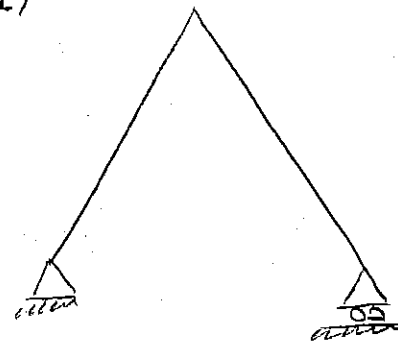


1) a)

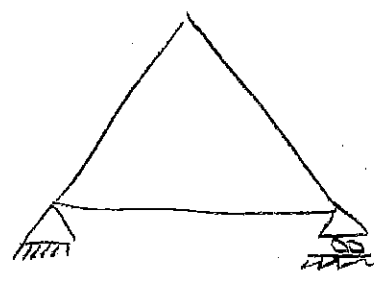


$$D_f = m + r - 2j$$

$$D_f = 2 + 3 - 2 \cdot 3$$

$$D_f = -1 < 0 // \Rightarrow \text{Unstable} //$$

b)

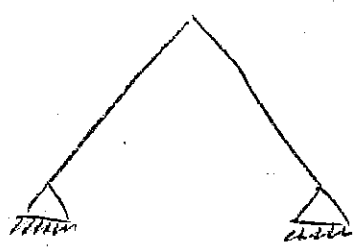


$$D_f = m + r - 2j$$

$$D_f = 3 + 3 - 2 \cdot 3$$

$$D_f = 0 // \Rightarrow \text{Determinate} //$$

c)

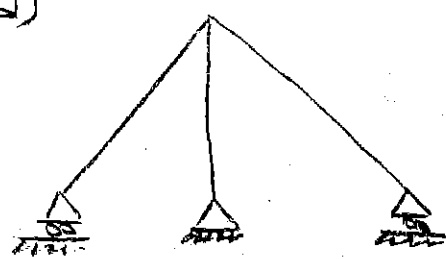


$$D_f = m + r - 2j$$

$$D_f = 2 + 2 - 2 \cdot 2$$

$$D_f = 0 // \Rightarrow \text{Determinate} //$$

d)

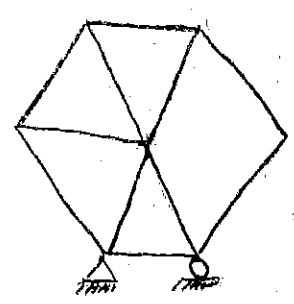


$$D_f = m + r - 2j$$

$$D_f = 3 + 4 - 2 \cdot 4$$

$$D_f = -1 < 0 // \Rightarrow \text{Unstable} //$$

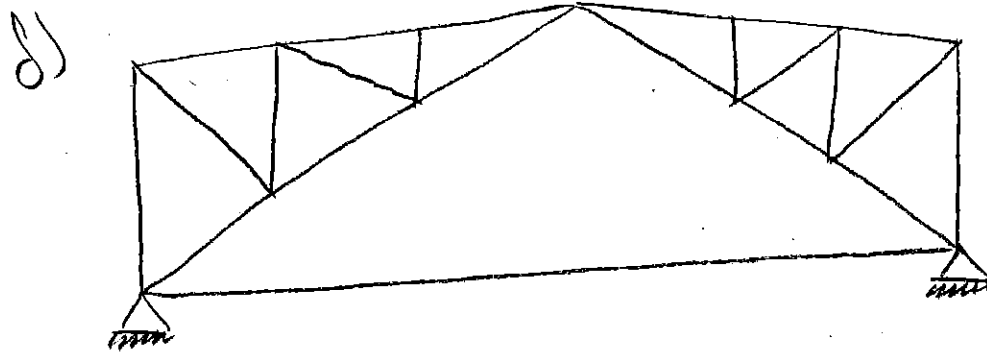
e)



$$D_f = m + r - 2j$$

$$D_f = 11 + 3 - 2 \cdot 7$$

$$D_f = 0 // \Rightarrow \text{Determinate} //$$

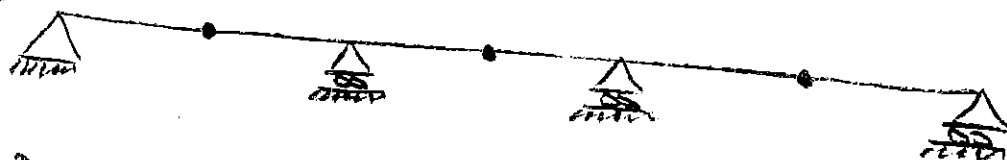


$$D_f = m + r - 2j$$

$$D_f = 23 + 4 - 2 \cdot 13$$

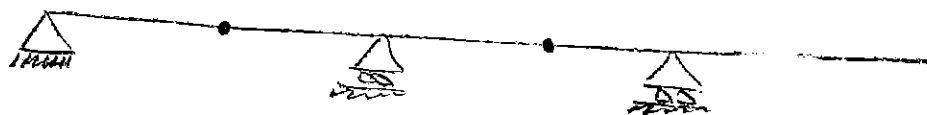
$$D_f = 1 // \Rightarrow \text{Indeterminate}$$

