

CE 382 - Reinforced Concrete Fundamentals

HOMEWORK 3

- 1) For C25 & S420 $f_{cd} = 17 \text{ MPa}$, $f_{yd} = 365 \text{ MPa}$.
 $k_m = 0.776$ $k_m = 133 \text{ mm}^2/\text{kN}$
 $k_p = 0.86$ $k_p = 291 \text{ mm}^2/\text{kN}$

K101 (Flanged Section)

$$K = \frac{b d^2}{M_d} = \frac{1000 \times 460^2}{275000} = 763 > k_p \quad \text{O.K.}$$

$$j'd = 0.9d = 414 \text{ mm} \rightarrow \text{Use this}$$

$$j'd = d - \frac{z}{2} = 390 \text{ mm}$$

$$A_s = \frac{M_d}{f_{yd} j'd} = \frac{275000}{0.365 \times 414} = 1820 \text{ mm}^2 \rightarrow \begin{array}{l} 4\phi 20 \text{ Bent} = 1256 \text{ mm}^2 \\ 4\phi 14 \text{ Straight} = 616 \text{ mm}^2 \\ \hline 1872 \text{ mm}^2 \checkmark \end{array}$$

Support 1 (Rectangular)

$$M_d = 245 - \frac{250 \times 0.4}{3} = 211.7 \text{ kNm}$$

$$K = \frac{300 \times 460^2}{211700} = 300 > k_p \quad \text{O.K.}$$

$$A_s = \frac{211700}{0.365 \times 0.86 \times 460} = 1466 \text{ mm}^2 \rightarrow \begin{array}{l} \text{Available K101 - } 4\phi 20 = 1256 \text{ mm}^2 \\ \text{Hanger - } 2\phi 12 = 226 \text{ mm}^2 \\ \hline 1482 \text{ mm}^2 \checkmark \end{array}$$

K102 (Flange)

$$K = \frac{1000 \times 460^2}{100000} = 2116 \text{ mm}^2/\text{kN} > K_e \quad \text{O.K.}$$

$$A_s = \frac{100000}{0.365 \times 414} = 662 \text{ mm}^2 \rightarrow \begin{array}{l} 2\phi 14 \text{ Straight} = 308 \text{ mm}^2 \\ 2\phi 20 \text{ Bent} = 628 \text{ mm}^2 \\ \hline 936 \text{ mm}^2 \checkmark \end{array}$$

Support 2 (Rect.)

$$M_d = 330 - \frac{270 \times 0.4}{3} = 294 \text{ kNm}$$

$$K = \frac{300 \times 460^2}{294000} = 216 < K_e \quad \text{Need Double Reinforcement}$$

$$M_1 = \frac{300 \times 460^2}{291} = 218.14 \text{ kNm} \quad M_2 = M_d - M_1 = 75.86 \text{ kNm}$$

$$A_{s1} = \frac{218140}{0.365 \times 0.86 \times 460} = 1511 \text{ mm}^2 \quad A_{s2} = A_s' = \frac{M_2}{f_{yd}(d-d')} = 435 \text{ mm}^2$$

$$A_s = 1511 + 435 = 2006 \text{ mm}^2$$

$$\begin{array}{l} \text{Top} \rightarrow \begin{array}{l} \text{K101} - 4\phi 20 \text{ Bent} = 1256 \text{ mm}^2 \\ \text{K102} - 2\phi 20 \text{ Bent} = 628 \text{ mm}^2 \\ \text{Hanger} - 2\phi 12 = 226 \text{ mm}^2 \\ \hline 2110 \text{ mm}^2 \checkmark \end{array} \end{array}$$

$$\begin{array}{l} \text{Bottom} \rightarrow \begin{array}{l} \text{K101} - 4\phi 14 \text{ Straight} = 616 \text{ mm}^2 \\ \text{K102} - 2\phi 14 \text{ Straight} = 308 \text{ mm}^2 \\ \hline 924 \text{ mm}^2 > A_s' \checkmark \end{array} \end{array}$$

K103 (Flange)

$$K = \frac{1000 \times 460^2}{200000} = 1058 > K_e \quad \text{O.K.}$$

$$A_s = \frac{200000}{0.365 \times 414} = 1324 \text{ mm}^2 \rightarrow \begin{array}{l} 4\phi 20 \text{ Bent} = 1256 \text{ mm}^2 \\ 2\phi 14 \text{ Straight} = 308 \text{ mm}^2 \\ \hline 1564 \text{ mm}^2 \checkmark \end{array}$$

