

Note: The dimension deviation can be up to  $\pm 75\text{mm}$  due to the tolerances and human errors as the dimensions are recorded manually.

All the dimensions must be checked on site by the contractor and such dimensions to be their responsibility.

Client Name

x

Project Address

XXXX

Section

## GARAGE CONVERSION

Stage

ARCHITECTURAL

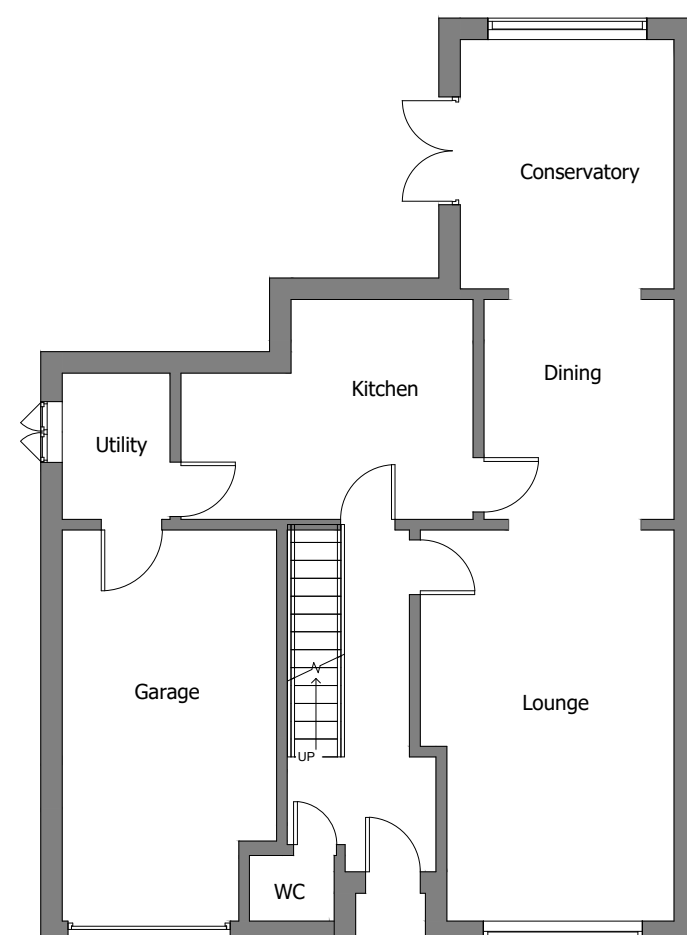
Drawing Title

## EXISTING PLANS

Drawing Status

FOR APPROVAL

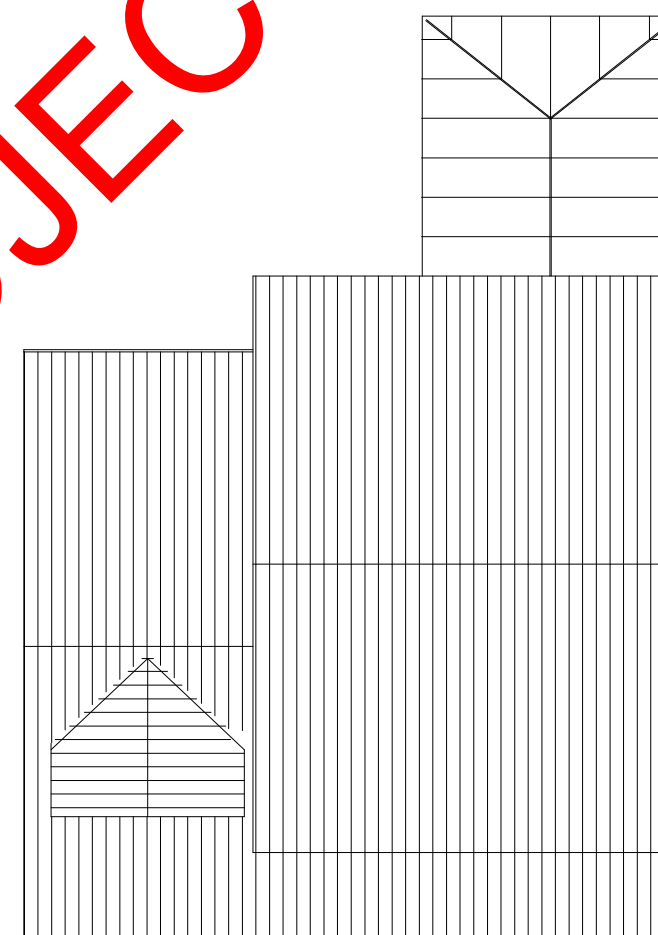
## Revisions and Notes



## GROUND FLOOR PLAN



## FIRST FLOOR PLAN



## ROOF PLAN

Project No.	2024-03-CR0 8XW
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Drawing No. **PLANNING-002**

Revision	00
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Scale at A3	1:100
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Date	06-03-24
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Designed	MM	Checked	MM
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Drawn	<b>MM</b>	Approved	<b>MM</b>
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Client Name

XX

Project Address

XXX

Section

## GARAGE CONVERSION

Stage

ARCHITECTURAL


Drawing Title


### EXISTING ELEVATIONS

Drawing Status

FOR APPROVAL

Revisions and Notes

4.Roof   
7600

3.Loft Existing 

---

5100

2.FF Existing 

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2439

1.GF Existing 

Project No.	2024-03-CR0 8XW
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Drawing No. **PLANNING-003**

Revision	00
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Scale at A3	1:100
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Date	06-03-24
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Designed	MM	Checked	MM
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Drawn	MM	Approved	MM
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Client Name

xxx00

Project Address

XXXX

Section

## GARAGE CONVERSION

Stage

ARCHITECTURAL

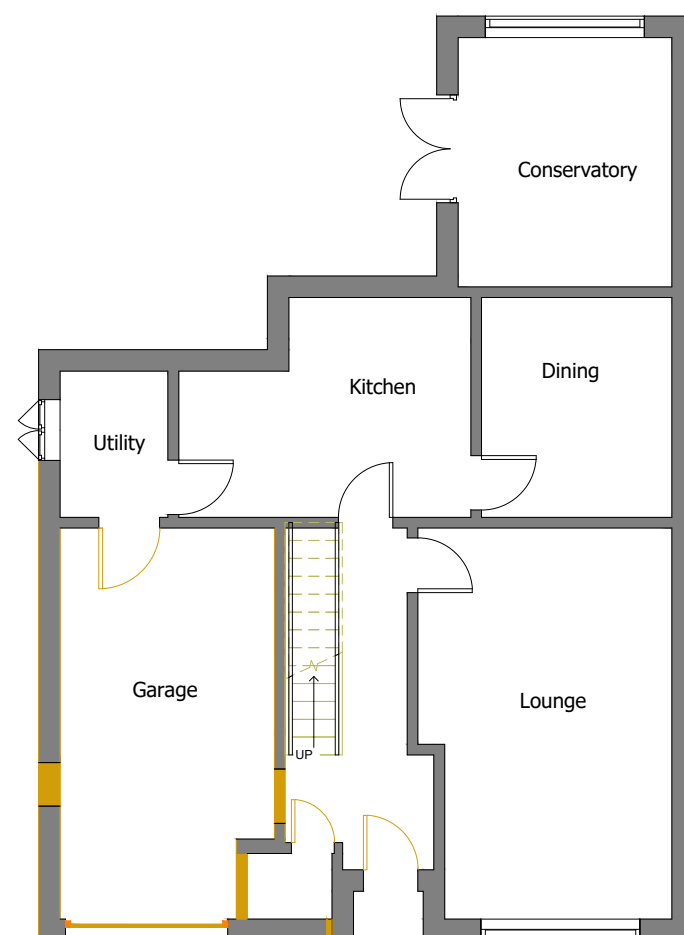
Drawing Title

## DEMO PLANS

Drawing Status

FOR APPROVAL

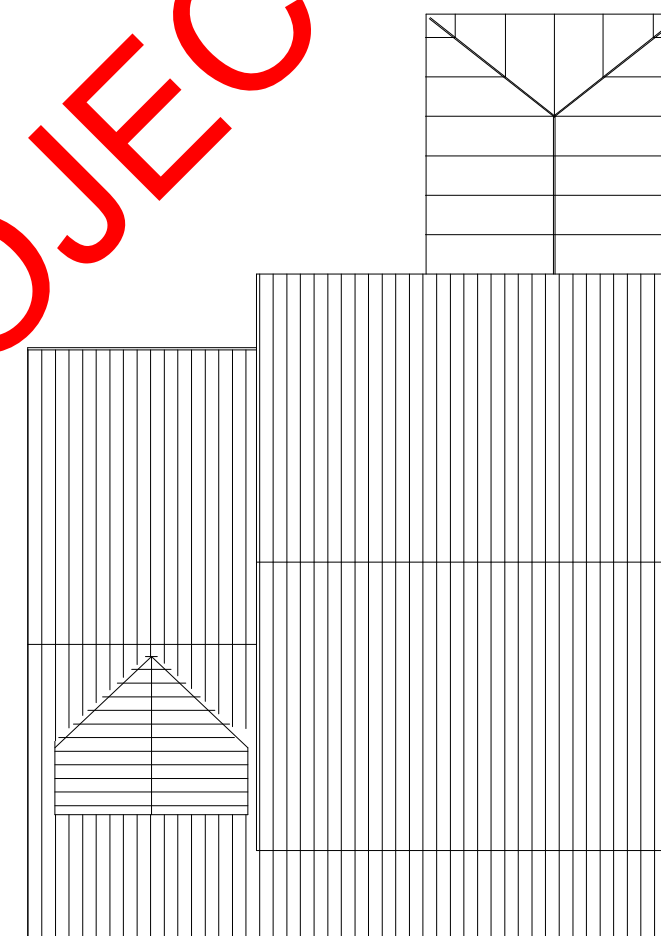
Revisions and Notes



## GROUND FLOOR PLAN



## FIRST FLOOR PLAN



## ROOF PLAN

Project No.	2024-03-CR0 8XW
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Drawing No. PLANNING-004

