{-1}^1 f(\xi) d\xi dy dz \approx \sum_{k=0}^{n_Q-1} W_k f(\xi_k): \text{ exact integration for } f(\xi) = \text{polynomial of order } < q_Q = 2\sqrt[3]{n_Q} - 1 !$$

$$\text{analog to 2D: } \hookrightarrow \text{e.g.: } q_Q = 1, n_Q = 1 \rightarrow W_0 = 1, \xi_0 = (0, 0, 0) \quad \left( \begin{array}{l} \text{e.g.: } q_Q = 3, n_Q = 8 \rightarrow W_{0-7} = 1, \xi_{0-7} = (\pm \frac{1}{\sqrt[3]{3}}, \pm \frac{1}{\sqrt[3]{3}}, \pm \frac{1}{\sqrt[3]{3}}) \end{array} \right)$$

(full integration (specific to mech. problem!):

$q_Q$  needed for exact integration of finite element that is undistorted ( $J = \text{const.}$ ), homogeneous, linear elastic

Q4 element:  $q_Q = 3, n_Q = 4$

Q8/Q9 element:  $q_Q = 5, n_Q = 9$  (from orders of  $N^a, J, B, E, C$ )  
 Brick element:  $q_Q = 3, n_Q = 8$

(example: Q4 element with  $q_Q = 1$  quadrature.

↪ below full integration order!



evaluate to same energy! bad, bc. solver is then unsure which is right. (called zero energy mode)

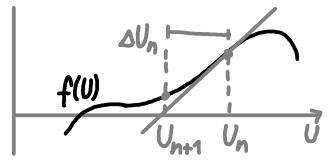
(if  $f(\xi)$  has order  $> q_Q$  or is no polynomial at all, then integration error appears! → still worth using this method because still more efficient than Riemann sums. just increase  $q_Q$  if needed. with  $q_Q \rightarrow \infty$  method is exact for any  $f(\xi)$ !)

### • simplicial quadrature (good for triangle/Tetrahedron element and polynomial func.)

[not discussed in class. see "look-up tables" online.]

## iterative solvers

general problem: solve  $\underline{f}(\underline{U}) = 0$  (e.g.  $\underline{f}(\underline{U}) = \underline{F}_{int}(\underline{U}) - \underline{F}_{ext}(\underline{U})$  from nonlinear mech. prob.)  
 iterative solver:  $\underline{U}_0$  initial guess (e.g.  $\underline{U}_0 = \underline{0}$ )  $\rightarrow$  loop  $\underline{U}_{n+1} = \underline{U}_n + \Delta \underline{U}_n$ , until  $\Delta \underline{U}_n < \text{threshold}$ .  $\begin{cases} n: \text{iteration counter} \\ \Delta \underline{U}_n: \text{update rule} \end{cases}$



### • Newton-Raphson (NR) method (exact solution after 1 iteration if $f(\underline{U})$ linear)

$$0 = \underline{f}(\underline{U}_{n+1}) = \underline{f}(\underline{U}_n + \Delta \underline{U}_n) = \underline{f}(\underline{U}_n) + \frac{\partial \underline{f}}{\partial \underline{U}}(\underline{U}_n) \Delta \underline{U}_n + O((\Delta \underline{U}_n)^2) \rightarrow \Delta \underline{U}_n = -[\frac{\partial \underline{f}}{\partial \underline{U}}(\underline{U}_n)]^{-1} \underline{f}(\underline{U}_n)$$

↑ Taylor expansion      ↑ assumed ≈ 0      ↑ =  $\underline{T}(\underline{U}_n)$

• advantage: quadratic convergence

• disadvantage: always have to recompute  $\underline{T}(\underline{U}_n)$  &  $\underline{T}$  not always invertible

### • Damped NR method

(like NR, but slower convergence and more stable)

$$\Delta \underline{U}_n = -\alpha [\frac{\partial \underline{f}}{\partial \underline{U}}(\underline{U}_n)]^{-1} \underline{f}(\underline{U}_n) = \alpha \Delta \underline{U}_n^{\text{NR}} \quad (0 < \alpha < 1)$$

### • Line search method

(same as damped NR, but optimize stepsize)

$$\Delta \underline{U}_n = \beta_n \Delta \underline{U}_n^{\text{NR}} \leftarrow \beta_n = \arg \min \| \underline{f}(\underline{U}_n - \beta_n \Delta \underline{U}_n^{\text{NR}}) \|^2$$

### • Conjugate Gradient (CG) method

(like "line search", but optimize dir. + dist. of next step)

$$\Delta \underline{U}_n = \gamma_n \underline{S}_n \leftarrow \gamma_n, \underline{S}_n = \arg \min \| \underline{f}(\underline{U}_n - \gamma_n \underline{S}_n) \|^2 \quad (\rightarrow \text{no } \underline{T}!)$$

### • Quasi-Newton (QN) method

(avoids recomputing and inverting  $\underline{T}(\underline{U}_n)$ )

done by approx.  $\underline{T}(\underline{U}_{n+1})$  based on  $\underline{T}(\underline{U}_n)$  and  $\underline{f}(\underline{U}_n)$ .

### • Nonlinear least squares

(beneficial for overconstrained systems)

$$\text{minimize } r(\underline{U}) = \| \underline{f}(\underline{U}) \|^2 \text{ (residual)} \Rightarrow \frac{\partial r}{\partial \underline{U}}(\underline{U}) = \dots = 0$$

### • Gradient flow method

(good because no need to compute  $\underline{T}$ )

$$\text{assume } \dot{\underline{U}} = -\underline{C} \cdot \underline{f}(\underline{U}) \rightarrow \underline{U}_{n+1} = \underline{U}_n - \underline{C} \cdot \underline{f}(\underline{U}_n)$$

## mech problem: boundary conditions (in finite element context)

(non-linear elastic)

• Neumann boundary conditions are considered in  $\underline{F} = \sum_e \underline{F}_e \rightarrow \underline{K}\underline{U} = \underline{F}$  (linear elastic) or  $\underline{F}_{ext} = \underline{F} = \sum_e \underline{F}_e \rightarrow \underline{F}_{int}(\underline{U}) - \underline{F}_{ext} = 0$

