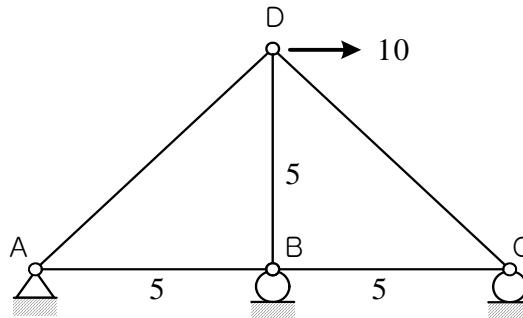




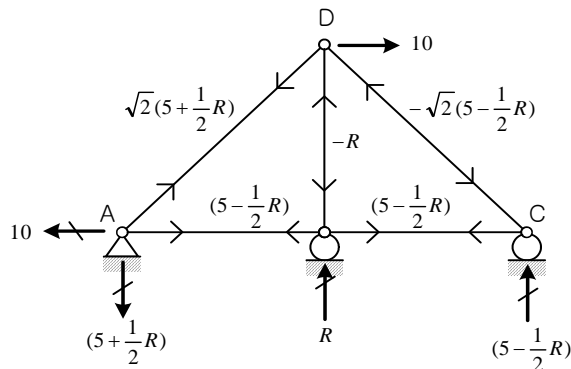
• The Principle of Least Work

The magnitudes of the redundants of an indeterminate structure must be such that the strain energy stored in the structure is a minimum.

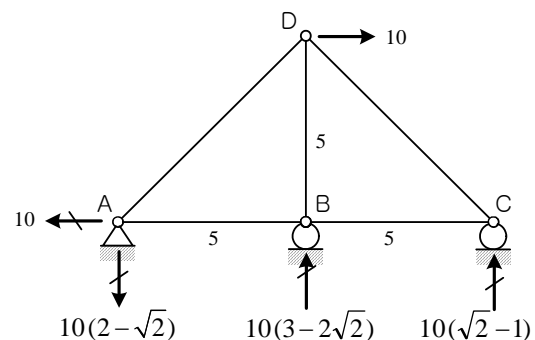
EX4) Determine the reaction at B. Assume the members are pin connected at their ends. ($AE=1$)



sol) Assume that the reaction at B is R and get the member forces.

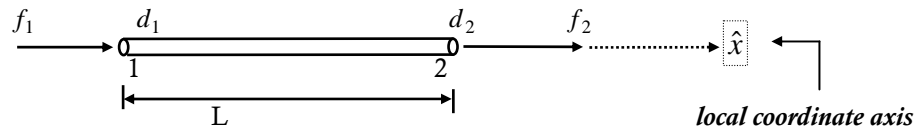


$$\begin{aligned}\frac{\partial U}{\partial R} &= \frac{FL}{EA} \frac{\partial F}{\partial R} \\ &= 5\sqrt{2} \left(5 + \frac{1}{2}R - 5 + \frac{1}{2}R\right) + 5 \left(R - 5 + \frac{1}{2}R\right) \\ &= \left(5\sqrt{2} + \frac{15}{2}\right)R - 25 \\ \therefore R &= \frac{25}{5\sqrt{2} + \frac{15}{2}} = 10(3 - 2\sqrt{2}) \\ \therefore \text{reaction at B} = R &= 10(3 - 2\sqrt{2})\end{aligned}$$



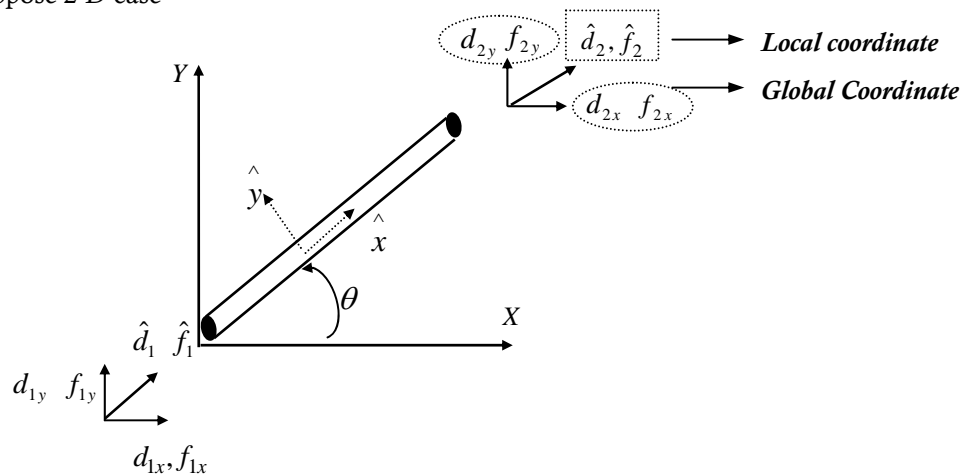
3.4. Coordinate Transformation

Recall that in 1-D bar element



$$\begin{Bmatrix} f_1 \\ f_2 \end{Bmatrix} = \frac{AE}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{Bmatrix} d_1 \\ d_2 \end{Bmatrix} \Rightarrow \hat{f}e = \hat{K}e\hat{e}$$

