## CE 382 - Reinforced Concrete Fundamentals HOMEWORK 2

1) a) 
$$\nabla_{c} = 13200 E_{c}$$
 $N_{c} + N_{s \pm} = 1500 \text{ kN}$ , From compatibility  $E_{c} = E_{s} = E$ 
 $N_{c} = \nabla_{c} \times A_{c}$ 
 $N_{c} = 13200 E$ 
 $N_{s \pm} = \nabla_{s \pm} \times A_{s \pm}$ 
 $\nabla_{s \pm} = 200000 E$ 
 $N_{c} = (19200 E) \times (300 \times 300) = 1728000 E \text{ kN}$ 
 $N_{s \pm} = (200000 E) \times (6 \times 5120^{2}) = 377000 E \text{ kN}$ 
 $E = 7.126 \times 10^{4}$ 

check 
$$\sqrt{s_{\pm}} = 200000E \le f_{4}$$

No = (26480 E - 3642080 E2) x (300x300) = 2383 200 E - 327 787 200 E2 LN Not = 377 000 E KN

-327 787 200 
$$E^2$$
 + 2760200  $E$  - 1500 = 0  $\Longrightarrow$   $E_{1,2} = \frac{-b \mp \sqrt{\Delta}}{2a}$ 

$$E_1 = 7.84 \times 10^{-3} \longrightarrow E_1 \text{ is not possible because it creates the maximum }$$

$$E_2 = 6.84 \times 10^{-4}$$

$$\text{Stress in steel which is fy and the equality}$$

$$E = 5.84 \times 10^{-4}$$

c) Assuming linear-elastic behaviour for concrete makes the stress in concrete smaller. As can be seen from the graph on the question sheet, for to=30 MPa & reaches higher value for non-linear curve, which increases its capacity.

e) Higher concrete strength and larger steel area causes higher axial load capacity.

$$f_{cc} = 0.85 \times 35 + 6 \times 2 \times 420 \times \frac{3112^2}{4} \times \frac{1}{250 \times 5} = 29.75 + \frac{2280}{5}$$

5) a) compatibility Est = E = E T = 200000 E MPa TC=8MPG Hognestad model -> E= 12680 + 460 fc = 28780 MPa Eco = 2fc = 2.43x10-3 Te=fe[2 Ec - (Ec)2]  $8 = 35 \left[ \frac{2E}{243 \times 10^{-3}} - \left( \frac{E}{243 \times 10^{-3}} \right)^{2} \right] \longrightarrow E = 3 \times 10^{-4}$ Ts = 60 MPa, T = 8MPa

N= 8x(400x600) + 60x 10 51202 = 2108.5 LN

b) T = 28 MPa, F = 28780 MPa, E = 2.43 × 10-3

 $28 = 35 \left[ \frac{2E}{0.00243} - \left( \frac{E}{0.00243} \right)^{2} \right] \implies E = 1.34 \times 10^{-3}$ 

Tst = 268 MPa, Tc = 28 MPa

N= 28x (400x600) + 268 x 10 51202 = 7562 kN

c) Ts = 420 MPa

420 = Ex 200 000 -> E = 2.1 × 10-3

Vc = 35 [ 2x2.1 - (2.1 )2 ] = 34.35 MPG

N=34.35 x (400×600) + 420×10 x 7202 = 3563.5 EN

d) Nor = 35x(400x600) + 420x10x31202 = 19719.5 LN

e) Direct tensile -> fct = 0.35 \fc = 2.1 MPa

Nor = 2.1 x (400x600) = 504 kN

6) 200

0.003

0.85 fe

$$k_1c_b$$
 $E_s = E_s$ 
 $E_s = E_$ 

$$\frac{c}{660} = \frac{0.003}{0.003 + E_S} \quad \text{and} \quad 0.85 f_c k_1 c b_w = A_S f_W$$

$$0.85 \times 30 \times 0.85 \times C \times 400 = 6 \times \frac{3122^2}{4} \times 420$$

 $\mathcal{E}_{s} > \mathcal{E}_{y} = 0.0021$   $\mathcal{E}_{s} = 0.015$   $\mathcal{E}_{s} = 0.015$   $\mathcal{E}_{s} = 0.015$ 

$$M_r = R_{5d} \times f_{4} \times \left(d - \frac{k_1 c}{2}\right) = 6 \times \frac{3722^2}{4} \times 420 \times \left(660 - \frac{0.85 \times 110.5}{2}\right)$$

Mr = 587.3 KNm