- body force:  $\underline{F}_e = \int_{\Omega_e} S \underline{C}^T \underline{b} dV$
- traction force:  $\underline{F}_e = \int_{\partial \Omega_e} \underline{C}^T \underline{t} dS$
- point force on node  $a$ :  $\underline{F}^a = \underline{P}$
- point force on point  $x$ :  $\underline{F}_e = \underline{C}(x)^T \underline{P}$
- spring force (small/large strain):  $\underline{F}_e$  func of  $\underline{U}$ !  $\rightarrow \underline{F}_{ext}(\underline{U})$

• Dirichlet boundary condition (for linear problem) are considered in rows of  $\underline{K}\underline{U} = \underline{F}$

want to enforce  $\underline{U}^3 = \hat{\underline{U}}^3$  on:

$$\begin{bmatrix} K_{11} & K_{12} & K_{13} & K_{14} \\ K_{21} & K_{22} & K_{23} & K_{24} \\ K_{31} & K_{32} & K_{33} & K_{34} \\ K_{41} & K_{42} & K_{43} & K_{44} \end{bmatrix} \cdot \begin{bmatrix} \underline{U}^1 \\ \underline{U}^2 \\ \underline{U}^3 \\ \underline{U}^4 \end{bmatrix} = \begin{bmatrix} F^1 \\ F^2 \\ F^3 \\ F^4 \end{bmatrix}$$

• substitution:

$$\begin{bmatrix} K_{11} & K_{12} & K_{13} & K_{14} \\ K_{21} & K_{22} & K_{23} & K_{24} \\ 0 & 0 & 1 & 0 \\ K_{41} & K_{42} & K_{43} & K_{44} \end{bmatrix} \cdot \begin{bmatrix} \underline{U}^1 \\ \underline{U}^2 \\ \underline{U}^3 \\ \underline{U}^4 \end{bmatrix} = \begin{bmatrix} F^1 \\ F^2 \\ \hat{U}^3 \\ F^4 \end{bmatrix}$$

(+ cheap to do  
- matrix ill conditioned)

• condensation:

$$\begin{bmatrix} K_{11} & K_{12} & & K_{14} \\ K_{21} & K_{22} & & K_{24} \\ & & K_{33} & \\ K_{41} & K_{42} & K_{43} & K_{44} \end{bmatrix} \cdot \begin{bmatrix} \underline{U}^1 \\ \underline{U}^2 \\ \underline{U}^4 \\ \underline{U}^4 \end{bmatrix} = \begin{bmatrix} F^1 \\ F^2 \\ \underline{F}^4 \\ F^4 \end{bmatrix} - \begin{bmatrix} K_{43} \\ K_{23} \\ & \\ K_{43} \end{bmatrix} \hat{U}_3$$

(+ matrix size smaller  
- expensive to do)

## FEM mech. problem on Q4 element (static + linear elastic + numerical quadrature)

$$N_e^a(\underline{\xi}) = \dots \rightarrow \underline{C}(\underline{\xi}) = \dots \rightarrow \underline{J}(\underline{\xi}) = \dots \rightarrow \underline{B}(\underline{\xi}) = \dots$$

$$\underline{K}_e = \int_{\Omega_e} \underline{B}(\underline{\xi})^T \underline{E} \underline{B}(\underline{\xi}) dV = \int_{-1}^1 \int_{-1}^1 \underline{B}(\underline{\xi})^T \underline{E} \underline{B}(\underline{\xi}) \det(\underline{J}(\underline{\xi})) d\underline{\xi} d\underline{\eta} = \sum_{k=0}^{n_q-1} W_k \underline{B}(\underline{\xi}_k)^T \underline{E} \underline{B}(\underline{\xi}_k) \det(\underline{J}(\underline{\xi}_k)) \quad (q_a=3, n_q=4 \text{ for Q4})$$

$$\underline{F}_e = \text{body force: } \int_{\Omega_e} S \underline{C}(\underline{\xi})^T \underline{b}(\underline{\xi}) dV = \int_{-1}^1 \int_{-1}^1 S \underline{C}(\underline{\xi})^T \underline{b}(\underline{\xi}) \det(\underline{J}(\underline{\xi})) d\underline{\xi} d\underline{\eta} = \sum_{k=0}^{n_q-1} W_k S \underline{C}(\underline{\xi}_k)^T \underline{b}(\underline{\xi}_k) \det(\underline{J}(\underline{\xi}_k)) \quad (q_a=3, n_q=4)$$

$$\text{traction force: } \int_{\partial \Omega_e} \underline{C}(\underline{\xi})^T \underline{t} dS = \int_{-1}^1 \int_{-1}^1 \underline{C}(\underline{\star})^T \underline{t}(\underline{\star}) \det(\underline{J}(\underline{\star})) d\underline{\star} = \sum_{k=0}^{n_q-1} W_k \underline{C}(\underline{\star}_k)^T \underline{t}(\underline{\star}_k) \det(\underline{J}(\underline{\star}_k)) \quad (q_a=3, n_q=2)$$

( $\star = \text{face } \pm \underline{\xi} \text{ or } \pm \underline{\eta}$ )

## Dynamic mech problem (displacement field $\underline{u} = \underline{u}(x, t)$ )

- **strong form (dynamic):**  $\operatorname{div}(\underline{\sigma}(\underline{u})) + g \underline{b} = g \ddot{\underline{u}} \text{ in } \Omega ; \underline{u} = \hat{\underline{u}} \text{ on } \Omega_0 \forall t ; \underline{\sigma}(\underline{u}) \cdot \underline{n} = \hat{\underline{t}} \text{ on } \Omega_N \forall t$   
 $\alpha_{ij,j}(\underline{u}) + g b_i = g \ddot{u}_i \text{ in } \Omega ; u_i = \hat{u}_i \text{ on } \Omega_0 \forall t ; \alpha_{ij}(\underline{u}) \cdot n_j = \hat{t}_i \text{ on } \Omega_N \forall t$