Now suppose 2-D case



Let $\hat{d} = \begin{Bmatrix} \hat{d}_1 \\ \hat{d}_2 \end{Bmatrix}$: nodal displacement in local coordinate system

$d = \begin{Bmatrix} d_{1x} \\ d_{1y} \\ d_{2x} \\ d_{2y} \end{Bmatrix}$: nodal displacement in global coordinate system

$$\text{then, } \hat{d}_1 = d_{1x} \cos \theta + d_{1y} \sin \theta \Rightarrow [\cos \theta \quad \sin \theta] \begin{Bmatrix} d_{1x} \\ d_{1y} \end{Bmatrix}$$

$$\hat{d}_2 = d_{2x} \cos \theta + d_{2y} \sin \theta \Rightarrow [\cos \theta \quad \sin \theta] \begin{Bmatrix} d_{2x} \\ d_{2y} \end{Bmatrix}$$

$$\begin{Bmatrix} \hat{d}_1 \\ \hat{d}_2 \end{Bmatrix} = \begin{bmatrix} \cos \theta & \sin \theta & 0 & 0 \\ 0 & 0 & \cos \theta & \sin \theta \end{bmatrix} \begin{Bmatrix} d_{1x} \\ d_{1y} \\ d_{2x} \\ d_{2y} \end{Bmatrix}$$

$$\therefore \hat{d}e = T \cdot de \quad \text{where, } T : \text{Transformation matrix (global} \Rightarrow \text{local value)}$$

As the same way

$$\hat{f}_e = T \cdot f_e$$

Now suppose the global components

$$\begin{Bmatrix} f_{1x} \\ f_{1y} \\ f_{2x} \\ f_{2y} \end{Bmatrix} = \begin{bmatrix} \cos \theta & 0 \\ \sin \theta & 0 \\ 0 & \cos \theta \\ 0 & \sin \theta \end{bmatrix} \begin{Bmatrix} \hat{f}_1 \\ \hat{f}_2 \end{Bmatrix}$$

$$f_e = T^T \hat{f}_e$$

$$\begin{aligned} f_e &= T^T \hat{f}_e \\ &= T^T \hat{K}_e \hat{d}_e \quad \Leftarrow (\hat{f}_e = \hat{K}_e \hat{d}_e) \\ &= T^T \hat{K}_e T d_e \quad \Leftarrow (\hat{d}_e = T d_e) \end{aligned}$$

Therefore, $K_e = T^T \hat{K}_e T$

By using the above results, we can express the element level equation in global coordinate system as,

$$f_e = K_e d_e \quad \text{where } K_e : \text{Element stiffness matrix in global coordinate system.}$$

Global K_e in 2-D truss element

$$\begin{aligned} K_e &= T^T \hat{K}_e T \\ &= \begin{bmatrix} \cos \theta & 0 \\ \sin \theta & 0 \\ 0 & \cos \theta \\ 0 & \sin \theta \end{bmatrix} \frac{AE}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{bmatrix} \cos \theta & \sin \theta & 0 & 0 \\ 0 & 0 & \cos \theta & \sin \theta \end{bmatrix} \\ &= \frac{AE}{L} \begin{bmatrix} \cos^2 \theta & \cos \theta \sin \theta & -\cos^2 \theta & -\cos \theta \sin \theta \\ \cos \theta \sin \theta & \sin^2 \theta & -\cos \theta \sin \theta & -\sin^2 \theta \\ -\cos^2 \theta & -\cos \theta \sin \theta & \cos^2 \theta & \cos \theta \sin \theta \\ -\cos \theta \sin \theta & -\sin^2 \theta & \cos \theta \sin \theta & \sin^2 \theta \end{bmatrix} \\ &= \frac{AE}{L} \begin{bmatrix} C^2 & CS & -C^2 & -CS \\ CS & S^2 & -CS & -S^2 \\ -C^2 & -CS & C^2 & CS \\ -CS & -S^2 & CS & S^2 \end{bmatrix} \quad \text{where, } \begin{matrix} C = \cos \theta \\ S = \sin \theta \end{matrix} \end{aligned}$$