

# Computer Methods in Applied Mechanics and Engineering

Enhanced Material Point Method to face dynamic problems: Local-maximum entropy approximation and explicit predictor-corrector scheme

On the dynamic assessment of the Local-Maximum Entropy Material Point Method through an Explicit Predictor-Corrector Scheme

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**Abstract**

Material Point Method (MPM) has arisen in the recent years as an alternative to Finite Element Method (FEM) under the large deformation regime. However, the simulation of shock waves propagation and other high frequency problems is still challenging under this approach due the incapability of the standard MPM time integration scheme to filter spurious noises. To overcome this limitation in this paper, an Explicit Predictor-Corrector time integration scheme has been proposed. Its **powerful** performance mitigates the presence of spurious oscillations with minimal dissipation in high frequency problems. Other source of numerical noise in MPM occurs when the material points cross computational grid boundaries, being this issue motivated by the lack of smoothness of the interpolation functions. This noise results in spurious local variations at the material points, where strain-stress fields are computed. This could lead to inaccurate solutions as well as aborted simulations in the worst cases. To overcome it, this document adopts the Local Maximum-Entropy (LME) approximation schemes as a robust substitute of the traditional shape function in MPM. LME approximation may be regarded as a *thermalization* of Delaunay triangulation which resolves the degenerate cases resulting from the lack or uniqueness of the triangulation. Furthermore, by modifying a regularization parameter they are able to behave finite element like or as a meshfree method. This capability allows to

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face a wide range of physics with a single shape function family. Finally this paper demonstrates the performance of both improvements thorough numerical examples.

*Keywords:* LME, MPM, Explicit predictor-corrector, Dynamic problems

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## 1. Introduction

Since the proposal of MPM by Sulsky *et al.* (1994) [1] as a generalization to solids of the Fluid Implicit Particle (FLIP) method [2], its popularity has increased due to its ability to deal with large strain regime without suffer  
5 mesh distortion inaccuracies. One of the main fields where this method is strong is the dynamic one since the original time integration scheme proposed was the Forward Euler (FE) [1], carried out in a explicit manner. However, in this type of problems, the main instabilities of the original MPM are even marked.

10 On the one hand, the main source of instability occurs when material points cross cell boundaries. This provoked the development of other interpolation techniques to overcome this limitation such as the Generalized Interpolation Material Point (GIMP) method Bardenhagen & Kober (2004) [3], which has demonstrated to have a good performance in the finite  
15 deformation regime. However, in the absence of a regular grid, construction of the weighting functions is only achieved at considerable effort and computational cost. Furthermore, as it is a voxel-based discretization technique, it is prone to suffer voxel domains overlap or gaps when the material point mesh becomes irregular, which can introduce severe inaccuracies as noticed  
20 Steffen *et al.* (2008)[4]. This is similar to the difficulty encountered by the finite element methods due to element distortion. A more robust alternative is the Dual Domain Material Point (DDMP) method proposed by Zhang *et al.* (2011) [5]. Unfortunately this method shows an unsatisfactory behaviour when particle/cell ratio decreases [6]. Therefore DDMP a large number of  
25 particles is needed for convergence, what makes the method very expensive. In recent years the employment of spline-lines as shape functions has gain popularity with the introduction of the B-Spline MPM proposed by Roel Tielen *et al.* (2017) [7], this technique allows the employment of unstructured set of nodes and particles. More recently, approximants derived from  
30 minimization has been introduced in to the MPM framework with the Conservative Taylor Least Squares (CTLTS) reconstruction proposed by Wobbes *et al.* (2018) [8]. Unfortunately, when particles are spread in a challenging way, the quality of the CTLTS approximation decreases locally.

This document adopts the Local Maximum-Entropy (LME), or Local

35 *Max-Ent* approximates, as a robust substitute of the aforementioned shape  
 functions in MPM. First introduced by Arroyo & Ortiz (2006) [9], it belongs  
 to the class of convex approximation schemes and provides a seamless transi-  
 tion between FEM and meshfree interpolations. The approximation scheme  
 is based on a compromise between minimizing the width of the shape func-  
 40 tion support and maximizing the information entropy of the approximation.  
 The LME approximation may be regarded as a regularization, or *thermaliza-  
 tion*, of Delaunay triangulation which effectively resolves the degenerate cases  
 resulting from the lack of uniqueness or the triangulation. LME basis func-  
 tions possess many desirable properties for meshfree algorithms. First of all,  
 45 they are entirely defined by the nodal set and the domain of analysis. They  
 are also non-negative, satisfy the partition of unity property, and provide an  
 exact approximation for related functions [9]. Important contributions on  
 the Maximum-Entropy have been made by Sukumar and coworkers [10] with  
 Cell-based techniques and the ones carried within the Optimal Transporta-  
 50 tion Meshfree (OTM) method. The latter methodology has been proven to  
 have a good performance under the dynamic regime by other researchers,  
 being important the contributions of Li *et al.* (2012) [11] and Navas *et al.*  
 (2018) [12, 13] in the explicit regime and Navas *et al.* [14, 15, 16] and Wrig-  
 gers and coworkers [17] with implicit schemes. And more recently, under  
 55 MPM framework, the work made by Wobbes *et al.* (2020) [18]. The proposed  
 research delves into the benefits of the regularization parameter,  $\beta$ , and the  
 analogy of the different obtained shape functions derive by the tuning of this  
 parameter and the traditional MPM ones.