- **weak form (dynamic):**  $\min_{\text{action}} A[\underline{u}] = \int_{t_1}^{t_2} \int_{\Omega} W(\underline{u}) - \underbrace{\int_{\Omega} \frac{g}{2} |\dot{\underline{u}}|^2 dV}_{\text{kin E}} - \underbrace{\int_{\Omega} g b \underline{u} dV}_{\text{pot E}} - \underbrace{\int_{\partial\Omega} \hat{\underline{t}} \cdot \underline{u} dS}_{\text{ext W}} dt \text{ subj.to: } \underline{u} = \hat{\underline{u}} \text{ on } \partial\Omega_0$

variation:  $0 = \delta A[\underline{u}] = \int_{t_1}^{t_2} \int_{\Omega} \alpha_{ij} \delta \epsilon_{ij} - g \dot{u}_i \delta u_i dV - \int_{\Omega} g b_i \delta u_i dV - \int_{\partial\Omega_N} \hat{t}_i \delta u_i dS dt \text{ subj.to: } u_i = \hat{u}_i \text{ on } \partial\Omega_D \forall t$   
 $= \int_{t_1}^{t_2} \int_{\Omega} \alpha_{ij} \delta \epsilon_{ij} + g \ddot{u}_i \delta u_i dV - \int_{\Omega} g b_i \delta u_i dV - \int_{\partial\Omega_N} \hat{t}_i \delta u_i dS dt \quad \begin{matrix} \delta u_i = 0 \text{ on } \partial\Omega_D \forall t \\ \delta u_i = 0 \text{ at } t_1, t_2 \forall x \end{matrix}$

func. discretization:  $\underline{u}(x, t) \approx \underline{u}^h(x, t) = \sum_{a=1}^n \underline{u}^a(t) N^a(x) = \underline{C}(x) \cdot \underline{U}^h(t) \quad (\text{subj.to: } \underline{u}^h = \hat{\underline{u}} \text{ on } \partial\Omega_D \forall t)$   
 $\delta \underline{u}(x, t) \approx \underline{v}^h(x, t) = \sum_{a=1}^n \underline{v}^a(t) N^a(x) = \underline{C}(x) \cdot \underline{V}^h(t) \quad (\text{subj.to: } \underline{v}^h = 0 \text{ on } \partial\Omega_D \forall t \text{ and } \underline{v}^h = 0 \text{ at } t_1, t_2 \forall x)$

variation approx.:  $0 = A[\underline{u}] = \int_{t_1}^{t_2} \int_{\Omega} \underline{\sigma}(\underline{u}^h) \cdot \underline{\delta \epsilon}(\underline{v}^h) + g \ddot{\underline{u}}^h \cdot \underline{v}^h dV - \int_{\Omega} g b \cdot \underline{v}^h dV - \int_{\partial\Omega_N} \hat{\underline{t}} \cdot \underline{v}^h dS dt \quad \forall \underline{v}^h$   
 $= \int_{t_1}^{t_2} \underline{V}^h \left( \int_{\Omega} \underline{B}^T \underline{\sigma}(\underline{u}^h) + g \underline{C}^T \underline{\ddot{u}}^h dV - \int_{\Omega} g \underline{b}^T \underline{b} dV - \int_{\partial\Omega_N} \underline{C}^T \underline{\hat{t}} dS \right) dt \quad \forall \underline{V}^h$

$\Rightarrow \underbrace{\int_{\Omega} g \underline{C}^T \underline{C} dV \ddot{\underline{u}}^h}_{\underline{M} \cdot \ddot{\underline{U}}^h} + \underbrace{\int_{\Omega} \underline{B}^T \underline{\sigma}(\underline{u}^h) dV}_{F_{\text{int}}(\underline{u}^h)} - \underbrace{\int_{\partial\Omega_N} \underline{C}^T \underline{\hat{t}} dS}_{F_{\text{ext}}} = 0$   
 $(\text{consistent mass matrix}) \quad \underline{M} \cdot \ddot{\underline{U}}^h + F_{\text{int}}(\underline{u}^h) - F_{\text{ext}} = 0 \quad \left( \begin{matrix} F_{\text{int}} = \underline{K} \cdot \underline{U}^h \\ \text{for linear elastic!} \end{matrix} \right)$

M example for FEM bar:

1D:  $N_e^1 = 1 - \frac{x}{L}; N_e^2 = \frac{x}{L} \rightarrow \underline{C} = [N_e^1 \ N_e^2] = [1 - \frac{x}{L} \ \frac{x}{L}]$   
 $\underline{M} = \int_{\Omega} g \underline{S} \underline{C}^T \underline{C} dV = \int_0^L g \underline{S} \underline{C}^T \underline{C} A dx = gLA \frac{1}{6} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix}$

2D:  $\underline{M} = gLA \frac{1}{6} \begin{bmatrix} 2 & 1 & 1 \\ 1 & 2 & 1 \\ 1 & 1 & 2 \end{bmatrix}$   
(explaination?)

3D:  $\underline{M} = gLA \frac{1}{6} \begin{bmatrix} 2 & 1 & 1 & 1 \\ 1 & 2 & 1 & 1 \\ 1 & 1 & 2 & 1 \\ 1 & 1 & 1 & 2 \end{bmatrix}$

M example for CST element:

$N_e^1 = \xi, N_e^2 = \eta, N_e^3 = 1 - \xi - \eta \rightarrow \underline{C} = \begin{bmatrix} \xi & \eta & 1-\xi-\eta \\ 0 & \xi & \eta \\ 1-\xi-\eta & 0 & 1-\xi-\eta \end{bmatrix}$   
 $\underline{M} = \int_{\Omega} g \underline{S} \underline{C}^T \underline{C} dV = \int_0^1 \int_0^{1-\xi} g \underline{S} \underline{C}^T \underline{C} \det(J) d\eta d\xi \cdot t_e \rightarrow \underline{M} = gA_e t_e \frac{1}{12} \begin{bmatrix} 2 & 1 & 1 & 1 \\ 1 & 2 & 1 & 1 \\ 1 & 1 & 2 & 1 \\ 1 & 1 & 1 & 2 \end{bmatrix}$

alternative M lump: concentrate mass on nodes  $\rightarrow \underline{M}$  is less dense, but has error (vanishes for small elements)  
Lumped mass matrix  $\hookrightarrow \underline{M} = \int_{\Omega} g \underline{S} \underline{C}^T \underline{C} dV = \frac{SV}{\# \text{nodes}} \sum_{\text{nodes}} \underline{C}(\text{node})^T \underline{C}(\text{node}) = \frac{SV}{\# \text{nodes}}$  II

