AE 737: Mechanics of Damage Tolerance

Lecture 2 - Common Stress Intensity Factors

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schedule

- 24 Jan - Common Stress Intensity Factors
- 29 Jan - Superposition, Compounding
- 31 Jan - Curved Boundaries, Homework 1 Due
- 5 Feb - Plastic Zone

office hours

- So far TBD students have participated in Doodle
- Take advantage of office hours, this is time that I have already set aside for you
- If the regular office hours do not work for your schedule, send me an e-mail and we can work out a time to meet
- While in person visits are often the most helpful, I will always try to answer questions as best as I can via e-mail

fracture mechanics

- In fracture mechanics we consider three different modes
- Mode I is known as the "opening mode"
- Mode II is known as the "sliding mode"
- Mode III is known as the "tearing mode"

fracture mechanics

Mode I:
Opening

Mode II:
Sliding

Mode III:
Tearing
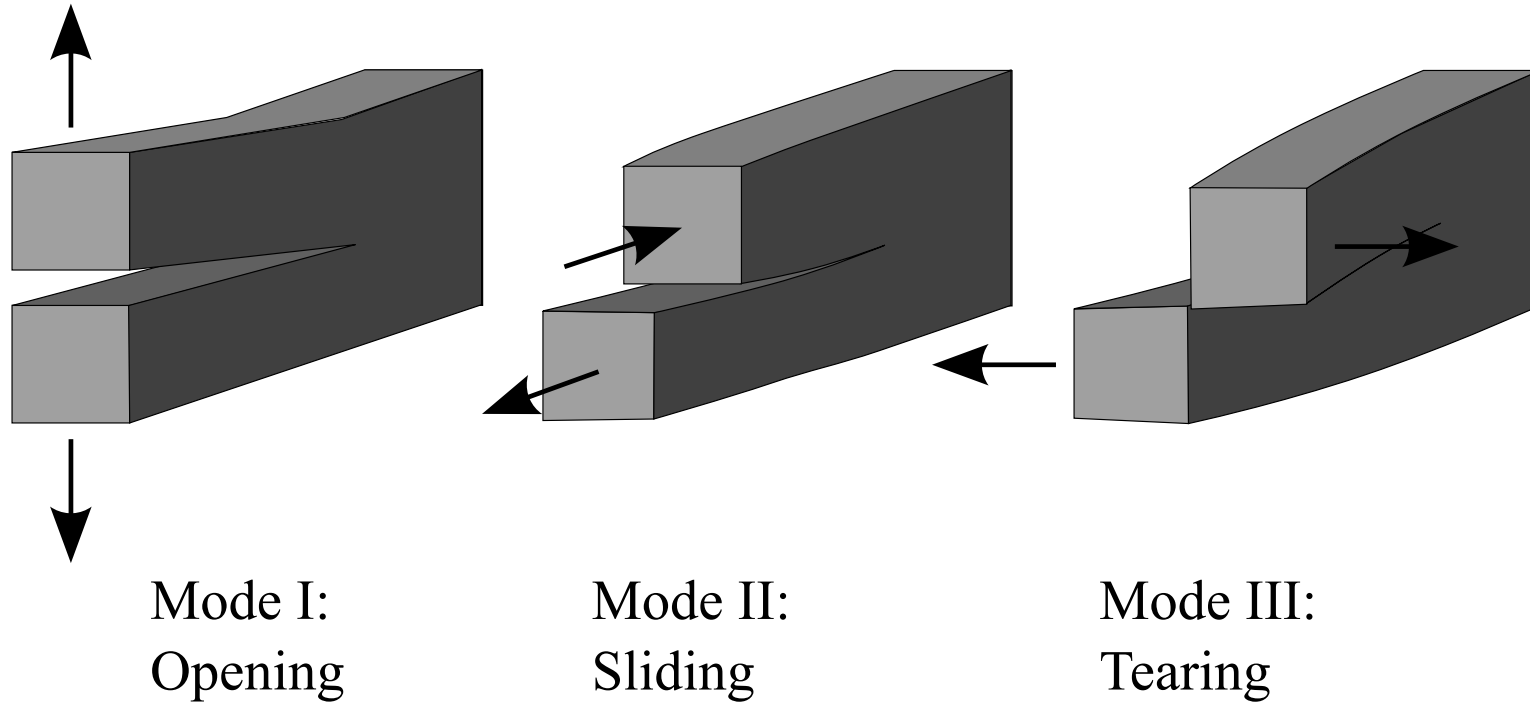
stress intensity

- A key finding from Linear Elastic Fracture Mechanics (LEFM) is known as the *Stress Intensity Factor*
- The stress intensity factor is often written in this form

$$K = \sigma\sqrt{\pi a}\beta$$

  - Where K is the stress intensity factor, σ is the applied stress, a is the crack length, and β is a dimensionless parameter depending on geometry

stress Intensity

- Be careful that although the notation is similar, the *Stress Intensity Factor* is different from the *Stress Concentration Factor* from strength of materials
- We are usually most concerned with Mode I, but there will be a unique stress intensity factor for each mode, we label these K_I, K_{II}, and K_{III}
- If no subscript is given, assume Mode I

stress intensity

- For brittle materials (where "linear" fracture mechanics assumptions hold true) we can find the full stress field near the crack in terms of the stress intensity factor

$$\sigma_x = \frac{K_I}{\sqrt{2\pi r}}\cos\frac{\theta}{2}\left(1 - \sin\frac{\theta}{2}\sin\frac{3\theta}{2}\right)$$

$$\sigma_y = \frac{K_I}{\sqrt{2\pi r}}\cos\frac{\theta}{2}\left(1 + \sin\frac{\theta}{2}\sin\frac{3\theta}{2}\right)$$

$$\tau_{xy} = \frac{K_I}{\sqrt{2\pi r}}\sin\frac{\theta}{2}\cos\frac{\theta}{2}\cos\frac{3\theta}{2}$$

mode II

- Similarly for Mode II we find

$$\sigma_x = \frac{-K_{II}}{\sqrt{2\pi r}}\sin\frac{\theta}{2}\left(2 + \cos\frac{\theta}{2}\cos\frac{3\theta}{2}\right)$$

$$\sigma_y = \frac{K_{II}}{\sqrt{2\pi r}}\sin\frac{\theta}{2}\cos\frac{\theta}{2}\cos\frac{3\theta}{2}$$

$$\tau_{xy} = \frac{K_{II}}{\sqrt{2\pi r}}\cos\frac{\theta}{2}\left(1 - \sin\frac{\theta}{2}\sin\frac{3\theta}{2}\right)$$
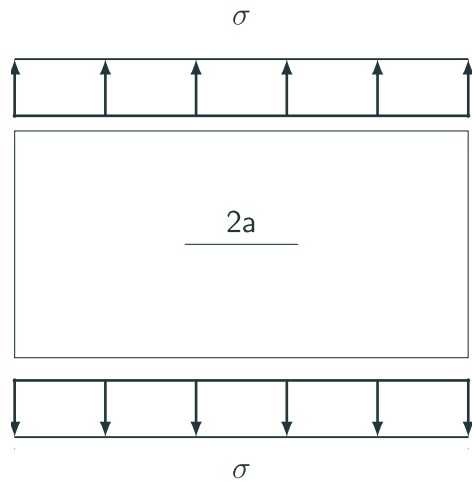
mode III