The aforementioned techniques are devoted to mitigate the “grid cross-  
 60 ing” error. Nevertheless, in the presence of shock waves spurious, numerical  
 noises appear despite of the employment of these techniques [19]. These  
 numerical inaccuracies, also known as wiggles, arise due to inaccuracies in  
 the time discretization technique. A simple approach to face those spurious  
 noises lies on the addition of nonphysical damping sources to the equilib-  
 65 rium equations. This approach has been widely employed in this and many  
 other numerical techniques. To avoid introducing this nonphysical sources,  
 many researchers has proposed alternative time integration schemes which  
 reduce the presence of high frequency noises by filtering them or increasing  
 somehow the accuracy. One of the first attempts of it was the proposal  
 70 of an implicit time integration scheme by Guilkey & Weiss [20]. More re-  
 cently Wang *et al.* [21] to downsize this spurious noises add a nonviscous  
 damping to the linear momentum balance equation, and later Charlton *et al.*  
 [22] extended this scheme to the GIMP approach introducing the Implicit  
 GIMP (iGIMP) method. However, the local damping introduced by [21] can  
 75 totally over-damp the solution in time-dependent simulations such as in con-

solidation process. Within the explicit time integration schemes, Lu *et al.* [23] introduced the time-discontinuous Galerkin method to control the spurious noises propagation, and later Tran & Solowski [19] proposed a generalised- $\alpha$  scheme for MPM with promising results but at the expense of increasing the computational effort. In this paper a less time consuming and high efficient explicit predictor-corrector integration method has been proposed. It consists in an accommodation of the traditional npc scheme, widely employed in Finite Element methods. This method has been chosen among other suitable alternatives as those proposed by Wilson *et al.* (1972) [24] or Chung & Hulbert (1993) [25] because its simplicity and its good performance dealing with solid dynamic problems under a meshfree framework in [13].

The aim of this document is to mitigate the spurious oscillations due to inaccuracies in both space and time discretization by the employment of a suitable combination of the LME family shape functions, and the proposal of an explicit predictor-corrector scheme. The advantages of this approach will be illustrated through several challenging test cases **on the elastic regime**: the propagation of shock waves in a elastic bar and the response of a block of soil gradually loaded with gravitational forces.

The article is organised as follows. Section 2 is devoted to describe the meshfree methodology adopted in this research, first MPM procedure is introduced in 2.1, second the explicit predictor-corrector time integration scheme is presented in 2.2, and third LME approximation scheme will be introduced in 2.3. In Section 3 applications to prove the numerical accuracy of the proposed approach are presented. Finally, conclusions and future research topics are exposed in Section 4.

## 2. The meshfree methodology

The aim of this section is to provide an overview of the standard explicit MPM algorithm [1]. Without losing generality, the method is composed by three main steps: (i) a variational recovery process, where particle data is projected to the grid nodes, (ii) an Eulerian step, where balance of momentum equation is expressed as a nodal equilibrium equation through a FEM-like procedure, and finally (iii) a Lagrangian advection of the particles. In consequence, MPM can be regarded as a Lagrangian-Eulerian method where particles carry on all the physical information and a set of background nodes is employed to compute the equilibrium equation. In what follows, we will adopt the following convention. Three kind of subscript or superscript are used within paper. The subscript  $\square_p$  is used to define a particle variable. While the subscript  $\square_I$  is reserved in this notation for denoting nodal variables. And finally, the superscript  $\square^\psi$  involves a virtual magnitude. For

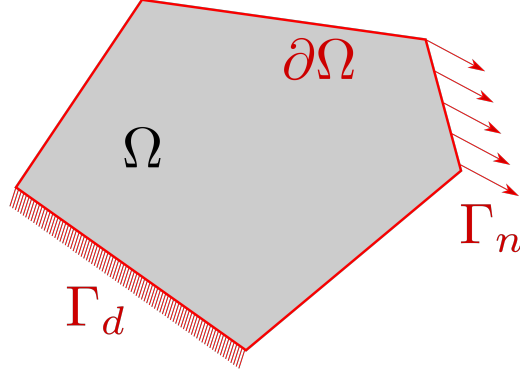


Figure 1: Description of the boundary-value-problem in a continuum. Red lines represents the closure  $\partial\Omega$  of the domain  $\Omega$  represented in gray.