Vibrations (small amplitude oscillation about a stable equilibrium)

• free vibration ( $F_{\text{ext}} = \text{const.}$ ) + linear elasticity:

$\underline{U}(t) = \underline{U}_0 + \underline{V}(t) \quad (\underline{V}(t): \text{small perturbation}; \underline{U}_0 \text{ equilibrium } K\underline{U}_0 = F)$

$\underline{M} \ddot{\underline{U}} + \underline{K} \underline{U} = F \rightarrow \underline{M} \ddot{\underline{V}} + \underline{K} \underline{V} = 0 \rightarrow \text{real solution: } \underline{V}(t) = C_1 e^{i\omega t} + C_2 e^{-i\omega t} = \hat{\underline{U}} \cos(\omega t + \varphi) \rightarrow$

$\rightarrow \text{insert solution: } (\underline{K} - \omega^2 \underline{M}) \hat{\underline{U}} \cos(\omega t + \varphi) = 0 \quad \forall t \rightarrow (\underline{M}^{-1} \underline{K} - \omega^2) \hat{\underline{U}} = 0 \rightarrow \omega_i^2 = E\lambda(\underline{M}^{-1} \underline{K}) ; \hat{U}_i = EV(\underline{M}^{-1} \underline{K}) \rightarrow$

$\rightarrow \text{multiple } \omega_i^2, \hat{U}_i \text{ solve equation. True solution is linear comb.: } \underline{V}(t) = \sum_j c_j \hat{U}_j \cos(\omega_j t + \varphi_j) \quad (\text{find } c_j, \varphi_j \text{ with IC: } V(0), \dot{V}(0))$

$c_j: \text{some const., since any } c_j \hat{U}_j \text{ is still an EV to } \omega_j^2$        $\varphi_j: \text{any phase of the vibration}$

w example for FEM bar:

$\underline{M} = \frac{m}{6} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} \rightarrow \hat{\underline{U}}_0 = \begin{pmatrix} 1 \\ 1 \end{pmatrix}, \hat{\underline{U}}_1 = \begin{pmatrix} 1 \\ -1 \end{pmatrix}$   
 $(\text{consistent}) \quad \omega_0 = 0, \omega_1 = 2\sqrt{3} \sqrt{\frac{EA}{mL}}$

$\underline{M} = \frac{m}{2} \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix} \rightarrow \hat{\underline{U}}_0 = \begin{pmatrix} 1 \\ 1 \end{pmatrix}, \hat{\underline{U}}_1 = \begin{pmatrix} 1 \\ -1 \end{pmatrix}$   
 $(\text{lumped}) \quad \omega_0 = 0, \omega_1 = 2 \sqrt{\frac{EA}{mL}}$

real bar  $\rightarrow \hat{\underline{U}}_0 = \begin{pmatrix} 1 \\ 1 \end{pmatrix}, \hat{\underline{U}}_1 = \begin{pmatrix} 1 \\ -1 \end{pmatrix}$   
 $\omega_0 = 0, \omega_1 = \pi \sqrt{\frac{EA}{mL}}$

## • forced vibrations ( $\underline{F}(t) = \underline{F}_0 + \hat{\underline{F}} \cos(\Omega t)$ ) + linear elasticity:

$$\underline{U}(t) = \underline{U}_0 + \underline{V}(t) \quad (\underline{V}(t): \text{small perturbation}; \underline{U}_0 \text{ equilibrium} \quad K\underline{U}_0 = \underline{F}_0)$$

$$\underline{M}\ddot{\underline{U}} + \underline{K}\underline{U} = \underline{F}(t) \rightarrow \underline{M}\ddot{\underline{V}} + \underline{K}\underline{V} = \hat{\underline{F}} \cos(\Omega t) \rightarrow \text{particular solution: } \underline{V}_{\text{part}}(t) = (\underline{K} - \Omega^2 \underline{M})^{-1} \hat{\underline{F}} \cos(\Omega t)$$

$$\rightarrow \underline{V}(t) = \underline{V}_{\text{homog}}(t) + \underline{V}_{\text{part}}(t) \quad (\text{homogen solution} = \text{free vibration}) \quad \leftarrow \text{if } \Omega = \omega; \text{ some eigenfreq. then } V_{\text{part}} \rightarrow \infty!$$

## • modal decomposition (use vibration solution to easily find solution for any $\underline{F}(t)$ )

idea:  $\underline{U}(t) = \sum z_i(t) \hat{\underline{U}_i}$  find coefficients  $z_i(t)$  instead of directly solving for  $\underline{U}(t)$  (is ok since  $\hat{\underline{U}_i}$  is basis of vector-space)

$$\hat{\underline{U}_j} \cdot (\underline{K} - \omega_j^2 \underline{M}) \hat{\underline{U}_i} - \hat{\underline{U}_i} \cdot (\underline{K} - \omega_j^2 \underline{M}) \hat{\underline{U}_j} = 0 \rightarrow (\hat{\underline{U}_j} \cdot \underline{K} \hat{\underline{U}_i} - \hat{\underline{U}_i} \cdot \underline{K} \hat{\underline{U}_j}) - (\hat{\underline{U}_j} \cdot \omega_j^2 \underline{M} \hat{\underline{U}_i} - \hat{\underline{U}_i} \cdot \omega_j^2 \underline{M} \hat{\underline{U}_j}) = 0 \rightarrow \begin{cases} \hat{\underline{U}_i} \cdot \underline{K} \hat{\underline{U}_j} = \hat{\underline{U}_j} \cdot \underline{K} \hat{\underline{U}_i} \\ \hat{\underline{U}_i} \cdot \underline{M} \hat{\underline{U}_j} = \hat{\underline{U}_j} \cdot \underline{M} \hat{\underline{U}_i} \end{cases} \quad \text{because } \underline{K}, \underline{M} \text{ symmetrical}$$

$$\rightarrow (\omega_i^2 - \omega_j^2) \hat{\underline{U}_i} \cdot \underline{M} \hat{\underline{U}_j} = 0 \rightarrow \text{if } i \neq j \rightarrow \omega_i^2 - \omega_j^2 \neq 0 \rightarrow \hat{\underline{U}_i} \cdot \underline{M} \hat{\underline{U}_j} = 0$$