- And for Mode III

$$\tau_{xz} = \frac{-K_{III}}{\sqrt{2\pi r}}\sin\frac{\theta}{2}$$

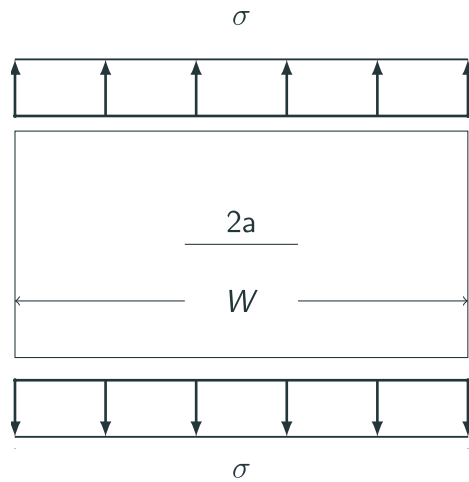$$\tau_{yz} = \frac{K_{III}}{\sqrt{2\pi r}}\cos\frac{\theta}{2}$$

common stress intensity factors

center crack, infinite width

$$K_I = \sigma\sqrt{\pi a}$$

σ

2a

σ

center crack, finite width

center crack, finite width

$$K_I = \sigma\sqrt{\pi a}\sqrt{\sec(\pi a / W)}$$
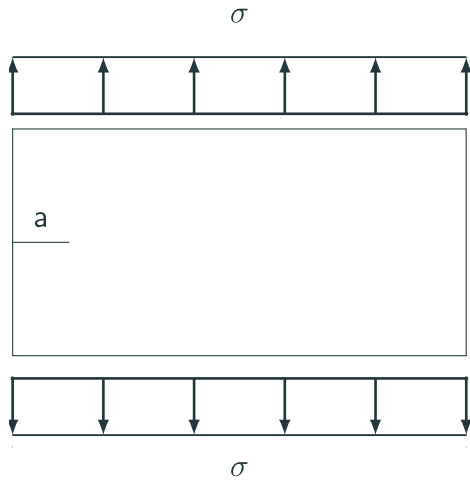
- Accurate within 0.3% for $2a / W \leq 0.7$
- within 1.0% for $2a / W = -.8$

$$K_I = \sigma\sqrt{\pi a}\left[1.0 - 0.025\left(\frac{2a}{W}\right)^2 + 0.06\left(\frac{2a}{W}\right)^4\right]\sqrt{\sec(\pi a / W)}$$
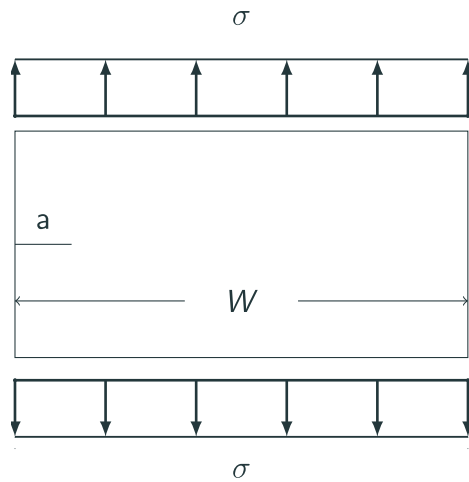
- Accurate within 0.1% for all crack lengths.

edge crack, semi-infinite width

$$K_I = 1.122\sigma\sqrt{\pi a}$$
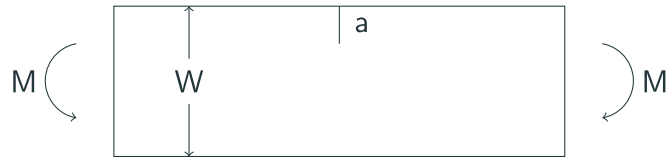
edge crack, finite width

edge crack, finite width

$$\beta = \left[1.122 - 0.231\frac{a}{W} + 10.55 \left(\frac{a}{W}\right)^2 - 21.71 \left(\frac{a}{W}\right)^3 + 30.82 \left(\frac{a}{W}\right) \right.$$

- Within 0.5% accuracy for $\frac{a}{W} < 0.6$

$$\beta = \frac{0.752 + 2.02\frac{a}{W} + 0.37 \left(1 - \sin\frac{\pi a}{2W}\right)^3}{\cos\frac{\pi a}{2W}} \sqrt{\frac{2W}{\pi a}\tan\frac{\pi a}{2W}}$$

- Within 0.5% accuracy for $0 < \frac{a}{W} < 1.0$

edge crack, bending moment

edge crack, bending moment

- The usual form for stress intensity still applies

$$K_I = \sigma\sqrt{\pi a}\beta$$

- Where $\sigma = \dfrac{6M}{tW^2}$

$$\beta = 1.122 - 1.40 \left(\frac{a}{W}\right) + 7.33 \left(\frac{a}{W}\right)^2 - 13.08 \left(\frac{a}{W}\right)^3 + 14.0 \left(\frac{a}{W}\right.$$

- valid within 0.2% accuracy for $\dfrac{a}{W} \leq 0.6$

edge crack, bending moment

$$\beta = \frac{0.923 + 0.199\left(1 - \sin\dfrac{\pi a}{2W}\right)^4}{\cos\dfrac{\pi a}{2W}}\sqrt{\frac{2W}{\pi a}\tan\frac{\pi a}{2W}}$$

- valid within 0.5% for any $\dfrac{a}{W}$

nominal bending stress

- The nominal bending stress is for rectangular cross-sections

- A more general form is given by $\sigma = \dfrac{Mc}{I}$

- Where for a rectangular cross-section, $c = W/2$ and $I = tW^3/12$ which simplifies as shown previously

center crack, finite width, splitting forces
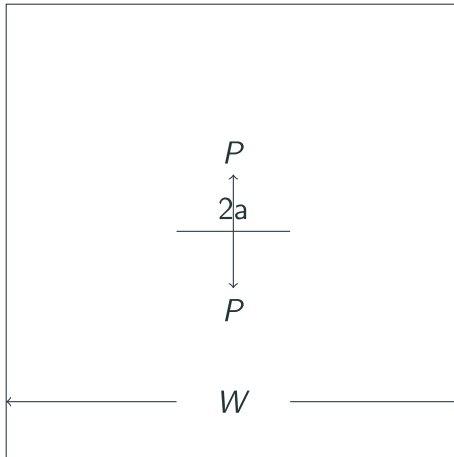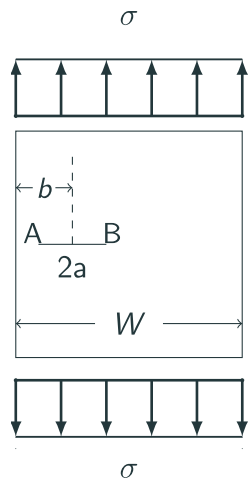
center crack, finite width, splitting forces

- With an applied load we use a slightly modified form for the stress intensity factor $K_I = \dfrac{P}{t\sqrt{\pi a}}\beta$