115 the operators, the convention is :  $\dot{\square}$  and  $\ddot{\square}$  for the first and second time derivative,  $\otimes$  means the dyadic operator,  $\square \cdot \square$  and  $\square : \square$  means the first and second contraction of a tensor,  $div(\square)$  denotes the divergence operator, and finally  $grad(\square)$  and  $grad^s(\square)$  denotes the gradient and its symmetric part. Einstein subscripts convention is adopted therefore repeated index means addition. 120 Following, the MPM methodology, the explicit predictor-corrector scheme and LME approximation shape functions are describe in subsection 2.1,2.2 and 2.3 respectively.

### 2.1. Derivation of MPM procedure

In MPM the continuum mechanics approach is considered. So on, let define a continuum  $\Omega$  occupied by an elastic body like the sketched in the figure 1, and  $\partial\Omega$  the boundaries of the domain defined by  $\partial\Omega = \Gamma_d \cup \Gamma_n$  and  $\Gamma_d \cap \Gamma_n = \emptyset$ . In this context the field  $\mathbf{u}$  allows to describe the *global state* of the system. Now the variable  $\phi = (\varepsilon, \sigma)$  is defined as the set of *local states* at any point of the continuum which can be derived from the field  $\mathbf{u}$  through the following set of governing equations and restrictions that must be satisfied. First (i) the *compatibility equation*, where the the strain field  $\varepsilon$  is extracted from  $\mathbf{u}$  is defined as:

$$\varepsilon = grad^s(\mathbf{u}), \quad (1)$$

together with essential boundary conditions of Dirichlet type  $\Gamma_d$ . An additional consideration over the strain field is the assumption of infinitesimal strain, therefore second order terms in the spatial derivatives can be neglected. Therefore, the stress field  $\sigma$  will be considered the corresponding conjugate variable for the strain field, being the one which satisfies (ii) the

conservation of momentum equation:

$$\rho \frac{D\mathbf{v}}{Dt} = \text{div}(\sigma) + \rho \mathbf{b} \quad (2)$$

together with the natural boundary conditions of the Neumann type  $\Gamma_n$ . An additional component will be (iii) the constitutive equation as a linear application from  $\mathfrak{R}^n$  to  $\mathfrak{R}^n$ , which relates the strain tensor with the stress tensor:

$$\sigma = \mathbf{D} : \varepsilon. \quad (3)$$

In this research, plane strain Linear Elasticity has been considered. Thus the constitutive tensor,  $\mathbf{D}$ , is the well known linear elastic one. The final restriction is (iv) the mass conservation, which can be obtained by setting to zero the total derivative of the density field,

$$\frac{D\rho}{Dt} = \dot{\rho} + \rho \text{div}(\mathbf{v}) = 0. \quad (4)$$

In order to obtain the variational statement of the problem, let us define a virtual displacement field such that

$$\mathbf{u}^\psi \in \mathcal{H}_0^1(\Omega) = \{\mathbf{u}^\psi \in \mathcal{H}^1 \mid \mathbf{u}^\psi = \mathbf{0} \text{ on } \Gamma_d\}; \quad (5)$$

satisfying that the Cauchy sequences are convergent in  $\Omega$  as well:

$$\int_{\Omega} \mathbf{u}^\psi d\Omega < \infty \quad \text{and} \quad \int_{\Omega} \varepsilon^\psi d\Omega < \infty. \quad (6)$$

The principle of virtual work states that the equilibrium solution to the boundary-value problem of elasticity is the function  $\mathbf{u} \in \mathcal{H}_0^1$  such that, for  $\mathbf{u}^\psi \in \mathcal{H}_0^1$ , the following holds:

$$\int_{\Omega} \rho \left( \frac{d\vec{v}}{dt} - \vec{b} \right) \cdot \vec{u}^\psi d\Omega = \int_{\Gamma_d} \vec{t} \cdot \vec{u}^\psi d\Gamma - \int_{\Omega} \sigma : \varepsilon^\psi d\Omega. \quad (7)$$