Revision	00
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Scale at A3	1:100
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Date 06-03-24

Designed	MM	Checked	MM
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Drawn	MM	Approved	MM
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Client Name

Project Address

Section  
GARAGE CONVERSION

Stage  
ARCHITECTURAL

Drawing Title

**DEMO ELEVATIONS**

Drawing Status

**FOR APPROVAL**

## Revisions and Notes

Project No.	2024-03-CR0 8XW
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Drawing No. **PLANNING-005**

Revision	00
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Scale at A3	1:100
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Date	06-03-24
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Designed	MM	Checked	MM
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Drawn	MM	Approved	MM
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Client Name

ANIL WANGOO

Project Address

5 IRIS CLOSE CR08XW

Section

## GARAGE CONVERSION

Stage

ARCHITECTURAL

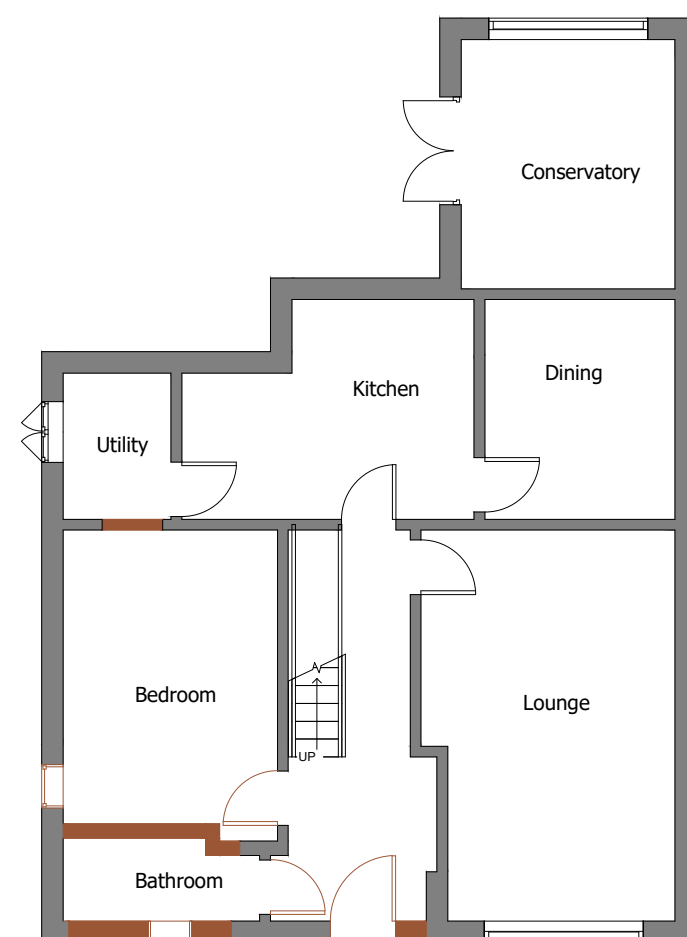
Drawing Title

## PROPOSED PLANS

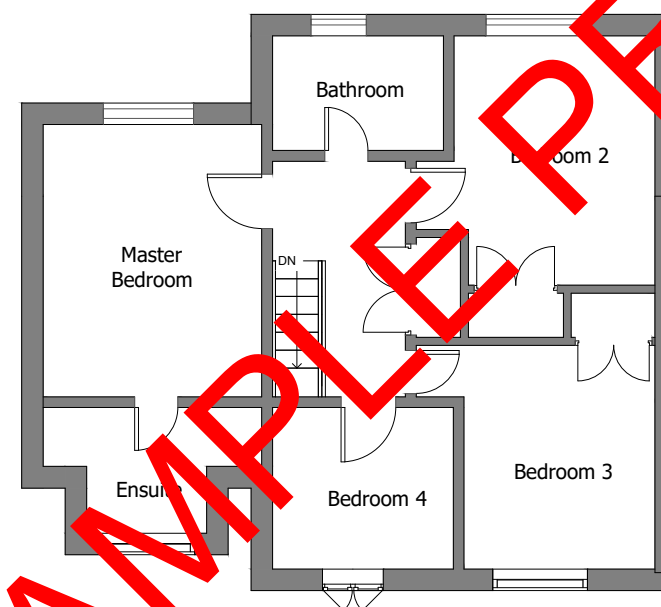
Drawing Status

FOR APPROVAL

## Revisions and Notes



## GROUND FLOOR PLAN



## FIRST FLOOR PLAN



## ROOF PLAN

Project No.

Drawing No. PLANNING-006

Revision	00
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Scale at A3	1:100
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Date	06-03-24
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Designed	MM	Checked	MM
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Drawn	MM	Approved	MM
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Phone No.: 02035763199

4. Roof ▼  
7600

3. Loft Existing ▼  
5100

2. FF Existing ▼  
2439

1. GF Existing ▼

The image shows a line drawing of a building's exterior wall. On the left, there is a set of double doors with transoms. To the right of the doors is a single window with a transom. The roofline is visible, showing a gabled structure. A large, diagonal red watermark reading 'SAMPLE PROJECT' is overlaid across the entire image.



Project Address

Stage

Drawing Status

FOR APPROVAL

## Revisions and Notes



## General Notes

1. All drawings to be read in conjunction with all relevant specifications, architect's drawings and services engineer's drawings.
2. For setting out refer to architect's drawings.
3. All dimensions are in millimetres unless noted otherwise.
4. Do not scale from the drawings or the computer digital data. Only figured dimensions to be used.

5. The contractor is to provide any temporary bracing necessary to maintain structural stability during construction.
6. The works have been designed and shall be constructed in accordance with the following codes. This list is not exhaustive and is only intended to list the principal codes:
  - a) BS EN 1991-1-1:2002, BS EN 1991-1-7:2006: Code of practice for dead and imposed loads.
  - b) BS EN 1991-1-4:2005+A1:2010: Code of practice for wind loads.
  - c) BS EN 1991-1-3:2003: Code of practice for imposed roof loads.
  - d) BS EN 1997-1:2004 : Code of practice for foundations.
  - e) BS EN 1992-1-1:2004: Structural use of concrete.
  - f) BS EN 1993-1-1:2005, BS EN 1993-1-5:2006, BS EN 1993-1-10:2005, BS EN 1993-5:2007, BS EN 1993-6:2007, BS EN 1993-1-8:2005: Structural use of steelwork in buildings.
  - g) PD 6697:2010, BS EN 1996-3:2006, BS EN 1996-2:2006, BS EN 1996-1-1:2005+A1:2012: Structural use of un-reinforced masonry.
  - h) BS EN 1995-1-1:2004+A1:2008: Structural use of timber.

7. The works have been designed for the finish state. The following superimposed loads have been used in the design:
  - Floor loads - See structural engineer's calculations.
  - Roof loads - See structural engineer's calculations.
8. All works shall comply with the Building Regulations and other relevant statutory notices e.g. Health and Safety Bylaws, COSHH etc
9. The client / appointed contractor must take their own assurances on:
  - a) Soil conditions on site and the gradient of land;
  - b) Suitability / existing methods of storm water drainage;
  - c) Trees (existing or removed) and their affect on foundations;
  - d) Position and condition of main sewer.

10. Extensions/alterations to existing structures are subject to revision depending upon such being fully exposed. The client/thier contractor must take their own assurances that any structure designated for demolition/removal are not load bearing or that alternative methods of permanent support are put in place prior to removal. Existing walls, lintels and foundations that are intended to take additional loads, must first be fully exposed and checked for adequacy prior to the commencement of works.

## Foundations

1. The depth of the proposed foundations are subject to ground conditions and building control approval. These are to be minimum 1000mm deep subject to be founding in good ground of minimum 100kN/m2 bearing stratum (based on London Clay).
2. The excavations should be free from any mature tree roots. If there are

- large trees in the vicinity then the foundations depth is to be in accordance with NHBC standards guidelines for building near trees.
3. Where new foundations are to abut existing foundations, a soft joint of 75mm is to be formed using 'Claymaster' or similar approved unless noted otherwise on the drawings.
  4. Any foundations deeper than 1.5m should have suspended floors to avoid any heave. Where the foundations are cast within highly shrinkable soils, then anti-heave precautions such as compressible materials or void formers are to be applied to the foundations.

