1 Introduction

...

The rest of this paper is structured as follows: In section 2 some background about conservation laws, their physical context and some general solution methods are provided. Later, in section 3, the focus shifts towards the their application of modelling traffic. There also some typical modelling approaches are provided. In that section we also introduce the notations of a network and how they can be used to model urban traffic flow. In section 4 we talk about how the traffic flow model on network can be solved numerically. In particular the notion of Riemann solvers are introduced and their usability is discussed. The application of Riemann solvers on toy examples as well as their application on a real traffic network in Munich are provided in section 5.

2 Scalar conservation laws

Many natural processes observed in nature, as well as practical problems in science and engineering are based on the conservation of physical values. In particular thoses processes can be described by a hyperbolic system of time-dependent, mostly non-linear, partial differential equations. In one space dimension the system can be written in a rather simple form

$$\frac{\delta}{\delta t}u(x,t) + \frac{\delta}{\delta x}f(u(x,t)) = 0$$

where $u : \mathcal{R} \times \mathcal{R} \to \mathcal{R}^n$ denotes n conserved quantities or state variables, e.g. densities or energy levels. In particular $u(x,t)$ represents the distribution of the conserved quantity on the space interval at every time step t. Furthermore, the total mass of an observed quantity on a space interval $[x_1, x_2]$ is given by $\int_{x_1}^{x_2} u(x,t)dx$ at a fixed time t. Knowing the mass distribution $u(x,t)$ at a certain time step, the specific movement of the quantities in space dimension is given by the so-called **flux function** $f : \mathcal{R}^n \to \mathcal{R}^n$. Typically the flux function is not linear in u, which results in a system of non-linear partial differential equations (for an example, see [LeVeque, 1992])

2.1 Derivation of scalar conservation laws

Conservation laws usually originate in physical principals such as the conservation of energy or mass. In modelling traffic flow this is based on the fact that during the movement of the traffic neither cars are created nor

destroyed and the total density of cars stays constant for all times t. Let u be the traffic density and $u(x,t)$ the density at a point x on an infinitely long road at some time t. Then the total amount of cars on a given space interval from x_1 to x_2 is given by the integral over the density as given in 2:

$$\text{amount of cars } = \int_{x_1}^{x_2} u(x,t)dx \tag{1}$$

For conserved quantities no mass is neither destroyed nor created. Therefore the mass on an interval can only change by inflowing and outflowing quantities. Let $v(x,t)$ be the velocity of the wave at a point x. Then the quantity mass crossing the point x at a time t is given by the **mass flux**

$$f(u) = u(x,t)\ v(x,t) \tag{2}$$

Since the total mass on the global space interval is preserved, the rate of change of the mass on the interval $[x_1, x_2]$ is defined by the difference of the in- and outfluxes of the interval, e.g.

$$
\begin{aligned}
\frac{d}{dt} \int_{x_1}^{x_2} u(x,t)dx &= f(u(x_1,t)) - f(u(x_2,t)) \\
&= u(x_1,t)v(x_1,t) - u(x_2,t)v(x_2,t)
\end{aligned}
$$

Rewriting this assuming differentiability of both functions u and f and integration over a time interval $[t_1, t_2]$ leads to the differential form of the conservation law:

$$\int_{t_1}^{t_2} \int_{x_1}^{x_2} \frac{d}{dt}u(x,t)\ dxdt = \int_{t_1}^{t_2} \int_{x_1}^{x_2} \frac{d}{dx}\{-u(x,t)v(x,t)\}\ dxdt \tag{3}$$

Since this has to hold for arbitrary time intervals $[t_1, t_2]$ on arbitrary sections $[x_1, x_2]$ we conclude the partial differential equation for the **conservation of mass**

$$u_t + (uv)_x = 0 \tag{4}$$

This conservation law can only be solved in isolation, if the velocity v is known *a priori* or as a function dependent on $u(x,t)$, cf. [LeVeque, 1992]. Typical representations for the velocity function $v(u)$ and the flux function $f(u)$, also called *characteristic function*, used in traffic flow modelling will be discussed in section 3.2.

In order to formulate a Cauchy-problem, the conservation law 4 has to be supplemented by suitable initial conditions.

2.2   Method of characteristics

2.3   Weak solutions

Definition - Fallunterscheidung

2.3.1   The Riemann problem

Cauchy problems with piecewise constant initial data providing a single discontinuity are known as Riemann problems.

