AE 760AA: Micromechanics and multiscale modeling

Lecture 2 - Tensor review, Anisotropic Elasticity

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schedule

- Jan 28 Tensor review, Anisotropic Elasticity
- Feb 30 Coordinate Transformation
- Feb 4 1D Micromechanics (HW1 Due)
- Feb 6 Orientation Averaging

2/61

outline

- index notation
- anisotropic elasticity

index notation

index notation

- Consider the following
- $s\hat{a}_{,,} = \hat{a}_{,,} a_1 x_1 \hat{a}_{...} + \hat{a}_{...} a_2 x_2 \hat{a}_{...} + \hat{a}_{...} \hat{a}_{...} + \hat{a}_{...} a_n x_n$
- Which we could also write as

$$s = \sum_{i=1}^n a_i x_i$$

• Using index notation, and EinsteinâTMs summation convention, we can also write this as $s\hat{a}_{,,} = \hat{a}_{,,} a_{i}x_{i}$

dummy index

- In index notation, a repeated index implies summation
- This index is also referred to as a dummy index
- It is called a âcdummy indexâ because the expression would have the same meaning with any index in its place
- i.e. i, j, k, etc. would all have the same meaning when repeated

dummy index

• Note, no index may be repeated more than once, thus the expression

$$s = \sum_{i=1}^n a_i b_i x_i$$

could not be directly written in index notation

free index

- Any index which is not repeated in an index notation expression is referred to as a free index
- The number of free indexes in an expression indicate the tensor order of that expression
- No free indexes = scalar expression (o-order tensor)
- One free index = vector expression (1st-order tensor)
- Two free indexes = matrix expression (2nd-order tensor)

index notation

free index vs. dummy index

- Free index is not repeated (on any term)
- Free index takes all values (1,2,3) Dummy index indicates
- ullet e.g. $u_i = \langle u_1, u_2, u_3 \rangle$
- Free indexes must match across e.g. $\sigma_{ii} = \sigma_{11} + \sigma_{22} + \sigma_{33}$ terms in an expression or equation
- Dummy index is repeated on at least one term
- summation over all values

 - Index can not be used more than twice in the same term ($A_{ij}B_{jk}C_{kl}$ is good, $A_{ij}B_{ij}C_{ij}$ is not)

9/61

dummy index

- The dummy index can be triggered by any repeated index in a *term*.
- Summation or not?
 - \bullet $a_i + b_{ij}c_j$
 - $lacksquare a_{ij}b_{ij}$
 - $lacksquare a_{ij} + b_{ij}c_j$

matrix multiplication

• How can we write matrix multiplication in index notation?

$$\left[egin{array}{ccc} a_{11} & a_{12} \ a_{21} & a_{22} \end{array}
ight] \left[egin{array}{ccc} b_{11} & b_{12} \ b_{21} & b_{22} \end{array}
ight] = \left[egin{array}{ccc} c_{11} & c_{12} \ c_{21} & c_{22} \end{array}
ight]$$

special symbols

kronecker delta

- For convenience we define two symbols in index notation
- Kronecker delta is a general tensor form of the Identity Matrix

$$\delta_{ij} = egin{cases} 1 & ext{if } i=j \ 0 & ext{otherwise} \end{bmatrix} = egin{bmatrix} 1 & 0 & 0 \ 0 & 1 & 0 \ 0 & 0 & 1 \end{bmatrix}$$

• Is also used for higher order tensors

kronecker delta

- ullet $\delta_{ij}=\delta_{ji}$
- ullet $\delta_{ii}=3$
- $\delta_{ij}a_j=a_i$
- $\delta_{ij}b_{ij}=b_{ii}$

alternating symbol

• alternating symbol or permutation symbol

$$\epsilon_{ijk} = \left\{ egin{array}{ll} & ext{if } ijk ext{ is an even permutation of 1,2,3} \ -1 & ext{if } ijk ext{ is an odd permutation of 1,2,3} \ 0 & ext{otherwise} \end{array}
ight.$$

alternating symbol

- This symbol is not used as frequently as the *Kronecker delta*
- For our uses in this course, it is enough to know that 123, 231, and 312 are even permutations
- 321, 132, 213 are odd permutations
- all other indexes are zero
- $\epsilon_{ijk}\epsilon_{imn} = \delta_{jm}\delta_{kn} \delta_{jn}\delta mk$

tensor algebra

substitution

- When solving tensor equations, we often need to manipulate expressions
- We need to make sure the correct indexes are used when substituting, for example

$$a_i = U_{im}b_m \tag{1}$$

$$b_i = V_{im}c_m \tag{2}$$

• To substitute (2) into (1), we first need to change indexes

substitution

- We need to change the free index, i, to m in (2)
- Since *m* is already used as the dummy index, we need to change that too