- With β in this case given as

$$\beta = \frac{1 - 0.5\left(\dfrac{a}{W}\right) + 0.975\left(\dfrac{a}{W}\right)^2 - 0.16\left(\dfrac{a}{W}\right)^3}{\sqrt{1 - \left(\dfrac{a}{W}\right)}}$$

offset crack

offset crack

$$K_{IA} = \sigma\sqrt{\pi a}\,\beta_C\beta_A \ \text{ and } \ K_{IB} = \sigma\sqrt{\pi a}\,\beta_C\beta_B$$

$$\beta_C = \sqrt{\sec\frac{\pi a}{W}}$$

offset crack

$$\beta_A = (1 - 0.025\lambda^2 + 0.6\lambda^4 - \gamma\lambda^{11})$$

$$\sqrt{\sec\left(\frac{\pi\lambda}{2}\right)\frac{\sin\left(2\lambda - 4\frac{a}{W}\right)}{2\lambda - 4\frac{a}{W}}}$$

$$\beta_B = (1 - 0.025\delta^2 + 0.06\delta^4 - \zeta\lambda^{30})$$

$$\left(1 + \frac{\sqrt{\sec\left(\frac{2\pi\lambda + 1.5\pi\delta}{7}\right)} - 1}{1 + 0.21\sin\left(8\tan^{-1}\left(\frac{\lambda - \delta}{\lambda + \delta}\right)^{0.9}\right)}\right)$$

offset crack

- The parameters λ, δ are given as
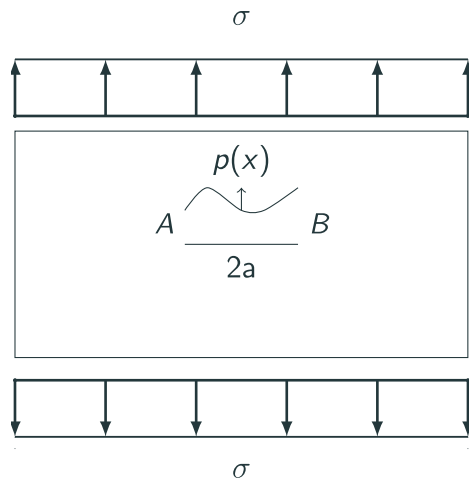
$$\lambda = \frac{a}{b}$$

$$\delta = \frac{a}{W - b}$$

offset crack

- And γ and ζ can be looked up on a table

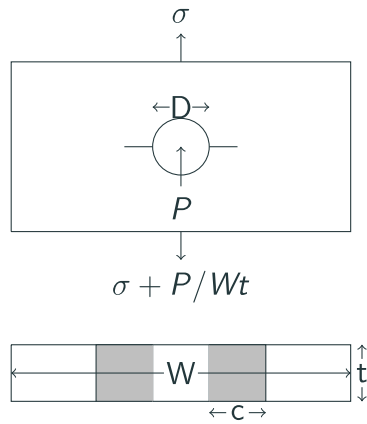| $\dfrac{b}{W}$	γ	ζ
0.1	0.382	0.114
0.25	0.136	0.286
0.4	0.0	0.0
0.5	0.0	0.0

non-uniform stress, infinite width

non-uniform stress, infinite width

- Stress intensity will be different at points A and B

$$K_{IA} = \int_{-a}^{a} \frac{p(x)\sqrt{a-x}}{\sqrt{\pi a}\sqrt{a+x}} dx$$

$$K_{IB} = \int_{-a}^{a} \frac{p(x)\sqrt{a+x}}{\sqrt{\pi a}\sqrt{a-x}} dx$$

cracks around a hole

$$\sigma$$

$$\sigma + P/Wt$$

cracks around a hole

- For symmetric through cracks under uniform applied stress, we have

$$\beta = \beta_1 + \beta_2$$

$$\beta_1 = F_{c/R}F_wF_{ww}$$

$$\beta_2 = \frac{\sigma_{br}}{\sigma}F_3F_wF_{ww}$$

$$F_{c/R} = \frac{3.404 + 3.8172\dfrac{c}{R}}{1 + 3.9273\dfrac{c}{R} - 0.00695\left(\dfrac{c}{R}\right)^2}$$

$$F_w = \sqrt{\sec\frac{\pi R}{W}\sec\frac{\pi(R + c)}{W}}$$

cracks around a hole

$$F_{ww} = 1 - \left(\left(1.32\frac{W}{D} - 0.14 \right)^{-\left(.98 + \left(0.1\frac{W}{D} \right)^{0.1} \right)} - 0.02 \right)$$

$$\left(\frac{2c}{W - D} \right)^{N}$$

$$F_3 = 0.098 + 0.3592e^{-3.5089\frac{c}{R}} + 0.3817e^{-0.5515\frac{c}{R}}$$
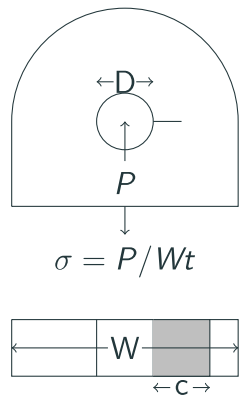
cracks around a hole

- Note that

$$\sigma_{br} = \frac{P}{Dt}$$

$$N = \frac{W}{D} + 2.5 \quad \text{when} \quad \frac{W}{D} < 2$$

$$N = 4.5 \quad \text{otherwise}$$

- Also R is the radius, $R = \frac{D}{2}$

cracks around a hole

$$\sigma = P/Wt$$

cracks around a hole

$$\beta = \beta_1 + \beta_2$$

$$\beta_1 = \beta_3 F_w F_{ww}$$

$$\beta_2 = \frac{\sigma_{br}}{\sigma} F_4 F_w F_{ww}$$

$$\beta_3 = 0.7071 + 0.7548\frac{R}{R + c} + 0.3415\left(\frac{R}{R + C}\right)^2 +$$

$$0.6420\left(\frac{R}{R + c}\right)^3 + 0.9196\left(\frac{R}{R + c}\right)^4$$

$$F_4 = 0.9580 + 0.2561\frac{C}{R} - 0.00193\left(\frac{C}{R}\right)^{2.5} - 0.9804\left(\frac{C}{R}\right)^{0.5}$$

