AE 737 - MECHANICS OF DAMAGE TOLERANCE

LECTURE 9

Dr. Nicholas Smith

Last Updated: February 18, 2016 at 2:57pm

Wichita State University, Department of Aerospace Engineering

OFFICE HOURS

- · No Office Hours Friday :(
- · Raincheck office hours Monday 3:00 5:00
- · Homework 4 due Thursday, 25 Feb.

SCHEDULE

- · 18 Feb Residual Strength
- · 23 Feb Residual Strength, Multiple Site Damage
- 25 Feb Multiple Site Damage, Mixed-mode Fracture, Homework
 4 Due, Homework 5 Assigned
- · 1 Mar Section 1 Review, Homework 5 Due
- · 3 Mar Section 1 Review, Homework 5 return
- 8 Mar Exam 1
- 10 Mar Exam return, Final Project discussion

OUTLINE

- 1. thickness effects
- 2. fracture toughness review
- 3. residual strength
- 4. fedderson approach
- 5. proof testing



 We already know there is a difference between plane strain and plane stress fracture toughness

- We already know there is a difference between plane strain and plane stress fracture toughness
- As a material gets thicker and thicker, it converges to the plane strain solution

- We already know there is a difference between plane strain and plane stress fracture toughness
- As a material gets thicker and thicker, it converges to the plane strain solution
- Thinner specimens tend towards the plane stress solution

- We already know there is a difference between plane strain and plane stress fracture toughness
- As a material gets thicker and thicker, it converges to the plane strain solution
- · Thinner specimens tend towards the plane stress solution
- When a specimen is thinner than some critical thickness, the material behavior is somewhat unknown

- We already know there is a difference between plane strain and plane stress fracture toughness
- As a material gets thicker and thicker, it converges to the plane strain solution
- · Thinner specimens tend towards the plane stress solution
- When a specimen is thinner than some critical thickness, the material behavior is somewhat unknown
- Some materials retain the constant plane stress fracture toughness

- We already know there is a difference between plane strain and plane stress fracture toughness
- As a material gets thicker and thicker, it converges to the plane strain solution
- · Thinner specimens tend towards the plane stress solution
- When a specimen is thinner than some critical thickness, the material behavior is somewhat unknown
- Some materials retain the constant plane stress fracture toughness
- · Others exhibit an unpredictable decrease in fracture toughness

- We already know there is a difference between plane strain and plane stress fracture toughness
- As a material gets thicker and thicker, it converges to the plane strain solution
- · Thinner specimens tend towards the plane stress solution
- When a specimen is thinner than some critical thickness, the material behavior is somewhat unknown
- Some materials retain the constant plane stress fracture toughness
- · Others exhibit an unpredictable decrease in fracture toughness
- · The phenomenon is not well-understood

• There is also a difference in the fracture surface between thin and thick specimens

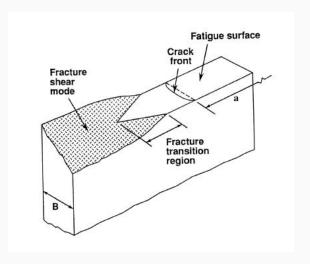
- There is also a difference in the fracture surface between thin and thick specimens
- \cdot Thin specimens (in plane stress region) fail due to slant fracture

- There is also a difference in the fracture surface between thin and thick specimens
- · Thin specimens (in plane stress region) fail due to slant fracture
- This actually indicates some mixed-mode conditions at failure

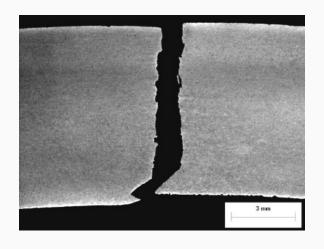
- There is also a difference in the fracture surface between thin and thick specimens
- · Thin specimens (in plane stress region) fail due to slant fracture
- · This actually indicates some mixed-mode conditions at failure
- Thick specimens fail due to square fracture (with a small shear tip near the edges)

- There is also a difference in the fracture surface between thin and thick specimens
- · Thin specimens (in plane stress region) fail due to slant fracture
- · This actually indicates some mixed-mode conditions at failure
- Thick specimens fail due to square fracture (with a small shear tip near the edges)
- This is more consistent with pure Mode I

SLANT FRACTURE



SHEAR LIP



FRACTURE TOUGHNESS REVIEW

GROUP REVIEW

- Group 1 Sketch K_R -curve (for ductile material), explain what it means, how to find K_C
- Group 2 Sketch K_c vs. crack length, explain what's happening
- Group 3 How can we determine whether a panel is in plane strain or plane stress?
- Group 4 Sketch K_R-curves for ductile and brittle materials, what is the difference?