$$\rightarrow \text{if } i = j \rightarrow \omega_i^2 - \omega_i^2 = 0 \rightarrow \hat{\underline{U}_i} \cdot \underline{M} \hat{\underline{U}_i} \neq 0 \rightarrow \text{scale } \hat{\underline{U}_i} \text{ st } \hat{\underline{U}_i} \cdot \underline{M} \hat{\underline{U}_i} = 1 \quad (\hat{\underline{U}_i} = \underline{U}_i / \sqrt{\hat{\underline{U}_i} \cdot \underline{M} \hat{\underline{U}_i}})$$

$$\hat{\underline{U}_j} \cdot (\underline{K} - \omega_j^2 \underline{M}) \hat{\underline{U}_i} = 0 \rightarrow \hat{\underline{U}_j} \cdot \underline{K} \hat{\underline{U}_i} = \omega_j^2 \hat{\underline{U}_j} \cdot \underline{M} \hat{\underline{U}_i} \rightarrow \text{if } i \neq j \rightarrow \hat{\underline{U}_j} \cdot \underline{M} \hat{\underline{U}_i} = 0 \rightarrow \hat{\underline{U}_j} \cdot \underline{K} \hat{\underline{U}_i} = 0$$

$$\rightarrow \text{if } i = j \rightarrow \hat{\underline{U}_j} \cdot \underline{M} \hat{\underline{U}_i} = 0 \rightarrow \hat{\underline{U}_j} \cdot \underline{K} \hat{\underline{U}_i} = \omega_j^2 \leftarrow \text{scaled } \hat{\underline{U}_i}$$

$$\boxed{\text{Rayleigh's formula:} \quad \omega_i^2 = \hat{\underline{U}_i} \cdot \underline{K} \hat{\underline{U}_i} / \hat{\underline{U}_i} \cdot \underline{M} \hat{\underline{U}_i}}$$

$$\underline{M}\ddot{\underline{U}} + \underline{K}\underline{U} = \underline{F}(t) \rightarrow \hat{\underline{U}_j} \cdot [\sum z_i(t) \underline{M} \hat{\underline{U}_i} + z_i(t) \underline{K} \hat{\underline{U}_i}] = \hat{\underline{U}_j} \cdot \underline{F}(t) \quad \forall j \rightarrow \sum [z_i(t) \hat{\underline{U}_j} \cdot \underline{M} \hat{\underline{U}_i} + z_i(t) \hat{\underline{U}_j} \cdot \underline{K} \hat{\underline{U}_i}] = \hat{\underline{U}_j} \cdot \underline{F}(t) \quad \forall j$$

$$\rightarrow \ddot{z}_j(t) + \omega_j^2 z_j(t) = \hat{\underline{U}_j} \cdot \underline{F}(t) \quad \forall j \rightarrow \text{system of equations becomes decoupled! easier to solve. } (\hat{\underline{U}_j} \text{ must be scaled!})$$

## • Time-stepping (dynamic mech. prob. with finite difference method in time) not same $\underline{C}$ as in Voigt notation!

$$\text{general dynamic mech. prob.: } \underline{M}\ddot{\underline{U}} + \underline{C}\dot{\underline{U}} + \underline{F}_{\text{int}}(\underline{U}) + \underline{F}_{\text{ext}}(t) = 0 \quad \left( \underline{C}: \text{damping matrix. e.g.: } \underline{C} = \alpha \underline{M} + \beta \underline{K} \quad \alpha: \text{damps low freq.; } \beta: \text{damps high freq.} \right)$$

## • explicit time integration (1st/2nd order central diff. scheme)

$$\underline{M} \cdot \frac{\underline{U}^{dt+1} - 2\underline{U}^a + \underline{U}^{a-1}}{\Delta t^2} + \underline{C} \cdot \frac{\underline{U}^{dt+1} - \underline{U}^a}{2\Delta t} + \underline{F}_{\text{int}}(\underline{U}^a) - \underline{F}_{\text{ext}}(t^a) = 0 \Rightarrow \left( \frac{\underline{M}}{\Delta t^2} + \frac{\underline{C}}{2\Delta t} \right) \cdot \underline{U}^{dt+1} = \frac{2\underline{M}}{\Delta t^2} \cdot \underline{U}^a + \left( \frac{\underline{C}}{2\Delta t} - \frac{\underline{M}}{\Delta t^2} \right) \cdot \underline{U}^{a-1} - \underline{F}_{\text{int}}(\underline{U}^a) + \underline{F}_{\text{ext}}(t^a)$$

$$\hookrightarrow \text{stability (without proof): } \Delta t \leq \Delta t_{\max} = 2/\omega_{\max} \leftarrow \text{highest eigenfreq.} \leftarrow \omega_{\max} \propto 1/\text{smallest element size}$$

## • Newmark- $\beta$ method (implicit FDM + special scheme)

$$\text{finite difference scheme: } \begin{aligned} \underline{U}^{dt+1} &= \underline{U}^a + \Delta t (\gamma \underline{\ddot{U}}^{dt+1} + (1-\gamma) \underline{\ddot{U}}^a) \\ (\beta, \gamma: \text{some constants}) \quad \underline{U}^{dt+1} &= \underline{U}^a + \Delta t \underline{\dot{U}}^a + \frac{\Delta t^2}{2} (2\beta \underline{\ddot{U}}^{dt+1} + (1-2\beta) \underline{\ddot{U}}^a) \end{aligned} \quad \begin{aligned} \underline{\ddot{U}}^{dt+1} &= \frac{1}{\beta \Delta t^2} (\underline{U}^{dt+1} - \underline{U}^a - \Delta t \underline{\dot{U}}^a) - \frac{1-2\beta}{2\beta} \underline{\ddot{U}}^a \\ \underline{\dot{U}}^{dt+1} &= \underline{U}^a + \Delta t (1-\gamma) \underline{\dot{U}}^a + \frac{\gamma}{\beta \Delta t} (\underline{U}^{dt+1} - \underline{U}^a + \Delta t \underline{\dot{U}}^a) - \gamma \Delta t \frac{1-2\beta}{2\beta} \underline{\ddot{U}}^a \end{aligned}$$

