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June 3, 2008

David D. Nesbit Nesbit Development 555 Gettysburg Pike, Suite C-100 Mechanicsburg, PA 17055

Re:

Trip Generation Assessment Hillcrest at Shepardstown

Upper Allen Township, Cumberland County

Dear David:

We have completed a trip generation assessment for the proposed rezoning of the Hillcrest at Shepardstown site. The proposed site is located along the east side of Gettysburg Road, south of Shepardstown. This trip generation assessment is provided to compare the trip generation estimates for the site with the current zoning and the proposed zoning.

The ITE Trip Generation Manual, published by the Institute of Transportation Engineers (2003), was used to estimate of trips that may be generated with the current and the proposed site zoning. The trip generation calculations summarized in the following tables are based upon the ITE Trip Generation Manual. Trip generation calculation worksheets are attached for reference.

Development with Current Zoning

Under existing zoning conditions, it is estimated that the site can be developed with approximately 15 single family dwelling units. Table 1 shows the trip generation calculations for the site if it were developed with single family housing.

Table 1. ITE Trip Generation Summary Current Zoning

| Land Use (Code) [Size] | Average Weekday Vehicle Trips (vpd) | AM Peak (vph) | | PM Peak (vph) | |
|---|--|---------------|------|---------------|------|
| | | Enter | Exit | Enter | Exit |
| Single Family Detached Housing (210) [15 units] | 182 | 5 | 15 | 12 | 7 |

Development with Proposed Zoning

The development proposal for Hillcrest at Shepardstown under the proposed zoning consists of 37 residential townhouses. Table 2 shows the trip generation calculations for the site if it were developed with the townhouses.

Table 2. ITE Trip Generation Summary Proposed Zoning

| Land Use (Code) | Average Weekday Vehicle Trips (vpd) | Weekday | | PM Peak (vph) | |
|--|--|---------|------|---------------|------|
| [Size] | | Enter | Exit | Enter | Exit |
| Residential Townhouses (230) [37 units] | 276 | 4 | 19 | 18 | 9 |

Summary of Trip Generation Estimates

Table 3 provides a comparison of the new trips that will be generated by the site under the current and proposed zoning.

Table 3. ITE Trip Generation Summary
Trip Generation Comparison

| The Generation Companison | | | | | |
|--|---------|---------------|------|---------------|------|
| Land Use (Code) [Size] Average Weekday Vehicle Trips (vpd) | Weekday | AM Peak (vph) | | PM Peak (vph) | |
| | | Enter | Exit | Enter | Exit |
| Residential Townhouses (230) [37 units] | 276 | 4 | 19 | 18 | 9 |
| Single Family Detached Housing (210) [15 units] | 182 | 5 | 15 | 12 | 7 |
| Additional Trips with Proposed Zoning | 94 | -1 | 4 | 6 | 2 |

Based upon the information summarized in Table 3, we conclude that the rezoning of the site to allow the proposed townhouse development will not have a significant impact on the trip generation potential of the site. The 37 townhouses projected to generate only three (3) more trips during the AM peak hour, and eight (8) more trips during the PM peak hour. The 94 additional daily trips breaks down to less than four (4) trips per hour over the 24-hour period.

Sight Distance Evaluation

Sight distances at the proposed site access location onto Gettysburg Road were evaluated to determine if available sight distances meet PENNDOT sight distance criteria outlined in Pennsylvania Code 67, Chapter 441.8 and PENNDOT Publication 212. Sight distances were measured and compared with the published safe sight distance criteria.

The posted speed limit (25 miles per hour) and approach grades on Gettysburg Road were used to determine whether adequate sight distance is available. A summary of sight distance criteria from Chapter 441 and Publication 212, as well as the sight distance measurements for the intersection can be found in Table 4.

Table 4. Sight Distance Evaluation Summary: Gettysburg Road and Site Access

| Cettysburg (toda tala ette / teeces | | | | | |
|-------------------------------------|---|-------------------------------------|------------------------|--------------------|------------|
| Criteria Source | Direction | Available Sight Distance (ft) | | ght Distance t) | Acceptable |
| | *************************************** | | Desirable ¹ | Minimum² | |
| Chapter 441 Section 441.8 | Left | 1,469 | 250 | 161 | YES |
| 0001017 44 1.0 | Right | 548 | 195 | 159 | YES |
| Publication 212 | Left | 1,469 | 147 | | YES |
| Form M-950S | Right | 548 | 14 | 15 | YES |

^{1 -} Chapter 441, Section 441.8(h)(1) Table 1.

As shown in Table 4, the sight distances observed at the proposed site access are expected to exceed PENNDOT stopping sight distance criteria.

Please give me a call if you have questions relative to the materials provided herein, or if you need anything additional.

Sincerely,

Gregory E. Creasy, P.E

Traffic Engineer

Attachments

FILE: Z:\Proposals\Nesbit Development\tgen_comparison.wpd



^{2 -} Chapter 441, Section 441.8(h)(2)(iv).

