

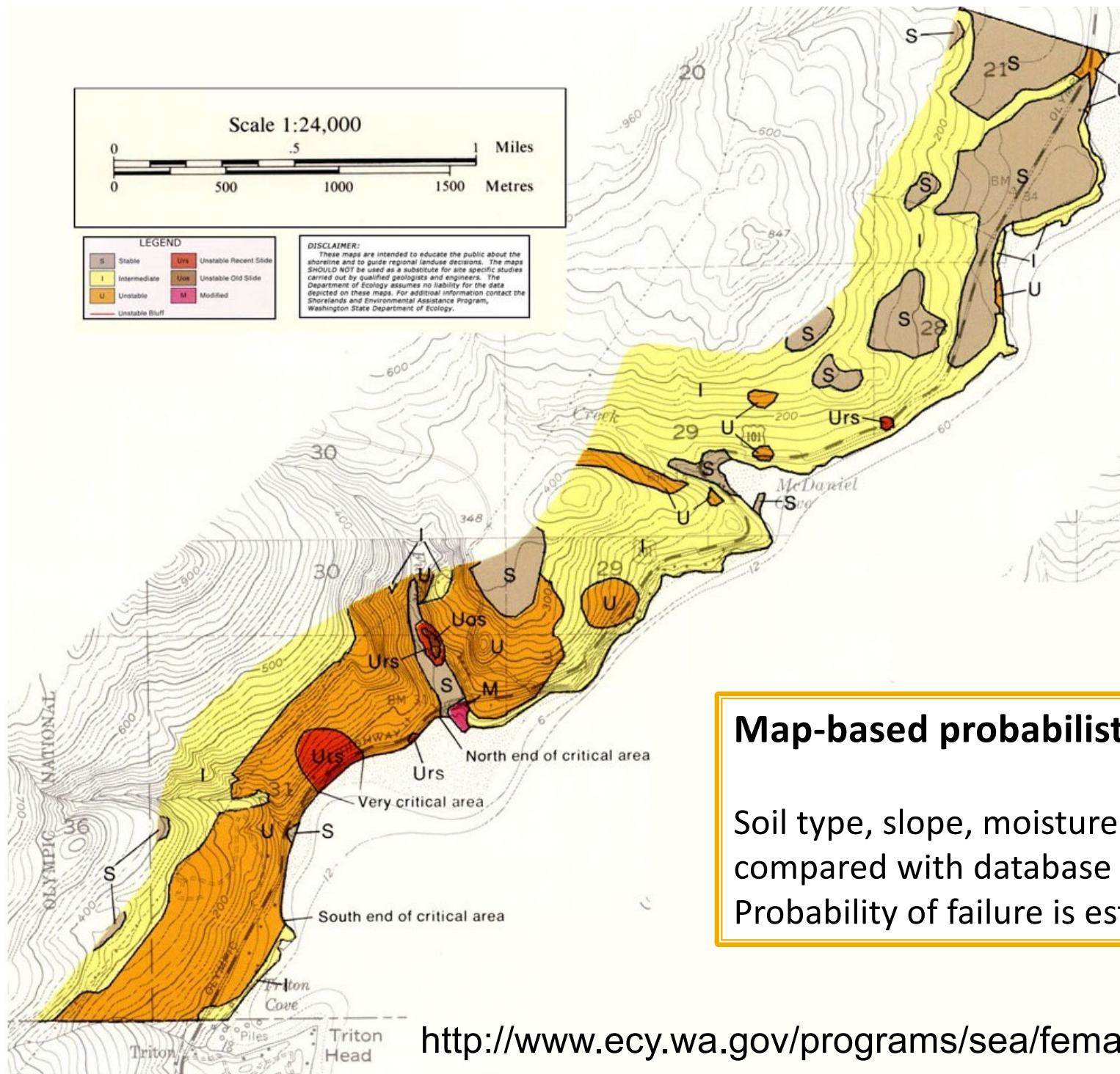
CE 544 – BRIGHAM YOUNG UNIVERSITY

Limit Equilibrium Procedures

Part 1

Slope Stability Analysis Methods

- Map-based probabilistic analysis
- Deformation analysis
- Limit equilibrium analysis



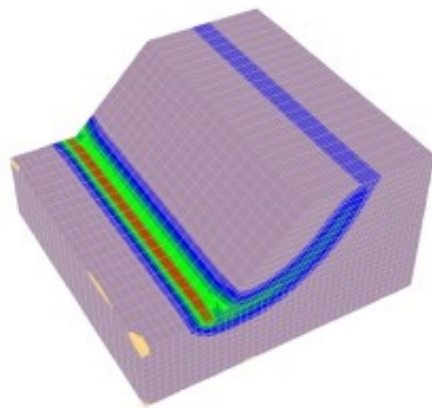
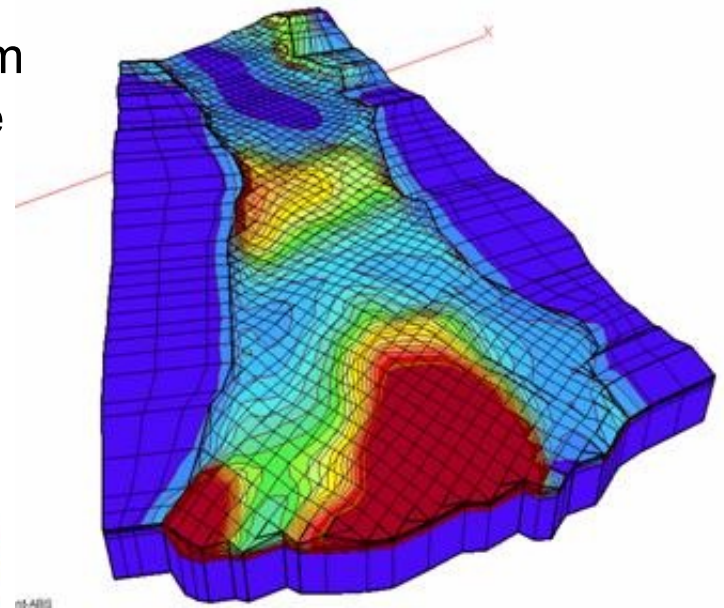
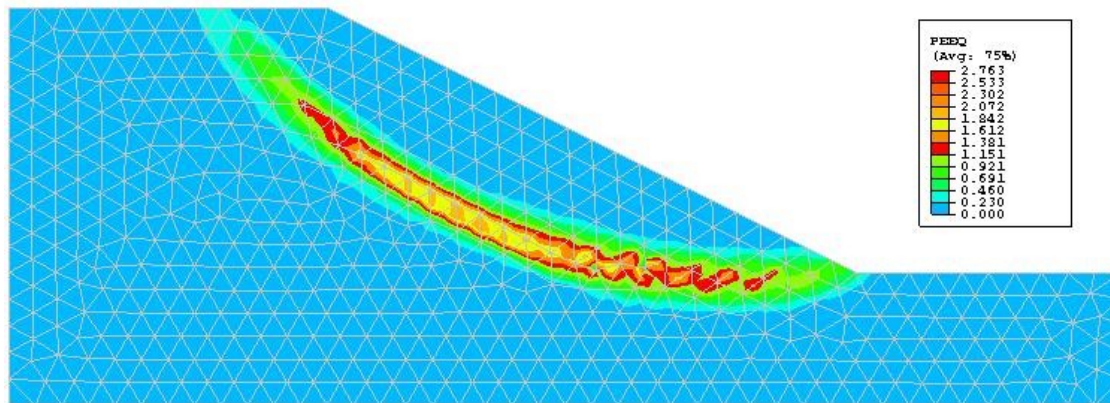
Map-based probabilistic analysis