2) a)



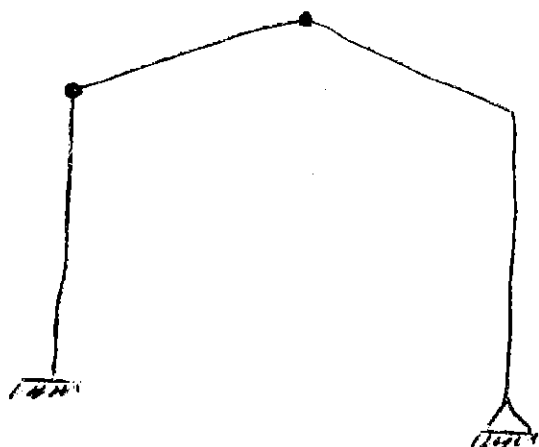
$$D_f = 3 \cdot n + r - e - x = 3 \cdot 0 + 5 - 3 - 3 = -1 < 0 \Rightarrow \text{Unstable}$$

b)



$$D_f = 3 \cdot n + r - e - x = 3 \cdot 0 + 4 - 3 - 2 = -1 < 0 \Rightarrow \text{Unstable}$$

c)

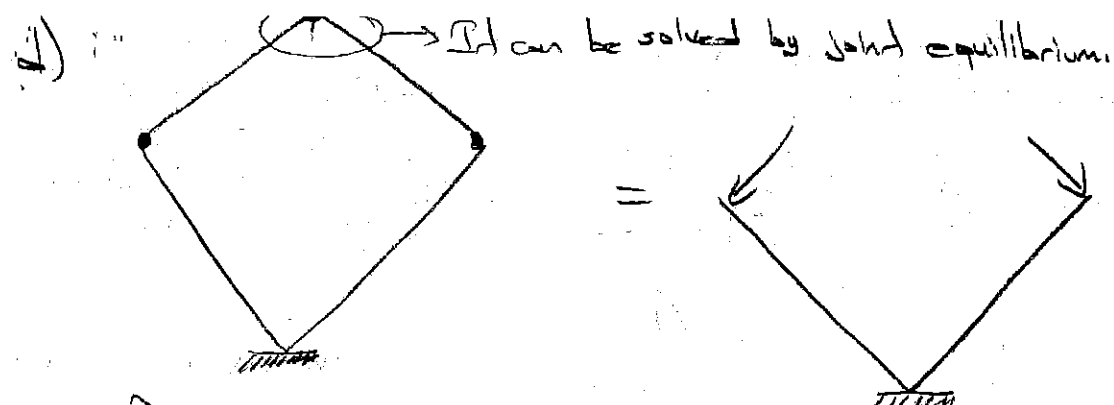


$$D_f = 3n + r - e - x$$

$$D_f = 3 \cdot 0 + 5 - 3 - 2$$

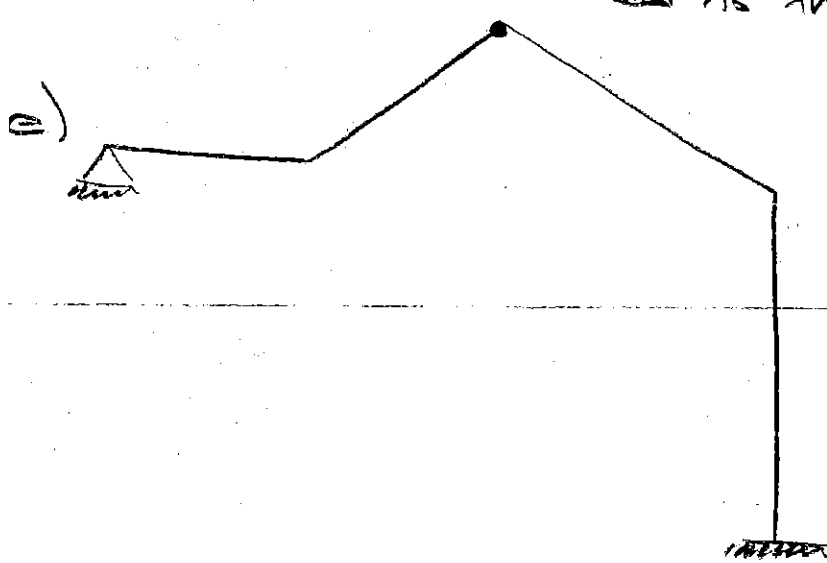
$$D_f = 0 // \Rightarrow \text{Determinate}$$

Note = In calculations, closed loops adjacent to foundation are not taken into account.