TODO:
- Definition schwache Loesung

2.3.2   The wave front tracking method

TODO:

- Fallunterscheidungen (vgl. [Garavello and Piccoli, 2006])

- Example for

  - shock
  - non-admissible shock
  - rarefaction wave

2.3.3   Admissibility

TODO: see [Dafermos, 2005]

- Lax-admissibility

3   Traffic flow modelling

3.1   The macroscopic Lighthill-Whitham-Richards model

3.2   Characteristic diagrams

3.3   The Riemann problem

4   Networks and Riemann solvers

So far we have only solved traffic flow problems on the real line. In order to analyze traffic flow more sophisticated problems under realistic scenarios, in

particular in urban environments, it is necessary to extend the mathematical theory and the representation of traffic networks. The method of choice is hereby to represent traffic networks via directed graphs. This directed graph consists of a set of edges, representing the roads, and a set of nodes representing junctions. Every junction is thereby characterized by a finite number of incoming and a finite number of outgoing roads.

4.1   Basic principles of networks

Rigorously we can define a network as followed, see e.g. **ZITAT EINFU-GEN**

Definition 4.1 *A network is a tuple $(\mathcal{N}, \mathcal{E})$, where $\mathcal{N}$ is a finite collection of vertices, and $\mathcal{E}$ is a finite collection of M edges. Each edge $e_i = [a_i, b_i] \in \mathbb{R}, i = 1...N$ is defined over a real interval. Furthermore, every node $n_j, j = 1...M$ is a union of two non-empty sets $IN(n_j)$ and $OUT(n_j)$, where $IN(J), OUT(J) \in \{1...N\}$.*

We further require the following properties:

1) For $n_j, n_i \in \mathcal{N}, j \neq i$ we require $IN(n_j) \cap IN(n_i) = \emptyset$ and $OUT(n_j) \cap OUT(n_i) = \emptyset$

2) If $e_i \notin \bigcup_{n \in \mathcal{N}} IN(n)$ then $b_i = \infty$, and if $e_i \notin \bigcup_{n \in \mathcal{N}} OUT(n)$ then $a_i = -\infty$

These assumptions only guarantee that the resulting network is indeed a valid graph. Requirement 1) implies that every road starts and ends in at most one junction. Requirement 2) then says that if a road does not start (end) at a junction then it is considered as an inflow (outflow) of the network.

Another method to represent a road network is via so-called **adjescency matrices** ([Biggs, 1993]).

Definition 4.2 *Let $\mathcal{N}$ be a set of edges with $|\mathcal{N}| = n$. Then the adjescency matrix is a square $n \times n$-matrix A with*

$$A_{ij} = \begin{cases} 1 & \text{if there exists a junction between road } i \text{ and } j \\ 0 & \text{else} \end{cases}$$

<u>Remark:</u>
Having given the network as in 4.1 one can easily derive the corresponding adjescency matrix as in 4.2, and vice versa. **BEING MORE SPECIFIC???   also about problem of non-unique indexing of junctions?**

Example 1 *Let us consider an example*

4.2 Riemann solver at junctions

5 Traffic flow at intersections

5.1 General setting

Consider a family of $n + m$ roads, all joining at a single junction. The indices $i \in \{1, ..., m\} =: \mathcal{I}$ hereby denote *incoming roads* and indices $j \in \{m + 1, ..., m + n\} =: \mathcal{O}$ denote *outgoing roads*. Then the evolution of the density of cars $\rho_k(x, t)$ on the $k-th$ road can be described by the conservation law

$$\rho_t + f_k(\rho)_x = 0. \tag{5}$$

- Maximum fluxes on incoming roads:

$$\omega_i(\rho) := \{ \begin{array}{ll} f(\rho) & \rho \leq \sigma \\ f(\sigma) & \rho > \sigma \end{array}$$

- Maximum fluxes on outgoing roads:

$$\omega_j(\rho) := \{ \begin{array}{ll} f(\sigma) & \rho \leq \sigma \\ f(\rho) & \rho > \sigma \end{array}$$

  This property ensures that Riemann problems on incoming roads are solved by waves with negative speed, and Riemann problems on outgoing roads are solved by waves with positive speed.

5.2 Junction model

Problem with uniqueness

5.3 Junction model with buffers

In order to be useful for the analysis of global optimization, the used traffic flow model at junctions should provide two crucial properties:

- Well posedness for $\mathcal{L}^\infty$ data

- Continuous solution w.r.t. weak convergence

Due to the ill-posedness of the general junction model in 5.2 for certain input data, we need to come up with a different approach.

