



moment of inertia gross without shear lag: 131997495506.10445

Buckling Proof according to EC 1993 Part 1-5

## 3.2 Effective width for elastic shear lag

Shear lag reduction for flange 1

Shear Lag is not neglectable

$\alpha_0$ : 1.7

Beta: 0.31395004434548573

Shear lag reduction for flange 3

Shear Lag is not neglectable

alpha\_0: 1.3

Beta: 0.44121107679637916

## 4.4 Plate elements without longitudinal stiffeners

Iteratively changing the widths until  $M_{Rd\_el\_eff}$  converges to a limit of 0.02

moment of inertia gross with shear lag: 60364468595.56615

moment of inertia eff without shear lag: 114100942203.74236

moment of inertia eff with shear lag: 54037465137.15008

## 4.5 Stiffened plate elements with longitudinal stiffeners

Side 2

### 4.5.2 Plate type behaviour

$\sigma_{cr} = 2555.282079228872$

$\Lambda = 0.26007581303875243$

$\rho_{Global} = 1.0$

### 4.5.3 Column type buckling behaviour

Column number 9

$A_{sl}=13548.45$ ,  $A_{sl\_eff}=13190.5$ ,  $I_{sl}=27291618.43$

$\sigma_{cr\_c}=260.93$

$e_1=72.7$ ,  $e_2=59.95$

All tension =False

## Buckling Values 9

$\beta_{A_c} = 0.9735800820732999$

$\lambda_{c\_bar} = 0.9363764569980191$

$\Phi_c = 1.0840396776701264$

$\chi_c = 0.6134044238586146$

## Critical buckling values

$\chi_c$ : 0.6134044238586146

$\sigma_{cr\_c}$ : 260.9387459785654

## 4.5.4 Interaction between plate and column buckling

all\_tension: False

$\rho_c = 1.0$

## Side 3

## 4.5.2 Plate type behaviour

$\sigma_{cr} = 763.4573757020646$

$\lambda$ : 0.5017485584496578

$\rho_{Global}$ : 1.0

## 4.5.3 Column type buckling behaviour

Column number 10

$A_{sl} = 16998.1$ ,  $A_{sl\_eff} = 13902.39$ ,  $I_{sl} = 27320365.52$

$\sigma_{cr\_c} = 208.2$

$e_1 = 84.87$ ,  $e_2 = 47.78$

All tension = False

## Buckling Values 10

$\beta_{A_c} = 0.8178792681216397$

$\lambda_{c\_bar} = 0.960806674379394$

$\Phi_{c\_c} = 1.10708338313521$

$\chi_{c\_c} = 0.6034751810906328$

Column number 11

$A_{sl} = 16998.1$ ,  $A_{sl\_eff} = 13902.39$ ,  $I_{sl} = 27320365.52$

$\sigma_{cr\_c} = 208.2$

$e_1 = 84.87$ ,  $e_2 = 47.78$

All tension = False

Buckling Values 11

$\beta_{A\_c} = 0.8178792681216398$

$\lambda_{c\_bar} = 0.960806674379394$

$\Phi_{c\_c} = 1.10708338313521$

$\chi_{c\_c} = 0.6034751810906328$

Critical buckling values

$\chi_{c\_c}$ : 0.6034751810906328

$\sigma_{cr\_c}$ : 208.20206822935626

## 4.5.4 Interaction between plate and column buckling

all\_tension: False

$\rho_{c\_c} = 1.0$

Side 4

## 4.5.2 Plate type behaviour

$\sigma_{cr} = 2555.282079228872$

$\lambda_{c\_c}$ : 0.26007581303875243

$\rho_{Global} = 1.0$

## 4.5.3 Column type buckling behaviour

Column number 12

$A_{sl}=13548.45$ ,  $A_{sl\_eff}=13190.5$ ,  $I_{sl}=27291618.43$

$\sigma_{cr\_c}=260.93$

$e_1=72.7$ ,  $e_2=59.95$

All tension =False

Buckling Values 12

$\beta_{A\_c}=0.9735800820732999$

$\lambda_{c\_bar}=0.9363764569980192$

$\Phi_{c}=1.0840396776701267$

$\chi_{c}=0.6134044238586144$

Critical buckling values

$\chi_{c}$ : 0.6134044238586144

$\sigma_{cr\_c}$ : 260.93874597856535

## 4.5.4 Interaction between plate and column buckling

all\_tension: False

$\rho_{c} = 1.0$

Resistance to shear and interaction shear force and bending moment for side 1

## 5. Resistance to shear

stiffened plate; EBPlate

$k_{\tau}$ : 1571.0214527033982

$\eta_3$ : 0.25205626496949346

## 7.1 Interaction between shear force, bending moment and axial force

Flange -> (7.1), comment (5)

$\eta_3 \leq 0.5$ ; no interaction needed

utilisation: -1

Proofing Resistance to shear for each subpanel

5. Resistance to shear