Soil type, slope, moisture content etc.
compared with database of past failures.
Probability of failure is estimated

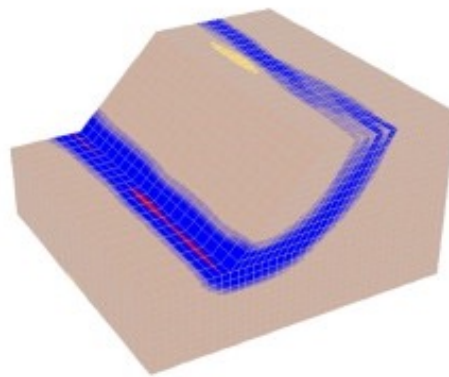
<http://www.ecy.wa.gov/programs/sea/femaweb/jefferson.htm>

Deformation Analysis

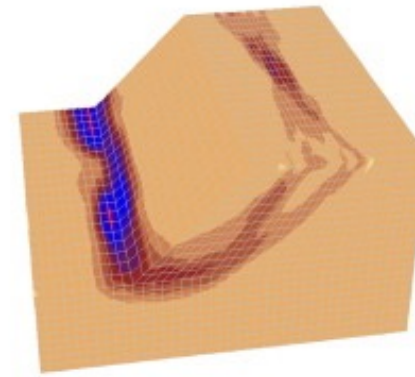
Stress-strain relations, peak-residual strength, anisotropy and other factors are combined to form a model for plastic deformation. Solved with finite element technique



(a) homogeneous, FOS=1.17



(b) $m=3$, FOS=1.14



(c) $m=1$, FOS=1.04

Limit Equilibrium Method

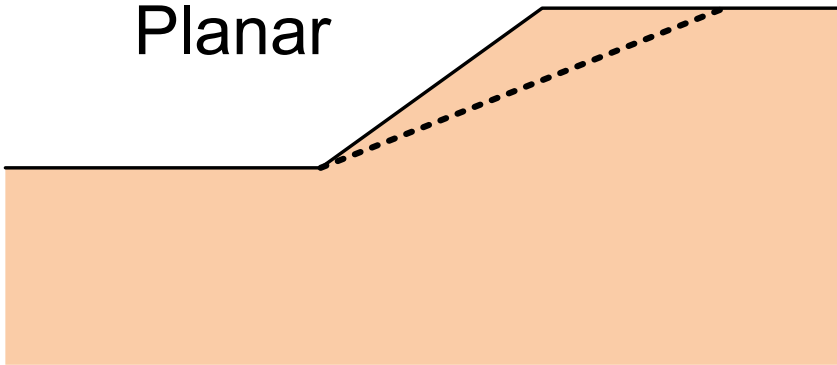
- Candidate failure surface is selected
- Stresses along failure surface are calculated assuming soil is at limit of static equilibrium (τ)
- Total available strength along surface is calculated (s)
- Factor of safety is computed as:

$$FS = \frac{s}{\tau}$$

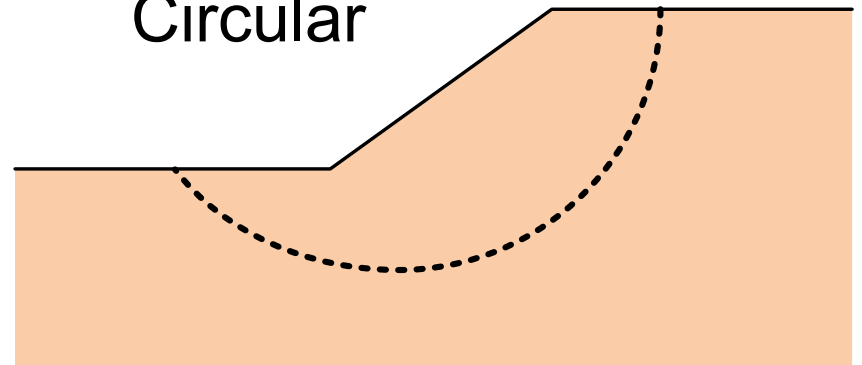
- Process is repeated until critical failure surface with minimum factor of safety is found