In [Bressan, 2014] a model is proposed where each intersection in the model includes a **buffer** with limited capacity. The current filling level of the buffer in front of the outgoing road $j \in \mathcal{O}$ may be denoted as the *queue length* $q_j(t)$. The rate of flux at which cars from incoming roads enter the intersection is controlled by the current length of the queues. The outgoing fluxes are governed by the queue length and the personal preference destination of the individual drivers, as well as the maximum permitted fluxes on the designated roads.

The main results of their analysis can be stated as:

I. If the queue lenghts q_j for every outgoing road are given, the initial boundary value problems on each road become decoupled and can be solved individually, first on every incoming road, and secondly on every outgoing road. The densities $\rho_k(t, x)$ on every road $k = 1...m + n$ can then be explicitly computed via a Lax type formula.

II. Given the densities, the lenghts q_j of the queues can be determined by balancing the influxes and outfluxes of the intersection. The queue lengths can finally be obtained as a fixpoint of a contractive transformation $q \rightarrow \Delta(q)$ where q needs to be Lipschitz continuous.

III. The buffer model is thus well-posed at intersections for general $\mathcal{L}^\infty$ data. It is also shown that the traffic flow model is continous w.r.t. weak convergence.

The interested reader is referred to [Bressan, 2014] for the proofs.

Setting of the buffer model - SBJ and MBJ

Let the general setting be as defined in 5.1. Further we include two realistic assumtions for the boundary values at the entrance and exit points of junctions:

i. **Driver's preferences** Θ_{ij} define the fraction of cars on road i that want to exit the junction in direction of road j.

$$\Theta_{ij} \in [0, 1], \qquad \sum_j \Theta_{ij} = 1$$

  ii. **Relative priorities η_i given to incoming roads** e.g. external effects like traffic lights

In general, $\Theta_{ij} = \Theta_{ij}(t, x)$ needs not to be necessarily constant, but can be time- and location-dependent. In the following it is assumed that the drivers' preferences are known in advance and that they do not change their itinerary throughout the network. Then the conservation law reads

$$(\Theta_{ij}\rho_i)_t + [\rho_i\Theta_{ij}v(\rho_i)]_x = 0$$

Using product rule and reordering yields

$$\rho_i[(\Theta_{ij})_t + v(\rho_i)(\Theta_{ij})_x] + \Theta_{ij}[(\rho_i)_t + (\rho_iv(\rho_i))_x] = 0$$

The second term vanishes as the general conservation law still needs to be fulfilled. For zero density on the road this is fulfilled trivially. For $\rho_i \neq 0$ we obtain the passive scalar transport equation along the flux:

$$(\Theta_{ij})_t + v(\rho_i)(\Theta_{ij})_x = 0 \tag{6}$$

A similar approach has been persued by []. In their intersection model the capacity of the queues is arbitrarily big. Cars wanting to ender road j but exceed the maximum outflux of the intersection are instead stored in the queue. As a consequence there is no backward propagation of queues and therefore no emergence of shocks on incoming roads.
The model by [Bressan, 2014] extends this model. Consider a single junction and let $M > 0$ be the maximum capacity of the queue. Then the incoming fluxes into the junction depend on the current degree of occupancy of the buffer, which is defined by

$$q = (q_j)_{j \in \mathcal{O}}, \qquad q \in \mathbb{R}^n$$

The Cauchy problem for traffic flow on a network can thus be formulated as

$$\begin{aligned} (\rho_i)_t + f_i(\rho_i)_x &= 0 \\ (\Theta_{ij})_t + v(\rho_i)(\Theta_{ij})_x &= 0 \end{aligned}$$

supplemented by initial conditions
$\vdots$
Conservation of mass yields the additional differential equation

$$\dot{q}_j = \sum_{i \in \mathcal{I}} \Theta_{ij}\bar{f}_i - \bar{f}_j \tag{7}$$

where $\bar{f}_j$ denote the boundary fluxes at the junction, $k \in \mathcal{I} \cup \mathcal{O}$.

We are now ready to state two sets of equations regarding the incoming and outgoing junction fluxes depending on the drivers' choices Θ_{ij} and the queue lenghts q_j.

The first model provides a shared buffer of capacity M for every outgoing junction. Incoming cars can cross the intersection governed by the amount of free space left in the queue, regardless of the car's destination. Once within the junction, cars leave at the maximum rate allowed by the outgoing road of their choice.