unstiffened plate; (A.5)

$k_\tau$ : 5.353840830449827

$\eta_3$ : 0.15703164083436558

$\eta_{3\_panel} < 1$ : pass subpanel

5. Resistance to shear

unstiffened plate; (A.5)

$k_\tau$ : 5.353840830449827

$\eta_3$ : 0.13987046328644648

$\eta_{3\_panel} < 1$ : pass subpanel

5. Resistance to shear

unstiffened plate; (A.5)

$k_\tau$ : 5.353840830449827

$\eta_3$ : 0.12270928573852742

$\eta_{3\_panel} < 1$ : pass subpanel

5. Resistance to shear

unstiffened plate; (A.5)

$k_\tau$ : 5.353840830449827

$\eta_3$ : 0.10554810819060838

$\eta_{3\_panel} < 1$ : pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.0883869306426893

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.07122575309477024

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.05406457554685117

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.03690339799893212

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.01974222045101305

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.03690339799893209

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.05406457554685115

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.07122575309477021

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.08838693064268928

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827



eta\_3: 0.10554810819060838

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.12270928573852743

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.13987046328644648

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.353840830449827

eta\_3: 0.15703164083436558

eta\_3\_panel < 1: pass subpanel

## Resistance to shear and interaction shear force and bending moment for side 2

## 5. Resistance to shear

stiffened plate; EBPlate

k\_tau: 21.01369360902256

eta\_3: 0.520359254539201

## 7.1 Interaction between shear force, bending moment and axial force

Web -> (7.1) without iterating

$\eta_3 > 0.5$ ; interaction needed

utilisation: 0.6031112266506108

## Resistance to shear and interaction shear force and bending moment for side 3

### 5. Resistance to shear

stiffened plate; EBPlate

$k_\tau$ : 94.90997656054232

$\eta_3$ : 0.23801478429570364

### 7.1 Interaction between shear force, bending moment and axial force

Flange -> (7.1), comment (5)

$\eta_3 \leq 0.5$ ; no interaction needed

utilisation: -1

### Proofing Resistance to shear for each subpanel

### 5. Resistance to shear

unstiffened plate; (A.5)

$k_\tau$ : 5.51015625

$\eta_3$ : 0.18360775955085792

$\eta_{3\_panel} < 1$ : pass subpanel

### 5. Resistance to shear

unstiffened plate; (A.5)

$k_\tau$ : 5.3625

$\eta_3$ : 0.09261316350554184

$\eta_{3\_panel} < 1$ : pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.480625

eta\_3: 0.05245412605513624

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.3625

eta\_3: 0.09261316350554184

eta\_3\_panel < 1: pass subpanel

## 5. Resistance to shear

unstiffened plate; (A.5)

k\_tau: 5.51015625

eta\_3: 0.18360775955085792

eta\_3\_panel < 1: pass subpanel

## Resistance to shear and interaction shear force and bending moment for side 4

## 5. Resistance to shear

stiffened plate; EBPlate

k\_tau: 21.01353383458647

eta\_3: 0.6004168070617667

## 7.1 Interaction between shear force, bending moment and axial force

Web -> (7.1) without iterating

eta\_3 > 0.5; interaction needed

utilisation: 0.6230257777471733

cross-section with  $b_{sup}=4000$ ,  $b_{inf}=3000$ ,  $h=2000$

trapezoid plate on side 1 with the number 1

$a=(2000, 0)$   $p_1=(1882, 0)$   $p_2=(1882, 0)$   $b=(1764, 0)$   $t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 2