Determinate

2 truss members are added to the system. (2nd indeterminate).

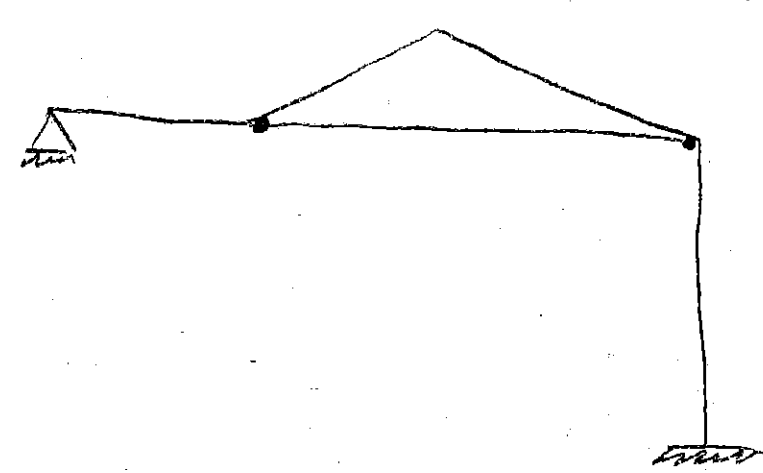


$$D_f = 3n + r - e - x$$

$$D_f = 0 + 5 - 3 - 1$$

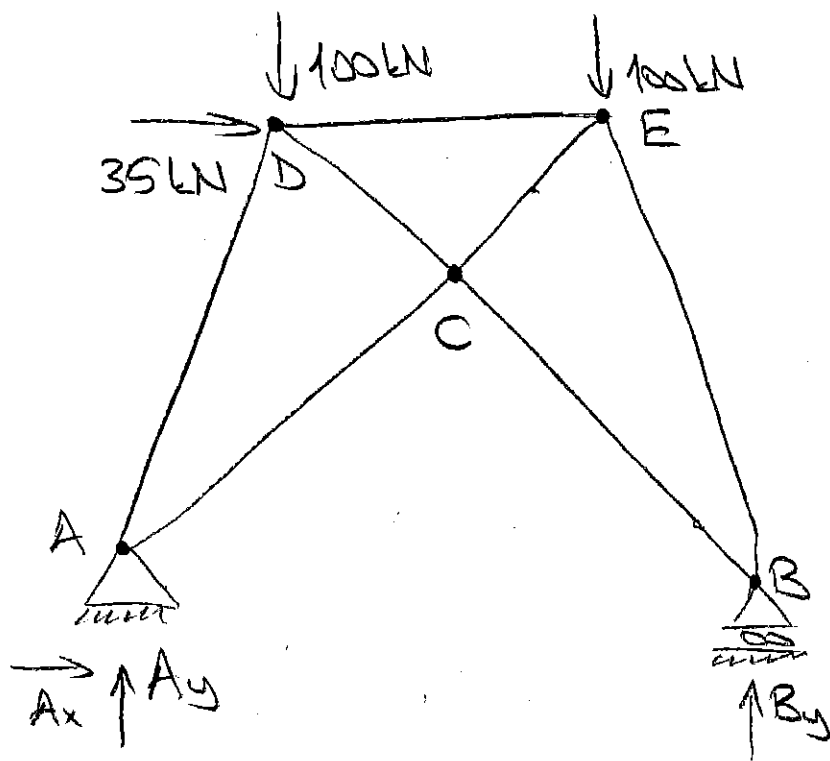
$$D_f = 1 \Rightarrow \text{Indeterminate}$$

If we add one truss member to the above structure, we obtain the provided structure.



Therefore, this structure is indeterminate (2nd degree).

3) Use unit dummy load method to obtain the displacement because this structure is statically determinate



$$\sum M_A = 0 \Rightarrow 35 \cdot 10 + 100 \cdot 4 + 100 \cdot 10 - 14 \cdot B_y = 0$$