125 Thus, equation (7), together with (3) and (4), represents the weak form formulation of the problem.

In order to obtain a finite set of equations, in contrast with the FEM, in MPM a double discretization procedure is performed. First, the continuum  $\Omega$  is discretized with a finite sum of material points (also denominated particles  
130 the manuscript). Each material point represents a part of the discretized domain  $\Omega_p \subset \Omega$  with  $p = 1, 2, \dots, N_p$  where  $N_p$  is the number of particles. The material point location,  $\vec{x}_p$ , is defined at the centroid of each  $\Omega_p$  (see

figure 2 for details). Initial values of position, velocity, mass, volume and stress denoted by  $\vec{x}_p$ ,  $\vec{v}_p$ ,  $m_p$ ,  $V_p$  and  $\sigma_p$  respectively are assigned to each material point, which also owns the virtual displacement field  $\mathbf{u}_p^\psi$ . Therefore, employing the definition of the material integral, where Riemann integral definition [26] is recovered as an addition of a finite set of points, and their volumes are interpreted as quadrature weights. Consequently, individual terms in (7) are solved as follows.

- Acceleration forces :

$$\int_{\Omega} \rho \frac{d\vec{v}}{dt} \cdot \mathbf{u}^\psi d\Omega = \frac{d\vec{v}_p}{dt} \cdot \mathbf{u}_p^\psi m_p. \quad (8)$$

- Internal forces :

$$\int_{\Omega} \sigma : \varepsilon^\psi d\Omega = \sigma_p : \varepsilon_p^\psi V_p. \quad (9)$$

- Body forces :

$$\int_{\Omega} \rho \vec{b} \cdot \mathbf{u}^\psi d\Omega = \vec{b}_p \cdot \mathbf{u}_p^\psi m_p. \quad (10)$$

- Loads :

$$\int_{\Gamma_d} \vec{t} \cdot \mathbf{u}^\psi d\Gamma = \int_{\Gamma_d} \rho \vec{t}^s \cdot \mathbf{u}^\psi d\Gamma = \vec{t}_p^s \cdot \mathbf{u}_p^\psi h^{-1} m_p, \quad (11)$$

where  $h$  is the thickness of the continuum in a 2D case. Following, the aforementioned second discretization procedure appears. A background mesh composed by a finite set of grid points with coordinates  $\vec{x}_I$ ,  $I = 1, 2, \dots, N_n$ , is generated, being  $N_n$  the number of grid nodes. **Spatial derivatives, such as gradients and divergences, are computed through the support of the background mesh.**

Introducing (8), (9), (10), (11) in (7), approximating the displacement field of the particle  $p$  as  $\vec{u}_p = N_{Ip} \mathbf{u}_I$ ,  $\vec{u}_p^\psi = N_{Ip} \mathbf{u}_I^\psi$ , and its gradient as  $\varepsilon_p = (\mathbf{u}_I \otimes \text{grad}(N_{Ip}))^s$ ,  $\varepsilon_p^\psi = (\mathbf{u}_I^\psi \otimes \text{grad}(N_{Ip}))^s$ . nodal balance of forces of the continuum yields:

$$\dot{\vec{p}}_I = \mathbf{m}_{IJ} \dot{\vec{v}}_J = \vec{f}_I^{int} + \vec{f}_I^{ext}, \quad (12)$$



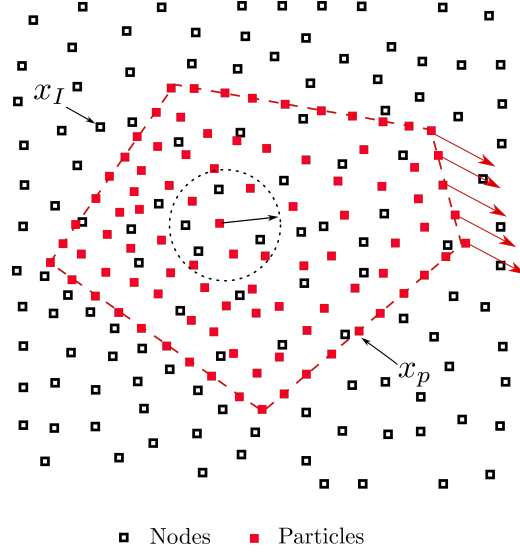


Figure 2: Description of the spatial discretization for domain presented in the figure 1. Blue mesh represent the background computational support, and the red mesh conforms the discretized continuum body.