Failure Surfaces

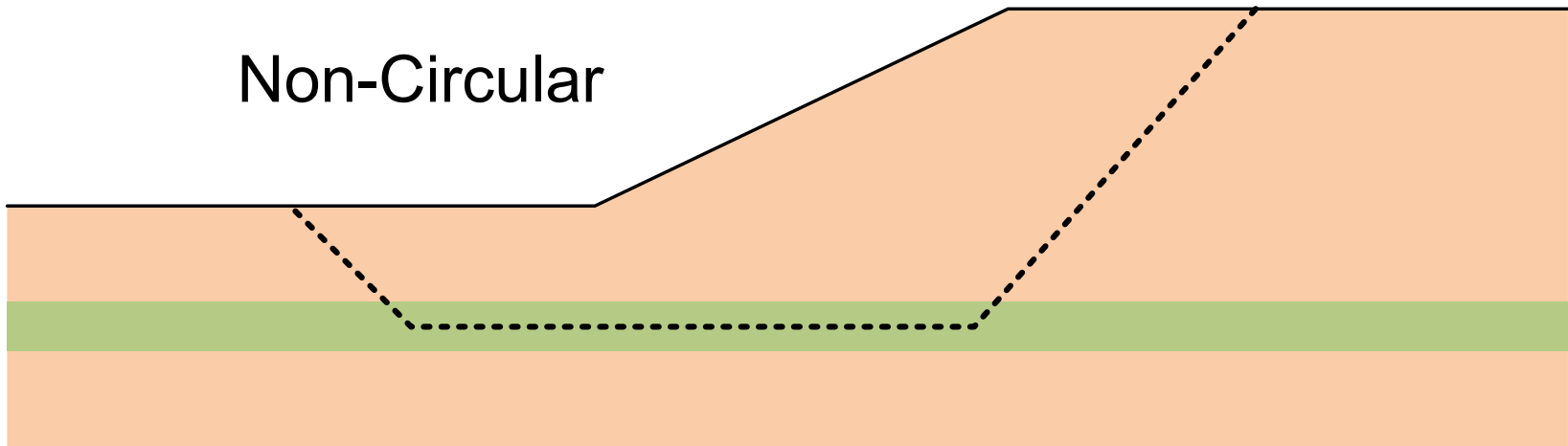
Planar



Circular



Non-Circular



Factor of Safety

$$FS = \frac{s}{\tau}$$

$$\tau = \frac{s}{FS}$$

$$\tau = \frac{c + \sigma \tan \phi}{FS}$$

$$\tau = \frac{c}{FS} + \frac{\sigma \tan \phi}{FS}$$

$$c_d = \frac{c}{FS}$$

$$\tan \phi_d = \frac{\tan \phi}{FS}$$

c_d = developed or mobilized cohesion

$\tan \phi_d$ = developed or mobilized friction

Equilibrium Conditions

- Static equilibrium must be satisfied
- Three conditions

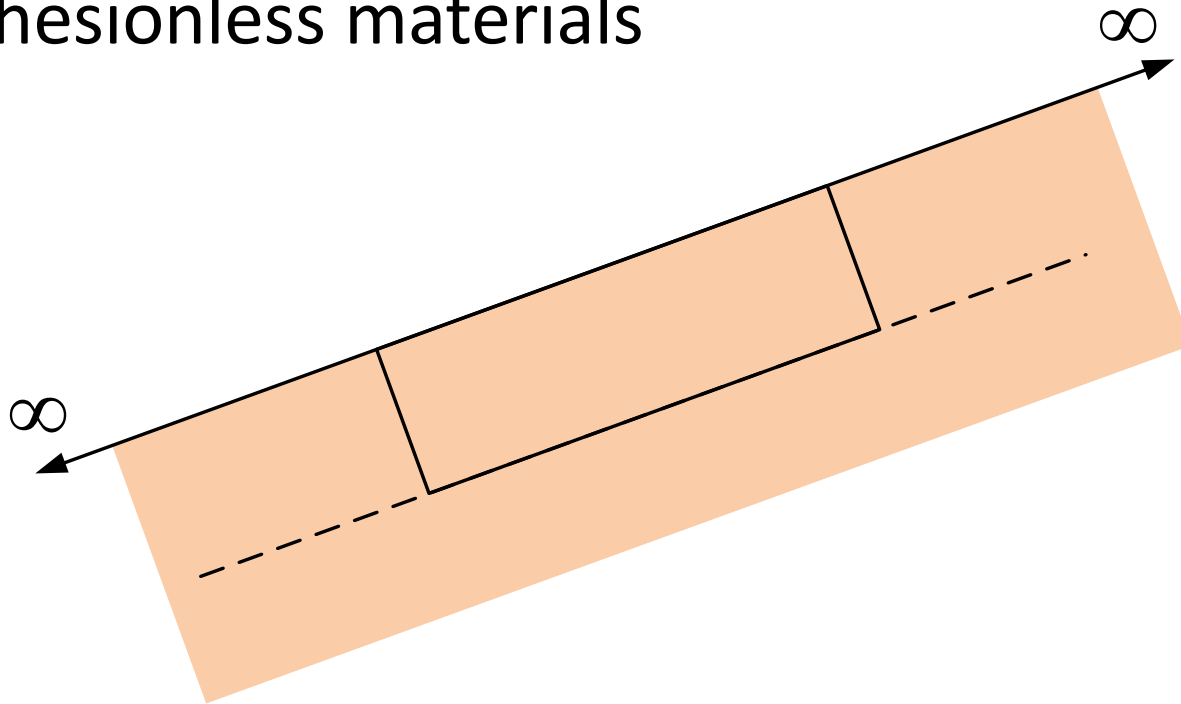
$$\Sigma F_x = 0 \quad \Sigma F_y = 0 \quad \Sigma M = 0$$

- Typically, # equations < # unknowns → simplifying assumptions must be used
- Some techniques satisfy all three conditions, some satisfy just a subset

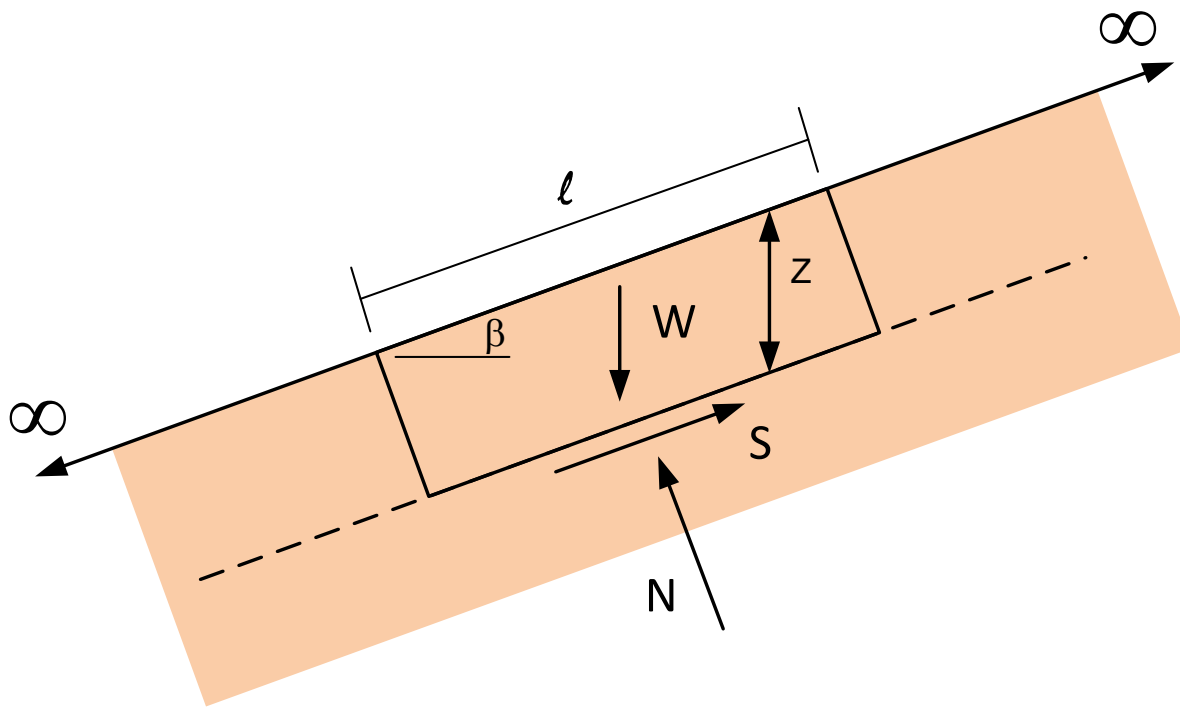
Infinite Slope Analysis

Infinite Slope Analysis

- Can be used as a reasonable approximation for cases with:
 - Shallow firm strata
 - Cohesionless materials



Infinite Slope Analysis



$$S = W \sin \beta$$

$$N = W \cos \beta$$

$$W = \gamma \ell z \cos \beta$$

$$S = \gamma \ell z \cos \beta \sin \beta$$

$$N = \gamma \ell z \cos^2 \beta$$

Infinite Slope Analysis

$$\tau = \frac{S}{\ell} = \frac{\gamma \ell z \cos \beta \sin \beta}{\ell} = \gamma z \cos \beta \sin \beta$$

$$\sigma = \frac{N}{\ell} = \frac{\gamma \ell z \cos^2 \beta}{\ell} = \gamma z \cos^2 \beta$$

$$F = \frac{S}{\tau}$$

$$F = \frac{c + \sigma \tan \phi}{\tau}$$

Infinite Slope Analysis

$$F = \frac{c + \gamma z \cos^2 \beta \tan \varphi}{\gamma z \cos \beta \sin \beta}$$