Single Buffer Junction model (SBJ): Consider a constant $M > 0$, describing the maximum capacity of the junction at any given time, and constants $c_i > 0, i \in \mathcal{I}$ accounting for priorities given to the differnt incoming roads.
We then require that the incoming fluxes $\bar{f}_i$ satisfy

$$\bar{f}_i = \min\{\omega_i, c_i(M - \sum_{j \in \mathcal{O}} q_j)\}, \qquad i \in \mathcal{I}$$

Furthermore, the outgoing fluxes $\bar{f}_j$ should satisfy

$$\bar{f}_j = \begin{cases} \omega_j & q_j > 0 \\ \min\{\omega_j, \sum_{i \in \mathcal{I}} \bar{f}_i \bar{\Theta}_{ij}\} & q_j = 0 \end{cases}$$

As seen, the outgoing fluxes are uniquely defined once the incoming fluxes are known in addition to the current status of the queue.

The second model uses n different buffers, one for every outgoing road. Once having entered the junction, cars are admitted to their desired road of destination depending on the length of the queue in front of the desired road.

Multiple Buffer Junction model (MBJ): Consider constants $M_j > 0, j \in \mathcal{O}$, describing the maximum capacitis of the buffers in front of the n outgoing roads of the junction at any given time, and constants $c_i > 0, i \in \mathcal{I}$ accounting for priorities given to the differnt incoming roads.
We then require that the incoming fluxes $\bar{f}_i$ satisfy

$$\bar{f}_i = \min\{\omega_i, \frac{c_i(M_j - q_j)}{\Theta_{ij}}, j \in \mathcal{O}\}, \qquad i \in \mathcal{I}$$

Furthermore, the outgoing fluxes $\bar{f}_j$ should satisfy

$$\bar{f}_j = \begin{cases} \omega_j & q_j > 0 \\ \min\{\omega_j, \sum_{i \in \mathcal{I}} \bar{f}_i \bar{\Theta}_{ij}\} & q_j = 0 \end{cases}$$

Limitations of this approach:

- Consider a trivial junction with one incoming and one outgoing road and initial data $\rho_{in} > \sigma$ and $\rho_{out} = 0$. Then the result won't be continuous.

6 Numerical approximation of conservation laws

[LeVeque, 1992] chapter 10, 12

Most naive approach: Finite difference method (or 2-stencil upwind method)

$$\rho(x_i, t_{k+1}) = \rho(x_i, t_k) + \frac{dt}{dx}(f(\rho(x_{i+1}, t_k) - f(\rho(x_i, t_k))) \tag{8}$$

Problem with discontinuous data:

- method might converge to a solution that does not solve the original PDE

TODO:

- Consistency

6.1 The Godunov method

[LeVeque, 1992]

The Godunov scheme, first proposed in [Godunov, 1959], uses the numerical solution U^n to define a piecewise constant function $\tilde{u}^n(x, t_n) = U_j^n$ on the grid $x_{j-\frac{1}{2}} < x < x_{j+\frac{1}{2}}$, where $x_j = x_0 + j\Delta x$.

At time t_n this agrees with the piecewise constant function $U_k(x, t_n)$ that has been introduced in section 6, but $\tilde{u}^n$, unlike U_k will not be constant over $t_n < t < t_{n+1}$. Instead we make use of the fact that $\tilde{u}^n$ is piecewise constant, which generates a sequence of Riemann problems. This enables us to solve the conservation law in an exact manner over short time intervals $t_n < t < t_{n+1}$.

pic from LeVeque p. 139

From this exact solution we obtain the approximate solution U_j^{n+1} at time t_{n+1} by averaging again over the space interval $\left[x_{j-\frac{1}{2}}, x_{j+\frac{1}{2}}\right]$:

$$U_j^{n+1} = \frac{1}{h} \int_{x_{j-\frac{1}{2}}}^{x_{j+\frac{1}{2}}} \tilde{u}^n(x, t_{n+1}) \tag{9}$$

These averages then define new piecewise constant functions $\tilde{u}^{n+1}(x, t_{n+1})$ and the process repeats. In practice these averages are computed by using the integral form of the conservation law. As it is assumed that $\tilde{u}^n$ is a weak solution, we know that

$$\int_{x_{j-\frac{1}{2}}}^{x_{j+\frac{1}{2}}} \tilde{u}^n(x, t_{n+1}) dx = \int_{x_{j-\frac{1}{2}}}^{x_{j+\frac{1}{2}}} \tilde{u}^n(x, t_n) dx + \int_{t_n}^{t_{n+1}} f(\tilde{u}^n(x_{j-\frac{1}{2}}, t)) dt$$
$$- \int_{t_n}^{t_{n+1}} f(\tilde{u}^n(x_{j+\frac{1}{2}}, t)) dt$$