### Notes for Timber

1. These notes are to be read in conjunction with relevant architect's and services engineer's drawings and specification.
2. All timber-work shall comply with BS EN 1995-1-1:2004+A1:2008.
3. Roof area:
  - a) Roof joists shall be grade C24. Evidence of grading shall be provided before work commences;
  - b) Blocking and battens shall be grade C24 softwood;
  - c) The sizes shown on the drawings are finished sizes;
  - d) In joint zones waness, shakes and knots are not permitted;
  - e) Timber to be carefully cut and planed to ensure tight fit and continuous bearing against metalwork;
  - f) All gaps between timber and metalwork to be resin-grouted, to the approval of the engineer.
4. All connectors, bolts, nails etc. shall be galvanised to BS 729.
5. Adhesive shall be to BS1204: Part 1: 1970, Type 1B/P.
6. All timber to be treated in accordance with the British Wood Preservative and Damp-proofing Association Commodity Specification for 40 years desired service life.

### Notes for Fire Resistance

1. These notes are to be read in conjunction with relevant architect's services engineer's drawings and specifications.
2. All habitable doors to stairs enclosure and the kitchen to be fitted with self-closing devices.
3. Any glazing within the stair enclosure, including glazing to doors, to be fire-resisting.
4. Mains powered interconnected smoke alarms to be provided to entrance lobby and all stairs landings.
5. Class 1 flame spread to be provided to all new walls and ceilings.

### Notes for Masonry

1. These notes are to be read in conjunction with relevant architect's services engineer's drawings and specifications.
2. All brickwork shall comply with PD 6697:2010, BS EN 1996-3:2006, BS EN 1996-2:2006, BS EN 1996-1-1:2005+A1:2012 .
3. All bricks shall have a minimum crushing strength of 20N/mm<sup>2</sup>.

4. Blockwork shall have a minimum crushing strength of 7N/mm<sup>2</sup>.
5. Mortar shall be a Class (ii) cement: lime putty: sand mix (1:1/2:4), unless indicated otherwise.
6. All vertical joints shall be completely filled. Bricks shall be laid frog up. The voids in perforated bricks shall be filled.
7. Fissured bricks or bricks with voids shall not be used.
8. Horizontal chases are prohibited. Vertical chases and builderswork holes shall be agreed with the architect.

## Notes for Structural Steelwork

1. These notes are to be read in conjunction with relevant architect's and services engineer's drawing and specifications.
2. All steelwork shall comply with BS EN 1993-1-1:2005, BS EN 1993-1-5:2006, BS EN 1993-1-10:2005, BS EN 1993-5:2007, BS EN 1993-6:2007, BS EN 1993-1-8:2005.
3. Unless noted otherwise stipulated structural steelwork shall conform to BS EN: Weldable structural steels.
4. Unless noted otherwise all steel shall be grade S355. Steel grade shall conform with EC-3.
5. Unless noted otherwise all butt welds shall be full penetration.  
Unless noted otherwise all fillet welds shall be full profile with a minimum leg length of 6mm.
7. Unless noted otherwise all ordinary bolt assemblies shall be Grade 8.8.
8. Unless noted otherwise all bolts shall be M16.
9. Unless noted otherwise all holding down bolts shall be M16 Grade 8.8 anchored a minimum of 200mm depth into the supporting concrete with a 100 x 100 x 8 thick washer plate at the embedded head of the bolt.
10. The clearance of base plates from supporting concrete shall be a minimum of 20mm and on completion of erection this shall be grouted solid under the full area of the base plate with 1:2 sand: cement grout.
11. Corrosion protection:
  - a) Surface protection - blast clean to SA 2.5 quality BS EN ISO 8501-1.
  - b) Prefabricator primer - epoxy zinc phosphate hb: 50 microns (DFT).
  - c) Finishing coat - see arch's spec.
  - d) See arch's specification for details on colour and texture.

12. Fire protection:
  - 30min - One layer of plasterboard and skim coat or intumescent paint to manufacturer's specification.
  - 60min - Two layers of plasterboard with joints staggered and skim coat or intumescent paint to manufacturer's specification.
13. Weather protection: Any steelwork exposed to external weather is either to be galvanized or stainless steel UNO.
14. All steel beams carrying load-bearing masonry walls wider than their flanges are to have 12mm thick top/bottom flange plates continuously welded along the length to suit the wall width UNO.

Note: The dimension deviation can be up to  $\pm 75\text{mm}$  due to the tolerances and human errors as the dimensions are recorded manually.

All the dimensions must check on site by contractor and such dimensions to be their responsibility

Client Name
Project Address
Section <b>GARAGE CONVERSION</b>
Stage <b>ARCHITECTURAL</b>
Drawing Title <b>GENERAL NOTES</b>
Drawing Status <b>FOR APPROVAL</b>

Project No.	2024-03-CR0 8XW		
Drawing No.	PLANNING-001		
Revision	00		
Scale at A3	1:100		
Date	06-03-24		
Designed	MM	Checked	MM
Drawn	MM	Approved	MM



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Client Name

Project Address

Section  
GARAGE CONVERSION

Stage  
ARCHITECTURAL

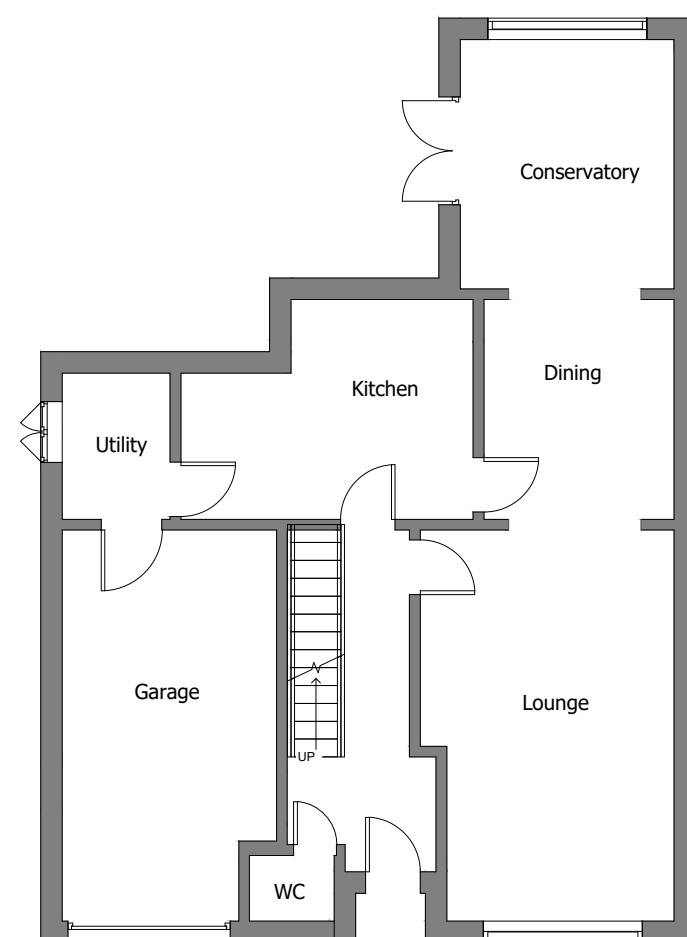
Drawing Title

**EXISTING PLANS**

Drawing Status

**FOR APPROVAL**

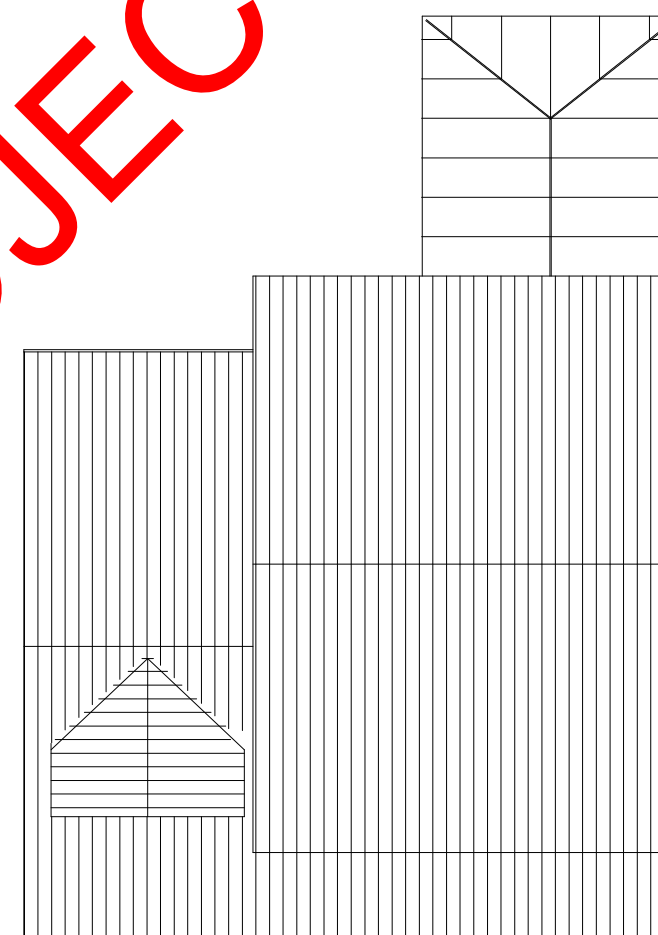
## Revisions and Notes



## GROUND FLOOR PLAN



## FIRST FLOOR PLAN



## ROOF PLAN

Project No.	2024-03-CR0 8XW		
Drawing No.	PLANNING-002		
Revision	00		
Scale at A3	1:100		
Date	06-03-24		
Designed	MM	Checked	MM
Drawn	MM	Approved	MM