$$\hookrightarrow \text{plug into mech. prob.: } \underline{M}\ddot{\underline{U}}^{dt+1} + \underline{C}\dot{\underline{U}}^{dt+1} + \underline{F}_{\text{int}}(\underline{U}^{dt+1}) - \underline{F}_{\text{ext}}(t^{dt+1}) = 0$$

$$\left( \frac{1}{\beta \Delta t^2} \underline{M} + \frac{\gamma}{\beta \Delta t} \underline{C} \right) \underline{U}^{dt+1} + \underline{F}_{\text{int}}(\underline{U}^{dt+1}) = \underline{F}_{\text{ext}}(t^{dt+1}) + \underline{M} \left( \frac{1}{\beta \Delta t^2} \underline{U}^a + \frac{1}{\beta \Delta t} \underline{\dot{U}}^a + \left( \frac{1}{2\beta} - 1 \right) \underline{\ddot{U}}^a \right) + \underline{C} \left( \frac{\gamma}{\beta \Delta t} \underline{U}^a + \left( \frac{\gamma}{\beta} - 1 \right) \underline{\dot{U}}^a + \Delta t \left( \frac{\gamma}{2\beta} - 1 \right) \underline{\ddot{U}}^a \right)$$

$$\underline{M}^* \underline{U}^{dt+1} + \underline{F}_{\text{int}}(\underline{U}^{dt+1}) = \underline{F}_{\text{ext}}^*(t^{dt+1}, \underline{U}^a, \underline{\dot{U}}^a, \underline{\ddot{U}}^a)$$

$$\text{algorithm: } \begin{cases} \text{solve } \underline{M}^* \underline{U}^{dt+1} + \underline{F}_{\text{int}} = \underline{F}_{\text{ext}} \text{ for } \underline{U}^{dt+1} \\ (\text{linear elastic: } (\underline{M}^* + \underline{K}) \underline{U}^{dt+1} = \underline{F}_{\text{ext}}) \\ \text{compute } \underline{\ddot{U}}^{dt+1} \text{ and } \underline{\dot{U}}^{dt+1} \text{ with } \underline{U}^{dt+1} \end{cases}$$

$$\text{stability: } \begin{cases} \text{if } \gamma \geq \frac{1}{2} \text{ and } \beta \geq \frac{1}{2}: \text{ stable for any } \Delta t \\ \text{if } \gamma \geq \frac{1}{2} \text{ and } \beta < \frac{1}{2}: \text{ stable for } \Delta t_{\max} \propto 1/\omega_{\max} \\ \text{else: unknown} \end{cases}$$

## • general:

$$\underline{\ddot{U}}(t) = \dots \text{ (for } t \in [t^a, t^{dt+1}])$$

$$\hookrightarrow \underline{\dot{U}}^{dt+1} = \underline{U}^a + \int_{t^a}^{t^{dt+1}} \underline{\ddot{U}}(t) dt$$

$$\hookrightarrow \underline{U}^{dt+1} = \underline{U}^a + \int_{t^a}^{t^{dt+1}} \underline{\dot{U}}(t) dt$$

## • average acceleration:

$$\underline{\ddot{U}}(t) = \frac{1}{2} (\underline{\ddot{U}}^{dt+1} + \underline{\ddot{U}}^a)$$

$$\hookrightarrow \underline{\dot{U}}^{dt+1} = \underline{U}^a + \frac{\Delta t}{2} (\underline{\ddot{U}}^{dt+1} + \underline{\ddot{U}}^a)$$

$$\hookrightarrow \underline{U}^{dt+1} = \underline{U}^a + \Delta t \underline{\dot{U}}^a + \frac{\Delta t^2}{4} (\underline{\ddot{U}}^{dt+1} + \underline{\ddot{U}}^a)$$

$$\hookrightarrow \beta = \frac{1}{4}, \gamma = \frac{1}{2}$$

## • linear acceleration:

$$\underline{\ddot{U}}(t) = \underline{\ddot{U}}^a + \frac{t-t^a}{\Delta t} (\underline{\ddot{U}}^{dt+1} - \underline{\ddot{U}}^a)$$

$$\hookrightarrow \underline{\dot{U}}^{dt+1} = \underline{U}^a + \frac{\Delta t}{2} (\underline{\ddot{U}}^{dt+1} + \underline{\ddot{U}}^a)$$

$$\hookrightarrow \underline{U}^{dt+1} = \underline{U}^a + \Delta t \underline{\dot{U}}^a + \frac{\Delta t^2}{6} (\underline{\ddot{U}}^{dt+1} + 2\underline{\ddot{U}}^a)$$

$$\hookrightarrow \beta = \frac{1}{6}, \gamma = \frac{1}{2}$$

## • explicit central diff.:

$$\underline{\ddot{U}}(t) = \underline{\ddot{U}}^a$$

$$\hookrightarrow \underline{\dot{U}}^{dt+1} = \underline{U}^a + \frac{\Delta t}{2} (\underline{\ddot{U}}^{dt+1} + \underline{\ddot{U}}^a)$$

$$\hookrightarrow \underline{U}^{dt+1} = \underline{U}^a + \Delta t \underline{\dot{U}}^a + \frac{\Delta t^2}{2} \underline{\ddot{U}}^a$$

$$\hookrightarrow \beta = 0, \gamma = \frac{1}{2}$$

### • HHT- $\alpha$ method (variation of Newmark- $\beta$ )

same as Newmark- $\beta$ , but instead of solving  $\underline{\underline{M}}\underline{\underline{U}}^{d+1} + \underline{\underline{C}}\underline{\underline{U}}^{d+1} + \underline{\underline{F}}_{int}(\underline{\underline{U}}^{d+1}) = \underline{\underline{F}}_{ext}(t^{d+1})$ , solve the following: ( $\alpha$ : some const.)

$$\underline{\underline{M}}\dot{\underline{\underline{U}}}^{d+1} + \underline{\underline{C}}[(1-\alpha)\dot{\underline{\underline{U}}}^d + \alpha\dot{\underline{\underline{U}}}^a] + (1-\alpha)\underline{\underline{F}}_{int}(\underline{\underline{U}}^d) + \alpha\underline{\underline{F}}_{int}(\underline{\underline{U}}^a) = (1-\alpha)\underline{\underline{F}}_{ext}(t^{d+1}) + \alpha\underline{\underline{F}}_{ext}(t^a)$$

→ substitute  $\dot{\underline{\underline{U}}}^{d+1}, \dot{\underline{\underline{U}}}^d$  with Newmark- $\beta$  formulas. → solve for  $\underline{\underline{U}}^{d+1}$

stability: if  $\alpha \leq \frac{1}{3}$  and  $\beta = \frac{(1+\alpha)^2}{4}$  and  $\gamma = \alpha + \frac{1}{2}$  → stable for any  $\Delta t$

(increasing  $\alpha$  removes high frequency noise!)