$a=(1764, 0)$   $p_1=(1647, 0)$   $p_2=(1647, 0)$   $b=(1529, 0)$   $t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 3

$a=(1529, 0)$   $p_1=(1411, 0)$   $p_2=(1411, 0)$   $b=(1294, 0)$   $t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 4

$a=(1294, 0)$   $p_1=(1176, 0)$   $p_2=(1176, 0)$   $b=(1058, 0)$   $t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 5

$a=(1058, 0)$   $p_1=(941, 0)$   $p_2=(941, 0)$   $b=(823, 0)$   $t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 6

$a=(823, 0)$   $p_1=(705, 0)$   $p_2=(705, 0)$   $b=(588, 0)$   $t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 7

$a=(588, 0)$   $p_1=(470, 0)$   $p_2=(470, 0)$   $b=(352, 0)$   $t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 8

$a=(352, 0)$   $p_1=(235, 0)$   $p_2=(235, 0)$   $b=(117, 0)t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 9

$a=(117, 0)$   $p_1=(0, 0)$   $p_2=(0, 0)$   $b=(-117, 0)t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 10

$a=(-117, 0)$   $p_1=(-235, 0)$   $p_2=(-235, 0)$   $b=(-352, 0)t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 11

$a=(-352, 0)$   $p_1=(-470, 0)$   $p_2=(-470, 0)$   $b=(-588, 0)t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 12

$a=(-588, 0)$   $p_1=(-705, 0)$   $p_2=(-705, 0)$   $b=(-823, 0)t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 13

$a=(-823, 0)$   $p_1=(-941, 0)$   $p_2=(-941, 0)$   $b=(-1058, 0)t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 14

$a=(-1058, 0)$   $p_1=(-1176, 0)$   $p_2=(-1176, 0)$   $b=(-1294, 0)t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 15

$a=(-1294, 0)$   $p_1=(-1411, 0)$   $p_2=(-1411, 0)$   $b=(-1529, 0)t=14.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-125.65$

trapezoid plate on side 1 with the number 16

$a=(-1529, 0)$   $p1=(-1647, 0)$   $p2=(-1647, 0)$   $b=(-1764, 0)t=14.0$

$\sigma_a_{red}=-125.65$   $\sigma_b_{red}=-125.65$

trapezoid plate on side 1 with the number 17

$a=(-1764, 0)$   $p1=(-1882, 0)$   $p2=(-1882, 0)$   $b=(-2000, 0)t=14.0$

$\sigma_a_{red}=-125.65$   $\sigma_b_{red}=-125.65$

trapezoid plate on side 2 with the number 18

$a=(-2000, 0)$   $p1=(-1741, 1032)$   $p2=(-1741, 1032)$   $b=(-1711, 1154)t=10.0$

$\sigma_a_{red}=-125.65$   $\sigma_b_{red}=45.26$

trapezoid plate on side 2 with the number 19

$a=(-1711, 1154)$   $p1=(-1671, 1315)$   $p2=(-1671, 1315)$   $b=(-1638, 1445)t=10.0$

$\sigma_a_{red}=45.26$   $\sigma_b_{red}=88.34$

trapezoid plate on side 2 with the number 20

$a=(-1638, 1445)$   $p1=(-1570, 1717)$   $p2=(-1554, 1780)$   $b=(-1500, 2000)t=10.0$

$\sigma_a_{red}=88.34$   $\sigma_b_{red}=170.43$

trapezoid plate on side 3 with the number 21

$a=(-1500, 2000)$   $p1=(-1258, 2000)$   $p2=(-916, 2000)$   $b=(-675, 2000)t=10.0$