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Client Name

Project Address

Section  
GARAGE CONVERSION

Stage  
ARCHITECTURAL

Drawing Title

**EXISTING ELEVATIONS**

Drawing Status

**FOR APPROVAL**

## Revisions and Notes

Project No.	2024-03-CR0 8XW
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Drawing No. **PLANNING-003**

Revision	00
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Scale at A3	1:100
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Date	06-03-24
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Designed	MM	Checked	MM
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Drawn	MM	Approved	MM
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Client Name \_\_\_\_\_

Project Address

Section

**GARAGE CONVERSION**

Stage  
ARCHITECTURAL

Drawing Title

**DEMO PLANS**

Drawing Status

**FOR APPROVAL**

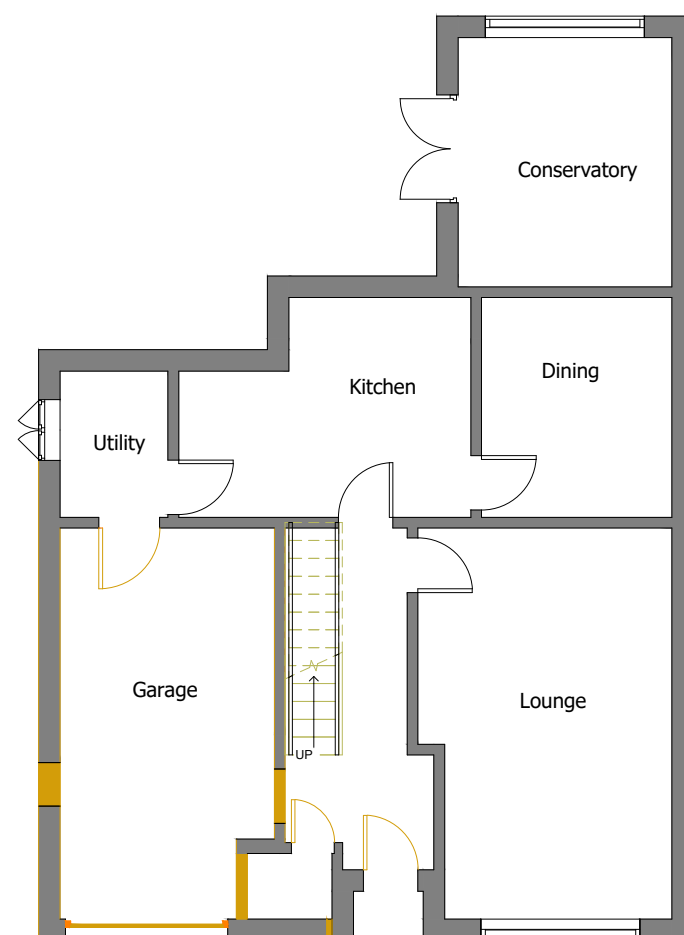
Revisions and Notes
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Drawing No. **PLANNING-004**

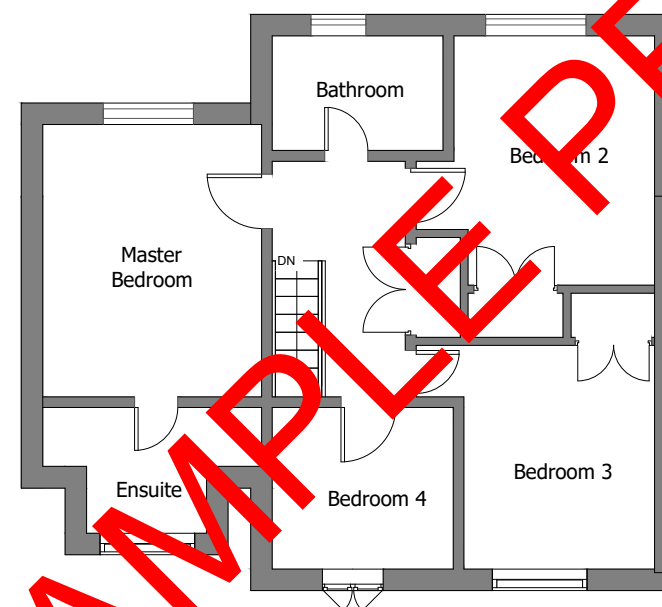
Scale at A3	1:100
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Designed	MM	Checked	MM
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Drawn  Approved 



## GROUND FLOOR PLAN



## FIRST FLOOR PLAN



## ROOF PLAN

**4. Roof** ▼  
7600

**3. Loft Existing** ▼  
5100

**2. FF Existing** ▼  
2439

**1. GF Existing** ▼

The image shows a black and white architectural elevation of a building facade. The building has a complex roofline with multiple gables and a section with vertical siding. On the left, there is a set of double doors with transoms. To the right, there is a single window with a decorative frame. A large, bold, red watermark reading 'SAMPLE PROJECT' is oriented diagonally across the entire image, from the bottom left to the top right.



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Client Name

Project Address

Section  
GARAGE CONVERSION

Stage  
ARCHITECTURAL

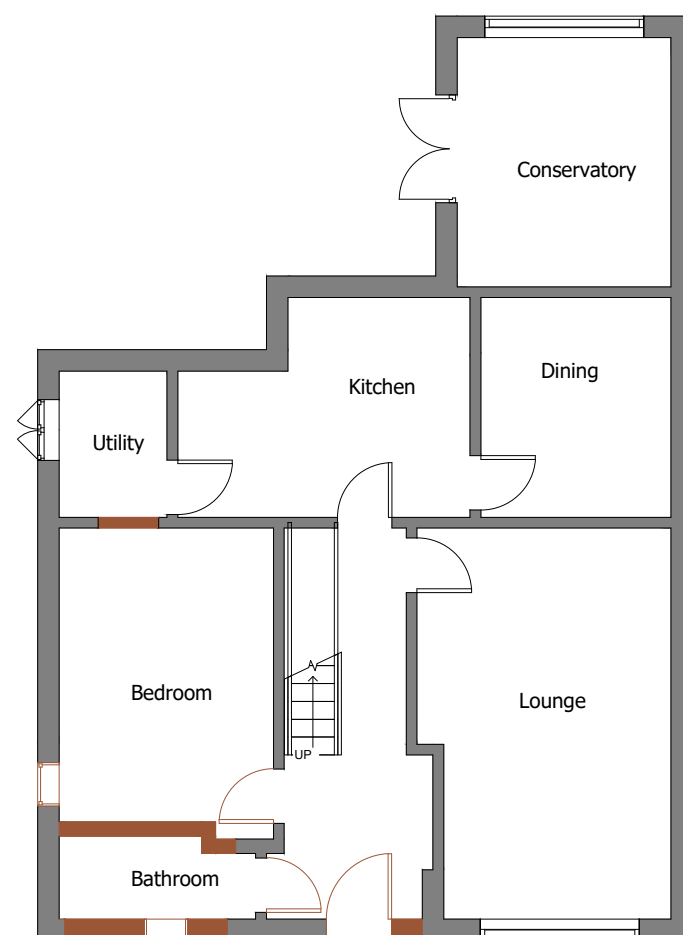
Drawing Title

**PROPOSED PLANS**

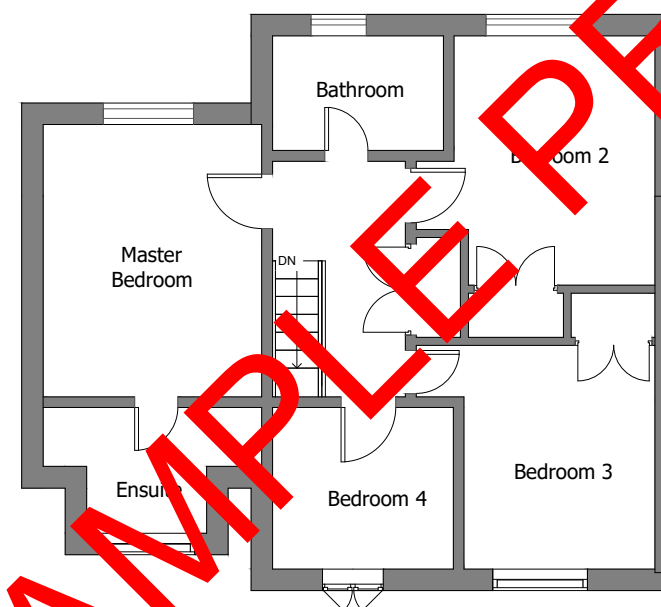
Drawing Status

**FOR APPROVAL**

## Revisions and Notes



## GROUND FLOOR PLAN



## FIRST FLOOR PLAN



## ROOF PLAN

Project No.	2024-03-CR0 8XW		
Drawing No.	PLANNING-006		
Revision	00		
Scale at A3	1:100		
Date	06-03-24		
Designed	MM	Checked	MM
Drawn	MM	Approved	MM



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ALL ELECTRICAL WORK REQUIRED TO MEET THE REQUIREMENTS OF PART P (ELECTRICAL SAFETY) MUST BE DESIGNED, INSTALLED, INSPECTED AND TESTED BY A COMPETENT PERSON REGISTERED UNDER A COMPETENT PERSON SELF CERTIFICATION SCHEME SUCH AS BRE CERTIFICATION LTD, BSI, NICEIC CERTIFICATION SERVICES OR ZURICH LTD. AN APPROPRIATE BS7671 ELECTRICAL INSTALLATION CERTIFICATE IS TO BE ISSUED FOR THE WORK BY A PERSON COMPETENT TO DO SO. A COPY OF A CERTIFICATE WILL BE GIVEN TO BUILDING CONTROL ON COMPLETION.