$$B_y = 125 \text{ kN}$$

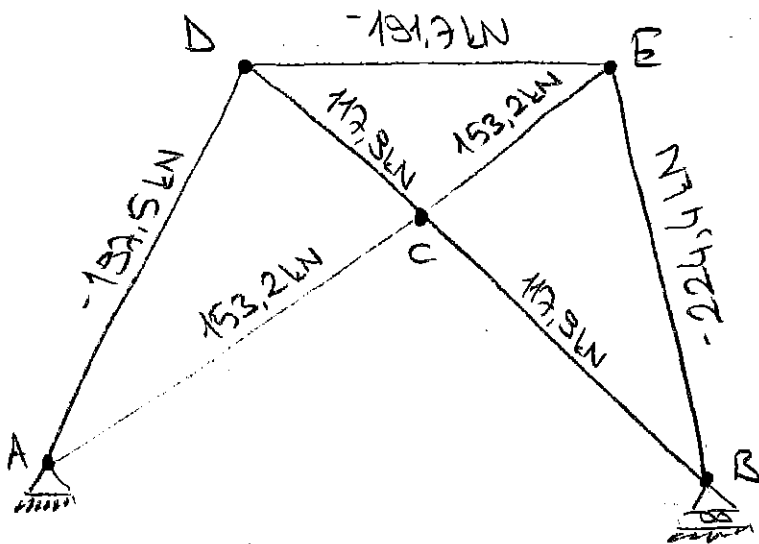
$$\sum F_y = 0 \Rightarrow A_y + B_y - 200 = 0$$

$$A_y = 75 \text{ kN}$$

$$\sum F_x = 0 \Rightarrow A_x + 35 = 0$$

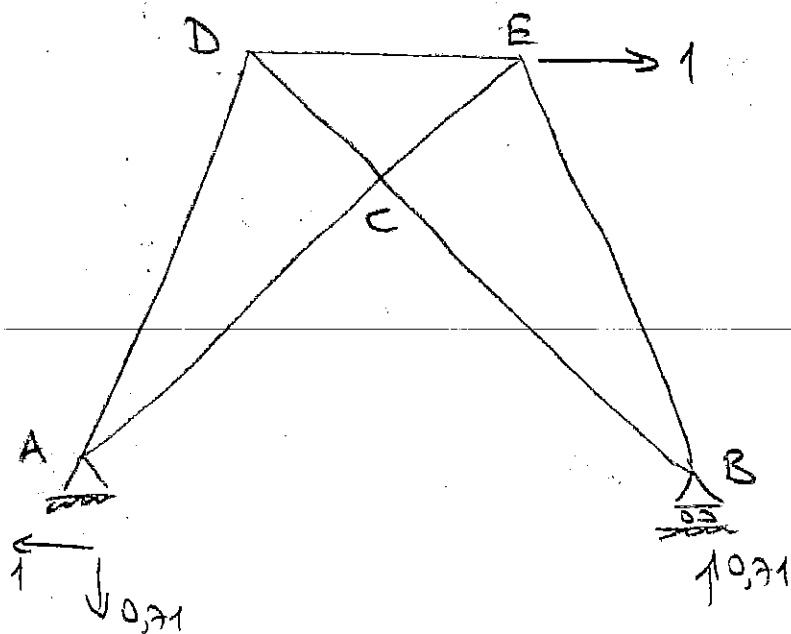
$$A_x = -35 \text{ kN}$$

The internal forces due to real forces are

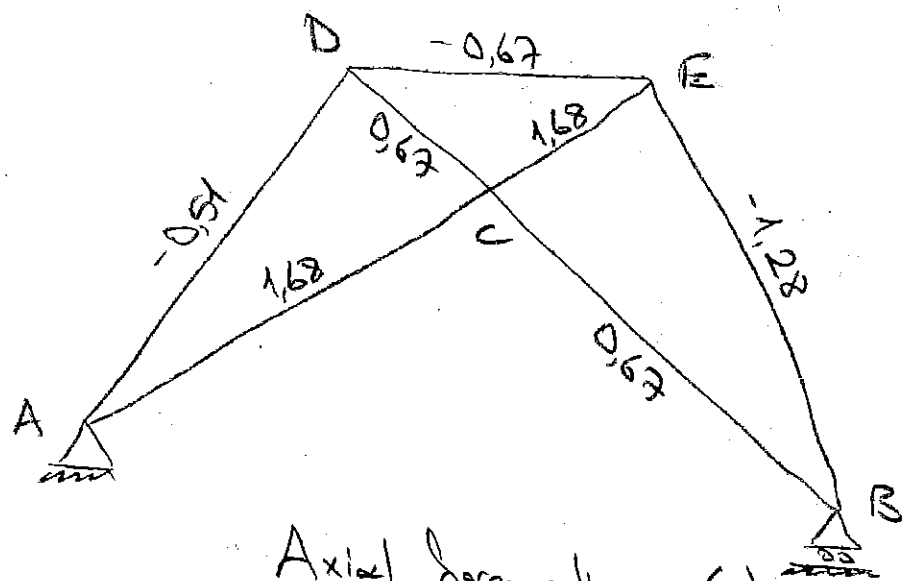


Axial force diagram (N)

Apply a unit force at point E.



