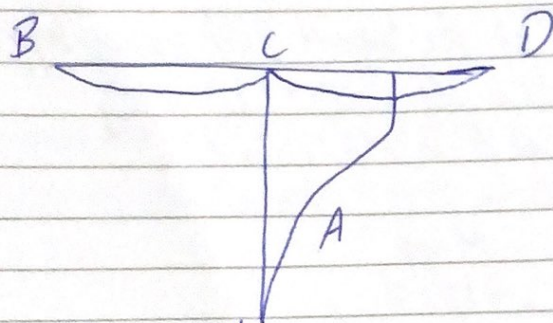


~~8/10~~

Assignment ①

a) the deflected shape of the given frame for $\alpha = 1$



b) for given $M = 20 \text{ kNm}$ and $\alpha = 1$

By applying equilibrium conditions

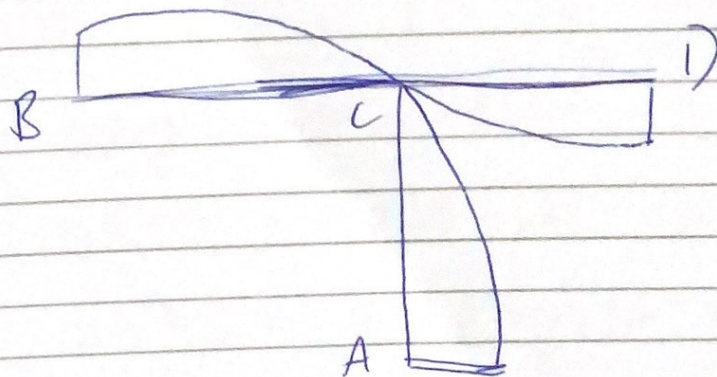
total horizontal forces $H = 0$, total vertical forces $V = 0$
and total bending moment acting in the given frame is
 $M = 0$

that is $M_A + M_D = 20 \text{ kNm}$ (applied moment)

Since $\alpha = 1$ the given frame is symmetrical therefore

$$M_A = M_D = 20/2 = 10 \text{ kNm}$$

therefore, bending moment diagram will be ..



Bending moment Diagram (BMD)

c) for any value of moment in terms of α will be applying equilibrium equations,

taking moment at the support B at roller support

moment at D is $M_D = V_D * (L + \alpha L)$

and moment at A is $M_A = V_A * L$

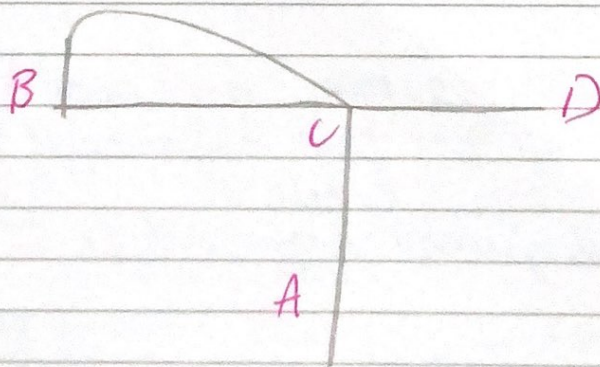
$$M_A + M_D = M_{app}$$

since $V_A = V_D = V$,

$$V(L + \alpha L) + VL = VL(2 + \alpha) = M_{app}$$

$$\text{therefore } M_{app} = VL(2 + \alpha) = \cancel{M_{app}}$$

$M_{applied}$



σ^2 ,

[missing Info the width of the beam]

Yield M_c/I or M/S

M = maximum moment in the beam

c = distance from neutral axis to the extreme fiber ($h/2$)

I = is the moment of inertia

S = elastic section modulus

~~M can be calculated~~

$$PL/4 + wL^2/8 \Rightarrow M = PL/4 + \gamma(9.81) b h L^2/8$$

$$M = 0.25 PL + 1.22625 \gamma b h L^2$$

M_c/I

$$M = 0.25 PL + 1.22625 \gamma b h L^2$$

$$c = h/2$$

$$I = b h^3/12$$

The resulting equation will be:-

$$\sigma = (0.25 PL + 1.2265 \gamma b h L^2) (h/2) (b h^3/12)$$

$$\sigma = (0.25 PL + 1.22625 \gamma h b L^2) (6/b h^2)$$