where  $\dot{\vec{p}}_I$  is the rate of momentum at grid node  $I$  and  $\mathbf{m}_{IJ}$ , the nodal mass matrix, is obtained through:

$$\mathbf{m}_{IJ} = N_{Ip} m_p N_{Jp}. \quad (13)$$

In order to improve the computational efficiency and stability, the nodal mass matrix (13) can be substituted by the lumped mass matrix  $\mathbf{m}_{IJ}^{lumped}$ . Following, internal and external forces are computed as follows,

$$\vec{f}_I^{int} = -\sigma_p \cdot \text{grad}(N_{Ip}) \frac{m_p}{\rho_p} \quad (14)$$

$$\vec{f}_I^{ext} = N_{Ip} \vec{b}_p m_p + N_{Ip} \vec{t}_p^s m_p h^{-1} \quad (15)$$

where  $\sigma_p = \sigma_p(\varepsilon_p)$  is the particle  $p$  stress field, which can be integrated employing the suitable constitutive model. The strain tensor rate,  $\dot{\varepsilon}_p$ , as a measure of the time derivative of the strain tensor, is updated employing the velocity at the background mesh by the equation:

$$\dot{\varepsilon}_p = \frac{\Delta \varepsilon_p}{\Delta t} = \frac{1}{2} [\text{grad}(N_{Ip}) \otimes \vec{v}_I + \vec{v}_I \otimes \text{grad}(N_{Ip})]. \quad (16)$$

Next, mass conservation is guaranteed by enforcing the null value of the material derivative of the density field  $\frac{D\rho}{Dt} = 0$ . This leads to a suitable equation to update the density field:

$$\dot{\rho} = -\rho \text{ trace}(\dot{\varepsilon}). \quad (17)$$

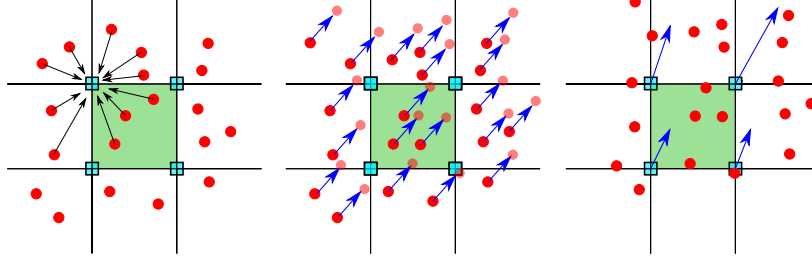


Figure 3: Description of the three steps in MPM standard algorithm.

Finally, to solve the equation (12), a second order **time** integration scheme is required. Therefore, time is discretized into a finite set of time steps  $k = 1 \dots, Nt$ , where  $k$  is the current time step and  $Nt$  is the total number of time steps. Once the nodal equilibrium equation is solved, the values at the nodes are interpolated back into the particles, which are advected to the new position through:

$$\dot{\vec{v}}_p = N_{Ip} \vec{a}_I, \quad \text{and} \quad \dot{\vec{x}}_p = N_{Ip} \vec{v}_I \quad (18)$$

Traditionally, Eqs. (12) and (18), are solved with an explicit forward Euler algorithm. In the following subsection, this and the proposed schemes are described .

## 155 2.2. MPM time integration scheme: the Explicit Predictor-Corrector proposal

As was stated previously, an explicit forward Euler algorithm has been utilized widely within the methodology. This scheme has been described in detail by many researchers [1], [27], [28] and can be sketched by the scheme of the figure 3. Other authors have proposed many others time integration alternatives like [20, 22, 19]. In the first publication on MPM [1], the nodal acceleration was employed to update the particles as

$$\mathbf{v}_p^{k+1} = \mathbf{v}_p^k + \Delta t N_{Ip}^k \vec{a}_I^k \quad (19)$$

$$\mathbf{x}_p^{k+1} = \mathbf{x}_p^k + \Delta t N_{Ip}^k \vec{v}_I^k. \quad (20)$$

However, as Andersen (2009)[28] pointed out, this algorithm has been shown to be numerically unstable due to that  $\mathbf{f}_I^{int,k}$  can be infinite for an infinitesimal nodal mass  $\mathbf{m}$ . This issue may lead to numerical issues when nodal acceleration is obtained in the evaluation of the Eqs. (20) and (19). Hence, a

corrected version of this algorithm was proposed by Zhang *et al.* (2016)[29]:

$$\mathbf{x}_p^{k+1} = \mathbf{x}_p^k + \Delta t \frac{N_{Ip}^k \vec{p}_I^k}{\mathbf{m}_I}, \quad (21)$$

$$\mathbf{v}_p^{k+1} = \mathbf{v}_p^k + \Delta t \frac{N_{Ip}^k \vec{f}_I^k}{\mathbf{m}_I}. \quad (22)$$

Delving into the improvement of the accuracy of the MPM explicit schemes, Tran & Solowski (2019)[19] presented a generalized- $\alpha$  scheme for MPM inspired in the explicit time integration algorithm proposed by Chung & Hulbert (1993)[25], but with the particularity that the acceleration is evaluated both in the beginning and the end of the time step.