$\sigma_a_{red}=170.43$   $\sigma_b_{red}=170.43$

trapezoid plate on side 3 with the number 22

$a=(-675, 2000)$   $p1=(-525, 2000)$   $p2=(-525, 2000)$   $b=(-375, 2000)t=10.0$

$\sigma_a_{red}=170.43$   $\sigma_b_{red}=170.43$

trapezoid plate on side 3 with the number 23

$a=(-375, 2000)$   $p1=(-138, 2000)$   $p2=(138, 2000)$   $b=(375, 2000)t=10.0$

$\sigma_a_{red}=170.43$   $\sigma_b_{red}=170.43$

trapezoid plate on side 3 with the number 24

a=(375, 2000) p1=(525, 2000) p2=(525, 2000) b=(675, 2000)t=10.0

sigma\_a\_red=170.43 sigma\_b\_red=170.43

trapezoid plate on side 3 with the number 25

a=(675, 2000) p1=(916, 2000) p2=(1258, 2000) b=(1500, 2000)t=10.0

sigma\_a\_red=170.43 sigma\_b\_red=170.43

trapezoid plate on side 4 with the number 26

a=(1500, 2000) p1=(1554, 1780) p2=(1570, 1717) b=(1638, 1445)t=10.0

sigma\_a\_red=170.43 sigma\_b\_red=88.34

trapezoid plate on side 4 with the number 27

a=(1638, 1445) p1=(1671, 1315) p2=(1671, 1315) b=(1711, 1154)t=10.0

sigma\_a\_red=88.34 sigma\_b\_red=45.26

trapezoid plate on side 4 with the number 28

a=(1711, 1154) p1=(1741, 1032) p2=(1741, 1032) b=(2000, 0)t=10.0

sigma\_a\_red=45.26 sigma\_b\_red=-125.65

stiffener plate on side 1 of stiffener nr 1 on stiffener plate position 2

a=(1529, 0) p1=(1564, 130) p2=(1564, 130) b=(1599, 260)t=10.0

sigma\_a\_red=-125.65 sigma\_b\_red=-87.16

stiffener plate on side 1 of stiffener nr 1 on stiffener plate position 3

a=(1599, 260) p1=(1647, 260) p2=(1647, 260) b=(1695, 260)t=10.0

sigma\_a\_red=-87.16 sigma\_b\_red=-87.16

stiffener plate on side 1 of stiffener nr 1 on stiffener plate position 4

a=(1695, 260) p1=(1729, 130) p2=(1729, 130) b=(1764, 0)t=10.0

sigma\_a\_red=-87.16 sigma\_b\_red=-125.65

stiffener plate on side 1 of stiffener nr 2 on stiffener plate position 2

a=(1058, 0) p1=(1093, 130) p2=(1093, 130) b=(1128, 260)t=10.0

sigma\_a\_red=-125.65 sigma\_b\_red=-87.16

stiffener plate on side 1 of stiffener nr 2 on stiffener plate position 3

a=(1128, 260) p1=(1176, 260) p2=(1176, 260) b=(1224, 260)t=10.0

sigma\_a\_red=-87.16 sigma\_b\_red=-87.16

stiffener plate on side 1 of stiffener nr 2 on stiffener plate position 4

a=(1224, 260) p1=(1259, 130) p2=(1259, 130) b=(1294, 0)t=10.0

sigma\_a\_red=-87.16 sigma\_b\_red=-125.65

stiffener plate on side 1 of stiffener nr 3 on stiffener plate position 2

a=(588, 0) p1=(623, 130) p2=(623, 130) b=(657, 260)t=10.0

sigma\_a\_red=-125.65 sigma\_b\_red=-87.16

stiffener plate on side 1 of stiffener nr 3 on stiffener plate position 3

a=(657, 260) p1=(705, 260) p2=(705, 260) b=(753, 260)t=10.0

sigma\_a\_red=-87.16 sigma\_b\_red=-87.16

stiffener plate on side 1 of stiffener nr 3 on stiffener plate position 4

a=(753, 260) p1=(788, 130) p2=(788, 130) b=(823, 0)t=10.0

sigma\_a\_red=-87.16 sigma\_b\_red=-125.65