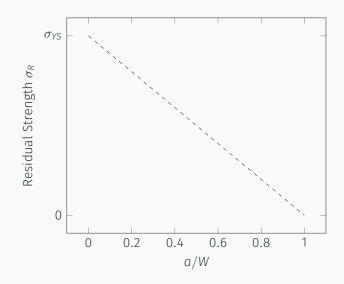
• In the last chapter we performed some basic residual strength analysis by checking for net section yield

- In the last chapter we performed some basic residual strength analysis by checking for net section yield
- As the crack grows, the area of the sample decreases, increasing the net section stress

- In the last chapter we performed some basic residual strength analysis by checking for net section yield
- As the crack grows, the area of the sample decreases, increasing the net section stress
- The residual strength, σ_R is given in terms of the gross area, so as the crack grows the residual strength due to yield decreases

- In the last chapter we performed some basic residual strength analysis by checking for net section yield
- As the crack grows, the area of the sample decreases, increasing the net section stress
- The residual strength, σ_R is given in terms of the gross area, so as the crack grows the residual strength due to yield decreases
- We can relate the net-section stress to σ_R by

$$\sigma_R = \sigma_{YS} \frac{A_{net}}{A_{gross}} \tag{9.1}$$

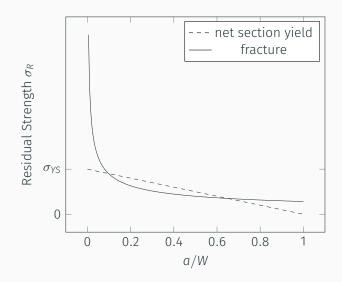


 $\boldsymbol{\cdot}$ For brittle fracture to occur, we need to satisfy the condition

• For brittle fracture to occur, we need to satisfy the condition

.

$$\sigma_R = \sigma_C = \frac{K_C}{\sqrt{\pi a}\beta} \tag{9.2}$$



 Within the same family of materials (i.e. Aluminum), there is generally a trade-off between yield stress and fracture toughness

- Within the same family of materials (i.e. Aluminum), there is generally a trade-off between yield stress and fracture toughness
- As we increase the yield strength, we decrease the fracture toughness (and vice versa)

- Within the same family of materials (i.e. Aluminum), there is generally a trade-off between yield stress and fracture toughness
- As we increase the yield strength, we decrease the fracture toughness (and vice versa)
- · Consider a comparison of the following aluminum alloys

- Within the same family of materials (i.e. Aluminum), there is generally a trade-off between yield stress and fracture toughness
- As we increase the yield strength, we decrease the fracture toughness (and vice versa)
- · Consider a comparison of the following aluminum alloys
 - 1. 7178-T6, $K_C = 43 \text{ ksi} \sqrt{\text{in.}}, \sigma_{YS} = 74 \text{ksi}$

- Within the same family of materials (i.e. Aluminum), there is generally a trade-off between yield stress and fracture toughness
- As we increase the yield strength, we decrease the fracture toughness (and vice versa)
- · Consider a comparison of the following aluminum alloys
 - 1. 7178-T6, $K_C = 43 \text{ ksi} \sqrt{\text{in.}}$, $\sigma_{YS} = 74 \text{ksi}$
 - 2. 7075-T6, $K_C = 68 \text{ ksi} \sqrt{\text{in.}}, \sigma_{YS} = 63 \text{ksi}$

- Within the same family of materials (i.e. Aluminum), there is generally a trade-off between yield stress and fracture toughness
- As we increase the yield strength, we decrease the fracture toughness (and vice versa)
- · Consider a comparison of the following aluminum alloys
 - 1. 7178-T6, $K_C = 43 \text{ ksi} \sqrt{\text{in.}}, \sigma_{YS} = 74 \text{ksi}$
 - 2. 7075-T6, $K_C = 68 \text{ ksi} \sqrt{\text{in.}}, \sigma_{YS} = 63 \text{ksi}$
 - 3. 2024-T3, $K_C = 144 \text{ ksi} \sqrt{\text{in.}}, \sigma_{YS} = 42 \text{ksi}$

• As an example let us consider an edge-cracked panel with W=6" and t=0.1"

- As an example let us consider an edge-cracked panel with W=6" and t=0.1"
- The net section yield condition will be given by

- As an example let us consider an edge-cracked panel with W=6" and t=0.1"
- · The net section yield condition will be given by

$$\sigma_{\rm C} = \sigma_{\rm YS} \frac{W - a}{W} = \sigma_{\rm YS} \frac{6 - a}{6}$$

- As an example let us consider an edge-cracked panel with W = 6" and t = 0.1"
- · The net section yield condition will be given by

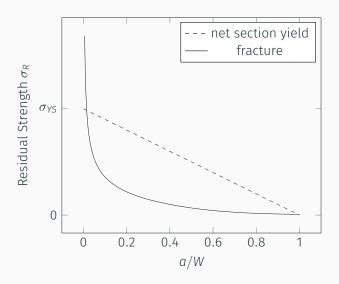
$$\sigma_{\rm C} = \sigma_{\rm YS} \frac{W - a}{W} = \sigma_{\rm YS} \frac{6 - a}{6}$$

