

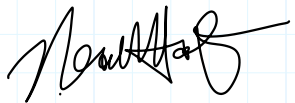
CIVE 3205

Steel 1

Partial Solution

BCPS-10

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Revisions:

- Mar 27/20: original posting

W310 x 158:

ASTM A992 steel (Table 6-8)

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$$A = 20100 \text{ mm}^2$$

$$I_x = 386 \times 10^6 \text{ mm}^4$$

$$r_x = 139$$

$$Z_x = 2670 \times 10^3 \text{ mm}^3$$

$$I_y = 125 \times 10^6 \text{ mm}^4$$

$$r_y = 78.9$$

$$J = 3770 \times 10^3 \text{ mm}^4$$

$$C_w = 2840 \times 10^9 \text{ mm}^6$$

$$d = 327 \text{ mm}$$

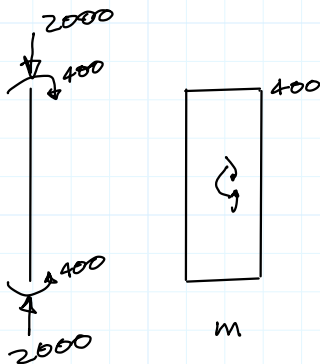
$$b = 310 \text{ mm}$$

$$t = 25.1 \text{ mm}$$

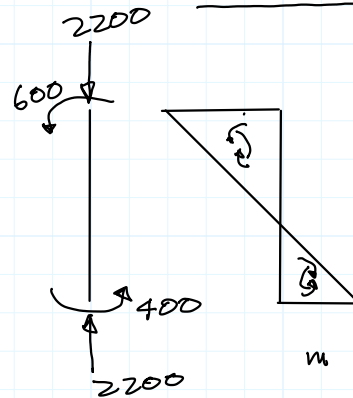
$$w = 15.5 \text{ mm}$$

$$d - 2t = 277 \text{ mm}$$

Case 1



Case 2



Section Class (See Table 4-7, p4-105)

$$\phi C_y = 6241$$

$$C_f / \phi C_y = 0.3205$$

flange - class 1
web - class 1

class 1

$$C_f / \phi C_y = 0.3525$$

flange - class 1
web - class 1

class 1

a) cross section

$$C_r = 6241 \text{ kN}$$

$$M_r = 829.0 \text{ kN-m}$$

$$\alpha = -\frac{400}{400} = -1$$

$$w_1 = 1.0$$

$$C_e = \frac{\pi^2 \times 200000 \times 386 \times 10^6}{4600^2}$$

$$C_e = 36010 \text{ kN}$$

$$C_r = 6241 \text{ kN}$$

$$M_r = 829.0 \text{ kN-m}$$

$$\alpha = +\frac{400}{600} = +0.667$$

$$w_1 = 0.333 \geq 0.4$$

$$w_1 = 0.4$$

$$C_e = 36010 \text{ kN}$$

$$U_{ix} = \frac{1.0}{1 - \frac{2000}{36010}} = 1.059$$

$$\text{use } U_{ix} = 1.059$$

$$\frac{2000}{6241} + \frac{0.85 \times 1.059 \times 400}{829.0} = 0.7548 < 1 \quad \text{OK}$$

$$U_{ix} = \frac{0.4}{1 - \frac{2200}{36010}} = 0.4260 \quad \frac{3}{5}$$

$$\text{use } U_{ix} = 1.0$$

$$\frac{2200}{6241} + \frac{0.85 \times 1 \times 600}{829.0} = 0.9677 < 1 \quad \text{OK.}$$

b) overall member strength

for C_r , use $k=1$, $L=4600$
 $r=r_y=78.9$

$$C_r = 4604 \text{ kN}$$

$$M_r = 829.0 \text{ kN-m}$$

$$U_{ix} = 1.059$$

$$\frac{2000}{4604} + \frac{0.85 \times 1.059 \times 400}{829.0} = 0.8687 < 1 \quad \text{OK}$$

$$C_r = 4604 \text{ kN}$$

$$M_r = 829.0 \text{ kN-m}$$

$$U_{ix} = 0.4260$$

$$\frac{2200}{4604} + \frac{0.85 \times 0.4260 \times 600}{829.0} = 0.7399 < 1 \quad \text{OK}$$

c) lateral torsional buckling strength

for C_r use weak axis.
 $k=1$ because pinned beams frame in.

$$\therefore C_r = 4604 \text{ kN}$$

for M_r : (§ 13.6)

$$\alpha_x = -1$$

$$w_2 = 1.0$$

$$L = 4600$$

$$M_{rx} = 829.0$$

$$U_{ix} = 1.059$$

$$\frac{2000}{4604} + \frac{0.85 \times 1.059 \times 400}{829} = 0.87 < 1 \quad \text{OK}$$

$$C_r = 4604 \text{ kN}$$

$$\alpha_x = +0.6667$$

$$w_2 = 2.58 > 2.5$$

$$\text{use } w_2 = 2.5$$

$$L = 4600$$

$$M_{rx} = 829.0$$

$$U_{ix} = 1.0$$

$$\frac{2200}{4604} + \frac{0.85 \times 1.0 \times 600}{829} = 1.09 \not< 1 \quad \text{NG}$$

also: $\frac{M_{fx}}{M_{rx}} \leq 1$
 $\frac{400}{829} \leq 1$ OK

$\frac{600}{829} < 1$ OK

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W310x158 is not adequate
w.r.t. lateral torsional buckling
in load case 2