INSTALL LOW ENERGY LIGHT FITTINGS THAT ONLY TAKE LAMPS HAVING A LUMINOUS EFFICIENCY BETTER THAN 80 LUMENS PER CIRCUIT WATT. ALL FIXED TO HAVE LIGHTING CAPACITY (LM) 185 X TOTAL FLOOR AREA, TO COMPLY WITH PART L OF THE CURRENT BUILDING REGULATIONS AND THE DOMESTIC BUILDING SERVICES COMPLIANCE GUIDE.

L3 FIRE ALARM SYSTEM TO BS 5839 TO BE INSTALLED (PROTECTION OF ESCAPE ROUTES). MAINS POWERED SMOKE DETECTORS(SD), WITH BACKUP BATTERY, TO BE FITTED IN HALLWAY AND UPPER LANDINGS, ALL LINKED TO EACH OTHER AND ON AN INDEPENDANT CIRCUIT WITH A SEPERATE FUSE.

ALL DOORS TO HABITABLE ROOMS WITHIN STAIR ENCLOSURE TO BE FD30 DOORS FITTED WITH A PERKOMATIC SELF CLOSER. 25X38MM DOOR STOPS GLUED AND SCREWED TO FRAME. THIS IS NOT REQUIRED FOR A TWO STORY (GROUND+FIRST FLOOR) BUILDING BUT STRONGLY RECOMMENDED. DOORS TO BE FULL PANEL DOORS WITH NO GLAZING.

ALL ELECTRICAL WIRING & INSTALLATIONS TO CONFORM TO BS7671 "REQUIREMENTS FOR ELECTRICAL INSTALLATIONS" AND ANY OTHER REGULATIONS APPLICABLE TO SIMILAR RESIDENTIAL HOUSES.

**(mev) MECHANICAL EXTRACT  
VENTILATOR**

Lounge

# GROUND FLOOR WALL & LINTEL PLAN

NEW AND REPLACEMENT WINDOWS TO BE DOUBLE GLAZED WITH 16-20MM ARGON GAP AND SOFT COAT LOW-E GLASS. WINDOW ENERGY RATING TO BE BAND B OR BETTER AND TO ACHIEVE U-VALUE OF 1.4 W/M<sup>2</sup>K. THE FLOOR AND WINDOW OPENINGS SHOULD BE LIMITED TO 25% OF THE EXTENSION FLOOR AREA PLUS THE AREA OF ANY EXISTING OPENINGS COVERED BY THE EXTENSION. INSULATED PASTERBOARD TO BE USED IN REVEALS TO ABUT JAMBS AND TO BE CONSIDERED WITHIN REVEAL DETAILS. FULLY INSULATED AND CONTINUOUS CAVITY CLOSERS TO BE USED AROUND REVEALS. WINDOWS AND DOOR THRESHLES TO BE TAPED TO SURROUNDING OPENINGS USING AIR SEALING TAPE. WINDOWS TO BE FITTED WITH TRICKLE VENTS TO PROVIDE ADEQUATE BACKGROUND VENTILATION IN ACCORDANCE WITH APPROVED DOCUMENT F.

AND REPLACEMENT DOORS TO ACHIEVE A U-VALUE OF 1.4W/M<sup>2</sup>K. GLAZED AREAS TO BE DOUBLE GLAZED WITH 16-20MM ARGON GAP AND SOFT LOW-E GLASS. GLASS TO BE TOUGHENED OR LAMINATED SAFETY GLASS TO BS 6206, BS EN 14179 OR BS EN ISO 12543-1 AND PART K OF THE CURRENT BUILDING REGULATIONS.

CARE SHALL BE TAKEN TO LIMIT THE OCCURRENCE OF THERMAL BRIDGING IN THE INSULATION LAYERS CAUSED BY GAPS WITHIN THE THERMAL ELEMENT, (I.E. AROUND WINDOWS AND DOOR OPENINGS). REASONABLE PROVISION SHALL ALSO BE MADE TO ENSURE THE EXTENSION IS CONSTRUCTED TO MINIMIZE UNWANTED AIR LEAKAGE THROUGH THE NEW BUILDING FABRIC.

ALL STAGES OF WORK ARE TO BE CHECKED AND AGREED ON SITE WITH THE BUILDING INSPECTOR BEFORE COVERING OVER.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL THE TEMPORARY WORKS, THE STABILITY OF THE EXISTING STRUCTURE, EARTHWORKS, EXCAVATIONS, ETC.; THE CONTRACTOR WILL ENSURE THAT THE BUILDINGS WILL BE ADEQUATELY SUPPORTED AT ALL STAGES OF CONSTRUCTION, INCLUDING ANY EARTHWORK SUPPORTS MADE NECESSARY BY EXCAVATIONS AND GROUND CONDITIONS. THE SUPPORTS AND PROPS TO BE PROVIDED TILL THE FINAL RETAINING WALL IS IN PLACE.

## Revisions and Notes

Drawn	MM	Approved	MM
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Min 65 - 75mm sand/cement screed to be provided over insulation and underfloor heating pipework

Pipework to installed directly to rigid insulation using proprietary clip rails and clips. spaced in accordance with pipe layout design

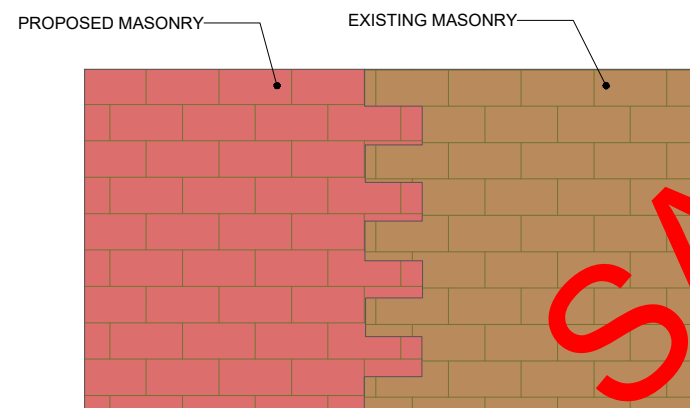
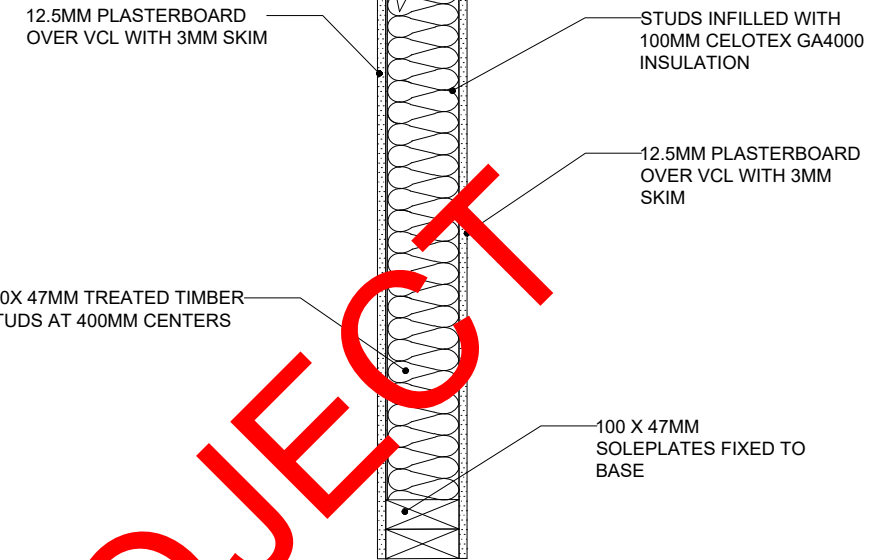
130mm Celotex XR4000 PIR INSULATION

100mm thick concrete slab

1200g damp proof membrane

150mm sand blinded hardcore

STUD WALL  
U-value 0.18 W/m<sup>2</sup>k



STRADDLE HANGER OR TWO STANDARD TYPE HANGERS JOINED TOGETHER WITH BEARING PLATE

TOP OF JOIST MAX 10MM HIGHER THAN TOP OF HANGER

GAP BETWEEN END OF JOIST AND HANGER NO MORE THAN 5MM

DO NOT UNDERSLING HANGER

JOIST TO FIT TIGHTLY INTO HANGER (MAX 6MM GAP), JOIST TO BE NOTCHED AT BOTTOM

ENSURE ALL THAT JOISTS ARE FULLY NAILED TO HANGER

WIDTH OF JOIST NOT TO BE CUT DOWN TO MAKE HANGER FIT

## STRADDLE TIMBER HANGER

Client Name
Project Address
Section GARAGE CONVERSION
Stage ARCHITECTURAL
Drawing Title STRUCTURAL DETAILS
Drawing Status FOR APPROVAL

## Revisions and Notes

Drawn	MM	Approved	MM
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Note: The dimension deviation can be up to  $\pm 75\text{mm}$  due to the tolerances and human errors as the dimensions are recorded manually.