cracks around a hole

$$F_W = \sqrt{\sec\frac{\pi R}{W}\sec\frac{\pi(R + c/2)}{W - c}}$$

$$F_{WW} = 1 - N^{-\frac{W}{D}}\left(\frac{2c}{W - D}\right)^{\frac{W}{D}+0.5}$$

$$N = 2.65 - 0.24\left(2.75 - \frac{W}{D}\right)^2$$

$$N \geq 2.275 \quad (\text{if } N < 2.275, \text{ let } N = 2.275)$$

Also note that R indicates radius, $R = \dfrac{D}{2}$

group problems

1. Find K_I for a center-cracked panel with $W/2a = 3$ and a uniformly applied remote stress, σ.

2. Find K_I for an edge-cracked panel with $W/a = 3$ and a uniformly applied remote stress, σ.

3. Find K_I for an edge-cracked panel with $W/a = 3$ and a remote bending moment, $M = tW^2\sigma/6$.

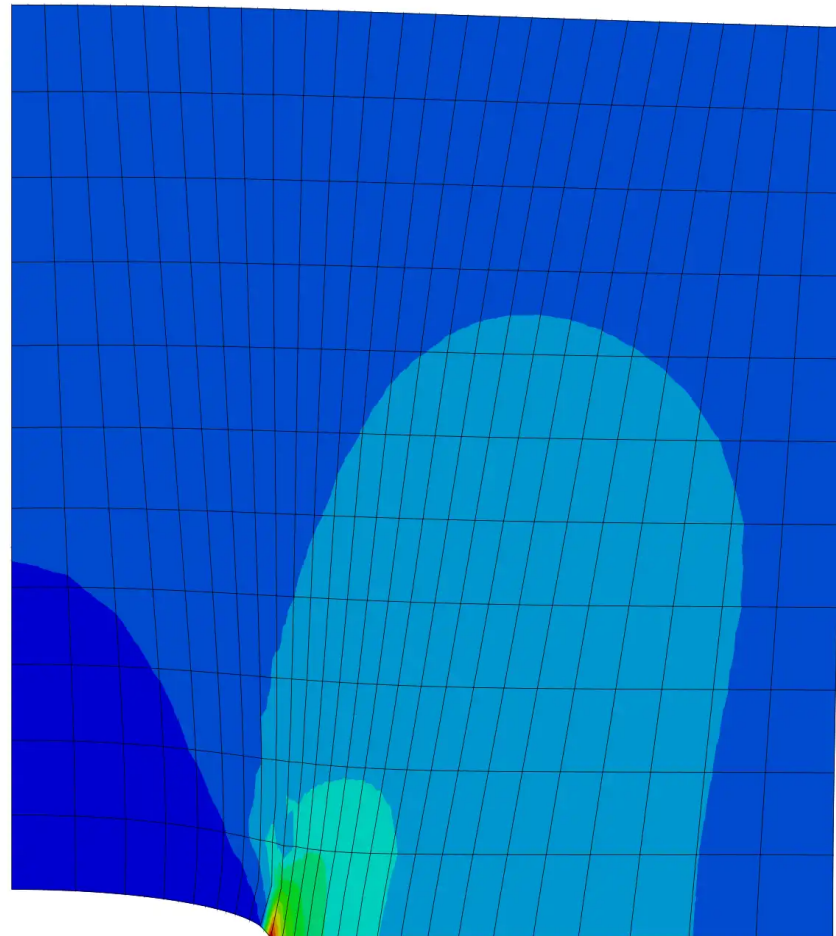4. Find K_I for a center-cracked panel with $W/2a = 3$ and a concentrated splitting force, $P = \sigma at$.

group problems

1. What do you think causes the difference (if any) in stress intensity between these panels?
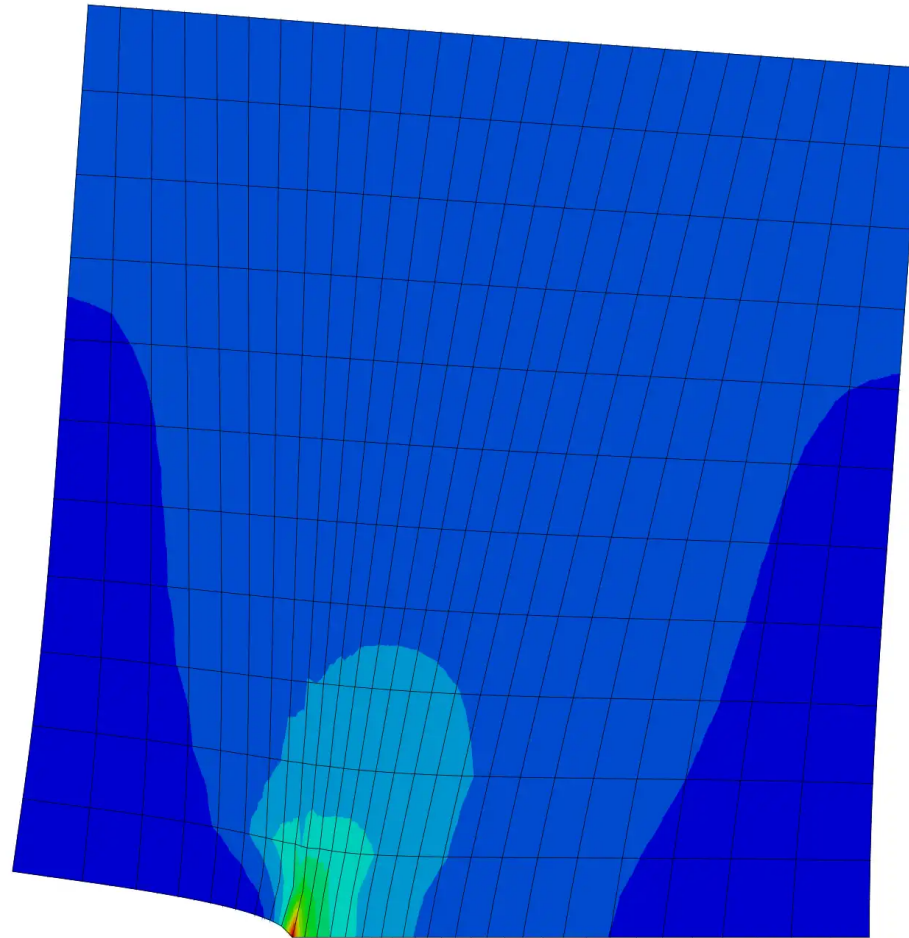
example 1

S, S22
(Avg: 75%)

```
6 70
6 14
5 57
5 01
4 45
3 88
3 32
2 76
2 19
1 63
1 07
0 50
−0.06
```

example 1

S, Mises
(Avg: 75%)
10.33
9.47
8.61
7.75
6.89
6.03
5.17
4.31
3.45
2.59
1.73
0.87
0.01

2D crack shapes

crack depth

- The previous stress intensity factors all assume a 2D problem (with a 1D crack)
- Through the thickness, it is assumed that the crack length is the same
- In many cases this is not an accurate assumption
- We will now consider 2D crack shapes and their effect on the stress intensity factor

elliptical flaw, infinite solid
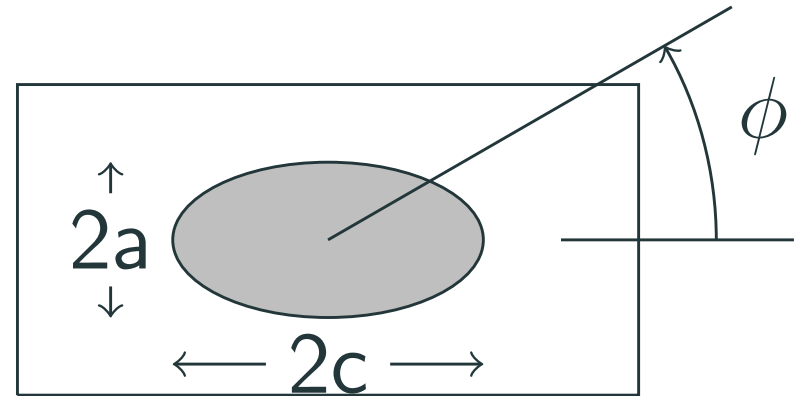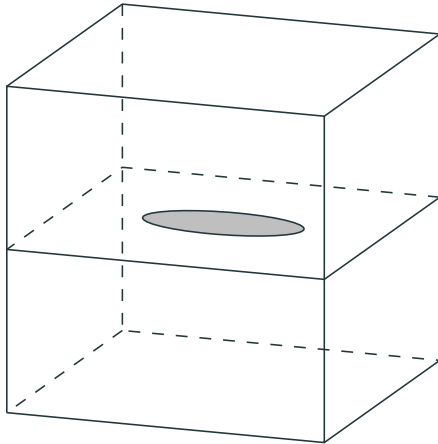
elliptical flaw, infinite solid