$$\mathbf{v}_p^{k+1} = \mathbf{v}_p^k + \Delta t N_{Ip}^k [(1 - \gamma) \mathbf{a}_I^k + \gamma \mathbf{a}_I^{k+1}], \quad (23)$$

$$\mathbf{x}_p^{k+1} = \mathbf{x}_p^k + N_{Ip}^k \left[ \Delta t \vec{v}_I^k + \Delta t^2 \left( \left( \frac{1}{2} - \beta \right) \vec{a}_I^k + \beta \vec{a}_I^{k+1} \right) \right], \quad (24)$$

$$\mathbf{a}_p^{k+1} = N_{Ip}^k \vec{a}_I^{k+1}. \quad (25)$$

This scheme has been proven to damps out the highest frequency noises [19]. However, it can present the same numerical instabilities as in (20),(19) when nodal masses become infinitesimal, and requires extra storage for nodal values of acceleration and previous steps.

In this section, an explicit predictor-corrector time integration scheme is proposed. It is based on the Newmark central differences explicit scheme, which is also denominated a-form  $\gamma = 0.5$  and  $\beta = 0$ . This method is devoted to solve a system of equations of the type

$$\mathbf{M}_{IJ} \ddot{\mathbf{d}}_J + \mathbf{C}_{IJ} \dot{\mathbf{d}}_J + \mathbf{K}_{IJ} \mathbf{d}_J = \mathbf{F}_I.$$

The nodal MPM stage allows to apply this method in the MPM framework in a similar manner that the one proposed by Tran *et al.* [19]. *Taking into account the predictor definition, it is possible to calculate nodal velocities and update particles position employing nodal values of velocity and acceleration.*

The predictor-corrector algorithm has been described in the classic literature [30], and its stability and computational advantages were widely validated by Liu [31]. The “classic” Newmark Predictor-Corrector (NPC) algorithm starts with a predicted value of the nodal velocities at the  $(k + 1)$ th time step, denoted by  $\vec{v}_I^{k+1}$ , which is calculated as follows:

$$\vec{v}_I^{k+1} = \vec{v}_I^k + (1 - \gamma) \Delta t \vec{a}_I^k. \quad (26)$$

The *user-defined* parameter  $\gamma \geq 0$  that appears in In (26), influences both the predictor accuracy and the stability of the algorithm. As pointed out Liu [31], the truncation error of the predictor formula is  $O(\Delta t^3)$  when  $\gamma = 0.5$ , and is unconditionally stable if  $0 < \gamma \leq 0.25$ .

To accommodate this step to MPM framework, it is necessary to get the nodal values of the velocity and acceleration throughout a variational recovery process where particles quantities are transferred to the mesh nodes. This technique arises as a generalization of the super-convergent recovery procedures described by Zienkiewicz & Zhu [32] (ZZ) in the context of FEM. In MPM, Gauss quadratures are not employed. However, integrals are computed following the Riemann integral definition, where each component of the summation corresponds to a particle of the discretization. Also Bardenhagen & Kober [3] proved that through this information-transference technique mass and momentum are conserved. So for a general particle variable  $\Phi_p$ , employing the ZZ technique, it is possible to get its nodal homologous  $\Phi_I$  as:

$$\Phi_I = \frac{m_p N_{Ip} \Phi_p}{m_I}. \quad (27)$$

Therefore, to get an analogous expression for (26) in the context of MPM, the procedure described in the equation (27) is employed, obtainen the following expression:

$$\vec{v}_I^{k+1} = \underbrace{\frac{N_{Ip}^k m_p \vec{v}_p^k}{m_I}}_{\vec{v}_I^k} + (1 - \gamma) \Delta t \underbrace{\frac{N_{Ip}^k m_p \vec{a}_p^k}{m_I}}_{\vec{a}_I^k}. \quad (28)$$

Nonetheless this way of computing the predictor stage can introduce instabilities due to numerical cancellation likewise the original Sulky algorithm. Thankfully, this can be avoided easily by the equivalent formulation proposed as follows:

$$\vec{v}_I^{k+1} = \frac{N_{Ip}^k m_p (\vec{v}_p^k + (1 - \gamma) \Delta t \vec{a}_p^k)}{m_I}. \quad (29)$$

This way of computing the nodal predictor is both numerically stable and minimize the computational effort. Once nodal velocities are obtained, the essential boundary conditions are imposed over  $\Gamma_d$ . And in the following, the “classic” MPM algorithm continues to reach to the equilibrium equation (12). Next, the *corrector* stage is introduced. Due to the fact that nodal velocities were obtained earlier, this step is computed in the same way as in FEM,

$$\vec{v}_I^{k+1} = \vec{v}_I^{pred} + \gamma \Delta t \frac{\vec{f}_I^{k+1}}{\mathbf{m}_I^{k+1}}. \quad (30)$$