• remarks: in general: implicit schemes underpredict  $w_i$ ; → use w/ consistent  $\underline{\underline{M}}$  (that overpredicts  $w_i$ )  
explicit schemes overpredict  $w_i$ ; → use w/ lumped  $\underline{\underline{M}}$  (that underpredicts  $w_i$ )

### Extensions

#### • finite kinematics (large deformations)

for NOT small deformations, classical stress/strain does not longer work! instead use e.g.:

$$\underline{x}(\underline{X}, t) = \underline{X} + \underline{v}(\underline{X}, t) \longrightarrow \underline{F} = \frac{\partial \underline{x}}{\partial \underline{X}} \quad (F_{ij} = \frac{\partial x_i}{\partial X_j}) \longrightarrow W = \text{func}(\underline{F}) \longrightarrow \underline{P} = \frac{\partial W}{\partial \underline{F}} \quad (P_{ij} = \frac{\partial W}{\partial F_{ij}})$$

( $x$ : deformed,  $X$  undeformed) ( $F$ : deformation gradient replaces  $\epsilon$ ) ( $W$ : strain energy density) ( $P$ : Piola-Kirchhoff stress tensor replaces  $\sigma$ )

#### • thermal problem

strong form:  $\kappa \nabla^2 T + g s = g c_v \dot{T}$  subj.to:  $T = \hat{T} \text{ on } \partial \Omega_D$  ( $T$ : temperature,  $s$ : heat source)  
 $-\kappa \nabla T \cdot n = \hat{q} \text{ on } \partial \Omega_N$  ( $\kappa$ : therm. conductivity,  $c_v$ : heat capacity)  
 $T = T_0 \text{ at } t=0$

weak form (static):  $\min I[T] = \int_{\Omega} \kappa/2 |\nabla T|^2 dV - \int_{\Omega} g s T dV + \int_{\Omega} \hat{q} T dS$  subj.to:  $T \in \mathcal{U} = \{H^1(\Omega) : T = \hat{T} \text{ on } \partial \Omega_D\}$   
 $0 = \delta I[T] = \int_{\Omega} \kappa T_i \delta T_i dV - \int_{\Omega} g s \delta T dV + \int_{\Omega} \hat{q} \delta T dS$  subj.to:  $T \in \mathcal{U}, \delta T \in \mathcal{U}_0 = \{H^1(\Omega) : T = 0 \text{ on } \partial \Omega_D\}$   
 $(T(\underline{x}) \approx T^h(\underline{x}) = \sum_{a=1}^n T^a N^a(\underline{x})) ; \delta T(\underline{x}) \approx V^h(\underline{x}) = \sum_{b=1}^n V^b N^b(\underline{x})$   
 $\underbrace{\int_{\Omega} \kappa N^a_i N^b_i dV}_{K^h} \underbrace{V^b - (\int_{\Omega} s N^b dV - \int_{\partial \Omega_N} \hat{q} N^b dS)}_{Q} = 0 \quad \forall V^b$

weak form (dyn.): ...  $\rightarrow \underline{\underline{C}} \dot{\underline{\underline{T}}}^h + \underline{\underline{K}} \underline{\underline{T}}^h - \underline{\underline{Q}} = 0$  ( $C^{ab} = \int_{\Omega} g c_v N^a N^b dV$ )

### Error sources (FEM)

- Discretization error: mesh only approximates real body  $\Omega$
- Numerical integration error: if numerical integration is used, error grows with element size and function "roughness"
- Solution error: when numerically solving a lin. eq.  $\underline{\underline{K}}\underline{\underline{U}}=\underline{\underline{F}}$  error is made if  $\underline{\underline{K}}$  is ill conditioned ( $\|\underline{\underline{K}}\| \cdot \|\underline{\underline{K}}^{-1}\| = |\lambda_{\max}/\lambda_{\min}| \gg 1$ )
- Approximation error: shape func. + coefficients can only approximate real function
- Truncation error: computers can only store numbers with finite precision
- Modelling error: model vs. reality mismatch

### Adaptive mesh refinement

(example with CST elements)

- 1) smoothen strains =  $\epsilon^*$ : e.g. assign "nodal strain" to all nodes by averaging strains of adjacent elements weighted by volume and interpolate these nodal strains over the elements
- 2) ZZ error estimation: compute error measure as integral over element of difference of smooth  $\epsilon^*$  and normal  $\epsilon$  squared. if ZZ error of an element is over a threshold split that element (e.g.: longest edge bisection)
- 3) refine mesh:

## Common pitfalls

- **poor mesh quality:**
  - appropriate mesh size for scale of expected stress changes
  - element size changes should be gradual
  - there must not be any "hanging nodes" (nodes that could move into an adjacent element)
  - avoid element distortions (elements should be close to reference shape)
- **wrong element type:**
  - completeness order of element must be sufficient
  - numerical integration must have full integration
- **linear vs. nonlinear:** if deformations are large, don't use linear analysis (this course)
- **boundary conditions:**
  - fix rigid body modes (don't allow entire body to rotate/translate)
  - avoid point loads as there  $\sigma \rightarrow \infty$  and FEA never converges
  - don't exploit symmetries in vibration analysis
- **do convergence study!**

## Einstein Notation

repeating index: sum over dimensions

index after comma: partial derivative in that dimension

$v, w$ : vectors ;  $T, S$ : matrices ;  $\phi, \psi$ : scalar ;  $e_i$ : basis vectors ;  $\epsilon_{ij}$ : Levi-Civita symbol