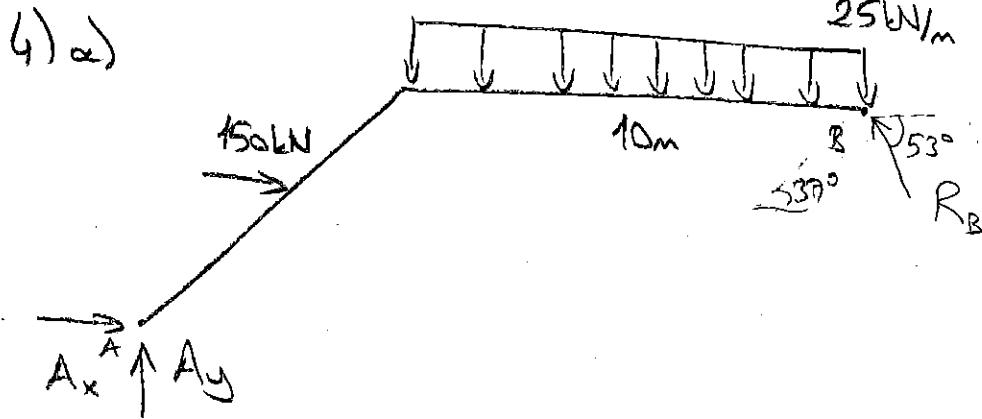
The internal forces due to the above fictitious load are



Member	Axial force diagram (n)			
	L (m)	N (kN)	n	N.n.L
AC	3,833	153,2	1,68	2547,77
AD	10,77	-137,5	-0,51	1084,81
BC	3,833	117,3	0,67	781,95
BE	10,77	-224,4	-1,28	3.033,43
DE	6	-131,7	-0,67	770,63
DC	4,243	117,3	0,67	335,17
CE	4,243	153,2	1,68	1.032,05

$$1. \Delta = \sum_{i=1}^{n=2} \frac{N_i \cdot \Delta L_i}{E \cdot A_i}$$

$$\Delta = \frac{9.705,87}{EA} \quad (\rightarrow)$$



$$\sum M_A = 0 \Rightarrow 0,6 \cdot R_B \cdot 8 + 0,8 \cdot R_B \cdot 16 - 25 \cdot 10 \cdot 11 - 150 \cdot 6 = 0$$

$$17,6 R_B = 3.350$$

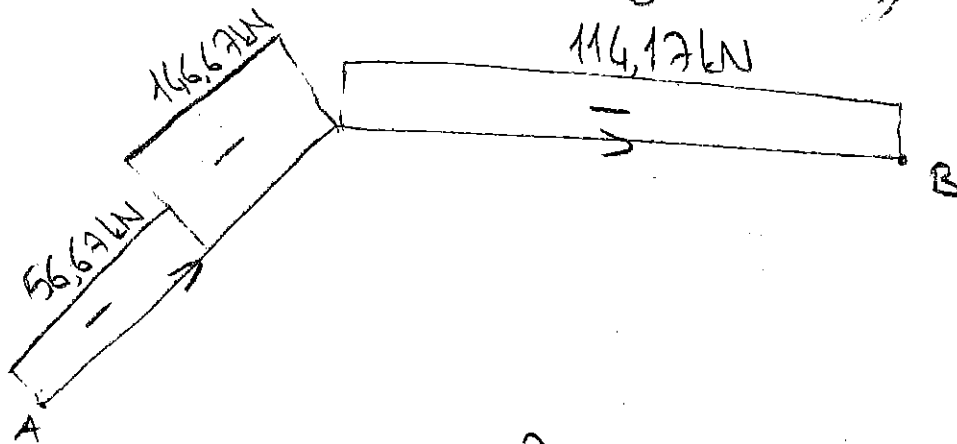
$$R_B = 190,33 \text{ kN}$$

$$\sum F_x = 0 \Rightarrow A_x + 150 - 0,6 \cdot R_B = 0$$

$$A_x = -35,83 \text{ kN}$$

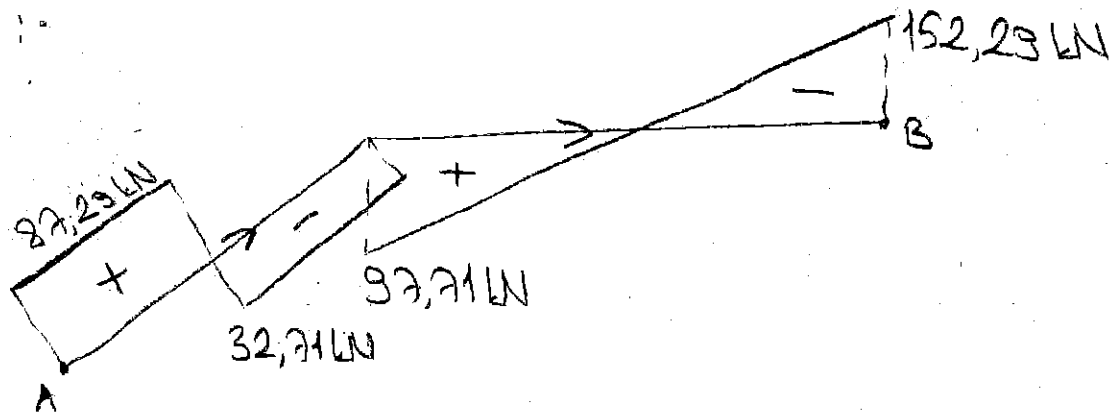
$$\sum F_y = 0 \Rightarrow A_y + 0,8 \cdot R_B - 25 \cdot 10 = 0$$