Finally updated particle kinetics are computed using nodal values as:

$$\vec{a}_p^{k+1} = \frac{N_{Ip}^k \vec{f}_I^k}{\mathbf{m}_I^k} \quad (31)$$

$$\vec{v}_p^{k+1} = \vec{v}_p^n + \Delta t \frac{N_{Ip}^k \vec{f}_I^k}{\mathbf{m}_I^k} \quad (32)$$

$$\vec{x}_p^{k+1} = \vec{x}_p^n + \Delta t N_{Ip}^k \vec{v}_I^k + \frac{1}{2} \Delta t^2 \frac{N_{Ip}^k \vec{f}_I^k}{\mathbf{m}_I^k}. \quad (33)$$

Notice that particle displacements are computed using the corrected nodal velocities as well as the accelerations with the velocities of the predictor. However, particle velocities and accelerations are computed using the corrected velocities. Therefore here we share similarities with the *leapfrog integration* where position is not updated at full time step, but the velocity is updated at half time steps. Notice also that, with this approach, the calculation of nodal momentum values are not required. Due to its simplicity, the proposed scheme can be implemented with minor modifications over the standard forward Euler. The full implementation is summarized in the algorithm .

### 2.3. Local Max-Ent approximants

The popularity of the MPM has increased notoriously during the recent years due to its ability to deal with large strain problems without mesh distortion issues inherent to mesh based methods like FEM, see Więckowski [33]. However, in the simulations made with the original MPM, numerical noises appear when particles cross the cell boundaries. Solving this issue is the main goal of the employment of the shape functions .

Local Maximum-Entropy (LME) approximation schemes were first introduced by Arroyo & Ortiz (2006)[9] and has been recently tested under MPM framework by Wobbes *et al.* (2020)[18]. The performed simulations of MPM within LME in [18] shows considerably more accurate stress approximations than traditional MPM schemes. However, how the regularization parameter  $\beta$  affects to the accuracy and stability of the solution is not assessed deeply in that research, which is considered interesting in the present manuscript; the tuning of this  $\beta$  parameter allows to make the comparison of the accuracy against analogous traditional MPM shape function.

The basic idea of the shape functions based on such an estimate is to interpret the shape function  $N_I(\vec{x})$  as a probability. This allows us to introduce two important limits: the principle of maximum-entropy (*max-ent*) statistical inference stated by [34], and the Delaunay triangulation which ensures the minimal width of the shape function.

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**Algorithm 1** Newmark Predictor-Corrector (NPC) scheme

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1: **Update mass matrix:**

$$\mathbf{m}_I = N_{Ip}^k m_p,$$

2: **Explicit Newmark Predictor:**

$$\vec{v}_I^{pred} = \frac{N_{Ip}^k m_p (\vec{v}_p^k + (1 - \gamma) \Delta t \vec{a}_p^k)}{m_I}$$

3: **Impose essential boundary conditions:**

At the fixed boundary, set  $\vec{v}_I^{pred} = 0$ .

4: **Deformation tensor increment calculation.**

$$\begin{aligned} \dot{\varepsilon}_p^{k+1} &= \left[ \vec{v}_I^{pred} \otimes \text{grad}(N_{Ip}^{k+1}) \right]^s \\ \Delta \varepsilon_p^{k+1} &= \Delta t \dot{\varepsilon}_p^{k+1} \end{aligned}$$

5: **Update the density field:**

$$\rho_p^{k+1} = \frac{\rho_p^k}{1 + \text{trace} [\Delta \varepsilon_p^{k+1}]}.$$

6: **Balance of forces calculation:**

Calculate the total grid nodal force  $\vec{f}_I^{k+1} = \vec{f}_I^{int,k+1} + \vec{f}_I^{ext,k+1}$  by evaluating (14) and (15) in the time step  $k + 1$ . In those nodes where  $\frac{\partial \mathbf{v}_I^k}{\partial t} \big|_{\Gamma_d} = 0$ , the acceleration is fixed to zero and nodal forces are stored as reactions.