•

$$b_m = V_{mj}c_j \tag{3}$$

• We can now make the substitution

•

$$a_i = U_{im} V_{mj} c_j \tag{4}$$

multiplication

- We need to be careful with indexes when multiplying expressions
- $p = a_m b_m$ and $q = c_m d_m$
- We can express, pq, but remember the dummy index cannot be repeated more than once
- $ullet pq
 eq a_m b_m c_m d_m$
- Instead we must change the dummy index in one of the expressions first
- $ullet pq = a_m b_m c_n d_n$

factoring

- In the following expression, we would like to factor out *n*, but it has different indexes
- $\sigma_{ij}n_j \lambda n_i = 0$
- Recall $\delta_{ij}a_j=a_i$ we can rewrite $n_i=\delta_{ij}n_j$
- $\sigma_{ij}n_j \lambda \delta_{ij}n_j = 0$
- $(\sigma_{ij} \lambda \delta_{ij})nj = 0$

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21/61

contraction

- σ_{ii} is the contraction of σ_{ij}
- This can often be a useful tool in solving tensor equations
- $\sigma_{ij} = \lambda \Delta \delta_{ij} + 2\mu E_{ij}$
- $\sigma_{ii} = 3\lambda\Delta + 2\mu E_{ii}$

tensor calculus

partial derivative

- We indicate (partial) derivatives using a comma
- In three dimensions, we take the partial derivative with respect to each variable $(x, \hat{a} \dagger y, \hat{a} \dagger z \text{ or } x_1, x_2, \text{ and } x_3)$
- For example a scalar property, such as density, can have a different value at any point in space
- $\bullet \ \rho = \rho(x_1,x_2,x_3)$
- $ullet
 ho_{,i} = rac{\partial}{\partial x_i}
 ho = \left\langle rac{\partial
 ho}{\partial x_1}, rac{\partial
 ho}{\partial x_2}, rac{\partial
 ho}{\partial x_3}
 ight
 angle$

partial derivative

• Similarly, if we take the partial derivative of a vector, it produces a matrix

$$u_{i,j} = rac{\partial}{\partial x_j} u_i = egin{bmatrix} rac{\partial u_1}{\partial x_1} & rac{\partial u_1}{\partial x_2} & rac{\partial u_1}{\partial x_3} \ rac{\partial u_2}{\partial x_1} & rac{\partial u_2}{\partial x_2} & rac{\partial u_2}{\partial x_3} \ rac{\partial u_3}{\partial x_1} & rac{\partial u_3}{\partial x_2} & rac{\partial u_3}{\partial x_3} \end{bmatrix}$$

dyadic notation

dyadic notation

- Dyadic notation is sometimes called tensor product notation
- Dyadic product: $C_{ij} = a_i b_j$ is written as $C = a \otimes b$
- Double dot product: $A_{ij}B_{ji}=c$ is written as $A\hat{\mathsf{a}}_{,,}:\hat{\mathsf{a}}_{,,}B\hat{\mathsf{a}}_{,,}=\hat{\mathsf{a}}_{,,}c$

transformation

linear transformation

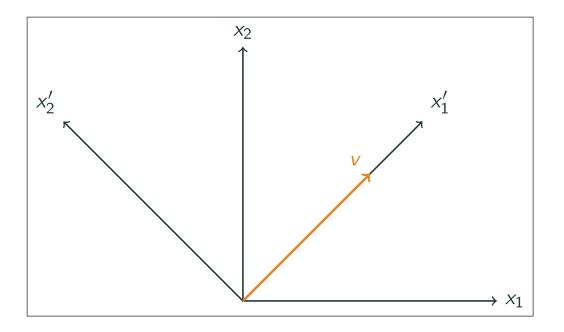
- Let us consider some transformation, **T**, which transforms any vector into another vector
- If we transform T a = c and T b = d
- We call **T** a linear transformation (and a tensor) if