- For an ellipse the stress intensity factor will vary with the angle, φ

$$K_I = \sigma\sqrt{\pi a}\beta$$

$$\beta = \sqrt{\frac{1}{Q}}f_\varphi$$

$$Q = 1 + 1.464\left(\frac{a}{c}\right)^{1.65} \qquad \text{if } a/c \leq 1$$

elliptical flaw, infinite solid

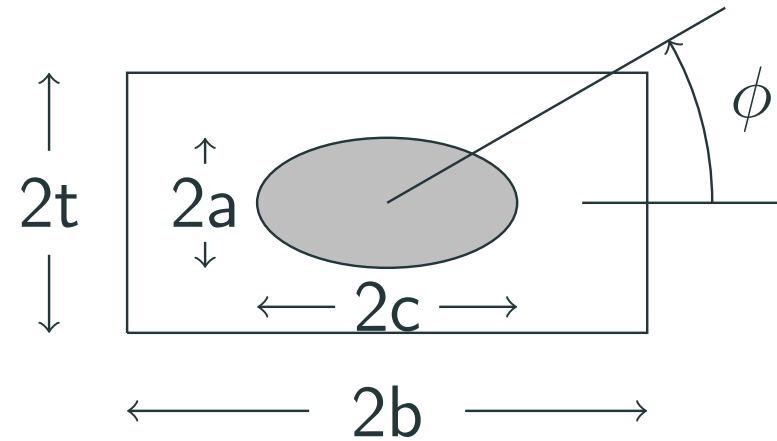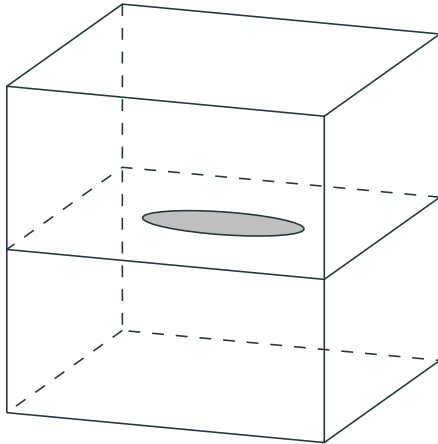- For an ellipse the stress intensity factor will vary with the angle, φ

$$Q = 1 + 1.464 \left(\frac{c}{a} \right)^{1.65} \qquad \text{if } a/c > 1$$

$$f_\varphi = \left(\left(\frac{a}{c} \right)^2 \cos^2\varphi + \sin^2\varphi \right)^{1/4} \qquad \text{if } a/c \leq 1$$

$$f_\varphi = \left(\cos^2\varphi + \left(\frac{c}{a} \right)^2 \sin^2\varphi \right)^{1/4} \qquad \text{if } a/c > 1$$

elliptical flaw, finite solid

finite solid

$$\beta = \sqrt{\frac{1}{Q}} F_e$$

$$F_e = \frac{\left(M_1 + M_2 \left(\frac{a}{t} \right)^2 + M_3 \left(\frac{a}{t} \right)^4 \right) g f_\varphi f_w}{\sqrt{\sec \left(\frac{\pi c}{2b} \sqrt{\frac{a}{t}} \right)}}$$

$$f_w = \sqrt{\sec \left(\frac{\pi c}{2b} \sqrt{\frac{a}{t}} \right)}$$

$$g = 1 - \frac{\left(\frac{a}{t} \right)^4 \left(2.6 - 2\frac{a}{t} \right)^{1/2}}{1 + 4\frac{a}{c}} \cos\varphi$$

elliptical flaw, finite solid

$$M_2 = \frac{0.05}{0.11 + \left(\dfrac{a}{c}\right)^{3/2}}$$

$$M_3 = \frac{0.29}{0.23 \left(\dfrac{a}{c}\right)^{3/2}}$$

elliptical flaw, finite solid

- If $a/c \leq 1$
  $$M_1 = 1$$

$$Q = 1 + 1.464 \left(\frac{a}{c}\right)^{1.65}$$

$$f_\varphi = \left(\left(\frac{a}{c}\right)^2 \cos^2\varphi + \sin^2\varphi\right)^{1/4}$$
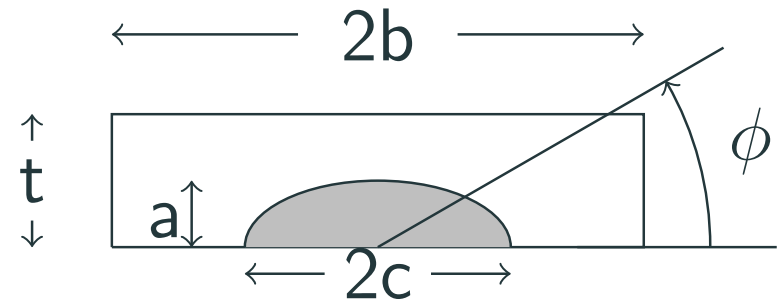
elliptical flaw, finite solid

- Otherwise (a / c > 1)

$$M_1 = \left(\frac{c}{a} \right)^{1/2}$$

$$Q = 1 + 1.464 \left(\frac{c}{a} \right)^{1.65}$$

$$f_\varphi = \left(\cos^2\varphi + \left(\frac{c}{a} \right)^2 \sin^2\varphi \right)^{1/4}$$

semi-elliptical surface flaw, finite body

semi-elliptical surface flaw, finite body

$$K_I = \sigma\sqrt{\pi a}\beta$$

$$\beta = \sqrt{\frac{1}{Q}}F_s$$

$$F_s = \frac{\left(M_1 + M_2\left(\frac{a}{t}\right)^2 + M_3\left(\frac{a}{t}\right)^4\right)gf_\varphi f_w}{\sqrt{\sec\left(\frac{\pi c}{2b}\sqrt{\frac{a}{t}}\right)}}$$

$$f_w = \sqrt{\sec\left(\frac{\pi c}{2b}\sqrt{\frac{a}{t}}\right)}$$

surface flaw, $\dfrac{a}{c} \leq 1$

$$M_1 = 1.13 - 0.09 \left(\frac{a}{c} \right)$$

$$M_2 = -0.52 + \frac{0.89}{0.2 + \dfrac{a}{c}}$$

$$M_3 = 0.5 - \frac{1}{0.65 + \dfrac{a}{c}} + 14 \left(1 - \frac{a}{c} \right)^4$$

surface flaw, $\dfrac{a}{c} \leq 1$

$$Q = 1 + 1.464 \left(\frac{a}{c} \right)^{1.65}$$

$$f_{\varphi} = \left(\left(\frac{a}{c} \right)^{2} \cos^{2}\varphi + \sin^{2}\varphi \right)^{1/4}$$

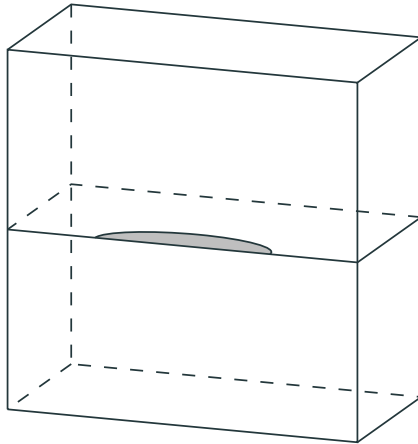$$g = 1 + \left(0.1 + 0.35 \left(\frac{a}{t} \right)^{2} \right) (1 - \sin\varphi)^{2}$$

surface flaw, $\dfrac{a}{c} > 1$

$$M_1 = \left(\dfrac{c}{a}\right)^{1/2}\left(1 + 0.04\dfrac{c}{a}\right)$$

$$M_2 = 0.2\left(\dfrac{c}{a}\right)^4$$

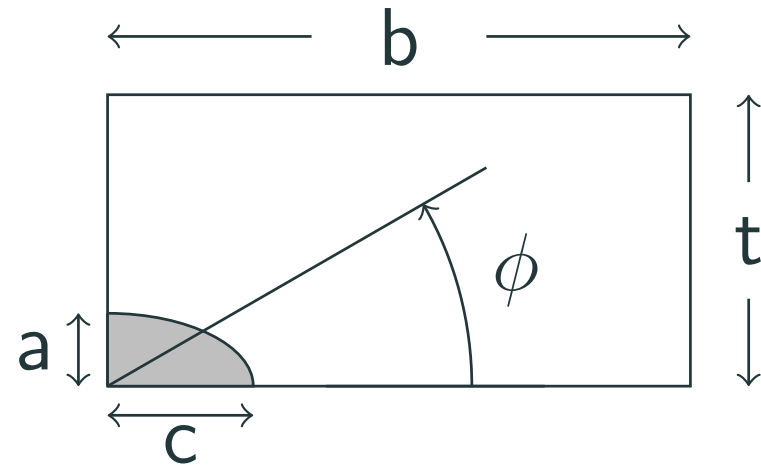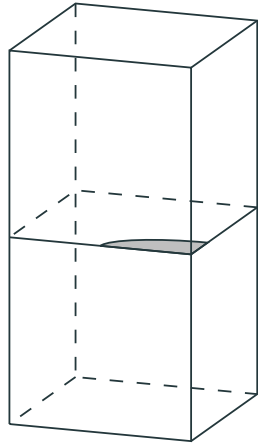$$M_3 = -0.11\left(\dfrac{c}{a}\right)^4$$