Using the notation of 9 we get

$$U_j^{n+1} = U_j^n - \frac{k}{h} \left[F(U_j^n, U_{j+1}^n) - F(U_{j-1}^n, U_j^n) \right] \tag{10}$$

with the **numerical flux**

$$F(U_j^n, U_{j+1}^n) := \frac{1}{k} \int_{t_n}^{t_{n+1}} f(\tilde{u}^n(x_{j+\frac{1}{2}}, t)) dt \tag{11}$$

Having the Godunov scheme written in the form of 10, one speaks of a method in *conservative form*. The function $\tilde{u}^n$ is constant on the line $x = x_{j+\frac{1}{2}}$ over the time interval $t_n \leq t \leq t_{n+1}$. We denote this value by $u^\star(U_j^n, U_{j+1}^n)$ which implies that the numerical flux reduces to

$$F(U_j^n, U_{j+1}^n) = f(u^\star(U_j^n, U_{j+1}^n)) \tag{12}$$

Finally we can write the **Godunov scheme** as

$$U_j^{n+1} = U_j^n - \frac{k}{h} \left[f(u^\star(U_j^n, U_{j+1}^n)) - f(u^\star(U_{j-1}^n, U_j^n)) \right] \tag{13}$$

TODO:

- Courant number

- deduction of the numerical flux values

Numerical fluxes for Riemann problems:
If we use the prior assumption that f is concave, we obtain for the numerical flux values

$$f(u^{\star}(U_j^n, U_{j+1}^n)) = \begin{cases} \min_{U_j^n \leq u \leq U_{j+1}^n} f(u) & \text{if } U_j^n \leq U_{j+1}^n \\ \min_{U_{j+1}^n \leq u \leq U_j^n} f(u) & \text{if } U_j^n > U_{j+1}^n \end{cases} \tag{14}$$

6.2 Godunov method at junctions

Suppose a traffic flow model for a network with one junction with initial conditions as in 5.1. Also consider a discretization $\{x_0...x_{6N_j}\}$ for every road $j \in \mathcal{I} \cup \mathcal{O}$.
Then for roads connected to the junction J at the right point - so for $i \in \mathcal{I}(J)$ we set

$$U_{N_i}^{n+1} = U_{N_i}^n - \frac{k}{h} \left[\bar{f}_i - f(u^{\star}(U_{N_i-1}^n, U_{N_i}^n)) \right] \tag{15}$$

Similarily for roads connected to the junction J at the left point - so for $j \in \mathcal{O}(J)$ we set

$$U_0^{n+1} = U_0^n - \frac{k}{h} \left[f(u^{\star}(U_0^n, U_1^n)) - \bar{f}_j \right] \tag{16}$$

where $\bar{f}_i, \bar{f}_j$ are the maximum junction fluxes defined in section 5.3, depending on the chosen model version.

6.3 Examples

7 Experiments

7.1 Single buffer junction model

7.1.1 Experiment 1

Let us consider a single junction of two incoming and two outgoing roads. Initially every road provides maximum flux density, namely $\rho_i = 0.5$, smoothly on the entire space. The buffer zone offers maximum capacity $M = 0.5$ and cars approaching the junction from both roads can cross it with equal priority value $c_i = 0.5$. The values of M and c were chosen such that cars can enter the junction with maximum flux in case of an empty buffer and a smaller

flux else (cf. 9.3.1). The goal of the experiment is to see the evolution of the traffic densities over time in case of a non-equal distribution of the traffic at the junction. Therefore we assume a transition matrix $TM = \begin{pmatrix} 0.2 & 0.4 \\ 0.8 & 0.6 \end{pmatrix}$.

Results:
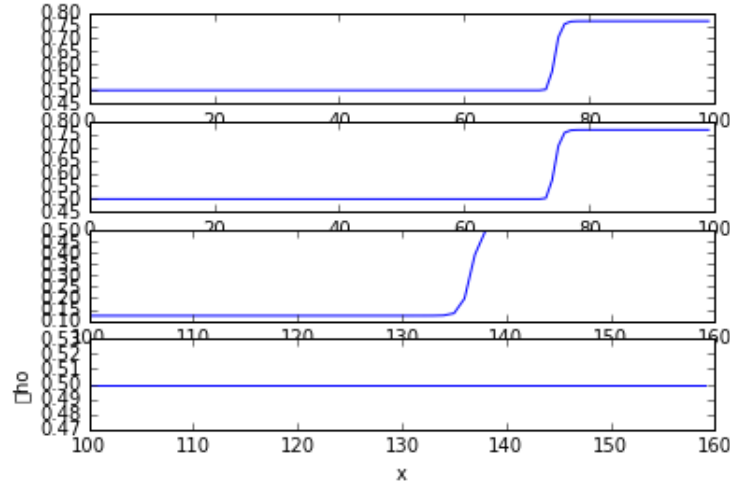 More cars want to enter road 4 $\rightarrow$ road would be congested $\rightarrow$ buffer builds
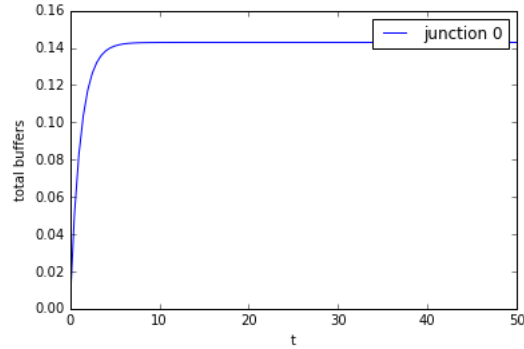