Support 3

$$M_d = 300 - \frac{195 \times 0.4}{3} = 276 \text{ kNm}$$

$$K = \frac{300 \times 460^2}{276000} = 230 < K_e \quad \text{Double Reinf.}$$

$$M_1 = \frac{300 \times 460^2}{291} = 218.14 \text{ kNm}$$

$$M_2 = M_d - M_1 = 57.86 \text{ kNm}$$

$$A_{s1} = \frac{218140}{0.365 \times 0.86 \times 460} = 1511 \text{ mm}^2$$

$$A_{s2} = A_s' = \frac{57860}{0.365 \times 420} = 378 \text{ mm}^2$$

$$A_s = A_{s1} + A_{s2} = 1889 \text{ mm}^2$$

Top \rightarrow K102 - 2 ϕ 20 Bent \pm = 628 mm²

K103 - 4 ϕ 20 Bent \pm = 1256 mm²

Hanger - 2 ϕ 12 = 226 mm²

+ 2110 mm² ✓

Bottom \rightarrow K102 - 2 ϕ 14 str = 308 mm²

K103 - 2 ϕ 14 str = 308 mm²

+ 616 mm² ✓

Support 4

$$M_d = 177 - \frac{170 \times 0.4}{3} = 154.3 \text{ kNm}$$

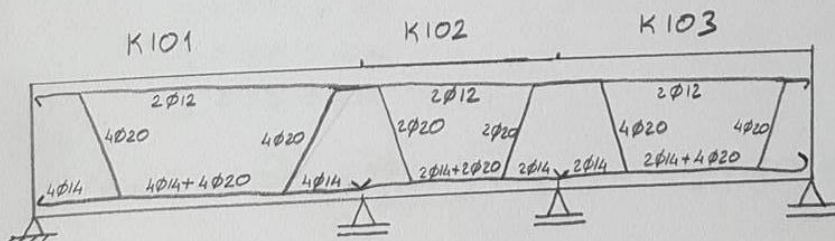
$$K = \frac{300 \times 460^2}{154300} = 411 > K_e \quad \text{O.K.}$$

$$A_s = \frac{154300}{0.365 \times 0.86 \times 460} = 1069 \text{ mm}^2$$

\rightarrow K103 - 4 ϕ 20 Bent \pm = 1256 mm²

Hanger - 2 ϕ 12 = 226 mm²

+ 1482 mm² ✓



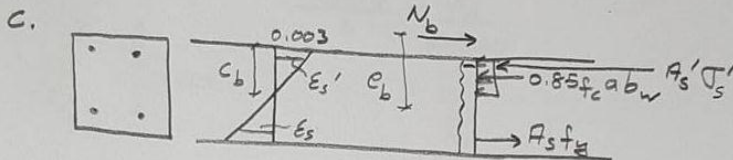
$$2) a. N_{or} = 0.85 f_{cd} A_c + A_s f_{yd} = 0.85 \times 17 \times (400 \times 400) + 4 \times \frac{\pi 24^2}{4} \times 365$$

$$N_{or} = 2972.5 \text{ kN}$$

b. Tension Capacity

$$\text{Direct Tensile} \rightarrow f_{ctd} = 0.35 \sqrt{17} = 1.44 \text{ MPa}$$

$$N_t = f_{ctd} \times A_c + A_s f_{yd} = 891 \text{ kN}$$



$$\text{At balance case } \epsilon_s = \epsilon_y = \frac{365}{200000} = 0.00183$$

$$\frac{0.003}{c_b} = \frac{0.00183}{360 - c_b} \rightarrow c_b = 223.6 \text{ mm}$$

$$\frac{0.003}{c_b} = \frac{\epsilon_s'}{c_b - 40} \rightarrow \epsilon_s' = 2.5 \times 10^{-3} > \epsilon_y \quad \text{Comp. steel yields}$$

$$N_b + A_s f_y = 0.85 f_c a b_w + A_s' \sigma_s'$$

$$N_b = 1218 \text{ kN}$$

$$M_b = 2 \times (A_s f_y) \times (160) + (0.85 f_c a b_w) \times (200 - \frac{a}{2}) = \boxed{221 \text{ kNm}}$$