$$\text{surface flaw}, \frac{a}{c} > 1$$

$$Q = 1 + 1.464 \left(\frac{c}{a} \right)^{1.65}$$

$$f_\varphi = \left(\cos^2\varphi + \left(\frac{c}{a} \right)^2 \sin^2\varphi \right)^{1/4}$$

$$g = 1 + \left(0.1 + 0.35 \left(\frac{c}{a} \right) \left(\frac{a}{t} \right)^2 \right)(1 - \sin\varphi)^2$$

corner flaw, finite body

corner flaw, finite body

$$K_I = \sigma\sqrt{\pi a}\,\beta$$

$$\beta = \sqrt{\frac{1}{Q}}F_c$$

$$F_c = \left(M_1 + M_2\left(\frac{a}{t}\right)^2 + M_3\left(\frac{a}{t}\right)^4\right)g_1 g_2 f_\varphi f_w$$

$$f_w = 1 - 0.2\lambda + 9.4\lambda^2 - 19.4\lambda^3 + 27.1\lambda^4$$

$$\lambda = \left(\frac{c}{b}\right)\left(\frac{a}{t}\right)^{1/2}$$

corner flaw, finite body, $\dfrac{a}{c} \leq 1$

$$M_1 = 1.08 - 0.03 \left(\dfrac{a}{c} \right)$$

$$M_2 = -0.44 + \dfrac{1.06}{0.3 + \dfrac{a}{c}}$$

$$M_3 = -0.5 + 0.25\dfrac{a}{c} + 14.8 \left(1 - \dfrac{a}{c} \right)^{1.5}$$

corner flaw, finite body, $\dfrac{a}{c} \leq 1$

$$Q = 1 + 1.464 \left(\frac{a}{c} \right)^{1.65}$$

$$f_{\varphi} = \left(\left(\frac{a}{c} \right)^2 \cos^2\varphi + \sin^2\varphi \right)^{1/4}$$

$$g_1 = 1 + \left(0.08 + 0.4 \left(\frac{a}{t} \right)^2 \right)(1 - \sin\varphi)^3$$

$$g_2 = 1 + \left(0.08 + 0.15 \left(\frac{a}{t} \right)^2 \right)(1 - \cos\varphi)^3$$

corner flaw, finite body, $\dfrac{a}{c} > 1$

$$M_1 = \left(\dfrac{c}{a}\right)^{1/2}\left(1.08 - 0.03\dfrac{c}{a}\right)$$

$$M_2 = 0.375\left(\dfrac{c}{a}\right)^4$$

$$M_3 = -0.25\left(\dfrac{c}{a}\right)^2$$

corner flaw, finite body, $\dfrac{a}{c} > 1$

$$Q = 1 + 1.464 \left(\frac{c}{a} \right)^{1.65}$$

$$f_{\varphi} = \left(\cos^2 \varphi + \left(\frac{c}{a} \right)^2 \sin^2 \varphi \right)^{1/4}$$

$$g_1 = 1 + \left(0.08 + 0.4 \left(\frac{c}{t} \right)^2 \right)(1 - \sin\varphi)^3$$

$$g_2 = 1 + \left(0.08 + 0.15 \left(\frac{c}{t} \right)^2 \right)(1 - \cos\varphi)^3$$

example 2
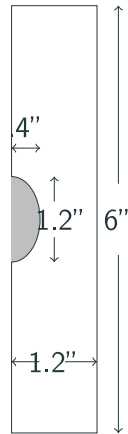
- Find maximum value of K_I for semi-elliptical surface flaw
- σ = 20 kpsi (in opening direction)

example 2

- Here we will use the formula for a semi-elliptical surface flaw
- In the first step we find `a / c = 0.4 / 0.6 < 1`, so we use that set of formulae
- A worked python notebook of this example can be found **here**

2D cracks at a hole

when to consider 2D crack shape

- When do we need to worry about 2D crack shape?
- The important factor is ratio of crack length to thickness
- When crack length is less than 5 times thickness, 2D shape effects are not negligible

cracks around a hole
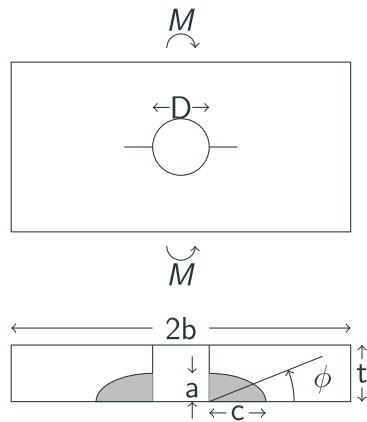
remote stress

$$K_I = \sigma\sqrt{\pi a}\,\beta$$

$$\beta = \sqrt{\frac{1}{Q}}\,F_{ch}$$

$$F_{ch} = \frac{\left(M_1 + M_2\left(\dfrac{a}{t}\right)^2 + M_3\left(\dfrac{a}{t}\right)^4\right)g_1 g_2 g_3 g_4 f_\varphi f_w}{}$$

$$f_w = \sqrt{\sec\left(\frac{\pi r}{2b}\right)\sec\left(\frac{\pi(2r + nc)}{4(b - c) + 2nc}\sqrt{\frac{a}{t}}\right)}$$

remote stress

$$g_2 = \frac{1 + 0.358\lambda + 1.425\lambda^2 - 1.578\lambda^3 + 2.156\lambda^4}{1 + 0.13\lambda^2}$$

$$\lambda = \frac{1}{1 + (c / r)\cos(0.85\varphi)}$$

Where n = number of cracks (1 or 2)

remote stress when $a/c \leq 1$

$$M_1 = 1.13 - 0.09(a/c)$$

$$M_2 = -0.54 + \frac{0.89}{0.2 + a/c}$$

$$M_3 = 0.5 - \frac{1}{0.65 + a/c} + 14(1 - a/c)^{24}$$

$$Q = 1 + 1.464(a/c)^{1.65}$$

remote stress when $a/c \leq 1$

$$g_1 = 1 + \left(0.1 + 0.35(a/t)^2 \right)(1 - \sin\varphi)^2$$

$$g_3 = (1 + 0.04(a/c)) \left(1 + 0.1(1 - \cos\varphi)^2 \right) \left(0.85 + 0.15(a/t)^{1/4} \right.$$