$$T(a+b) = Ta + Tb$$

$$T(\alpha a) = \alpha Ta$$

• Where α is any arbitrary scalar and a, b are arbitrary vectors

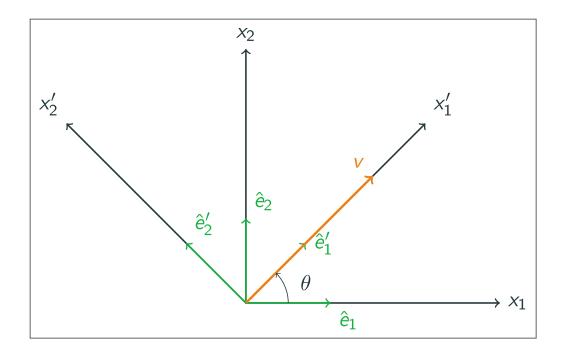
coordinate transformation in two dimensions



coordinate transformation in two dimensions

- The vector, v, remains fixed, but we transform our coordinate system
- In the new coordinate system, the x_2' portion of v is zero.
- To transform the coordinate system, we first define some unit vectors.
- \hat{e}_1 is a unit vector in the direction of x_1 , while \hat{e}'_1 is a unit vector in the direction of x'_1

coordinate transformation in two dimensions



coordinate transformation in two dimensions

- For this example, let us assume $v\hat{a}_{,,}=\hat{a}_{,,}\langle 2,\hat{a}^{\dagger}2\rangle$ and $\theta\hat{a}_{,,}=\hat{a}_{,,}45^{\circ}$
- We can write the transformed unit vectors, \hat{e}'_1 and \hat{e}'_2 in terms of \hat{e}_1 , \hat{e}_2 and the angle of rotation, θ .

$$\hat{e}_1' = \langle \hat{e}_1 \cos \theta, \hat{e}_2 \sin \theta \rangle$$
 $\hat{e}_2' = \langle -\hat{e}_1 \sin \theta, \hat{e}_2 \cos \theta \rangle$

coordinate transformation in two dimensions

- We can write the vector, v, in terms of the unit vectors describing our axis system
- $v = v_1 \hat{e}_1 + v_2 \hat{e}_2$
- (note: $\hat{e}_1 = \langle 1, 0 \rangle$ and $\hat{e}_2 = \langle 0, 1 \rangle$)
- $v = \langle 2, 2 \rangle = 2\langle 1, 0 \rangle + 2\langle 0, 1 \rangle$

coordinate transformation in two dimensions

- When expressed in the transformed coordinate system, we refer to v'
- $ullet v' = \langle v_1 \cos heta + v_2 \sin heta, -v_1 \sin heta + v_2 \cos heta
 ule{}$
- $v'=\langle 2\sqrt{2},0\rangle$
- We can recover the original vector from the transformed coordinates:
- $\bullet \ v = v_1' \hat{e}_1' + v_2' \hat{e}_2'$
- (note: $\hat{e}_1' = \langle \frac{\sqrt{2}}{2}, \frac{\sqrt{2}}{2} \rangle$ and $\hat{e}_2' = \langle -\frac{\sqrt{2}}{2}, \frac{\sqrt{2}}{2} \rangle$)
- $ullet v=2\sqrt{2}\langle rac{\sqrt{2}}{2},rac{\sqrt{2}}{2}
 angle, 0\langle -rac{\sqrt{2}}{2},rac{\sqrt{2}}{2}
 angle=\langle 2,2
 angle$

general coordinate transformation

- Coordinate transformation can become much more complicated in three dimensions, and with higher-order tensors
- It is convenient to define a general form of the coordinate transformation in index notation
- We define Q_{ij} as the cosine of the angle between the x_i' axis and the x_j axis.
- ullet This is also referred to as the "direction cosine" $Q_{ij} = \cos(x_i',x_j)$

mental and emotional health warning

- Different textbooks flip the definition of Q_{ij} (Elasticity and Continuum texts have opposite definitions, for example)
- The result gives the transpose
- Always use equations (next slide) from the same source as your Q_{ij} definition