$$A_y = 97,74 \text{ kN}$$

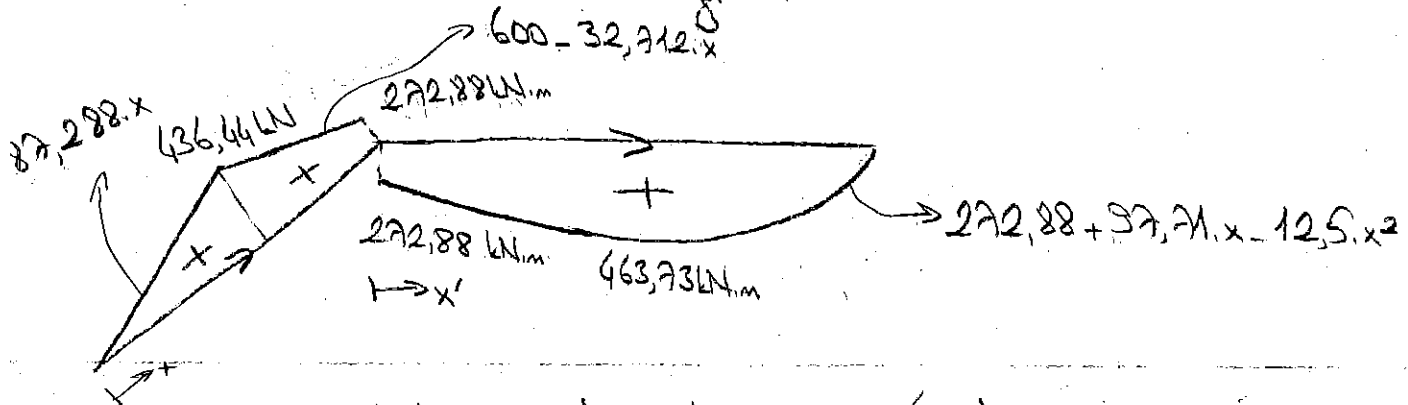


$\left( \begin{array}{c} \uparrow \\ \boxed{+} \\ \downarrow \end{array} \right)$   
 Positive sign convention

Axial force diagram (N)

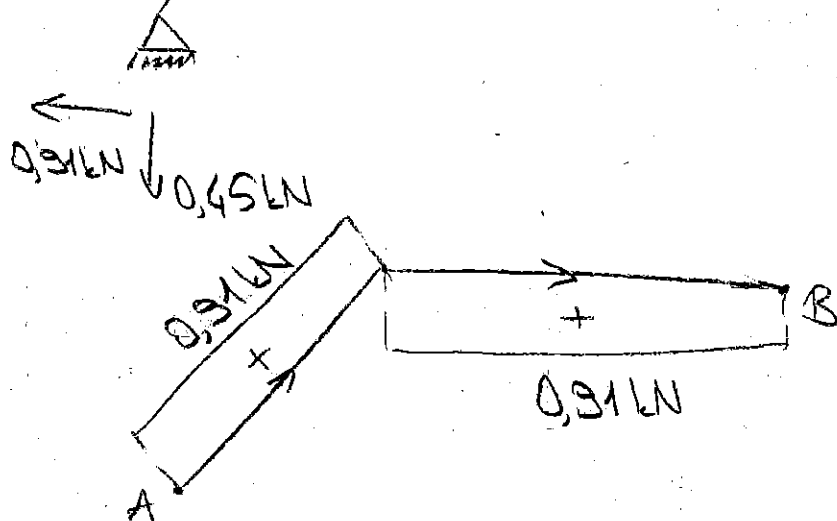
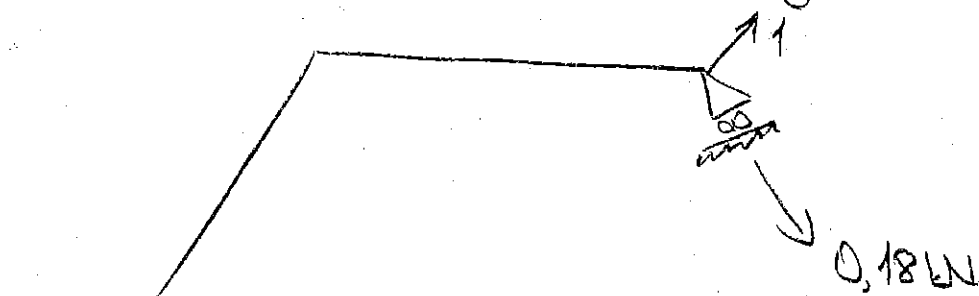