Figure 1: Traffic distribution after $t = 100s$.

Figure 2: Buffer status over time up to $t = 100s$.

up over time.
Obviously the question arises if the total mass of cars on the network is

preserved. It is apparent that the total mass on the network is preserved, as

Figure 3: Evolution of the total mass of traffic on the network up to time $t = 100s$.

long as the total influx equals the total outflux of the network. Considering incoming and outgoing roads as half-lines, the total mass is preserved.

Remark:
As we can only consider a finite part of the network, we have to require the compatibility condition $\int_{\delta\Omega} f \cdot n \ ds = 0$ TRUE???? in order to preserve the amount of cars on the network.

A case where the compatibility condition is not satisfied can by easily constructed by taking the setting of Example 1 but leaving one outgoing road empty. In this case more cars are entering the network - in particular, the incoming flow equals $2f(0.5)$ - compared to the number of cars initially leaving the network - $f(0.5)$ from road 3, as road 4 is initially empty. This implicates an increase of the total traffic mass on the network (see figure 4).

8   Optimization

Average travel time: [Colombo et al., 2011] From the point of view of drivers, key quantities determining the quality of traffic are related to the time necessary to reach the destination.

- total quantity of vehicles entering the road between $t = t_0, t = t_{end}$:
  $Q_{in} = \int_{t_0}^{t_{end}} q_0(t)dt$, where q_0 is the inflow into the road.
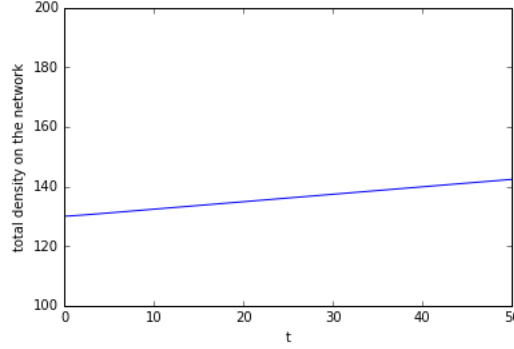
Figure 4:   Evolution of the total mass of traffic on the network for non-fulfilled compatibility condition.

- avg. time to reach point $\bar{x}$ is then given by $T(\bar{x}) := \frac{1}{Q_{in}} \int_{t_0}^{t_{end}} t f(\rho(\bar{x}, t)) dt$

- approximation over a finite sum over the discretization of t

- Problem on networks: considering a population starting at road i and ending at road j. then it is possible that cars starting at road $k \neq i$ also cross the end of road j, so they are counted for T but not in Q_{in}

8.1   Modelling traffic lights

As in section 5.1 stated, we con model traffic lights assigning priority values η_i for incoming roads $i \in \mathcal{I}(J)$ into a junction J.

$$\sum_{i \in \mathcal{I}(J)} \eta_i = 1 \tag{17}$$

A first naive approach is to combine η with the preference values of Θ_{ij} by multiplying $\Theta\eta$ to obtain a new distribution matrix for every junction.

Experiments

Observations

- the preference values Θ_{ij} only affect the outgoing fluxes of the junction -see section 5.3, not the incoming. As a result, as long as no queues emerge there will be no backward probagation of queues if cars have to wait at a red phase.
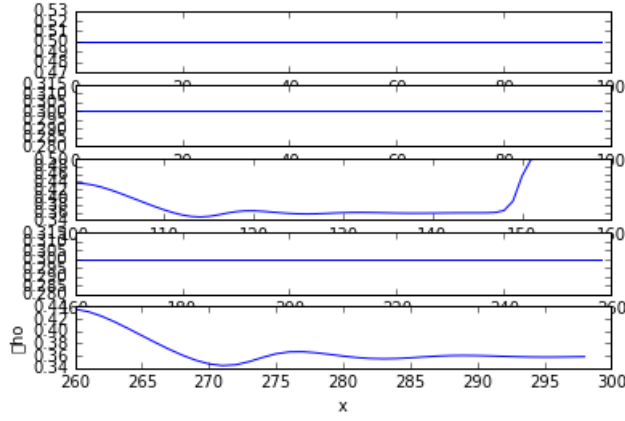
Figure 5: cap