general coordinate transformation

- We can transform any-order tensor using Q_{ij}
- Vectors (first-order tensors): $v'_i = Q_{ij}v_j$
- Matrices (second-order tensors): $\sigma'_{ij} = Q_{im}Q_{jn}\sigma_{mn}$
- ullet Fourth-order tensors: $C'_{ijkl}=Q_{im}Q_{jn}Q_{ko}Q_{lp}C_{mnop}$

general coordinate transformation

• We can use this form on our 2D transformation example

$$egin{aligned} Q_{ij} &= \cos(x_i', x_j) \ &= egin{bmatrix} \cos(x_1', x_1) & \cos(x_1', x_2) \ \cos(x_2', x_1) & \cos(x_2', x_2) \end{bmatrix} \ &= egin{bmatrix} \cos heta & \cos(90 - heta) \ \cos(90 + heta) & \cos heta \end{bmatrix} \ &= egin{bmatrix} \cos heta & \sin heta \ -\sin heta & \cos heta \end{bmatrix} \end{aligned}$$

general coordinate transformation

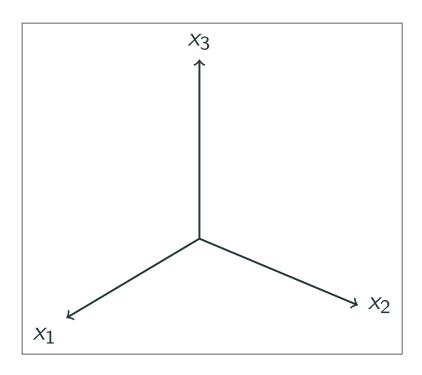
- We can similarly use Q_{ij} to find tensors in the original coordinate system
- Vectors (first-order tensors): $v_j = Q_{ji}v_i'$
- Matrices (second-order tensors): $\sigma_{mn} = Q_{mi}Q_{nj}\sigma'_{ij}$
- Fourth-order tensors: $C_{mnop} = Q_{mi}Q_{nj}Q_{ok}Q_{pl}C'_{ijkl}$

general coordinate transformation

- We can derive some interesting properties of the transformation tensor, Q_{ij}
- We know that $v_i' = Q_{ij}v_j$ and that $v_j = Q_{ji}v_i'$
- If we substitute (changing the appropriate indexes) we find:
- $v_j = Q_{ji}Q_{ik}v_k$
- We can now use the Kronecker Delta to substitute $v_j = \delta_{jk} v_k$
- $\bullet \ \delta_{jk}v_k=Q_{ji}Q_{ik}v_k$

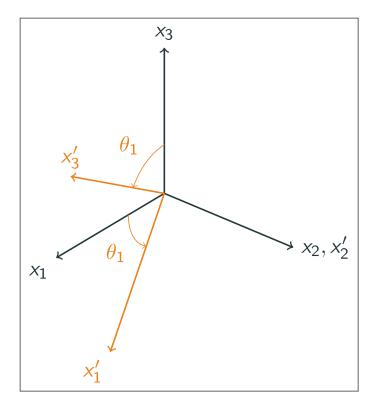
examples

example

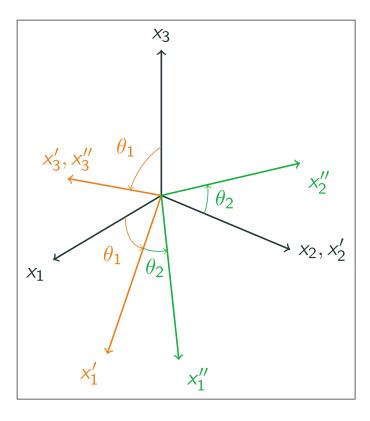


- Find Q_{ij}^1 for rotation of 60° about x_2
- Find Q_{ij}^2 for rotation of 30° about x_3'
- Find e_i'' after both rotations

example



example



example

$$ullet Q_{ij}^1 = \cos(x_i',x_j) \ ullet Q_{ij}^2 = \cos(x_i'',x_j') \ Q_{ij}^1 = egin{bmatrix} \cos 60 & \cos 90 & \cos 150 \ \cos 90 & \cos 0 & \cos 90 \ \cos 30 & \cos 90 & \cos 60 \end{bmatrix} \ Q_{ij}^2 = egin{bmatrix} \cos 30 & \cos 60 & \cos 90 \ \cos 30 & \cos 60 & \cos 90 \ \cos 120 & \cos 30 & \cos 90 \ \cos 90 & \cos 90 & \cos 90 \end{bmatrix} \ \end{pmatrix}$$