All the dimensions must be checked on site by the contractor and such dimensions to be their responsibility.

Client Name

Project Address

Section  
GARAGE CONVERSION

Stage

ARCHITECTURAL

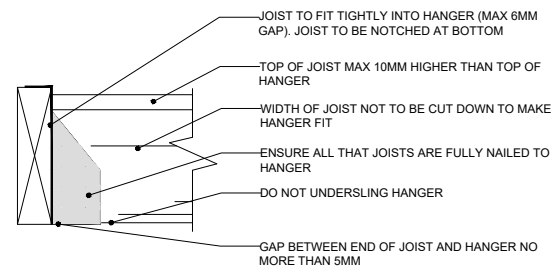
Drawing Title

**STRUCTURAL DETAILS**

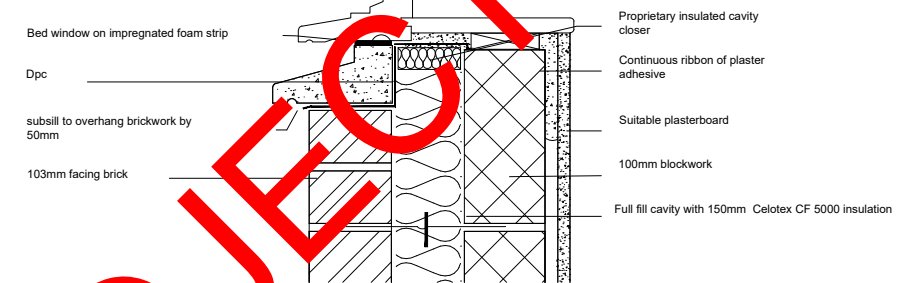
Drawing Status

FOR APPROVAL

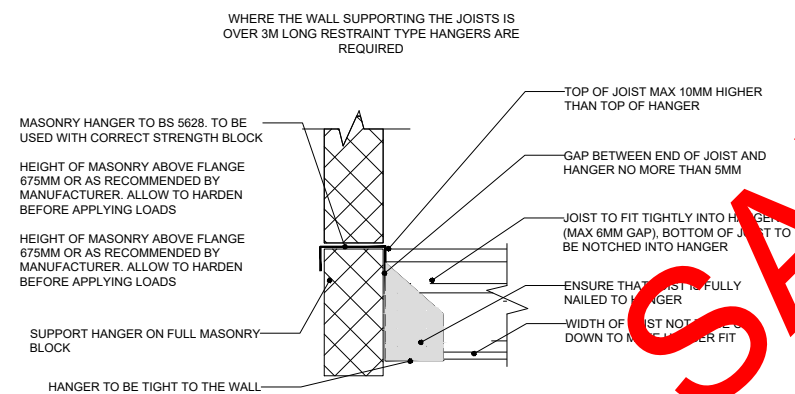
## Revisions and Notes



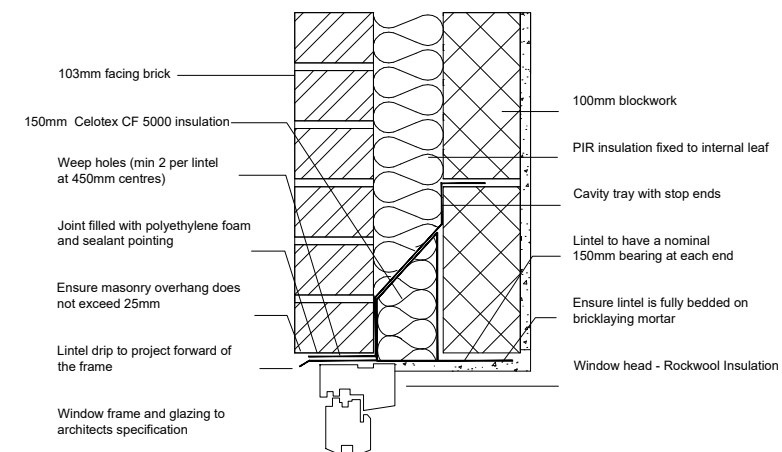
## STANDARD TIMBER HANGER



WINDOW SILL



## RESTRAINT MASONRY HANGER



## WINDOW HEAD AND LINTEL

Project No. 2024-03-CR0 8XW

Drawing No. **PLANNING-010**

Revision 00

Scale at A3                      N.T.S

Date 06-03-24

Designed	MM	Checked	MM
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Drawn	MM	Approved	MM
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## STRUCTURAL CALCULATION REPORT

**Client Name** AXXXX

**Client Address** XXXX

**Project Reference:** 2024-03- CR08XW

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Project: xxx

Sheet No./Rev.  
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Job Ref. 2024-03-  
CR08XW

Structural Engineer  
MM

Date  
22/03/2024

### Document Control:

Purpose/Status	Date	Rev.	Comments	Structural Engineer
Approval Issue	22/03/2024		B'Regs Issue	MM

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Structural Engineer

MM

Date  
22/03/2024

## Project Information

Design Codes – Eurocodes and their respective National Annexes:

BS EN 1990. Eurocode 0: 'Basis of structural design'

BS EN 1991. Eurocode 1: 'Actions on structures'

BS EN 1992. Eurocode 2: 'Design of concrete structures'

BS EN 1993. Eurocode 3: 'Design of steel structures'

BS EN 1995. Eurocode 5: 'Design of timber structures'

BS EN 1996. Eurocode 6: 'Design of masonry structures' BS EN

1997. Eurocode 7: 'Geotechnical Design'

## ASSUMPTIONS

THE FOLLOWING ASSUMPTIONS ARE MADE ABOUT THE SITE. THEY ARE TO BE CHECKED ON SITE BY THE CONTRACTOR AND BUILDING CONTROL OFFICER PRIOR TO THE START OF THE WORKS. ANY DIFFERENCES ARE TO BE REPORTED TO PEPP IMMEDIATELY.

1. The existing masonry is assumed to be minimum 3.6N/mm<sup>2</sup> blockwork in a 1:2:8 mortar
2. Floor joists are assumed to span as indicated on the drawings.
3. The external walls are assumed to be cavity brickwork.

## NOTES

Contractors to check all dimensions before ordering any steel.

All materials and workmanship must fully comply with all relevant current British Standard and Codes of practice.



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Project: xxx

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## ITEMS


### 1. LOADING DETAILS

### 2. TIMBER DESIGN

- Stud Wall 47 x 100 @ 400 c/c (C-24)
- Double Trimmer (2) (47x200) (C-24)

### 3. LINTEL

- IG L1/HD 100 (For Cavity Wall)
- IG Box HD 100 (For Solid Wall)

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## 1. LOADING DETAILS

### PITCHED ROOF

Clay Tiles	=	0.65	KN/m <sup>2</sup>
Felt and battens	=	0.05	KN/m <sup>2</sup>
Timber rafters	=	0.1	KN/m <sup>2</sup>
Insulations and other membranes	=	0.1	KN/m <sup>2</sup>
Ceiling and services	=	0.2	KN/m <sup>2</sup>
<b>Total dead load on the slope</b>	=	<b>1.1</b>	<b>KN/m<sup>2</sup></b>
<b>Live Load</b>	=	<b>0.6</b>	<b>KN/m<sup>2</sup></b>

### LOFT FLOOR


Plywood Flooring	=	0.15	KN/m <sup>2</sup>
Timber Joists	=	0.2	KN/m <sup>2</sup>
Insulation	=	0.1	KN/m <sup>2</sup>
Ceiling and services	=	0.2	KN/m <sup>2</sup>
Partitions	=	0.5	KN/m <sup>2</sup>
<b>Total dead load</b>	=	<b>1.10</b>	<b>KN/m<sup>2</sup></b>
<b>Live Load</b>	=	<b>1.5</b>	<b>KN/m<sup>2</sup></b>