Summary of Trip Generation Calculation For 15 Dwelling Units of Single Family Detached Housing March 27, 2008

| | Average Rate | | Adjustment Factor | |
|---------------------------|-----------------|------|----------------------|-----|
| Avg. Weekday 2-Way Volume | 12.10 | 0.00 | 1.00 | 182 |
| 7-9 AM Peak Hour Enter | 0.33 | 0.00 | 1.00 | 5 |
| 7-9 AM Peak Hour Exit | 1.00 | 0.00 | 1.00 | 15 |
| 7-9 AM Peak Hour Total | 1.33 | 0.00 | 1.00 | 20 |
| 4-6 PM Peak Hour Enter | 0.82 | 0.00 | 1.00 | 12 |
| 4-6 PM Peak Hour Exit | 0.48 | 0.00 | 1.00 | 7 |
| 4-6 PM Peak Hour Total | 1.30 | 0.00 | 1.00 | 19 |
| Saturday 2-Way Volume | 11.79 | 0.00 | 1.00 | 177 |
| Saturday Peak Hour Enter | 0.87 | 0.00 | 1.00 | 13 |
| Saturday Peak Hour Exit | 0.74 | 0.00 | 1.00 | 11 |
| Saturday Peak Hour Total | 1.62 | 0.00 | 1.00 | 24 |

Note: A zero indicates no data available. The above rates were calculated from these equations:

| 24-Hr. 2-Way Volume: | LN(T) = .92LN(X) + | $2.71, R^2 =$ | 0.96 |
|------------------------|---------------------|---------------|------|
| 7-9 AM Peak Hr. Total: | T = .7(X) + 9.43 | | |
| | $R^2 = 0.89 , 0.25$ | Enter, 0.75 | Exit |
| 4-6 PM Peak Hr. Total: | LN(T) = .9LN(X) + | .53 | |
| | $R^2 = 0.91, 0.63$ | Enter, 0.37 | Exit |
| AM Gen Pk Hr. Total: | T = .7(X) + 12.05 | | |
| | $R^2 = 0.89$, 0.26 | Enter, 0.74 | Exit |
| PM Gen Pk Hr. Total: | LN(T) = .89LN(X) + | .61 | |
| | $R^2 = 0.91, 0.64$ | Enter, 0.36 | Exit |
| Sat. 2-Way Volume: | LN(T) = .94LN(X) + | $2.63, R^2 =$ | 0.93 |
| Sat. Pk Hr. Total: | T = .89(X) + 10.93 | | |
| | $R^2 = 0.9, 0.54$ | Enter, 0.46 | Exit |
| Sun. 2-Way Volume: | T = 8.83(X) + -9.76 | $R^2 = 0.94$ | |
| Sun. Pk Hr. Total: | LN(T) = .89LN(X) + | . 44 | |
| | $R^2 = 0.88 , 0.53$ | Enter, 0.47 | Exit |
| | | | |

Source: Institute of Transportation Engineers
Trip Generation, 7th Edition, 2003.

TRIP GENERATION BY MICROTRANS

Summary of Trip Generation Calculation For 37 Dwelling Units of Residential Condominium / Townhouse June 02, 2008

| | Average | Standard | Adjustment | Driveway |
|---------------------------|---------|-----------|------------|----------|
| | Rate | Deviation | Factor | Volume |
| Avg. Weekday 2-Way Volume | 7.45 | 0.00 | 1.00 | 276 |
| 7-9 AM Peak Hour Enter | 0.11 | 0.00 | 1.00 | 4 |
| 7-9 AM Peak Hour Exit | 0.52 | 0.00 | 1.00 | 19 |
| 7-9 AM Peak Hour Total | 0.63 | 0.00 | 1.00 | 23 |
| 4-6 PM Peak Hour Enter | 0.48 | 0.00 | 1.00 | 18 |
| 4-6 PM Peak Hour Exit | 0.24 | 0.00 | 1.00 | 9 |
| 4-6 PM Peak Hour Total | 0.72 | 0.00 | 1.00 | 27 |
| Saturday 2-Way Volume | 15.19 | 0.00 | 1.00 | 562 |
| Saturday Peak Hour Enter | 0.78 | 0.00 | 1.00 | 29 |
| Saturday Peak Hour Exit | 0.66 | 0.00 | 1.00 | 25 |
| Saturday Peak Hour Total | 1.44 | 0.00 | 1.00 | 53 |

Note: A zero indicates no data available. The above rates were calculated from these equations:

| 24-Hr. 2-Way Volume: | $LN(T) = .85LN(X) + 2.55, R^2 = 0.83$ LN(T) = .8LN(X) + .26 |
|------------------------|--|
| 7-9 AM Peak Hr. Total: | $R^2 = 0.76$, 0.17 Enter, 0.83 Exit |
| 4-6 PM Peak Hr. Total: | LN(T) = .82LN(X) + .32 $R^2 = 0.8$, 0.67 Enter, 0.33 Exit |
| AM Gen Pk Hr. Total: | LN(T) = .82LN(X) + .17 |
| PM Gen Pk Hr. Total: | $R^2 = 0.8$, 0.18 Enter, 0.82 Exit T = .34(X) + 38.31 |
| PM Gen Pk HI. 10tal. | $R^2 = 0.83$, 0.64 Enter, 0.36 Exit |
| Sat. 2-Way Volume: | $T = 3.62(X) + 427.93, R^2 = 0.84$ T = .29(X) + 42.63 |
| Sat. Pk Hr. Total: | $R^2 = 0.84$, 0.54 Enter, 0.46 Exit |
| Sun. 2-Way Volume: | $T = 3.13(X) + 357.26, R^2 = 0.88$ T = .23(X) + 50.01 |
| Sun. Pk Hr. Total: | $R^2 = 0.78$, 0.49 Enter, 0.51 Exit |

Source: Institute of Transportation Engineers Trip Generation, 7th Edition, 2003.

TRIP GENERATION BY MICROTRANS