Shear diagram

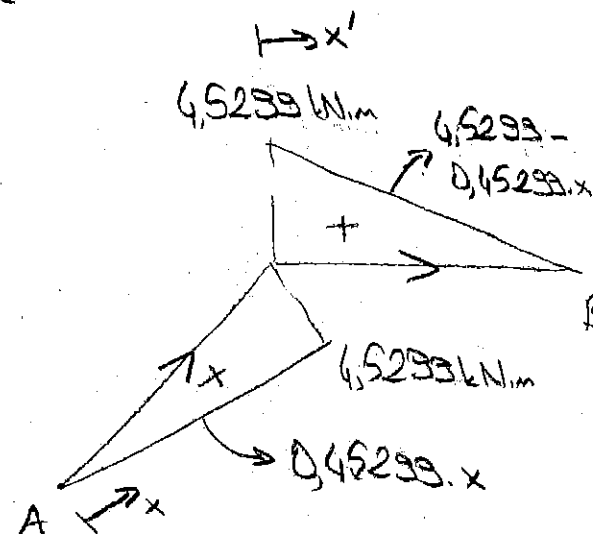


Moment diagram (M)

b) Apply a fictitious load of 1 along support B.

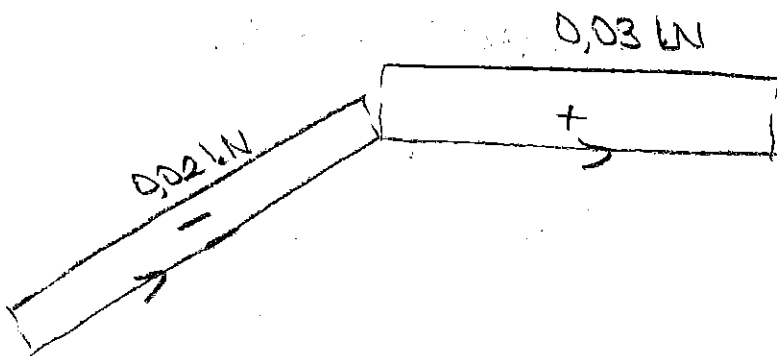
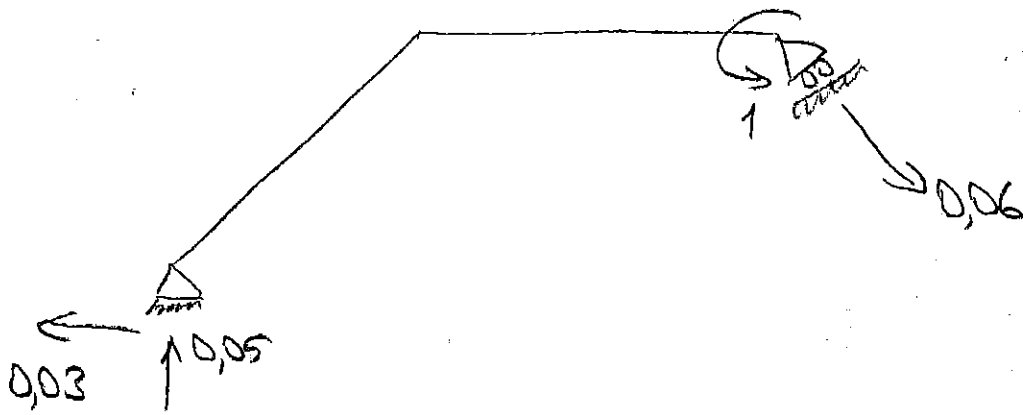


Axial force diagram ( $n_1$ )

