



Buckling Proof according to EC 1993 Part 1-5

## 3.2 Effective width for elastic shear lag

Shear lag reduction for flange 1

Shear Lag is not neglectable

$\alpha_0$ : 1.6

Beta: 0.6051075457633321

Shear lag reduction for flange 3

Shear Lag is not neglectable

alpha\_0: 1.0

Beta: 0.8741258741258742

## 4.4 Plate elements without longitudinal stiffeners

Iteratively changing the widths until  $M_{Rd\_el\_eff}$  converges to a limit of 0.02

## 4.5 Stiffened plate elements with longitudinal stiffeners

Side 2

4.5.4 Interaction between plate and column buckling

all\_tension: False

rho\_c = 1

Side 3

4.5.4 Interaction between plate and column buckling

all\_tension: True

rho\_c = 1

Side 4

4.5.4 Interaction between plate and column buckling

all\_tension: False

rho\_c = 1

Resistance to shear and interaction shear force and bending moment for side 1

5. Resistance to shear

stiffened plate; EBPlate

k\_tau: 1198.6503368036094

eta\_3: 0.03388146579817934

7.1 Interaction between shear force, bending moment and axial force

Flange -> (7.1), comment (5)

eta\_3 <= 0.5; no interaction needed

utilisation: -1

Proofing Resistance to shear for each subpanel

5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.03859186939482459

eta\_3\_panel < 1: pass subpanel

5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.03376788572047151

eta\_3\_panel < 1: pass subpanel

5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.02894390204611844

eta\_3\_panel < 1: pass subpanel

5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.024119918371765373

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.019295934697412295

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.014471951023059225

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.009647967348706151

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.004823983674353078

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 4.8072742965247425e-18

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.004823983674353068

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.009647967348706144

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.014471951023059215

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.019295934697412295

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.024119918371765373

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.02894390204611845

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.03376788572047151

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.03859186939482459

eta\_3\_panel < 1: pass subpanel

Resistance to shear and interaction shear force and bending moment for side 2

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 6.4025

eta\_3: 0.2538074578374027

7.1 Interaction between shear force, bending moment and axial force

Web -> (7.1) without iterating

eta\_3 <= 0.5; no interaction needed

utilisation: -1

## Resistance to shear and interaction shear force and bending moment for side 3

5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 7.59

eta\_3: 0.14549351534325097

7.1 Interaction between shear force, bending moment and axial force

Flange -> (7.1), comment (5)

eta\_3 <= 0.5; no interaction needed

utilisation: -1

Proofing Resistance to shear for each subpanel

5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 7.59

eta\_3: 0.12717964627906553

eta\_3\_panel < 1: pass subpanel

## Resistance to shear and interaction shear force and bending moment for side 4

### 5. Resistance to shear

unstiffened plate; (A.5)

$k_{\tau}$ : 6.4025

$\eta_3$ : 0.2538074578374027

### 7.1 Interaction between shear force, bending moment and axial force

Web -> (7.1) without iterating

$\eta_3 \leq 0.5$ ; no interaction needed

utilisation: -1