stiffener plate on side 1 of stiffener nr 4 on stiffener plate position 2

a=(117, 0) p1=(152, 130) p2=(152, 130) b=(187, 260)t=10.0

sigma\_a\_red=-125.65 sigma\_b\_red=-87.16

stiffener plate on side 1 of stiffener nr 4 on stiffener plate position 3

a=(187, 260) p1=(235, 260) p2=(235, 260) b=(283, 260)t=10.0

sigma\_a\_red=-87.16 sigma\_b\_red=-87.16

stiffener plate on side 1 of stiffener nr 4 on stiffener plate position 4



$a=(283, 260)$   $p_1=(318, 130)$   $p_2=(318, 130)$   $b=(352, 0)t=10.0$

$\sigma_{a\_red}=-87.16$   $\sigma_{b\_red}=-125.65$

stiffener plate on side 1 of stiffener nr 5 on stiffener plate position 2

$a=(-352, 0)$   $p_1=(-318, 130)$   $p_2=(-318, 130)$   $b=(-283, 260)t=10.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-87.16$

stiffener plate on side 1 of stiffener nr 5 on stiffener plate position 3

$a=(-283, 260)$   $p_1=(-235, 260)$   $p_2=(-235, 260)$   $b=(-187, 260)t=10.0$

$\sigma_{a\_red}=-87.16$   $\sigma_{b\_red}=-87.16$

stiffener plate on side 1 of stiffener nr 5 on stiffener plate position 4

$a=(-187, 260)$   $p_1=(-152, 130)$   $p_2=(-152, 130)$   $b=(-117, 0)t=10.0$

$\sigma_{a\_red}=-87.16$   $\sigma_{b\_red}=-125.65$

stiffener plate on side 1 of stiffener nr 6 on stiffener plate position 2

$a=(-823, 0)$   $p_1=(-788, 130)$   $p_2=(-788, 130)$   $b=(-753, 260)t=10.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-87.16$

stiffener plate on side 1 of stiffener nr 6 on stiffener plate position 3

$a=(-753, 260)$   $p_1=(-705, 260)$   $p_2=(-705, 260)$   $b=(-657, 260)t=10.0$

$\sigma_{a\_red}=-87.16$   $\sigma_{b\_red}=-87.16$

stiffener plate on side 1 of stiffener nr 6 on stiffener plate position 4

$a=(-657, 260)$   $p_1=(-623, 130)$   $p_2=(-623, 130)$   $b=(-588, 0)t=10.0$

$\sigma_{a\_red}=-87.16$   $\sigma_{b\_red}=-125.65$

stiffener plate on side 1 of stiffener nr 7 on stiffener plate position 2

$a=(-1294, 0)$   $p_1=(-1259, 130)$   $p_2=(-1259, 130)$   $b=(-1224, 260)t=10.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-87.16$

stiffener plate on side 1 of stiffener nr 7 on stiffener plate position 3

$a=(-1224, 260)$   $p1=(-1176, 260)$   $p2=(-1176, 260)$   $b=(-1128, 260)t=10.0$

$\sigma_{a\_red}=-87.16$   $\sigma_{b\_red}=-87.16$

stiffener plate on side 1 of stiffener nr 7 on stiffener plate position 4

$a=(-1128, 260)$   $p1=(-1093, 130)$   $p2=(-1093, 130)$   $b=(-1058, 0)t=10.0$

$\sigma_{a\_red}=-87.16$   $\sigma_{b\_red}=-125.65$

stiffener plate on side 1 of stiffener nr 8 on stiffener plate position 2

$a=(-1764, 0)$   $p1=(-1729, 130)$   $p2=(-1729, 130)$   $b=(-1695, 260)t=10.0$

$\sigma_{a\_red}=-125.65$   $\sigma_{b\_red}=-87.16$

stiffener plate on side 1 of stiffener nr 8 on stiffener plate position 3

$a=(-1695, 260)$   $p1=(-1647, 260)$   $p2=(-1647, 260)$   $b=(-1599, 260)t=10.0$