Opposingly cars that cannot exit the junction due to redphase are just lost as the total amount of traffic decreases rapidly.
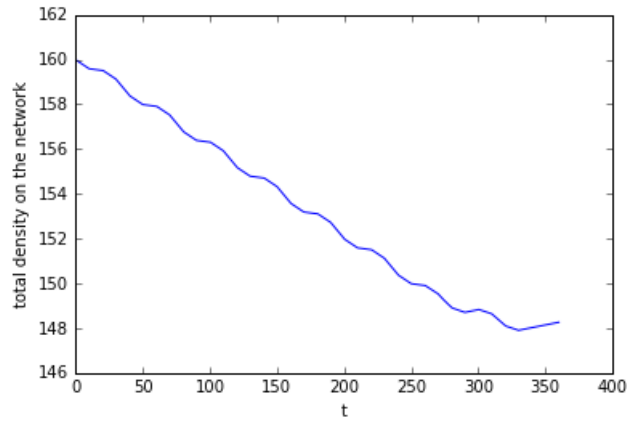

Figure 6: cap

9 Additional Notes - still to be included

9.1 Setting

9.1.1 Scalar conservation law

Consider a single junction consisting of m incoming and n outgoing roads. The corresponding index sets are $\mathcal{I} = \{1, ..., m\}$ and $\mathcal{O} = \{m+1, ..., m+n\}$, respectively. On road k the scalar conservation law is thus defined as:

$$\rho_t + f_k(\rho)_x = 0$$

In the infinite setting we have $x \in [-\infty, 0]$ for incoming and $x \in [0; \infty]$ for outgoing roads, $t \geq 0$.
The flux function on road k is defined via $f_k(\rho) = \rho * v(\rho)$, where $v(\rho)$ denotes the average speed of cars on the road and $\rho \in [0, \rho^{jam}]$ the density of cars.

In order to solve the corresponding Riemann problem on the network, boundary conditions have to imposed. Thus we define

9.2 Networks

9.2.1 Traffic flow on networks

- complex networks:

  - nodes $\mathcal{N}$ represent junctions
  - roads $i \in \mathcal{I}, j \in \mathcal{O}$ represent roads **TODO: differenciation between in-and outgoing maybe later**
  - $A \in \mathbb{R}^{(\geq+\ltimes)\times(\geq+\ltimes)}$ adjacency matrix representing the network, e.g. $A = \begin{pmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \\ 1 & 1 & 0 \end{pmatrix}$ would represent the network displayed in 7.
  - In contrast to the adjacency matrix, transition matrices are defined only for single junctions. Let J be a junction with m incoming and n outgoing roads. Then the transition matrix

$TM_J \in \mathbb{R}^{n \times m}$ is defined by

$$TM_J = \begin{pmatrix} P_{m+1,1} & \cdots & P_{m+1,m} \\ \vdots & \ddots & \vdots \\ P_{m+n,1} & \cdots & P_{m+n,m} \end{pmatrix}$$

From this adjacency matrix one can easily derive the transition matrices for every single junction. In the stated example one would conclude

$$TM_J = \begin{pmatrix} 1 & 1 \end{pmatrix}$$

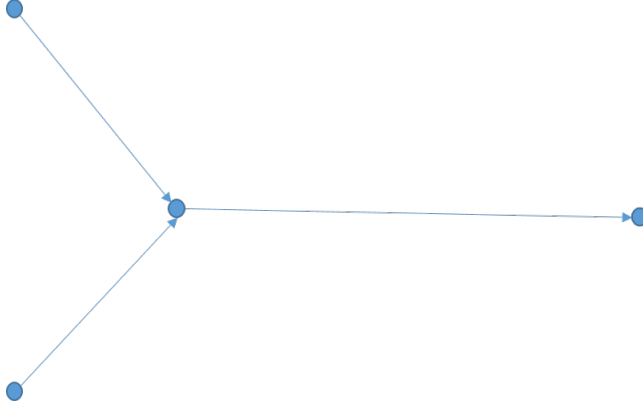as the transition matrix representing the single merge.