### FIRST FLOOR

Plywood Flooring	=	0.15	KN/m <sup>2</sup>
Timber Joists	=	0.2	KN/m <sup>2</sup>
Insulation	=	0.05	KN/m <sup>2</sup>
Ceiling and services	=	0.2	KN/m <sup>2</sup>
Partitions	=	0.5	KN/m <sup>2</sup>
<b>Total dead load</b>	=	<b>1.10</b>	<b>KN/m<sup>2</sup></b>
<b>Live Load</b>	=	<b>1.5</b>	<b>KN/m<sup>2</sup></b>

### WALL LOADS

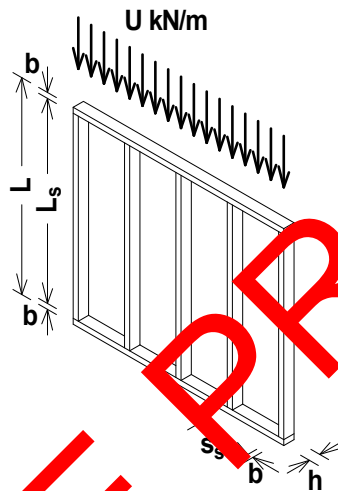
Brick wall (102 mm)	=	2	KN/m <sup>2</sup>
Brick wall with plaster	=	1.9	KN/m <sup>2</sup>
Glazing	=	0.5	KN/m <sup>2</sup>

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## 2. TIMBER DESIGN

- Stud Wall 47 x 100 @ 400 c/c (C-24)

### TIMBER STUD DESIGN (BS5268-2:2002)



#### Stud details

Stud breadth;  $b = 47 \text{ mm}$   
 Stud depth;  $h = 100 \text{ mm}$   
 Number of studs;  $N_s = 1$


#### Strength class C24 timber (Table 8 BS5268:Pt 2:2002)

#### Section properties

Cross sectional area;  $A = N_s \times b \times h = 4700 \text{ mm}^2$   
 Section modulus;  $Z = N_s \times b \times h^2 / 6 = 78333 \text{ mm}^3$   
 Moment of inertia in the major axis;  $I_x = N_s \times b \times h^3 / 12 = 3916667 \text{ mm}^4$   
 Moment of inertia in the minor axis;  $I_y = N_s \times h \times b^3 / 12 = 865192 \text{ mm}^4$   
 Radius of gyration in the major axis;  $r_x = \sqrt{I_x / A} = 28.9 \text{ mm}$   
 Radius of gyration in the minor axis;  $r_y = \sqrt{I_y / A} = 13.6 \text{ mm}$

#### Panel details - Studs restrained by sheathing in the plane of the panel

Panel height;  $L = 2400 \text{ mm}$   
 Stud length;  $L_s = L - (2 \times b) = 2306 \text{ mm}$   
 Standard stud spacing;  $s_s = 400 \text{ mm}$   
 Panel opening;  $O = 0 \text{ mm}$   
 Loaded panel length;  $s = \max(s_s, (O + s_s) / 2) = 400 \text{ mm}$   
 Effective length in the major axis;  $L_{ex} = 0.85 \times L_s = 1960 \text{ mm}$   
 Slenderness ratio;  $\lambda = L_{ex} / r_x = 67.90$

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#### Vertical loading details

Roof UDL;  
Floor UDL;  
Imposed floor load duration;

#### Modification factors

Section depth factor;  
Load sharing factor;

#### Consider axial compression without bending under medium term loads

Load duration factor;  
Vertical loading;

#### Check compressive stress on stud

Compression member factor;  
Compression parallel to grain;  
Permissible compressive stress;  
Applied compressive stress;

#### Dead loads

$U_{r,d} = 5.00$  kN/m;  
 $U_{f,d} = 5.00$  kN/m;

#### Long term

$K_7 = (300 \text{ mm} / h)^{0.11} = 1.13$   
 $K_8 = 1.10$

#### Imposed loads

$U_{r,i} = 5.00$  kN/m  
 $U_{f,i} = 5.00$  kN/m

$$F = (U_{r,d} + U_{f,d} + U_{r,i} + U_{f,i}) \times s = 6.00 \text{ kN}$$

$$K_{12} = 0.57$$

$$\sigma_c = 7.900 \text{ N/mm}^2$$

$$\sigma_{c_{adm}} = \sigma_c \times K_7 \times K_8 \times K_{12} = 6.200 \text{ N/mm}^2$$

$$\sigma_{c_{max}} = F / (N_s \times b \times h) = 1.702 \text{ N/mm}^2$$

**PASS - Applied compressive stress under medium term loads is within permissible limits**

#### Check compressive stress on rail

Bearing stress modification factor;  
Compression perpendicular to grain (no wane);  
Permissible compressive stress;  
Applied compressive stress;

$$K_4 = 1.24$$

$$\sigma_{cp1} = 2.400 \text{ N/mm}^2$$

$$\sigma_{cp1_{adm}} = \sigma_{cp1} \times K_3 \times K_4 = 3.717 \text{ N/mm}^2$$

$$\sigma_{cp1_{max}} = F / (N_s \times b \times h) = 1.702 \text{ N/mm}^2$$

**PASS - Applied compressive stress under medium term loads is within permissible limits**

#### Consider axial compression without bending under long term loads

Load duration factor;  
Vertical loading;

$$K_3 = 1.00$$

$$F = (U_{r,d} + U_{f,d} + U_{f,i}) \times s = 6.00 \text{ kN}$$

#### Check compressive stress on stud

Compression member factor;  
Compression parallel to grain;  
Permissible compressive stress;  
Applied compressive stress;

$$K_{12} = 0.61$$

$$\sigma_c = 7.900 \text{ N/mm}^2$$

$$\sigma_{c_{adm}} = \sigma_c \times K_3 \times K_8 \times K_{12} = 5.301 \text{ N/mm}^2$$

$$\sigma_{c_{max}} = F / (N_s \times b \times h) = 1.277 \text{ N/mm}^2$$

**PASS - Applied compressive stress under long term loads is within permissible limits**

#### Check compressive stress on rail

Bearing stress modification factor;  
Compression perpendicular to grain (no wane);  
Permissible compressive stress;  
Applied compressive stress;


$$K_4 = 1.24$$

$$\sigma_{cp1} = 2.400 \text{ N/mm}^2$$

$$\sigma_{cp1_{adm}} = \sigma_{cp1} \times K_3 \times K_4 = 2.974 \text{ N/mm}^2$$

$$\sigma_{cp1_{max}} = F / (N_s \times b \times h) = 1.277 \text{ N/mm}^2$$

**PASS - Applied compressive stress under long term loads is within permissible limits**

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- **Double Trimmer** (2) (47 x 200) (C-24)

**TIMBER BEAM ANALYSIS & DESIGN TO BS5268-2:2002**







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Project: xxxxxxxxE CR08XW UK

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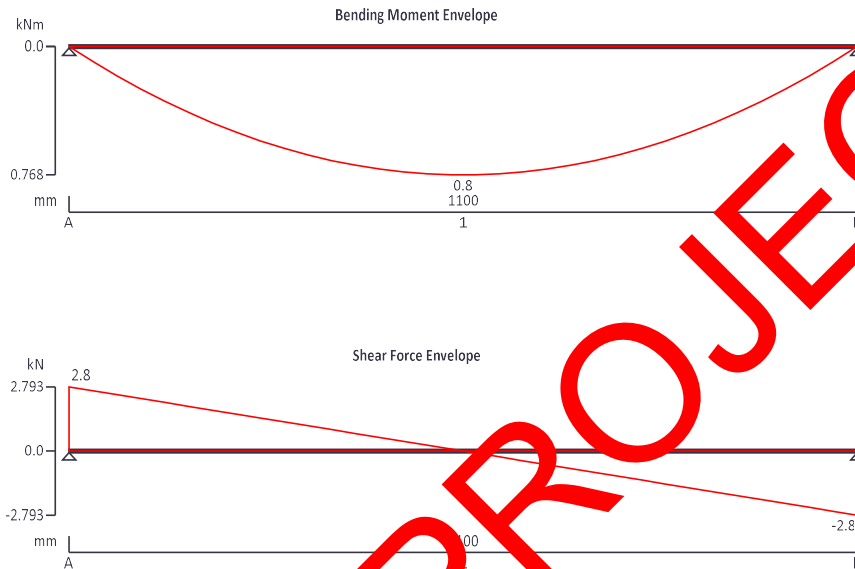
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#### Applied loading

##### Beam loads

SW

Dead self weight of beam = 1

DL

Dead full UDL 3.000 kN/m

LL

Imposed full UDL 2.000 kN/m

##### Load combinations

Load combination 1

Support A

Dead = 1.00

Imposed = 1.00

Span 1

Dead = 1.00

Imposed = 1.00

Support B

Dead = 1.00

Imposed = 1.00

##### Analysis results

Maximum moment;

$M_{max} = 0.768$  kNm;

$M_{min} = 0.000$  kNm

Design moment;

$M = \max(\text{abs}(M_{max}), \text{abs}(M_{min})) = 0.768$  kNm

Maximum shear;

$F_{max} = 2.793$  kN;

$F_{min} = -2.793$  kN

Design shear;

$F = \max(\text{abs}(F_{max}), \text{abs}(F_{min})) = 2.793$  kN

Total load on beam;