$\sigma_{a\_red}=-87.16$   $\sigma_{b\_red}=-87.16$

stiffener plate on side 1 of stiffener nr 8 on stiffener plate position 4

$a=(-1599, 260)$   $p1=(-1564, 130)$   $p2=(-1564, 130)$   $b=(-1529, 0)t=10.0$

$\sigma_{a\_red}=-87.16$   $\sigma_{b\_red}=-125.65$

stiffener plate on side 2 of stiffener nr 9 on stiffener plate position 2

$a=(-1638, 1445)$   $p1=(-1551, 1398)$   $p2=(-1551, 1398)$   $b=(-1456, 1348)t=10.0$

$\sigma_{a\_red}=88.34$   $\sigma_{b\_red}=73.98$

stiffener plate on side 2 of stiffener nr 9 on stiffener plate position 3

$a=(-1456, 1348)$   $p1=(-1478, 1260)$   $p2=(-1478, 1260)$   $b=(-1505, 1154)t=10.0$

$\sigma_{a\_red}=73.98$   $\sigma_{b\_red}=45.26$

stiffener plate on side 2 of stiffener nr 9 on stiffener plate position 4

$a=(-1505, 1154)$   $p1=(-1608, 1154)$   $p2=(-1608, 1154)$   $b=(-1711, 1154)t=10.0$

$\sigma_{a\_red}=45.26$   $\sigma_{b\_red}=45.26$

stiffener plate on side 3 of stiffener nr 10 on stiffener plate position 2

a=(-375, 2000) p1=(-398, 1904) p2=(-398, 1904) b=(-425, 1800)t=10.0

sigma\_a\_red=170.43 sigma\_b\_red=140.82

stiffener plate on side 3 of stiffener nr 10 on stiffener plate position 3

a=(-425, 1800) p1=(-525, 1800) p2=(-525, 1800) b=(-625, 1800)t=10.0

sigma\_a\_red=140.82 sigma\_b\_red=140.82

stiffener plate on side 3 of stiffener nr 10 on stiffener plate position 4

a=(-625, 1800) p1=(-651, 1904) p2=(-651, 1904) b=(-675, 2000)t=10.0

sigma\_a\_red=140.82 sigma\_b\_red=170.43

stiffener plate on side 3 of stiffener nr 11 on stiffener plate position 2

a=(675, 2000) p1=(651, 1904) p2=(651, 1904) b=(625, 1800)t=10.0

sigma\_a\_red=170.43 sigma\_b\_red=140.82

stiffener plate on side 3 of stiffener nr 11 on stiffener plate position 3

a=(625, 1800) p1=(525, 1800) p2=(525, 1800) b=(425, 1800)t=10.0

sigma\_a\_red=140.82 sigma\_b\_red=140.82

stiffener plate on side 3 of stiffener nr 11 on stiffener plate position 4

a=(425, 1800) p1=(398, 1904) p2=(398, 1904) b=(375, 2000)t=10.0

sigma\_a\_red=140.82 sigma\_b\_red=170.43

stiffener plate on side 4 of stiffener nr 12 on stiffener plate position 2

a=(1711, 1154) p1=(1608, 1154) p2=(1608, 1154) b=(1505, 1154)t=10.0

sigma\_a\_red=45.26 sigma\_b\_red=45.26

stiffener plate on side 4 of stiffener nr 12 on stiffener plate position 3

a=(1505, 1154) p1=(1478, 1260) p2=(1478, 1260) b=(1456, 1348)t=10.0

sigma\_a\_red=45.26 sigma\_b\_red=73.98

stiffener plate on side 4 of stiffener nr 12 on stiffener plate position 4

a=(1456, 1348) p1=(1551, 1398) p2=(1551, 1398) b=(1638, 1445)t=10.0

sigma\_a\_red=73.98 sigma\_b\_red=88.34

moment of inertia including all reductions: 54037465137.15008