$v$	$[v]_i = v_i$
$v \cdot w$	$v_i w_i$
$v \otimes w$	$[v \otimes w]_{ij} = v_i w_j$
$v \times w$	$[v \times w]_k = v_i w_j \epsilon_{ijk}$
$v \cdot (w \times x)$	$v_i w_j x_k \epsilon_{ijk}$
$T$	$[T]_{ij} = T_{ij}$
$T^\top$	$[T^\top]_{ij} = T_{ji}$
$Tv$	$[Tv]_i = T_{ij} v_j$
$T^\top v$	$[T^\top v]_i = T_{ji} v_j$
$v \cdot Tw$	$v_i T_{ij} w_j$
$TS$	$[TS]_{ik} = T_{ij} S_{jk}$
$T^\top S$	$[T^\top S]_{ik} = T_{ji} S_{jk}$
$TS^\top$	$[TS^\top]_{ik} = T_{ij} S_{kj}$
$T \cdot S$	$T_{ij} S_{ij}$
$T \times v$	$[T \times v]_{il} = T_{ij} v_k \epsilon_{jkl}$
$T \otimes v$	$[T \otimes v]_{ijk} = T_{ij} v_k$
$v \times T$	$[v \times T]_{il} = v_j T_{ki} \epsilon_{jkl}$
$v \otimes T$	$[v \otimes T]_{ijk} = v_i T_{jk}$
$\text{tr } T$	$T_{ii}$
$\det(T)$	$\epsilon_{ijk} T_{i1} T_{j2} T_{k3}$
$\text{grad } \phi$	$[\text{grad } \phi]_i = \phi_{,i}$
$\text{grad } v$	$[\text{grad } v]_{ij} = v_{i,j}$
$\text{grad } T$	$[\text{grad } T]_{ijk} = T_{ij,k}$
$\text{div } v$	$v_{i,i}$
$\text{div } T$	$[\text{div } T]_i = T_{ij,j}$
$\text{curl } v$	$[\text{curl } v]_k = v_{i,j} \epsilon_{jik}$
$\text{curl } T$	$[\text{curl } T]_{ij} = T_{ik,l} \epsilon_{lkj}$
$\text{div}(\text{grad } \phi)$	$[\text{div}(\text{grad } \phi)]_i = \phi_{,kk}$
$\text{div}(\text{grad } v)$	$[\text{div}(\text{grad } v)]_i = v_{i,kk}$

divergence theorem:

$$\int_{\Omega} \text{div}(\underline{v}) dV = \int_{\partial\Omega} \underline{v} \cdot \underline{n} dS \Leftrightarrow \int_{\Omega} v_{i,i} dV = \int_{\partial\Omega} v_i n_i dS$$

$$\int_{\Omega} \text{div}(\underline{T}) dV = \int_{\partial\Omega} \underline{T} \cdot \underline{n} dS \Leftrightarrow \int_{\Omega} T_{ij,j} dV = \int_{\partial\Omega} T_{ij} n_j dS$$

$$\int_{\Omega} \text{div}(\underline{v}\phi) dV = \int_{\Omega} \text{div}(\underline{v})\phi + \underline{v} \cdot \text{grad}(\phi) dV = \int_{\partial\Omega} \underline{v} \cdot \underline{\phi} n dS$$

$$\int_{\Omega} (v_i \phi)_{,i} dV = \int_{\Omega} v_{i,i} \phi + v_i \cdot \phi_{,i} dV = \int_{\partial\Omega} v_i \phi n_i dS$$

$$\int_{\Omega} \text{div}(\underline{T}\underline{v}) dV = \int_{\Omega} \text{div}(\underline{T})\underline{v} + \underline{T} \cdot \text{grad}(\underline{v}) dV = \int_{\partial\Omega} \underline{T} \cdot \underline{v} n dS$$

$$\int_{\Omega} (T_{ij} v_j)_{,i} dV = \int_{\Omega} T_{ij,j} v_i + T_{ij} v_{i,j} dV = \int_{\partial\Omega} T_{ij} n_j v_i dS$$

divergence product rules:

$$\text{div}(\underline{v}\phi) = \text{div}(\underline{v})\cdot\phi + \underline{v} \cdot \text{grad}(\phi) \Leftrightarrow (v_i \phi)_{,i} = v_{i,i} \cdot \phi + v_i \cdot \phi_{,i}$$

$$\text{div}(\underline{T}\cdot\underline{v}) = \text{div}(\underline{T})\cdot\underline{v} + \underline{T} \cdot \text{grad}(\underline{v}) \Leftrightarrow (T_{ij} v_j)_{,i} = T_{ij,j} v_i + T_{ij} v_{i,j}$$

integration by part:

$$\int_a^b \phi \Psi_{,x} dx = [\phi\Psi]_a^b - \int_a^b \phi_{,x} \Psi dx \quad (1D)$$

$$\int_a^b \phi \frac{d\Psi}{dx} dx = [\phi\Psi]_a^b - \int_a^b \phi \frac{d\phi}{dx} \Psi dx$$

$$\int_{\Omega} \Psi \cdot \phi_{,i} dV = \int_{\partial\Omega} \Psi \cdot \phi \cdot n dS - \int_{\Omega} \Psi_{,i} \cdot \phi dV$$

$$\int_{\Omega} \phi \text{div}(\underline{v}) dV = \int_{\partial\Omega} \phi \underline{v} \cdot \underline{n} dS - \int_{\Omega} \phi \text{grad}(\phi) \cdot \underline{v} dV$$

$$\int_{\Omega} \phi v_{i,i} dV = \int_{\partial\Omega} \phi v_i n_i dS - \int_{\Omega} \phi_{,i} v_i dV$$

$$\int_{\Omega} \underline{v} \cdot \text{div}(\underline{T}) dV = \int_{\partial\Omega} \underline{v} \cdot \underline{T} \cdot \underline{n} dS - \int_{\Omega} \underline{v} \cdot \text{grad}(\underline{T}) \cdot \underline{n} dV$$

$$\int_{\Omega} v_i T_{ij,j} dV = \int_{\partial\Omega} v_i T_{ij} n_j dS - \int_{\Omega} v_{i,j} T_{ij} dV$$