$W_{tot} = 5.585$  kN

Reactions at support A;

$R_{A\_max} = 2.793$  kN;

$R_{A\_min} = 2.793$  kN

Unfactored dead load reaction at support A;

$R_{A\_Dead} = 1.693$  kN


Unfactored imposed load reaction at support A;

$R_{A\_Imposed} = 1.100$  kN

Reactions at support B;

$R_{B\_max} = 2.793$  kN;

$R_{B\_min} = 2.793$  kN

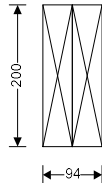
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Unfactored dead load reaction at support B;

$R_{B\_Dead} = 1.693 \text{ kN}$

Unfactored imposed load reaction at support B;

$R_{B\_Imposed} = 1.100 \text{ kN}$



#### Timber section details

Breadth of sections;

$b = 47 \text{ mm}$

Depth of sections;

$h = 200 \text{ mm}$

Number of sections in member;

$N = 2$

Overall breadth of member;

$b_b = N \times b = 94 \text{ mm}$

Timber strength class;

C24

#### Member details

Service class of timber;

2

Load duration;

Long term

Length of span;

$L_{s1} = 1100 \text{ mm}$

Length of bearing;

$L_b = 100 \text{ mm}$

#### Section properties

Cross sectional area of member;

$A = N \times b \times h = 18800 \text{ mm}^2$

Section modulus;

$Z_x = N \times b \times h^2 / 6 = 626667 \text{ mm}^3$

$Z_y = h \times (N \times b)^2 / 6 = 294533 \text{ mm}^3$

Second moment of area;

$I_x = N \times b \times h^3 / 12 = 62666667 \text{ mm}^4$

$I_y = h \times (N \times b)^3 / 12 = 13843067 \text{ mm}^4$

Radius of gyration;

$i_x = \sqrt{I_x / A} = 57.7 \text{ mm}$

$i_y = \sqrt{I_y / A} = 27.1 \text{ mm}$

#### Modification factors

Duration of loading - Table 17;

$K_3 = 1.00$

Bearing stress - Table 18;

$K_4 = 1.00$

Total depth of member - cl.2.10.6;

$K_7 = (300 \text{ mm} / h)^{0.11} = 1.05$

Load sharing - cl.2.9;

$K_8 = 1.00$

#### Lateral support - cl.2.10.8

Ends held in position

3.00

Permissible depth-to-breadth ratio - Table 19;

$h / (N \times b) = 2.13$


Actual depth-to-breadth ratio;

**PASS - Lateral support is adequate**

#### Compression perpendicular to grain

Permissible bearing stress (no wane);

$\sigma_{c\_adm} = \sigma_{cp1} \times K_3 \times K_4 \times K_8 = 2.400 \text{ N/mm}^2$

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Applied bearing stress;

$$\sigma_{c\_a} = R_{A\_max} / (N \cdot b \cdot L_b) = 0.297 \text{ N/mm}^2$$

$$\sigma_{c\_a} / \sigma_{c\_adm} = 0.124$$

**PASS - Applied compressive stress is less than permissible compressive stress at bearing**

**Bending parallel to grain**

Permissible bending stress;

$$\sigma_{m\_adm} = \sigma_m \cdot K_3 \cdot K_7 \cdot K_8 = 7.842 \text{ N/mm}^2$$

Applied bending stress;

$$\sigma_{m\_a} = M / Z_x = 1.225 \text{ N/mm}^2$$

$$\sigma_{m\_a} / \sigma_{m\_adm} = 0.156$$

**PASS - Applied bending stress is less than permissible bending stress**

**Shear parallel to grain**

Permissible shear stress;

$$\tau_{adm} = \tau \cdot K_3 \cdot K_4 \cdot K_5 = 710 \text{ N/mm}^2$$

Applied shear stress;

$$\tau_a = 3 \cdot F / (2 \cdot A) = 0.213 \text{ N/mm}^2$$

$$\tau_a / \tau_{adm} = 0.314$$

**PASS - Applied shear stress is less than permissible shear stress**

**Deflection**

Modulus of elasticity for deflection;

$$E_{min} = 7200 \text{ N/mm}^2$$

Permissible deflection;

$$\delta_{adm} = \min(551 \text{ in}, 0.003 \cdot L_{s1}) = 3.300 \text{ mm}$$

Bending deflection;

$$\delta_{b\_s1} = 0.215 \text{ mm}$$

Shear deflection;


$$\delta_{v\_s1} = 0.109 \text{ mm}$$

Total deflection;

$$\delta_a = \delta_{b\_s1} + \delta_{v\_s1} = 0.323 \text{ mm}$$

$$\delta_a / \delta_{adm} = 0.098$$

**PASS - Total deflection is less than permissible deflection**

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### 3. LINTEL

- **IG L1/HD 100** (For Cavity walls with 90-105mm cavity)

We had taken loadings being applied on our lintel:

- Floor Load
- Cavity wall Load

Our load derivation for each source for lintel is as follows;

#### Floor Load;

Dead Load:  $1.1 \text{ kN/m}^2$

Live Load:  $0.6 \text{ kN/m}^2$

Tributary Length =  $1.5 \text{ m}$

Dead Load (UDL) =  $1.1 \times 1.5 = 1.65 \text{ kN/m}$

Live Load (UDL) =  $0.6 \times 1.5 = 0.96 \text{ kN/m}$

#### Cavity Wall Load;

Dead Load:  $4.4 \text{ kN/m}^2$

Dead Load (UDL) =  $11.2 \text{ kN/m}$

**Total Dead Load (UDL): = 12.85 KN/m**

**Total Live Load (UDL): = 1 KN/m**

**Total Load (UDL) = 13.85 KN/m**

Hence

Required Load Carrying Capacity = 13.85 KN Required


Length = 1000 (with bearing on both sides)

IG L1/HD 100 Lintel for 100 mm cavity of 1000 mm as Per Requirement can carry = 22 KN

Load Carrying capacity of Provided Lintel = 22 KN

Hence,

**Provided > Required**

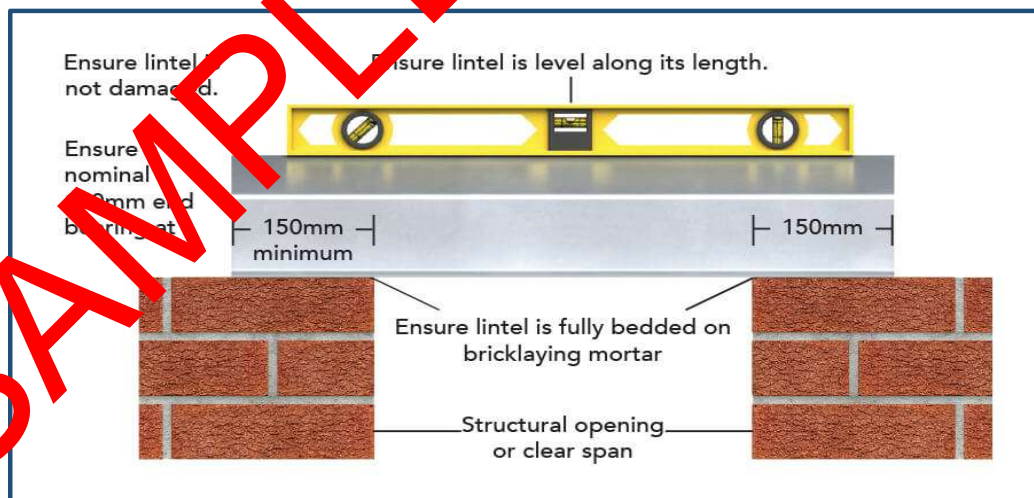
 <p>02 TOWERFIELDS WESTERHAM ROAD BROMLEY, BR2 6HF Email: <a href="mailto:info@Pearlepp.co.uk">info@Pearlepp.co.uk</a> Website: <a href="http://www.pearlepp.co.uk">www.pearlepp.co.uk</a> Tel no. 02035763199</p>	Project: XXXX		Sheet No./Rev. 13
	Job Ref. 2024-03- CR08XW	Structural Engineer MM	Date 22/03/2024


Select a suitable lintel according to the span

Length	Height	Thickness	Total UDL KN 3:1	Total UDL KN 19:1
600	110	2.9	30	22
1200				
1350	135	2.9	30	22
1500				
1650	163	2.9	40	35
2100				
2250	203	2.9	40	35
2550				
2700	203	3.2	40	35
3000				
3150	203	3.2	40	32
3600				
3750	203	3.2	33	28
4200				

### Installation

Provide a minimum bearing of 150 mm on both sides



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	Job Ref. 2024-03- CR08XW	Structural Engineer  MM	Date 22/03/2024

• **IG Box HD 100**

(For solid wall)

Select a suitable lintel according to the span

Length (mm)	Height (mm)	Thickness (mm)	Total UDL (kN)
600	150	2.5	50
1200			
1350	150	2.5	45
1800			
1950	215	2.5	50
2400			
2550	215	2.5	40
2700			

**Installation**

Provide a minimum bearing of 150 mm on both sides