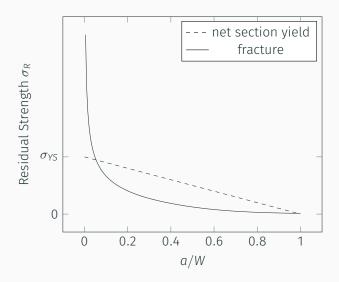
And the fracture condition by

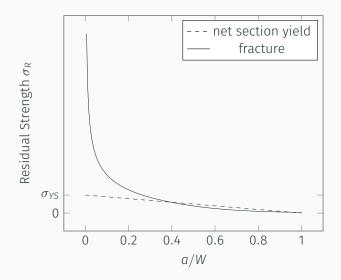
$$\sigma_{\rm C} = \frac{K_{\rm C}}{\sqrt{\pi a}\beta}$$

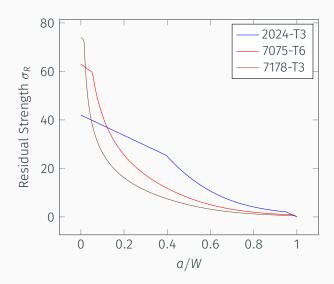
With

$$\beta = 1.12 - 0.231 \left(\frac{a}{W}\right) + 10.55 \left(\frac{a}{W}\right)^2 - 21.72 \left(\frac{a}{W}\right)^3 + 30.39 \left(\frac{a}{W}\right)^4$$









 \cdot Uses a different grain nomenclature

• Uses a different grain nomenclature

$$\begin{array}{c|cc} K_{C} & \sigma_{YS} \\ \hline L\text{-T} & L \\ \hline T\text{-L} & L\text{-T} \end{array}$$

· Uses a different grain nomenclature

$$\begin{array}{c|cc} K_C & \sigma_{YS} \\ \hline L-T & L \\ \hline T-L & L-T \end{array}$$

 A-Basis vs. B-Basis values are reported (A = 99% of population will meet/exceed value, B = 90% of population)

· Uses a different grain nomenclature

$$\begin{array}{c|cc} K_C & \sigma_{YS} \\ \hline L-T & L \\ \hline T-L & L-T \end{array}$$

- A-Basis vs. B-Basis values are reported (A = 99% of population will meet/exceed value, B = 90% of population)
- S-Basis no statistical information available, standard value to be used

- \cdot F_{tu} ultimate tensile strength
- F_{ty} tensile yield strength
- \cdot F_{cy} compressive yield strength
- F_{su} ultimate shear strength
- \cdot F_{bru} ultimate bearing strength
- \cdot F_{bry} bearing yield strength
- E tensile Young's Modulus
- \cdot E_c compressive Young's Modulus
- · G shear modulus
- μ Poisson's ratio



 Unfortunately, the method we described above does not quite match experimental results

- Unfortunately, the method we described above does not quite match experimental results
- Fedderson proposed an alternative, where we connect the net-section yield and brittle fracture curves with a tangent line

- Unfortunately, the method we described above does not quite match experimental results
- Fedderson proposed an alternative, where we connect the net-section yield and brittle fracture curves with a tangent line
- · This approach agrees very well with experimental data

- Unfortunately, the method we described above does not quite match experimental results
- Fedderson proposed an alternative, where we connect the net-section yield and brittle fracture curves with a tangent line
- · This approach agrees very well with experimental data
- Note: We could do something similar when the crack is very long, but we are generally less concerned with this region (failure will have already occurred)

 Proof testing is a way to use the concept of residual strength to check the size of a defect from manufacturing

- Proof testing is a way to use the concept of residual strength to check the size of a defect from manufacturing
- Due to the fatigue life of a certain panel, and/or an inspection cycle that we have prescribed for that part, we determine an "acceptable" initial flaw size, a_0

- Proof testing is a way to use the concept of residual strength to check the size of a defect from manufacturing
- Due to the fatigue life of a certain panel, and/or an inspection cycle that we have prescribed for that part, we determine an "acceptable" initial flaw size, a₀
- We then determine a load which would cause failure at this crack length

- Proof testing is a way to use the concept of residual strength to check the size of a defect from manufacturing
- Due to the fatigue life of a certain panel, and/or an inspection cycle that we have prescribed for that part, we determine an "acceptable" initial flaw size, a_0
- We then determine a load which would cause failure at this crack length
- This is the "proof load"

- Proof testing is a way to use the concept of residual strength to check the size of a defect from manufacturing
- Due to the fatigue life of a certain panel, and/or an inspection cycle that we have prescribed for that part, we determine an "acceptable" initial flaw size, a_0
- We then determine a load which would cause failure at this crack length
- · This is the "proof load"
- If the part does not fail in the proof test, we can assume that the largest flaw in the material is a_0

- Suppose we are concerned about edge cracks in a panel with $\sigma_{YS} = 65$ ksi, W = 5"
- We have determined that the largest allowable crack is 0.4"
- The fracture toughness of this panel is $K_c = 140 \text{ ksi}\sqrt{\text{in.}}$
- · We can find the proof load

$$\sigma_{c} = \frac{K_{c}}{\sqrt{\pi a_{0}}\beta}$$

$$= \frac{140}{\sqrt{\pi 0.4}(1.161)}$$

$$= 107.6$$

 So the proof load would need to induce a gross section stress of 107.6 ksi.