Total Stress Analysis

$$F = \frac{c' + (\gamma z \cos^2 \beta - u) \tan \varphi'}{\gamma z \cos \beta \sin \beta}$$

Effective Stress Analysis

Infinite Slope Analysis

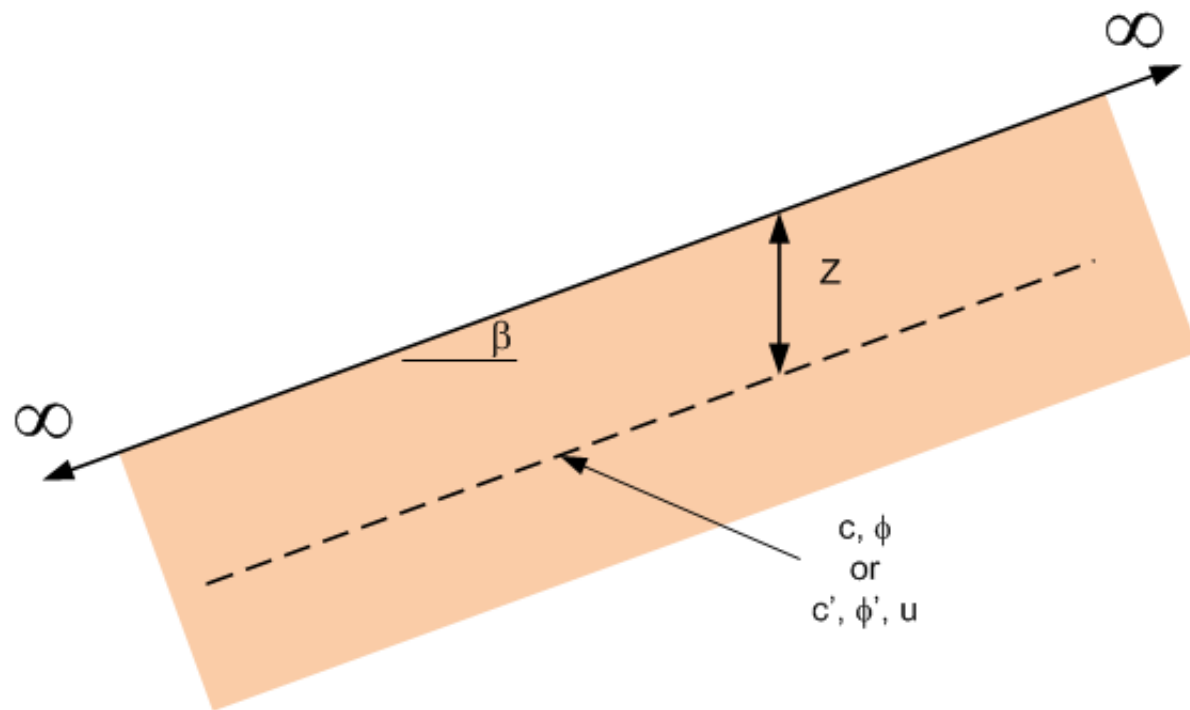
For $c=0$, $u=0$:

$$F = \frac{\gamma z \cos^2 \beta \tan \varphi}{\gamma z \cos \beta \sin \beta}$$

$$F = \frac{\cos \beta \tan \varphi}{\sin \beta}$$

$$F = \frac{\tan \varphi}{\tan \beta}$$

Infinite Slope Analysis



Total Stress Analysis

- Non-submerged slopes

$$FS = \frac{c}{\gamma z \sin(2\beta)} + \frac{\tan\phi}{\tan\beta}$$

- Submerged slopes

$$FS = \frac{c}{\gamma' z \sin(2\beta)}$$

$$c = S_u, \phi = 0$$

Effective stress analysis

- General case

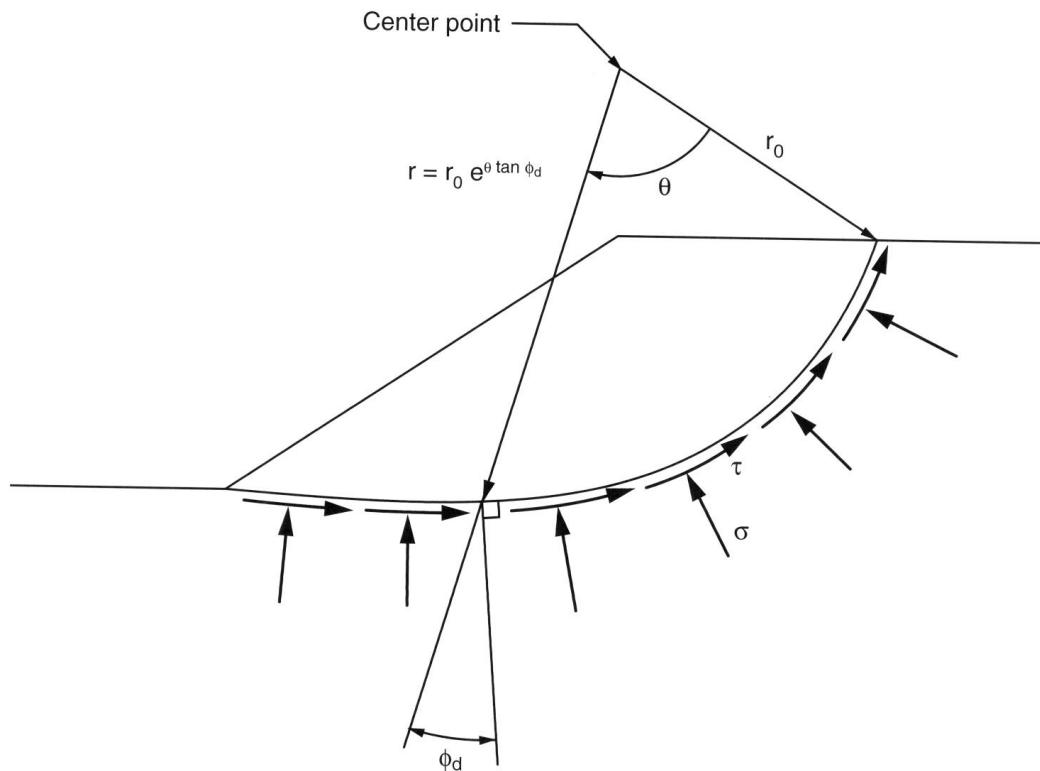
$$FS = \frac{(\bar{c} - u \tan \bar{\phi})}{\gamma z} \frac{2}{\sin(2\beta)} + \frac{\tan \bar{\phi}}{\tan \beta}$$

- Submerged slopes

$$FS = \frac{\bar{c}}{\gamma' z} \frac{2}{\sin(2\beta)} + \frac{\tan \bar{\phi}}{\tan \beta}$$