example

- We now use Q_{ij} to find \hat{e}'_i and \hat{e}''_i
- First, we need to write \hat{e}_i in a manner more consistent with index notation
- We will indicate axis direction with a superscript, e.g. $\hat{e}_1 = e_i^1$
- $ullet e_i' = Q_{ij}^1 e_j$
- $ullet e_i''=Q_{ij}^2e_j'$
- How do we find e_i'' in terms of e_i ?

anisotropic elasticity

stiffness

- In 3D, Hookeâ $^{ ext{TM}}$ s Law for linearly elastic materials is $\sigma_{ij} = C_{ijkl}\epsilon_{kl}$
- For isotropic materials, C_{ijkl} can be expressed in terms of two constants
- In general (anisotropic materials) more constants are needed and we use the full tensor

engineering notation

- Fourth-order tensors are cumbersome to write, we often use engineering notation
- σ and ϵ are written as vectors and C_{ijkl} is written as a matrix.
- NOTE: Although σ , ϵ and C_{ijkl} are tensors, their counterparts in engineering notation are NOT formal tensors
- This means that the usual transformation laws do not apply

engineering notation

$\lceil \sigma_{11} \rceil$		lacksquare	C_{1122}	C_{1133}	C_{1123}	C_{1113}	C_{1112}	$\lceil E_{11} \rceil$
$\mid \sigma_{22} \mid$		C_{1122}	C_{2222}	C_{2233}	C_{2223}	C_{1322}	C_{1222}	$\mid E_{22} \mid$
$\mid \sigma_{33} \mid$	_	C_{1133}	C_{2233}	C_{3333}	C_{2333}	C_{1333}	C_{1233}	$\mid E_{33} \mid$
$\mid \sigma_{23} \mid$		C_{1123}	C_{2223}	C_{2333}	C_{2323}	C_{1323}	$egin{array}{c} C_{1233} \ C_{1223} \end{array}$	$2E_{23}$
$\mid \sigma_{13} \mid$		C_{1113}	C_{1322}	C_{1333}	C_{1323}	C_{1313}	C_{1213}	$2E_{13}$
$\lfloor \sigma_{12} \rfloor$							C_{1212}	$\lfloor 2E_{12} floor$

51/61

compliance

$\lceil E_{11} \rceil$		$\lceil S_{1111} ceil$	S_{1122}	S_{1133}	S_{1123}	S_{1113}	S_{1112}	$\lceil \sigma_{11} ceil$
$oxed{E_{22}}$		S_{1122}	S_{2222}	S_{2233}	S_{2223}	S_{1322}	S_{1222}	σ_{22}
E_{33}		S_{1133}	S_{2233}	S_{3333}	S_{2333}	S_{1333}	S_{1233}	σ_{33}
$2E_{23}$	_	S_{1123}	S_{2223}	S_{2333}	S_{2323}	S_{1323}	S_{1223}	σ_{23}
$\mid 2E_{13} \mid$		S_{1113}	S_{1322}	S_{1333}	S_{1323}	S_{1313}	S_{1213}	σ_{13}
$\lfloor 2E_{12} floor$		$igs S_{1112}$				S_{1213}		$\lfloor\sigma_{12} floor$

physical interpretation

• If we now consider the case of uniaxial tension, we see that

$$E_{11} = S_{1111}\sigma_{11} \ E_{22} = S_{1122}\sigma_{11} \ E_{33} = S_{1133}\sigma_{11} \ 2E_{23} = S_{1123}\sigma_{11} \ 2E_{13} = S_{1113}\sigma_{11} \ 2E_{12} = S_{1112}\sigma_{11}$$

