

# OMAR SHEMY PORTFOLIO

## SUBJECT: Wind Load Calculation

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2024 JAN 19	202401	DEMO PORTFOLIO	O.S.	1

### WIND LOAD CALCULATION

$$I_w := 1.0$$

$$q := 0.6 \quad \text{for 1/50 year exceedence}$$

$$h := 19 \leq 20 \text{ and } h < 66.5 \text{ (smaller planar dimension)}$$

$$h := \frac{19}{2} = 9.5 \quad 9.5 > 6 \quad \therefore \text{OK}$$

$$C_e := \left( \frac{9.5}{10} \right)^{0.2} = 0.99 \quad C_e \geq 0.9 \quad \therefore \text{OK}$$

$$C_t := 1.0 \quad \text{For no hills and escarpments}$$

References(from NBCC 2015):  
Table 4.1.7.3.  
1.1.3 Appendix C  
4.1.7.3 sentence b)

Load case a

Main Wall

$$C_p C_{g1} := 0.75$$

$$C_p C_{g2} := -0.55$$

$$P_{windward\_mainwall} := 1.0 \cdot 0.6 \cdot 0.99 \cdot 1.0 \cdot 0.75 = 0.446$$

$$P_{leeward\_mainwall} := 1.0 \cdot 0.6 \cdot 0.99 \cdot 1.0 \cdot 0.55 = 0.327$$

Edge(High zone)

$$C_p C_{g3} := 1.15$$

$$C_p C_{g4} := -0.8$$

$$P_{windward\_edge} := 1.0 \cdot 0.6 \cdot 0.99 \cdot 1.0 \cdot 1.15 = 0.683$$

$$P_{leeward\_edge} := 1.0 \cdot 0.6 \cdot 0.99 \cdot 1.0 \cdot 0.8 = 0.475$$

4.1.7.4

4.1.7.6

Load case b

Main wall parallel to wind

Notes checking:

4.1.7.6-A

(3) Positive coefficients denote forces toward the surface, negative coefficient denote forces away from the surface

(5) The eave height, H=19m for roof slope<7 deg

(6) End zone:

$$y := 2 \cdot 6.65 = 13.3 \quad y > 6 \quad \therefore \text{OK}$$

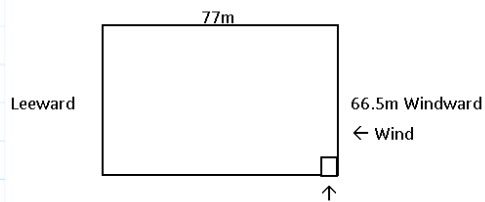
$$(7) z := \min(0.1 \cdot 66.5, 0.4 \cdot 19) = 6.65$$

$$0.04 \cdot 66.5 = 2.66$$

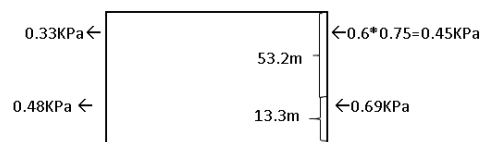
$$6.65 > 2.66 \quad \therefore \text{OK}$$

(8) B/H<5 checked

Corner Orientation 1:  
Main wall parallel to wind:



Case A:

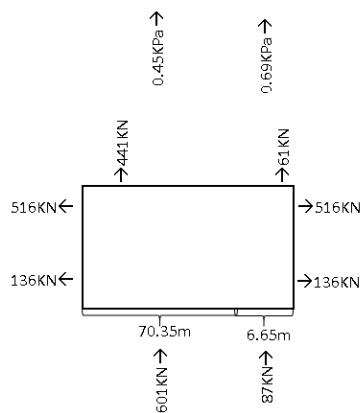
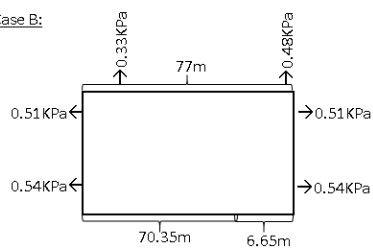


$$B/H = \frac{77}{19} = 4.05$$

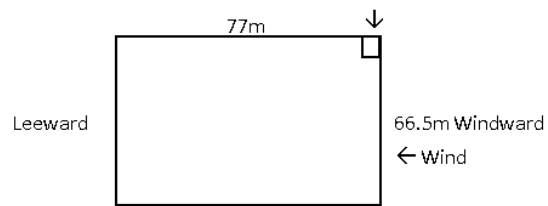
H=19



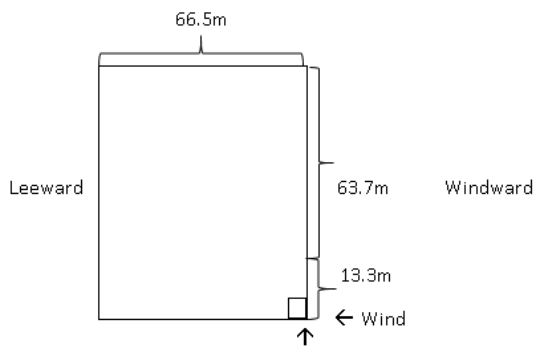
Case B:



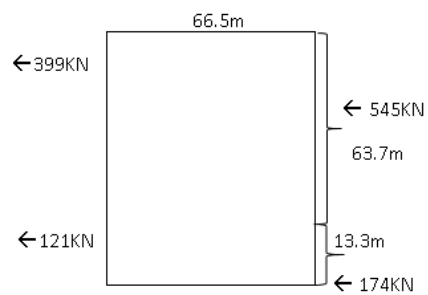
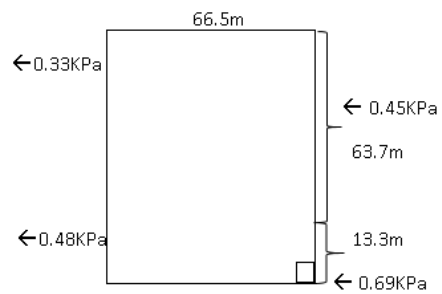
Corner orientation 2 (mirror to orientation 1):



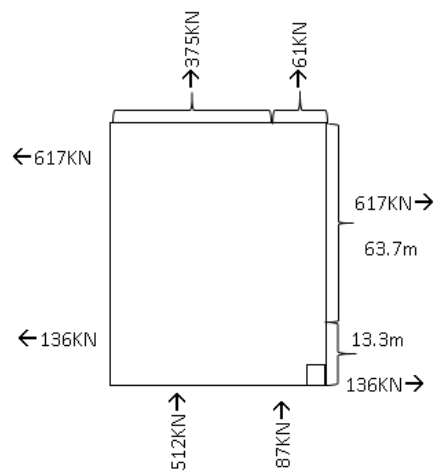
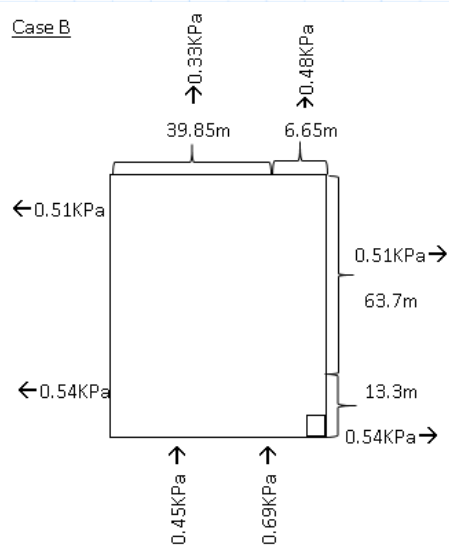
Corner orientation 3:



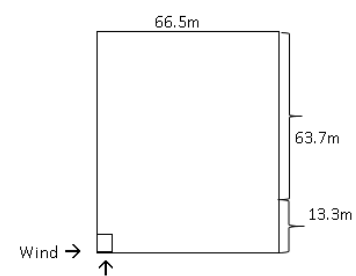
Case A



Case B



Corner orientation 4:



Mirror to orientation 3

Most critical:

Orientation 3,4: load case A dominates:

$$399 \text{ kN} + 545 \text{ kN} = 944 \text{ kN}$$

$$121 \text{ kN} + 174 \text{ kN} = 295 \text{ kN}$$

Comparing to:

$$512 \text{ kN} + 375 \text{ kN} = 887 \text{ kN}$$

$$87 \text{ kN} + 61 \text{ kN} = 148 \text{ kN}$$

Load case 1,2: load case B dominates:

$$441 \text{ kN} + 601 \text{ kN} = (1.042 \cdot 10^3) \text{ kN}$$

$$87 \text{ kN} + 61 \text{ kN} = 148 \text{ kN}$$

Comparing to :

$$445 \text{ kN} + 334 \text{ kN} = 779 \text{ kN}$$

$$121 \text{ kN} + 174 \text{ kN} = 295 \text{ kN}$$

Hangar walls:

$$Z := \min(0.1 \cdot 54, 0.4 \cdot 19) = 5.4$$

$$5.4 > 1 \quad \therefore \text{OK}$$

$$\text{e: } Area := 5.4 \cdot 19 = 102.6$$

$$\text{w: } Area_{w1} := (54 - Z \cdot 2) \cdot 19 = 820.8$$

$$Area_{w2} := (77 - Z \cdot 2) \cdot 19 = 1.258 \cdot 10^3$$

$C_p C_g$  coefficients:

$$\text{e: } +1.3 \quad -1.5$$

$$\text{w: } +1.3 \quad -1.5$$

Hangar roof:

$$\text{C: } Z^2 = 29.16$$

$$\text{S: } Area_{S1} := (54 - Z \cdot 2) \cdot Z = 233.28$$

$$Area_{S2} := (77 - Z \cdot 2) \cdot Z = 357.48$$

$$\text{r: } Area_r := (54 - Z \cdot 2) \cdot (77 - Z \cdot 2) = 2.86 \cdot 10^3$$

$C_p C_g$  coefficient

$$\text{C: } 0.3 \quad -2.0$$

$$\text{S: } 0.3 \quad -2.0$$

$$\text{r: } 0.3 \quad -1.5$$

$$1 \cdot (0.6) \quad 1 \cdot 1 \cdot 0.3 = 0.18$$

$$1 \cdot 0.6 \cdot 1 \cdot 1 \cdot -2 = -1.2$$

$$1 \cdot 0.6 \cdot 1 \cdot 1 \cdot -1.5 = -0.9$$

Interior pressure:

$$h = 9.5$$

$$9.5 < 20 \quad \therefore \text{ case b)}$$

References  
(NBCC 2015)  
4.1.7.6-B

4.1.7.3-7

$$C_{ei} := \left( \frac{9.5}{10} \right)^{0.2} = 0.99 \quad 0.99 > 0.9 \therefore OK$$

4.1.7.3-5

$$C_t := 1.0$$

4.1.7.4-1

$$C_{gi} := 2.0$$

4.1.7.3-10

$C_{pi}$  ranges from -0.45 to 0.3

4.1.7.7-1 Table 4.1.7.7

$$P_{int\_positive} := 1.0 \cdot 0.6 \cdot 1.0 \cdot 2.0 \cdot 0.99 \cdot 0.3 = 0.356$$

$$P_{int\_negative} := -0.54$$

Summary Surface	$P_{ext}$		$P_{int}$		Netpressure	
	+	-	+	-	+	-
e	0.78	-0.9	0.36	-0.54	1.14	-1.44
w	0.78	-0.9	0.36	-0.54	1.14	-1.44
c	0.18	-1.2	0.36	-0.54	0.54	-1.74
s	0.18	-1.2	0.36	-0.54	0.54	-1.74
r	0.18	-0.9	0.36	-0.54	0.54	-1.44

Office walls:

$$Z := \min(0.1 \cdot 12.5, 0.4 \cdot 9.91) = 1.25$$

$$1.25 > 1 \therefore OK$$

$$e: Area := 1.25 \cdot 9.91 = 12.388$$

$$w: Area_{w1} := (54 - Z \cdot 2) \cdot 9.91 = 510.365$$

$$Area_{w2} := (12.5 - Z \cdot 2) \cdot 9.91 = 99.1$$

$C_p C_g$  coefficients:

$$e: +1.4 \quad -1.7$$

$$w: +1.3 \quad -1.5$$

$C_p C_g$  coefficient

$$Area = 12.5 \cdot 55 = 687.5$$

$$r: 0.3 \quad -1.5$$

References  
(NBCC 2015)  
4.1.7.6-B

4.1.7.6 C

Interior pressure:

4.1.7.3-7

$$h := 6$$

$$6 < 20 \therefore \text{case b)}$$

$$C_{ei} := \left( \frac{6}{10} \right)^{0.2} = 0.903 \quad 0.903 > 0.9 \therefore OK \quad 4.1.7.3-5$$

$$C_t := 1.0 \quad 4.1.7.4-1$$

$$C_{gi} := 2.0 \quad 4.1.7.3-10$$

$$C_{pi} \text{ ranges from } -0.45 \text{ to } 0.3 \quad 4.1.7.7-1 \text{ Table } 4.1.7.7$$

$$P_{int\_positive} := 1.0 \cdot 0.6 \cdot 1.0 \cdot 2.0 \cdot 0.903 \cdot 0.3 = 0.325$$

$$P_{int\_negative} := -0.49$$

Summary Surface	$P_{ext}$		$P_{int}$		$Netpressure$	
	+	-	+	-	+	-
e	0.76	-0.92	0.33	-0.49	1.25	-1.24
w	0.70	-0.81	0.33	-0.49	1.19	-1.13
r	0.16	-0.81	0.33	-0.49	0.65	-1.13