Log Spiral Technique

Log Spiral Technique



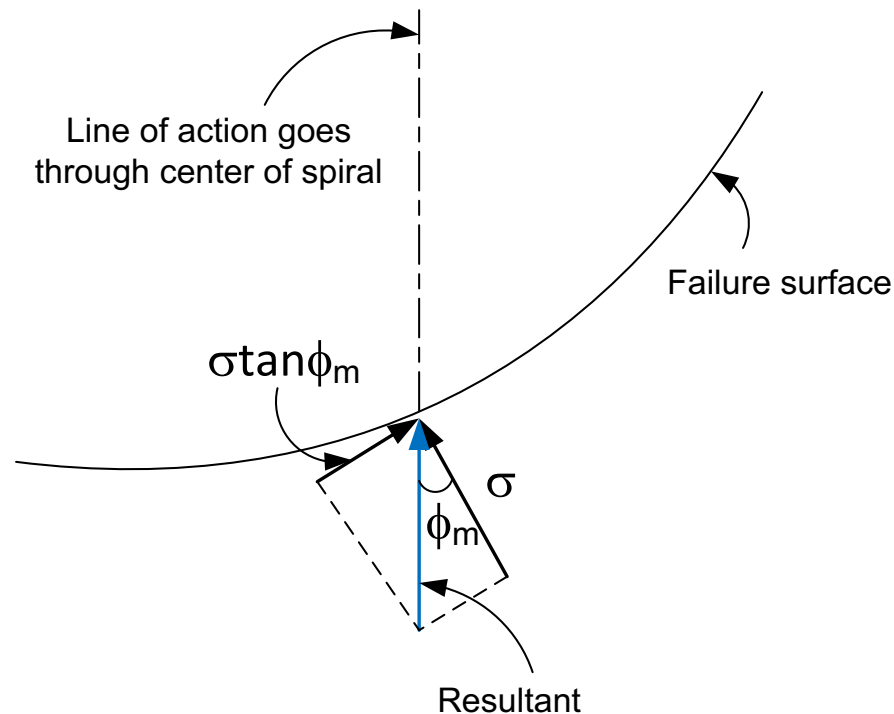
$$\tau = \frac{c}{FS} + \frac{\sigma \tan \phi}{FS}$$

$$c_d = \frac{c}{FS} \quad \tan \phi_d = \frac{\tan \phi}{FS}$$

c_d = developed or mobilized cohesion

$\tan \phi_d$ = developed or mobilized friction angle

Log Spiral Technique



Sum moments about center point:

- (1) Moment due to wt of soil mass, W
- (2) Moment due to mobilized cohesion C_m
- (3) Moments due to mobilized friction and normal forces cancel

(1) and (2) give you one equation with one unknown (F), but geometry depends on F so problem is solved iteratively.

Log Spiral Summary

- Achieves statically determinate solution by assuming log spiral shape for slope
- Explicitly satisfies moment equilibrium; implicitly satisfies force equilibrium
 - Complete equilibrium
 - Relatively accurate

Log Spiral Summary, cont.

- Best procedure for homogenous slopes
- Tough to solve by hand
- Popular method for
 - Charts
 - Simple programs

Swedish Method ($\phi=0$)

Swedish Method ($\phi=0$)

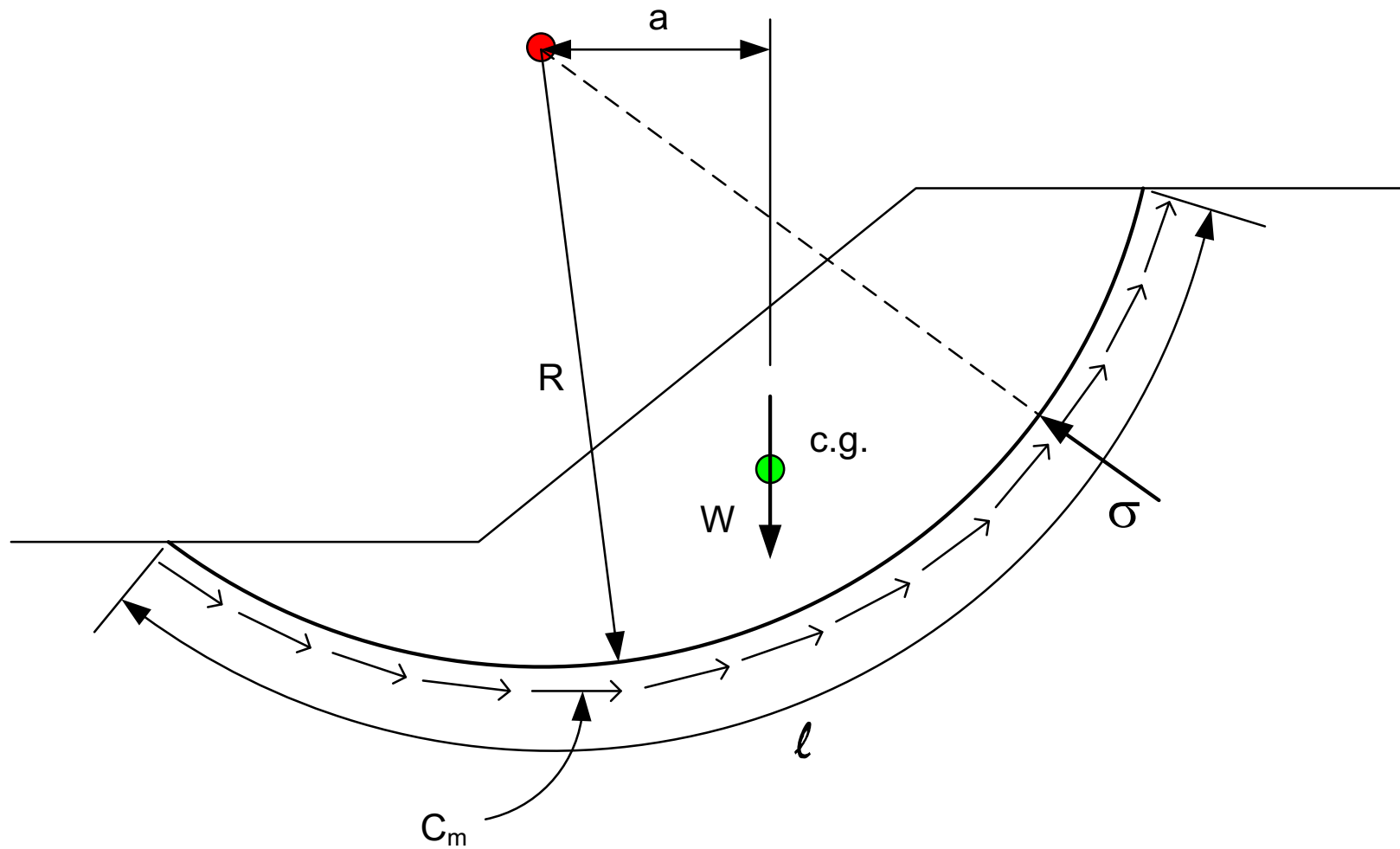
- Applicable to
 - Saturated soil
 - Undrained conditions
- Can be thought of as special case of log spiral:

$$r = r_0 e^{\theta \tan \phi_m^0}$$

(i.e. circular surface)

$$r = r_0 e^0 = r_0$$

Swedish Method



Swedish Method

Sum moments about the center of the circle:

$$W_a - c_m \ell R = 0$$

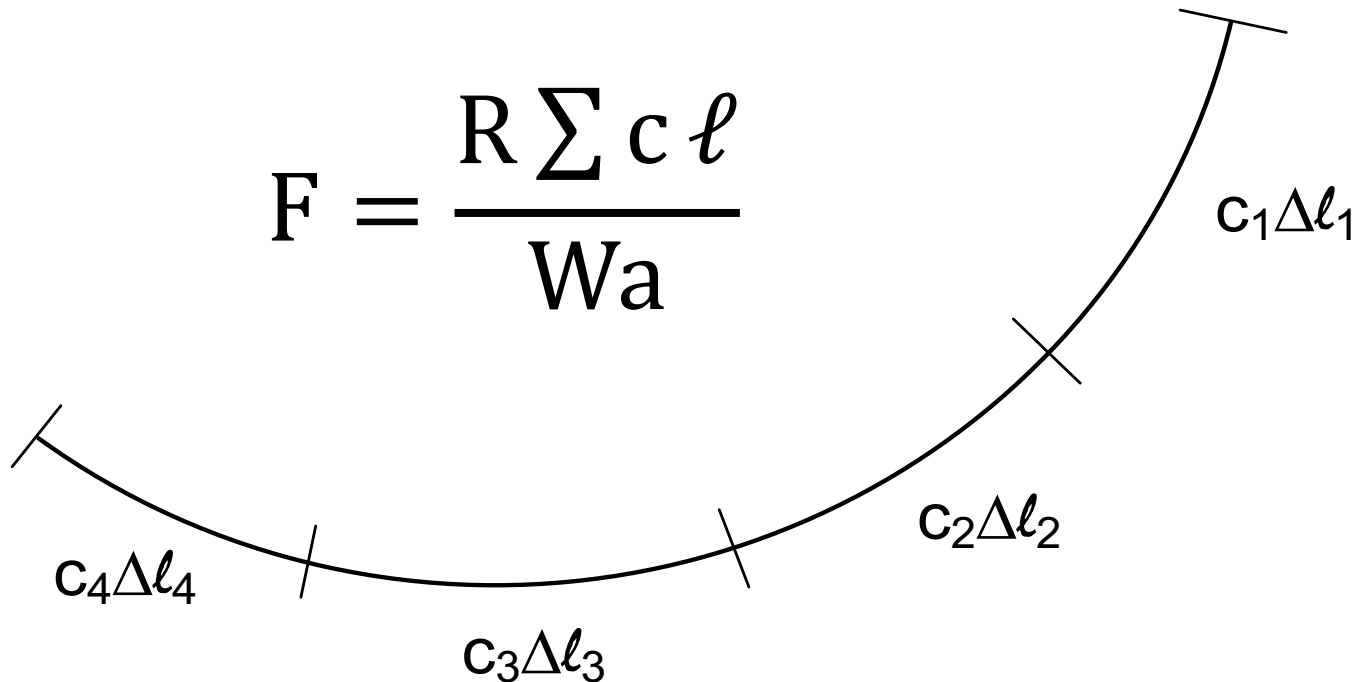
$$W_a = c_m \ell R = \frac{c}{F} \ell R$$

$$F = \frac{c \ell R}{W_a} = \frac{\text{Available resisting moment}}{\text{Actual driving moment}}$$

Swedish Method

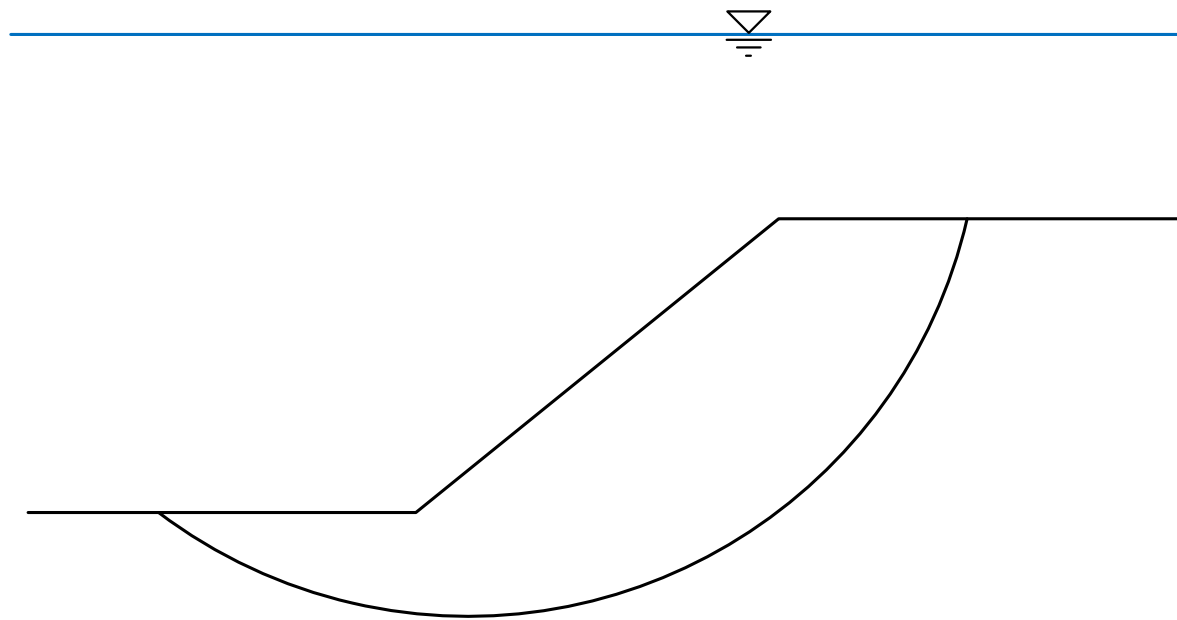
If c varies along the slope, break up slope and sum the individual moments:

$$F = \frac{R \sum c \ell}{W a}$$



Swedish Method

Submerged Slopes



Use submerged unit wt. to account for buoyant force

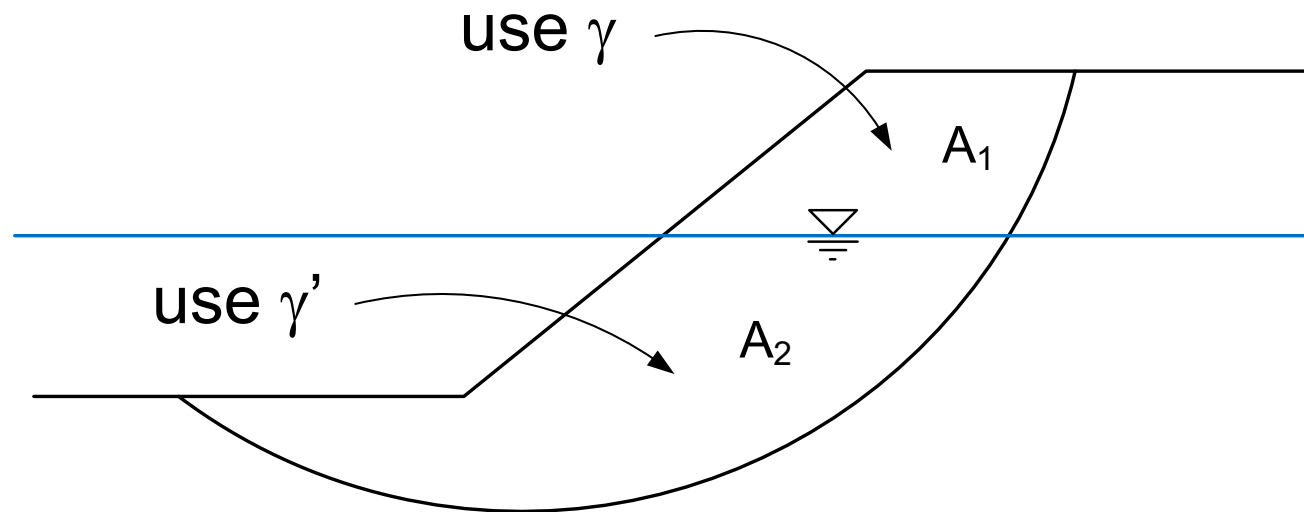
$$W_a = (\gamma - \gamma_w) A_a$$

$$W_a = \gamma' A_a$$

A = Area of sliding mass

Swedish Method

Partially Submerged Slopes



$$F = \frac{c\ell R}{\gamma A_1 a_1 + \gamma' A_2 a_2}$$

Circular Surface, $\phi \neq 0$

Σ Moments about center:

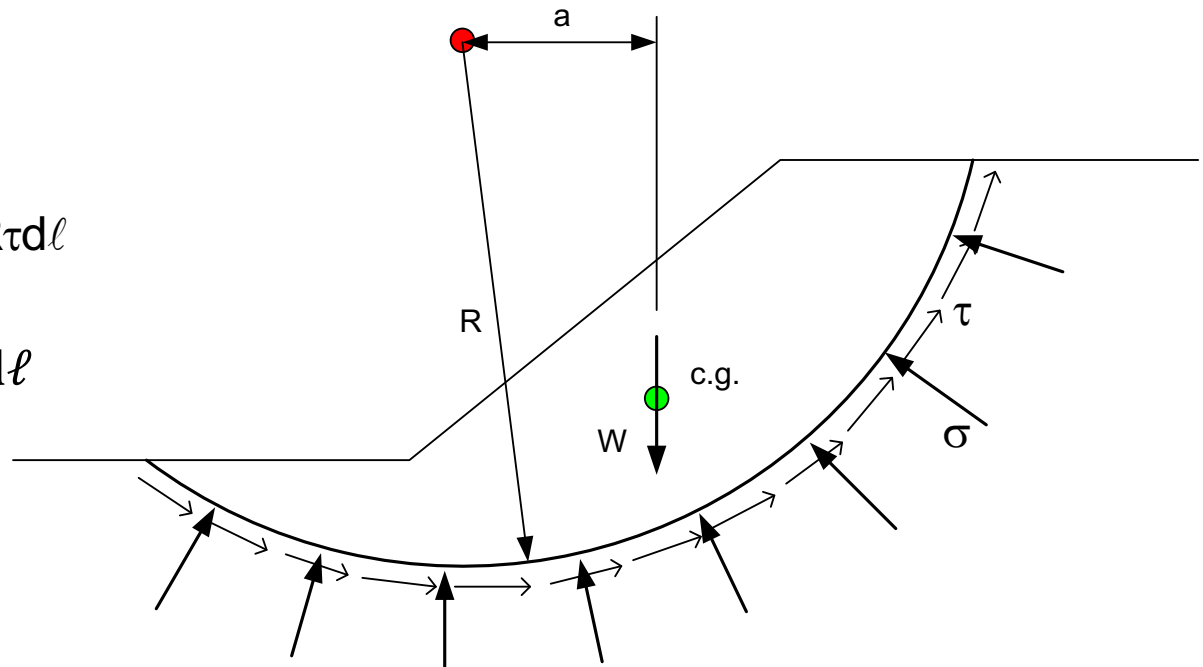
- 1) Due to weight, $M_\gamma = Wa$
- 2) Due to normal stress, $M_\sigma = 0$
- 3) Due to shear stress, $M_\tau = \int R\tau d\ell$

$$\sum M = M_\gamma - M_\tau = Wa - \int R\tau d\ell$$

$$\tau = \frac{c'}{F} + (\sigma - u) \frac{\tan\phi'}{F}$$

$$Wa - R \int \left[\frac{c'}{F} + (\sigma - u) \frac{\tan\phi}{F} \right] d\ell = 0$$

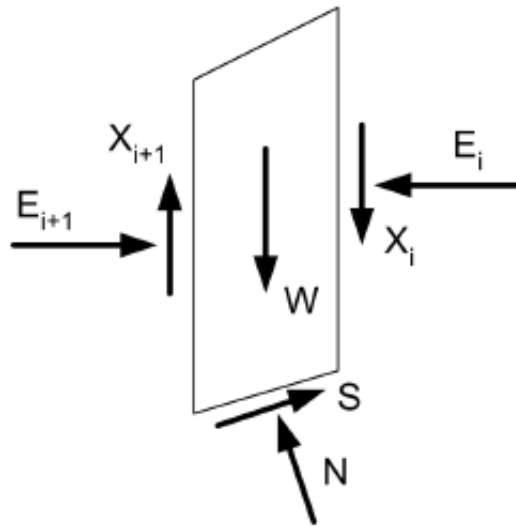
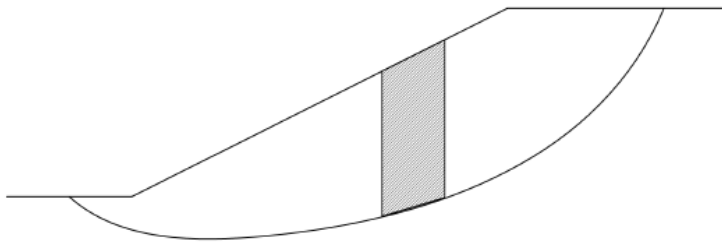
$$F = \frac{R \int [c' + (\sigma - u) \tan\phi] d\ell}{Wa}$$



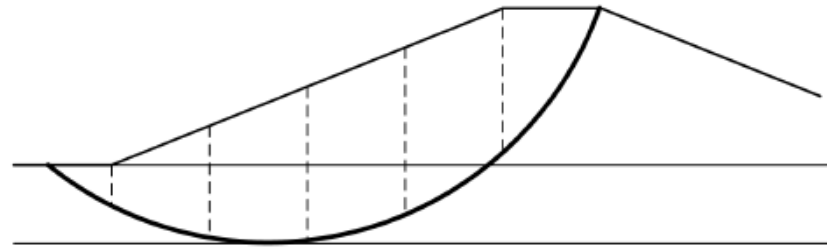
Note: σ varies along the surface and is unknown so you can't solve directly for F