$$d. F_c = 0.85 \times 17 \times 0.85 \times 400 = 4913 \text{ N}$$

$$F_s = 2 \times \frac{\pi 24^2}{4} \times 365 = 330244 \text{ N}$$

$$F_s' = 2 \times \frac{\pi 24^2}{4} \times \sigma_s' = 304.8 \times 200000 \times (0.003 - \frac{0.12}{c}) = 542880 - \frac{21715200}{c}$$

$$N = 0 = F_c + F_s' - F_s \rightarrow c = 48.35 \text{ mm}$$

$$F_c = 237.5 \text{ kN}$$

$$F_s = 330.2 \text{ kN}$$

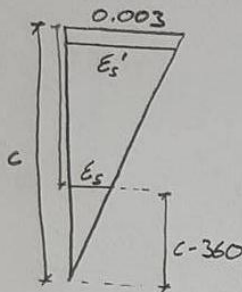
$$F_s' = 93.7 \text{ kN}$$

$$M = F_c \times (200 - \frac{0.85 \times c}{2}) + F_s \times 160 + F_s' \times 160 = \boxed{110.4 \text{ kNm}}$$

e. $N = 0.85 N_{or} = 0.85 \times 2972.5 = 2527 \text{ kN}$

$N > N_b \rightarrow$ Compression Failure

Assume all section is in compression.



$\epsilon_s' > \epsilon_y$ by definition.

$$\frac{0.003}{c} = \frac{\epsilon_s}{c-360}$$

$$F_c = 0.85 \times 17 \times 0.85c \times 400 = 4913c$$

$$F_s' = 365 \times 2 \times \frac{\pi 24^2}{4} = 330252$$

$$F_s = 2 \times \frac{\pi 24^2}{4} \times 200000 \times \left(0.003 - \frac{1.08}{c}\right) = 542880 - \frac{135436800}{c}$$

$$N = 2527000 = F_c + F_s' + F_s \rightarrow c = 429.3 \text{ mm}$$

$c > h$ Assumption is true.

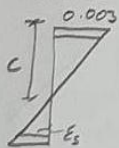
$$\left. \begin{array}{l} F_c = 2109 \text{ kN} \\ F_s' = 330.2 \text{ kN} \\ F_s = 87.7 \text{ kN} \end{array} \right\}$$

$$M = F_c \times \left(200 - \frac{a}{2}\right) + F_s' \times 160 + F_s \times 160 = \boxed{103.9 \text{ kNm}}$$

f. $N = 0.45 N_{or} = 1337.6 \text{ kN}$

$N > N_b \rightarrow$ Compression Failure

Assume bottom steel is under tension.



$$\frac{0.003}{c} = \frac{\epsilon_s}{360-c}$$

$$F_c = 4913c$$

$$F_s' = 330252$$

$$F_s = \frac{135436800}{c} - 542880$$

$$N = F_c + F_s' - F_s$$

$$\rightarrow c = 252.2 \text{ mm}$$

$c < h$ Assumption is true

$$F_c = 1239 \text{ kN}$$

$$F_s' = 330.2 \text{ kN}$$

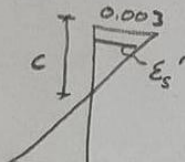
$$F_s = 232 \text{ kN}$$

$$M = F_c \times \left(200 - \frac{a}{2}\right) + 160 \times [F_s + F_s'] = \boxed{205 \text{ kNm}}$$

$$R \cdot N = 0.15 N_{or} = 0.15 \times 2972.5 = 446 \text{ kN}$$

$N < N_b \rightarrow$ Tension Failure

Assume $\sigma_s = f_y$ and $\sigma_s' < f_y$



$$\frac{0.003}{c} = \frac{\epsilon_s'}{c-40} \quad \epsilon_s' = 0.003 - \frac{0.12}{c}$$

$$F_c = 4913c$$

$$F_s = 330244$$

$$F_s' = A_s' \times 200000 \times \epsilon_s' = 542880 - \frac{21715200}{c}$$

$$N = F_c + F_s - F_s' \rightarrow c = 75.7 \text{ mm}$$

From compatibility $\epsilon_s' = 0.0014 < \epsilon_y$
 $\epsilon_s = 0.011 > \epsilon_y$

Assumption is true.

$$F_c = 371.9 \text{ kN}$$

$$F_s = 330.2 \text{ kN}$$

$$F_s' = 256 \text{ kN}$$

$$M = F_c \times \left(200 - \frac{0.85c}{2}\right) + 160(F_s + F_s') = 156.2 \text{ kNm}$$