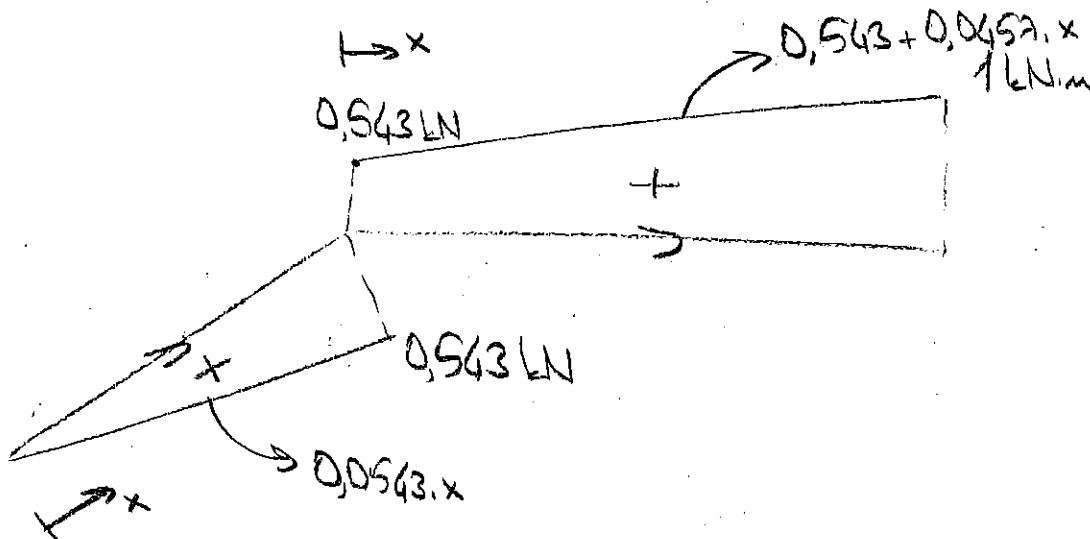


Moment diagram ( $m_1$ )

Apply a fictitious moment of 1 at support B.



Axial force diagram ( $n_2$ )



Moment diagram ( $m_2$ )



The displacement at the skew roller support B is

$$1. \Delta = \int = \int \frac{M_1 m_1 dx}{EI}$$

$$\Delta = \int_0^5 \frac{87,288 \cdot x \cdot 0,453 \cdot x dx}{EI} + \int_5^{10} \frac{(600 - 32,712 \cdot x) \cdot 0,453 \cdot x dx}{EI} +$$

$$\int_0^{10} \frac{(272,88 + 97,71x - 12,5x^2) \cdot (4,53 \cdot 0,453 \cdot x) dx}{EI}$$

$$\Delta = \frac{1}{EI} \cdot \left( 13,180 \cdot x^3 \Big|_0^5 + (135,89x^2 - 5,694 \cdot x^3) \Big|_5^{10} + \right.$$

$$\left. (1.236,12 \cdot x + 15350 \cdot x^2 - 33,63 \cdot x^3 + 1,42 \cdot x^4) \Big|_0^{10} \right)$$

$$\Delta = \frac{1}{EI} \cdot (1647,52 + 5.870,3 + 8.838,89)$$

$$\Delta = \frac{16.356,71}{EI} \quad (\rightarrow)$$

The rotation at the skew roller support B is

$$1. \theta = \int = \int \frac{M_1 m_2 dx}{EI}$$

$$\theta = \int_0^5 \frac{87,288 \cdot x \cdot 0,0543 \cdot x dx}{EI} + \int_5^{10} \frac{(600 - 32,712 \cdot x) \cdot 0,0543 \cdot x dx}{EI} +$$

$$\int_0^{10} \frac{(272,88 + 97,71x - 12,5x^2) \cdot (0,543 + 0,0453 \cdot x) dx}{EI}$$

$$\theta = \frac{1}{EI} \cdot \left( 1,58 \cdot x^3 \Big|_0^5 + (-0,592 \cdot x^3 + 16,29) \Big|_5^{10} + (148,2 \cdot x + 32,76 \cdot x^2 - 0,77 \cdot x^3 - \right. \textcircled{9}$$

$$0,143 \times 4 \Big|_0^{10} = \frac{197,49 + 703,67 + 2555,92}{EI} = \frac{3,45708}{EI} \quad (\nearrow)$$

c) Include the axial deformations and recalculate the displacement

$$\begin{aligned} 1. \Delta &= \int = \int \frac{M \cdot m_1 \cdot dx}{EI} + \int \frac{N \cdot n_1 \cdot dx}{EA} \\ &= \frac{16,356,71}{EI} + \frac{-56,67 \cdot 0,9315}{EA} + \frac{-146,67 \cdot 0,9315}{EA} + \frac{-114,12 \cdot 0,931 \cdot 10}{EA} \\ &= \frac{16,356,71}{EI} - \frac{7,856,58}{EI} \\ &= \frac{8,500,13}{EI} \quad (\nearrow) \end{aligned}$$

The rotation is

$$\begin{aligned} 1. \theta &= \int = \int \frac{M \cdot m_2 \cdot dx}{EI} + \int \frac{N \cdot n_2 \cdot dx}{EA} \\ &= \frac{3,45708}{EI} + \frac{-56,67 \cdot -0,025}{EA} + \frac{-146,67 \cdot -0,025}{EA} + \frac{-114,12 \cdot 0,03 \cdot 10}{EA} \\ &= \frac{3,45708}{EI} - \frac{55,668}{EI} \\ &= \frac{3,401,41}{EI} \end{aligned}$$