Figure 7: Simple merge of two roads

— **Driver's preferences** Θ_{ij} define the fraction of cars on road i that want to exit the junction in direction of road j.

$$\sum_j \Theta_{ij} = 1$$

— **Relative priorities given to incoming roads** e.g. external effects like traffic lights

9.2.2 The Riemann problem on networks

9.3 Models

9.3.1 Single buffer junction model (SBJ)

As the example before has shown there is no hope that the previous approach guarantees a unique solution for arbitrary initial conditions and as-

signed preference values Θ_{ij}. We can circumvent this behavior by modelling a junction not as a fixed point, but instead as a buffer with fixed capacity that can "store" cars in a queue.

Consider a constant $M > 0$, describing the maximum capacity of the junction at any given time, and constants $c_i > 0, i \in \mathcal{I}$ accounting for priorities given to the differnt incoming roads.
We then require that the incoming fluxes $\bar{f}_i$ satisfy

$$\bar{f}_i = \min\{\omega_i, c_i(M - \sum_{j \in \mathcal{O}} q_j), \qquad i \in \mathcal{I}$$

Furthermore, the outgoing fluxes $\bar{f}_j$ should satisfy

$$\bar{f}_j = \{ \begin{array}{ll} \omega_j & q_j > 0 \\ \min\{\omega_j, \sum_{i \in \mathcal{I}} \bar{f}_i \bar{\Theta}_{ij}\} & q_j = 0 \end{array}$$

As seen, the outgoing fluxes are uniquely defined one the incoming fluxes are known in addition to the current status of the queue.

The queue thus changes its length by

$$\dot{q}_j = \sum_{i \in \mathcal{I}} \bar{f}_i \bar{\Theta}_{ij} - \bar{f}_j \tag{18}$$

Question:

- choice of M and c

- Discretization of 18 $\rightarrow$ difference quotient?

9.3.2   Simulation: preliminaries

- adjacency matrix

- inflow-outflow vectors

- capacity M for all junctions

- priorities c_i for all outgoing roads for every junction

9.3.3   Example: 2-2 junction

Consider a junction with 2 incoming and 2 outgoing roads and the corresponding transition matrix $TM = \begin{pmatrix} 0.4 & 0.2 \\ 0.6 & 0.8 \end{pmatrix}$ and inital densities $\rho_i(0, x) = 0.5, i = 1...4$. The queue has the following properties: $q(0) = 0, M = 1, c_3 = c_4 = 0.5$. We also have incoming fluxes into road 1 and road 2 of $\rho_{in} = 0.5$.
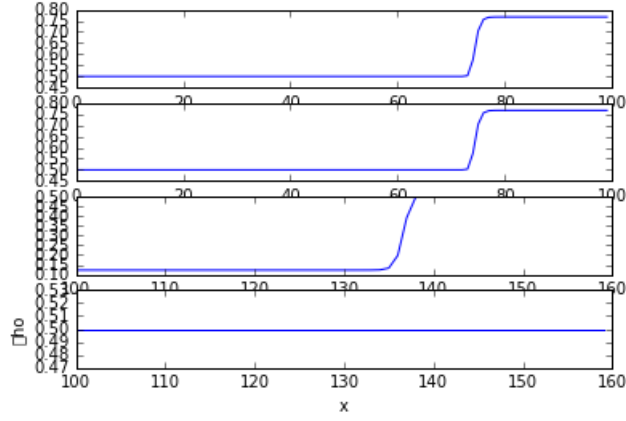**Numerical solution using Godunov scheme**

Figure 8: Traffic distribution on the roads after t=100s. First two plots show the density on the incoming roads, the bottom two plots show the outgoing roads

Obervations:

- jam emerges with idential speed for both incoming roads

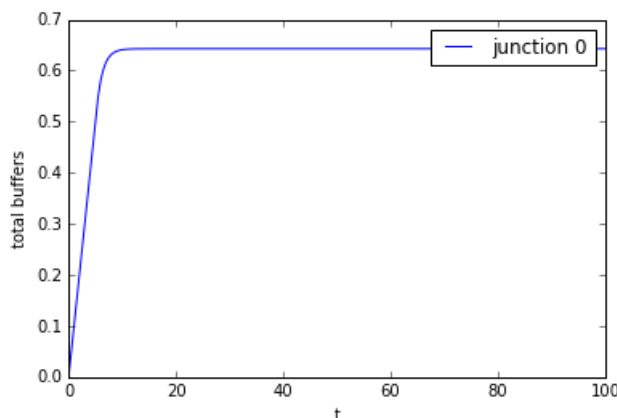- Buffer builds up in front of road 4, since the incoming flux is bigger than the maximum outgoing flux

Figure 9: Evolution of the total queue length over time

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