General Method of Slices

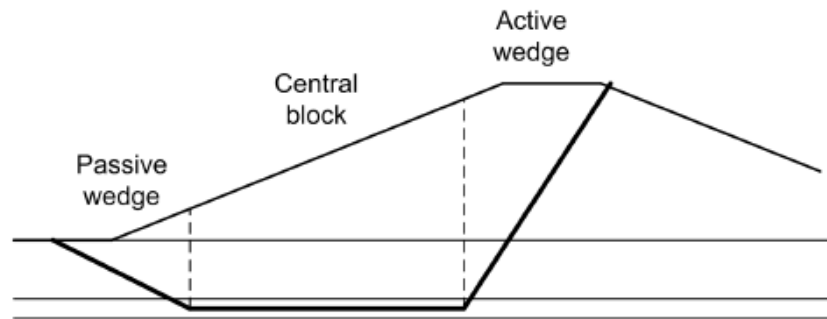
Method of Slices



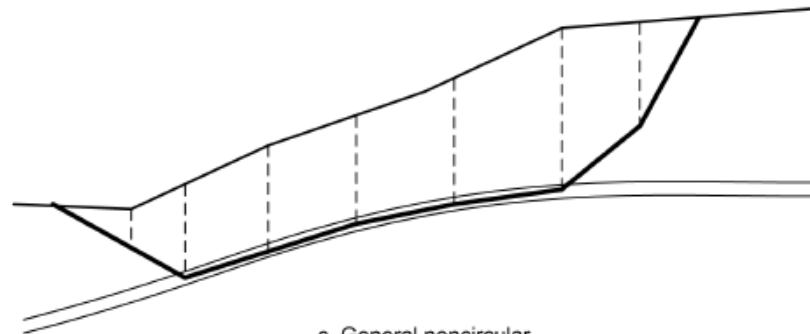
$$F = \frac{\sum [c' \Delta l + (W \cos \alpha - u \Delta l \cos^2 \alpha) \tan \phi']}{\sum W \sin \alpha}$$



a. Circular

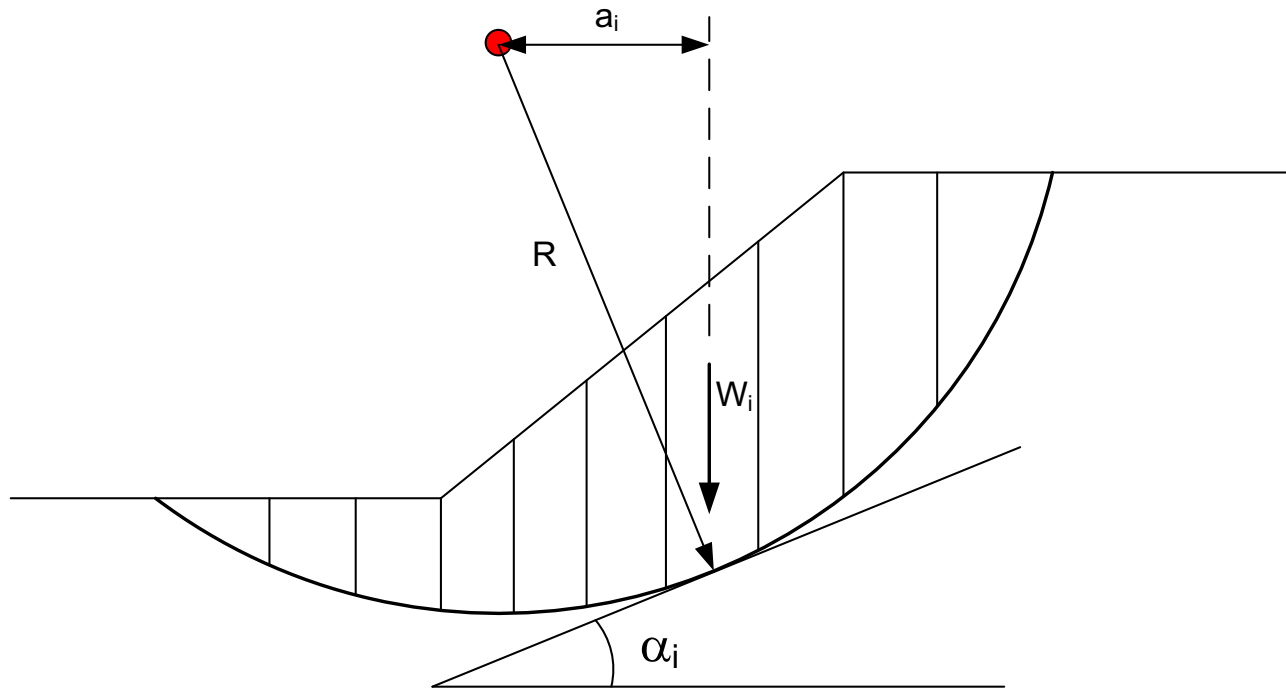


b. Wedge

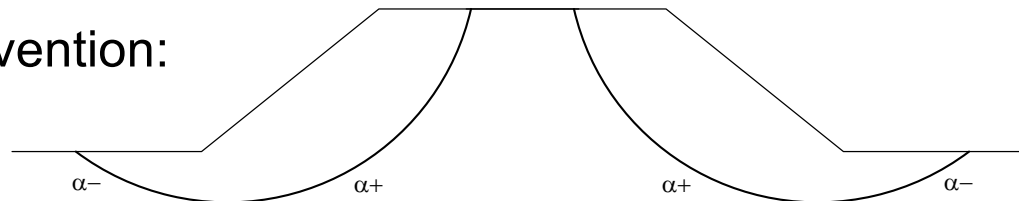


c. General noncircular

General Method of Slices



Sign convention:



General Method of Slices

Moment arm:

$$a_i = R \sin \alpha_i$$

Driving moment:

$$M_d = R \sum W_i \sin \alpha_i$$

Resisting moment:

$$M_r = \sum R S_i = R \sum S_i$$

$$M_r = R \sum \tau_i \Delta \ell_i$$

$$M_r = R \sum \frac{s_i \Delta \ell_i}{F}$$

$$M_d = M_r$$

$$R \sum W_i \sin \alpha_i = R \sum \frac{s_i \Delta \ell_i}{F}$$

$$\sum W_i \sin \alpha_i = \sum \frac{s_i \Delta \ell_i}{F}$$

$$F = \frac{\sum s_i \Delta \ell_i}{\sum W_i \sin \alpha_i}$$

$$s = c + \sigma \tan \phi$$

$$F = \frac{\sum (c + \sigma \tan \phi) \Delta \ell_i}{\sum W_i \sin \alpha_i} \quad \leftarrow \text{General eq.}$$

General Method of Slices

If $\phi=0$

$$F = \frac{\sum (c + \sigma \tan \phi) \Delta \ell_i}{\sum W_i \sin \alpha_i}$$

$$F = \frac{\sum c \Delta \ell_i}{\sum W_i \sin \alpha_i}$$

This equation is often called
the “Swedish Method”

Compare to Swedish Circle equation:

$$F = \frac{c \ell R}{W a}$$