$$g_4 = 1 - 0.7(1 - a/t)(a/c - 0.2)(1 - a/c)$$

$$f_\varphi = \left((a/c)^2\cos^2\varphi + \sin^2\varphi \right)^{1/4}$$

remote stress when $a/c > 1$

$$M_1 = \sqrt{c/a}\,(1 + 0.04(c/a))$$

$$M_2 = 0.2(c/a)^4$$

$$M_3 = -0.11(c/a)^4$$

$$Q = 1 + 1.464 \left(\frac{c}{a}\right)^{1.65}$$

remote stress when $a/c > 1$

$$g_1 = 1 + \left(0.1 + 0.35(c/a)(a/t)^2 \right)(1 - \sin\varphi)^2$$

$$g_3 = (1.13 - 0.09(c/a)) \left(1 + 0.1(1 - \cos\varphi)^2 \right) \left(0.85 + 0.15(a/t) \right.$$
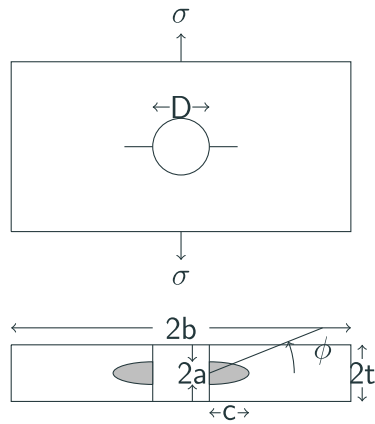
$$g_4 = 1$$

$$f_\varphi = \left(\cos^2\varphi + \left(\frac{c}{a} \right)^2 \sin^2\varphi \right)^{1/4}$$

remote stress

- The same formulas apply for both symmetric cracks (n = 2) and a single crack (n = 1) with one additional correction factor applied to the single crack case

$$K_{I,single} = \sqrt{\frac{4/\pi + ac/2tr}{4/\pi + ac/tr}} K_{I,symmetric}$$

surface cracks around a hole

remote stress

$$K_I = \sigma\sqrt{\pi a}\,\beta$$

$$\beta = \sqrt{\frac{1}{Q}}\,F_{sh}$$

$$F_{sh} = \left(M_1 + M_2\left(\frac{a}{t}\right)^2 + M_3\left(\frac{a}{t}\right)^4\right)g_1 g_2 g_3 f_\varphi f_w$$

$$f_w = \sqrt{\sec\left(\frac{\pi r}{2b}\right)\sec\left(\frac{\pi(2r+nc)}{4(b-c)+2nc}\sqrt{\frac{a}{t}}\right)}$$

remote stress

$$M_2 = \frac{0.05}{0.11 + (a/c)^{3/2}}$$

$$M_3 = \frac{0.29}{0.23 + (a/c)^{3/2}}$$

Where n = number of cracks (1 or 2)

remote stress

$$g_1 = 1 - \frac{(a/t)^4(2.6 - 2a/t)^{1/2}}{1 + 4a/c}\cos\varphi$$

$$g_2 = \frac{1 + 0.358\lambda + 1.425\lambda^2 - 1.578\lambda^3 + 2.156\lambda^4}{1 + 0.08\lambda^2}$$

$$\lambda = \frac{1}{1 + (c/r)\cos(0.9\varphi)}$$

$$g_3 = 1 + 0.1(1 - \cos\varphi)^2(1 - a/t)^{10}$$

remote stress $a / c \leq 1$

$$Q = 1 + 1.464(a / c)^{1.65}$$

$$M_1 = 1$$

$$f_\varphi = \left(\left(\frac{a}{c} \right)^2 \cos^2\varphi + \sin^2\varphi \right)^{1/4}$$

remote stress $a/c > 1$

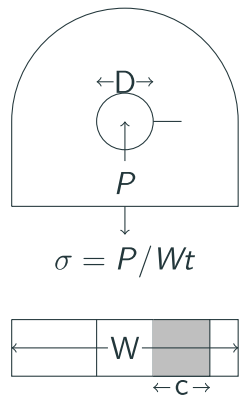$$Q = 1 + 1.464(c/a)^{1.65}$$

$$M_1 = \sqrt{c/a}$$

$$f_\varphi = \left(\cos^2\varphi + \left(\frac{c}{a} \right)^2 \sin^2\varphi \right)^{1/4}$$

single-crack correction

- When the surface crack is only on one side of the hole, we use the same correction as for corner cracks

$$K_{I,single} = \sqrt{\frac{4/\pi + ac/2tr}{4/\pi + ac/tr}} K_{I,symmetric}$$

edge crack on a lug

$$\sigma = P/Wt$$

edge crack on a lug

$$K_I = \sigma_{br}\sqrt{\pi c}\,\beta$$

$$\beta = \left(\frac{G_0 D}{2W} + G_1 \right) G_W G_L G_2$$

$$z = \left(1 + \frac{2C}{D} \right)^{-1}$$

$$G_0 = 0.7071 + 0.7548z + 0.3415z^2 + 0.642z^3 + 0.9196z^4$$

$$G_1 = 0.078z + 0.7588z^2 - 0.4293z^3 + 0.0644z^4 + 0.651z^5$$

$$G_L = \left(\sec\left(\frac{\pi D}{2W} \right) \right)^{1/2}$$

edge crack on a lug

$$\lambda = \frac{\pi}{2}\left(\frac{D+c}{W-c}\right)$$

$$G_W = (\sec\lambda)^{1/2}$$

$$b = \frac{W-D}{2}$$

$$A_1 = 0.688 + 0.772\frac{D}{W} + 0.613\left(\frac{D}{W}\right)^2$$

$$A_2 = 4.948 - 17.318\frac{D}{W} + 16.785\left(\frac{D}{W}\right)^2$$

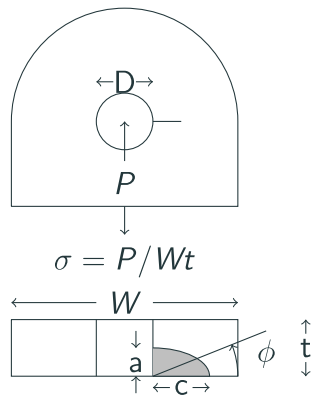# edge crack on a lug

$$A_3 = -14.297 + 62.994\frac{D}{W} - 69.818\left(\frac{D}{W}\right)^2$$

$$A_4 = 12.35 - 58.644\frac{D}{W} + 66.387\left(\frac{D}{W}\right)^2$$

$$G_2 = A_1 + A_2\frac{c}{b} + A_3\left(\frac{c}{b}\right)^2 + A_4\left(\frac{c}{b}\right)^3$$

corner crack on a lug

$$\sigma = P/Wt$$

corner crack on a lug

$$\beta = \left(\frac{G_0 D}{2W} + G_1 \right) G_W$$

$$z = \left(1 + 2\frac{c}{D}\cos(0.85\varphi) \right)^{-1}$$

$$f_0(z) = 0.7071 + 0.7548z + 0.3415z^2 + 0.642z^3 + 0.9196z^4$$

$$f_1(z) = 0.078z + 0.7588z^2 - 0.4293z^3 + 0.0644z^4 + 0.651z^5$$

$$G_0 = \frac{f_0(z)}{d_0}$$

$$d_0 = 1 + 0.13z^2$$

corner crack on a lug

$$g_p = \left(\frac{W + D}{W - D} \right)^{1/2}$$

$$G_1 = f_1(z) \left(\frac{g_p}{d_0} \right)$$

$$G_w = M_0 g_1 g_3 g_4 f_\varphi f_w f_x$$

$$v = \frac{a}{t}$$

corner crack on a lug

$$\lambda = \frac{\pi}{2}\sqrt{v} \left(\frac{D + c}{W - c} \right)$$

$$f_w = \left(\sec\lambda \sec\frac{\pi D}{2W} \right)^{1/2}$$

$$x = \frac{a}{c}$$

corner crack on a lug $a/c \leq 1$

$$f_\varphi = \left(\left(\frac{a}{c}\cos\varphi \right)^2 + \sin^2\varphi \right)^{1/4}$$