• S_{1111} is familiar, acting like $1/E_Y$

poisson's ratio

- ullet For isotropic materials we defined Poisson's ratio as $u=-E_{22}/E_{11}$
- For anisotropic materials, we can have a different Poisson's ratio acting in different directions
- We define $\nu_{ij} = -E_{jj}/E_{ii}$ (with no summation), the ratio of the transverse strain in the j direction when stress is applied in the i direction
- For this example we can find ν_{12} and ν_{13} as

$$u_{12} = -E_{22}/E_{11} = -S_{1122}/S_{1111}$$
 $u_{13} = -E_{33}/E_{11} = -S_{1133}/S_{1111}$

poisson's ratio

- Note that we cannot, in general, say that $\nu_{12}=\nu_{21}$
- However, due to the symmetry of the stiffness/compliance tensors, we know that

$$egin{aligned}
u_{21}E_x &=
u_{12}E_y \
u_{31}E_x &=
u_{13}E_z \
u_{32}E_y &=
u_{23}E_z \end{aligned}$$

• Where E_x referâTMs to the YoungâTMs Modulus in the *x*-direction, etc.

shear coupling coefficients

- An unfamiliar effect is that shear strains can be introduced from a normal stress
- We define shear coupling coefficients as $\eta_{1112}=\eta_{16}=-2E_{12}/E_{11}$ due to σ_{11}
- These coupling terms can also effect shear strain in a different plane from the applied shear stress
- Like the Poisson's ratio, these are not entirely independent $\eta_{61}E_x=\eta_{16}G_6$
- Where G_6 is the shear modulus in the 12 plane

shear coupling coefficients

- Shear coupling coefficients are sometimes placed in two groups
- Coefficients of mutual influence relate shear stress to normal strain and normal stress to shear strain
- Chentsov coefficients relate shear stress in one plane to shear strain in another plane
- In general we can say

$$egin{aligned} \eta_{nm}E_m &= \eta_{mn}G_n & \quad & (\mathrm{m}=1,2,3) \; (\mathrm{n}=4,5,6) \ & ext{and} \ &\eta_{nm}G_m &= \eta_{mn}G_n & \quad & (\mathrm{m},\mathrm{n}=4,5,6) & \quad & m
eq n \end{aligned}$$

orthotropic symmetry

$\lceil \sigma_{11} \rceil$		$igcap C_{1111}$	C_{1122}	C_{1133}	0	0	0]	$\lceil E_{11} ceil$
σ_{22}		C_{1122}	C_{2222}	C_{2233}	0	0	0	E_{22}
σ_{33}		C_{1133}	C_{2233}	C_{3333}	0	0	0	E_{33}
σ_{23}	_	0	0	0	C_{2323}	0	0	$2E_{23}$
σ_{13}		0	0	0	0	C_{1313}	0	$2E_{13}$
$oxedsymbol{oldsymbol{\sigma}}_{12}$		0	0	0	0	0	C_{1212}	$2E_{12}$

transversely isotropic symmetry

$\lceil \sigma_{11} \rceil$		$lacksquare$ C_{1111}	C_{1122}	C_{1133}	0	0	0	$ig \lceil E_{11} ig ceil$
$\mid \sigma_{22} \mid$	1	C_{1122}	C_{1111}	C_{1133}	0	0	0	E_{22}
σ_{33}		C_{1133}	C_{1133}	C_{3333}	0	0	0	E_{33}
$\mid \sigma_{23} \mid$	_	0	0	0	C_{1313}	0	0	$2E_{23}$
σ_{13}		0	0	0	0	C_{1313}	0	$2E_{13}$
$\lfloor \sigma_{12} \rfloor$		0	0	0	0	0	$1/2(C_{1111}-C_{2222})$	$ig\lfloor 2E_{12}ig floor$

isotropic symmetry

$$\begin{bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{33} \\ \sigma_{23} \\ \sigma_{13} \\ \sigma_{12} \end{bmatrix} = \frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1-\nu & \nu & \nu & 0 & 0 & 0 \\ \nu & 1-\nu & \nu & 0 & 0 & 0 \\ \nu & \nu & 1-\nu & 0 & 0 & 0 \\ 0 & 0 & 0 & \frac{1}{2}(1-2\nu) & 0 & 0 \\ 0 & 0 & 0 & 0 & \frac{1}{2}(1-2\nu) & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{1}{2}(1-2\nu) \end{bmatrix} \begin{bmatrix} E_{11} \\ E_{22} \\ E_{33} \\ 2E_{23} \\ 2E_{13} \\ 2E_{12} \end{bmatrix}$$

next class

• Next class we will develop transformation laws for engineering stress/strain and stiffness