$$f_x = \left(1 + 1.464 \left(\frac{a}{c} \right)^{1.65} \right)^{-1/2}$$

$$M_0 = (1.13 - 0.09x) + \left(-0.54 + \frac{0.89}{0.2 + x} \right)v^2 \colorbox{red!50}{\$+}$$

$$\left(0.5 - \frac{1}{.65 - x} + 14(1 - x^{24}) \right)v^4$$

corner crack on a lug $a / c \leq 1$

$$g_1 = 1 + \left(0.1 + 0.35v^2\right)(1 - \sin\varphi)^2$$

$$g_3 = (1 + 0.04x)\left(1 + 0.1(1 - \cos\varphi)^2\right)\left(0.85 + 0.15v^{1/4}\right)$$

$$g_4 = 1 - 0.7(1 - v)(x - 0.2)(1 - x)$$

corner crack on a lug $a / c > 1$

$$f_\varphi = \left(\left(\frac{ac}{c} \sin\varphi \right)^2 + \cos^2\varphi \right)^{1/4}$$

$$f_x = \left(1 + 1.464 \left(\frac{c}{a} \right)^{1.65} \right)^{-1/2}$$

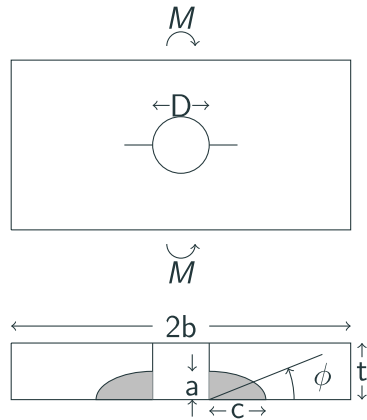$$M_0 = x^{-1/2} + 0.04x^{-3/2} + 0.2x^{-4}v^2 - 0.11x^{-4}v^4$$

corner crack on a lug $a / c > 1$

$$g_1 = 1 + \left(0.1 + \frac{0.35}{x}v^2 \right)(1 - \sin\varphi)^2$$

$$g_3 = \left(1.13 + \frac{0.09}{x} \right)\left(1 + 0.1(1 - \cos\varphi)^2 \right)\left(0.85 + 0.15v^{1/4} \right)$$

$$g_4 = 1$$

symmetric corner cracks under bending

corner cracks under bending

$$\sigma_b = \frac{Mt}{2I}$$

$$I = \frac{bt^3}{6}$$

$$\beta = H_{ch} \left(\frac{a}{cQ} \right)^{1/2} F_{ch}$$

corner cracks under bending

$$H_{ch} = H_1 + (H_2 - H_1)\sin^p\varphi$$

$$H_1 = 1 + G_{11}(a/t) + G_{12}(a/t)^2 + G_{13}(a/t)^3$$

$$H_2 = 1 + G_21(a/t) + G_{22}(a/t)^2 + G_{23}(a/t)^3$$

corner cracks under bending

$$F_{ch} = \left(M_1 + M_2(a/t)^2 + M_3(a/t)^4 \right) g_1 g_2 g_3 g_4 f_\varphi f_w$$

$$\lambda = \frac{1}{1 + (c/r)\cos(0.85\varphi)}$$

$$g_2 = \frac{1 + .358\lambda + 1.425\lambda^2 - 1.578\lambda^3 + 2.156\lambda^4}{1 + 0.13\lambda^2}$$

corner cracks under bending $a/c \leq 1$

$$M_1 = 1.13 - 0.09(a/c)$$

$$M_2 = -0.54 + \frac{0.89}{0.2 + a/c}$$

$$M_3 = 0.5 - \frac{1}{0.65 + a/c} + 14(1 - a/c)^4$$

$$Q = 1 + 1.464(a/c)1.65$$

corner cracks under bending $a/c \leq 1$

$$g_1 = 1 + \left(0.1 + (a/t)v^2 \right)(1 - \sin\varphi)^2$$

$$g_3 = (1 + 0.04(a/c)) \left(1 + 0.1(1 - \cos\varphi)^2 \right) \left(0.85 + 0.15(a/t)^{1/4} \right.$$

$$g_4 = 1 - 0.7(1 - a/t)(a/c - 0.2)(1 - a/c)$$

corner cracks under bending

$$f_\varphi = \left(\left(\frac{a}{c} \cos\varphi \right)^2 + \sin^2\varphi \right)^{1/4}$$

$$G_{11} = -0.43 - 0.74a/c - 0.84(a/c)^2$$

$$G_{12} = 1.25 - 1.19a/c + 4.39(a/c)^2$$

$$G_{13} = -1.94 + 4.22a/c - 5.51(a/c)^2$$

corner cracks under bending

$$G_{21} = -1.5 - 0.04a/c - 1.73(a/c)^2$$

$$G_{22} = 1.71 - 3.17a/c + 6.84(a/c)^2$$

$$G_{23} = -1.28 + 2.71a/c - 5.22(a/c)^2$$

$$p = 0.1 + 1.3a/t + 1.1a/c - 0.7(a/c)(a/t)$$

corner cracks under bending $a/c > 1$

$$M_1 = (c/a)^{1/2}(1 + 0.04c/a)$$

$$M_2 = 0.2(c/a)^4$$

$$M_3 = -0.11(c/a)^4$$

$$Q = 1 + 1.464(c/a)^{1.65}$$

$$g_1 = 1 + \left(0.10.35(c/a)(a/t)^2\right)(1 - \sin\varphi)^2$$

$$g_3 = (1.13 - 0.09(c/a))\left(1 + 0.1(1 - \cos\varphi)^2\right)\left(0.85 + 0.15(a/t)\right)$$

$$g_4 = 1$$

corner cracks under bending

$$f_\varphi = \left(\left(\cos^2\varphi + \frac{c}{a}\sin\varphi \right)^2 \right)^{1/4}$$

$$G_{11} = -2.07 + 0.06c/a$$

$$G_{12} = 4.35 + 0.16c/a$$

$$G_{13} = -2.93 - 0.3c/a$$
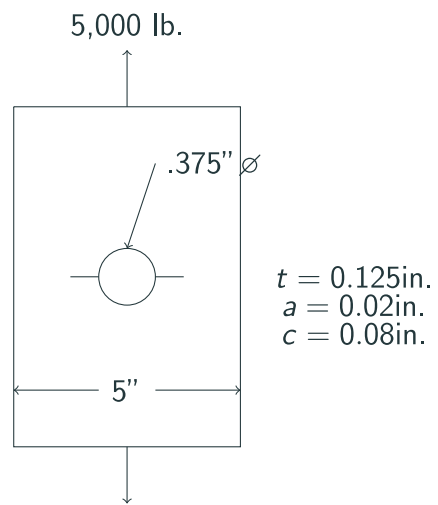
corner cracks under bending

$$G_{21} = -3.64 + 0.37c/a$$

$$G_{22} = 5.87 - 0.49c/a$$

$$G_{23} = -4.32 + 0.53c/a$$

$$p = 0.2 + c/a + 0.6a/t$$

example 3

5,000 lb.

.375" ⌀

$t = 0.125$in.
$a = 0.02$in.
$c = 0.08$in.

5"

example

- Case 1 - symmetric through cracks
- Case 2 - single through crack
- Case 3 - symmetric corner cracks
- Case 4 - single corner crack
- Case 5 - symmetric surface cracks
- Case 6 - single surface crack
- Viewable **here**