7: **Explicit Newmark Corrector:**

$$\vec{v}_I^{k+1} = \vec{v}_I^{pred} + \gamma \Delta t \frac{\vec{f}_I^{k+1}}{\mathbf{m}_I^{k+1}}$$

8: **Update particles lagrangian quantities:**

$$\begin{aligned} \vec{a}_p^{k+1} &= \frac{N_{Ip}^k \vec{f}_I^k}{\mathbf{m}_I^k} \\ \vec{v}_p^{k+1} &= \vec{v}_p^n + \Delta t \frac{N_{Ip}^k \vec{f}_I^k}{\mathbf{m}_I^k} \\ \vec{x}_p^{k+1} &= \vec{x}_p^n + \Delta t N_{Ip}^k \vec{v}_I^k + \frac{1}{2} \Delta t^2 \frac{N_{Ip}^k \vec{f}_I^k}{\mathbf{m}_I^k} \end{aligned}$$

9: **Reset nodal values**

---

This approximation scheme represents an optimal compromise, in the sense of Pareto, between the *unbiased statistical inference* based on the nodal data which leads to the principle of *Maximum-Entropy* stated by Jaynes [34], and the definition of local shape functions of *least width* the least biased shape functions.

Taking the definition of entropy as a measure of how uncertainty a random variable is averaged on all its possible outcomes. And adopting the Shannon's entropy as the starting point:

$$H(p_1(\vec{x}), \dots, p_n(\vec{x})) = - \sum_{I=1}^{N_n} p_I(\vec{x}) \log p_I \quad (34)$$

where  $p_I(\vec{x})$  is the probability, equivalent to the mentioned shape function  $N_I(\vec{x})$ , satisfying both the zeroth and first-order consistency. The least-biased approximation scheme is given by

$$\begin{aligned} \text{(LME) Maximize } H(N_I) &\equiv - \sum_I^{N_n} N_I(\vec{x}) \log N_I \\ \text{subject to } &\begin{cases} N_I \geq 0, \text{ I}=1, \dots, n \\ \sum_{I=1}^{N_n} N_I = 1 \\ \sum_{I=1}^{N_n} N_I \vec{x}_I = \vec{x} \end{cases} \end{aligned}$$

On the other hand, the control of the shape function width and its decay with distance away from the corresponding nodes is a desirable property. To reach to this objective [9] propose the following linear program,

$$\begin{aligned} \text{(RAJ) For fixed } \vec{x} \text{ minimize } U(\vec{x}_p, N_I) &\equiv \sum_I N_I |\vec{x}_p - \vec{x}_I|^2 \\ \text{subject to } &\begin{cases} N_I \geq 0, \text{ I}=1, \dots, n \\ \sum_{I=1}^{N_n} N_I = 1 \\ \sum_{I=1}^{N_n} N_I \vec{x}_I = \vec{x} \end{cases} \end{aligned}$$

To reach to a compromise between two competing objectives, a Pareto set is

defined by [9] as,

$$\begin{aligned}
& (\text{LME})_\beta \text{ For fixed } \vec{x} \text{ minimize } f_\beta(\vec{x}, N_I) \equiv \beta U(\vec{x}, N_I) - H(N_I) \\
& \text{subject to } \begin{cases} N_I \geq 0, \text{ I}=1, \dots, n \\ \sum_{I=1}^{N_n} N_I = 1 \\ \sum_{I=1}^{N_n} N_I \vec{x}_I = \vec{x} \end{cases}
\end{aligned}$$

The regularization or *thermalization* parameter between the two criterion,  $\beta$ , has Pareto optimal values in the range  $\beta \in (0, \infty)$ . The unique solution of the local *max-ent* problem  $\text{LME}_\beta$  is:

$$N_I^*(\vec{x}) = \frac{\exp \left[ -\beta |\vec{x} - \vec{x}_I|^2 + \vec{\lambda}^* \cdot (\vec{x} - \vec{x}_I) \right]}{Z(\vec{x}, \vec{\lambda}^*)} \quad (35)$$

where

$$Z(\vec{x}, \vec{\lambda}) = \sum_{I=1}^{N_n} \exp \left[ -\beta |\vec{x} - \vec{x}_I|^2 + \vec{\lambda} \cdot (\vec{x} - \vec{x}_I) \right] \quad (36)$$

205 being  $\vec{\lambda}^*(\vec{x})$  the unique minimiser for the function  $\log Z(\vec{x}, \vec{\lambda})$ . The traditional way to obtain such a minimiser is using Eq. (37) to calculate small increments of  $\partial \vec{\lambda}$  in a Newton-Raphson approach.  $\mathbf{J}$  is defined as the Hessian matrix, obtained by:

$$\mathbf{J}(\vec{x}, \vec{\lambda}, \beta) \equiv \frac{\partial \vec{r}}{\partial \vec{\lambda}} \quad (37)$$

$$\vec{r}(\vec{x}, \vec{\lambda}, \beta) \equiv \frac{\partial \log Z(\vec{x}, \vec{\lambda})}{\partial \vec{\lambda}} = \sum_I^{N_n} p_I(\vec{x}, \vec{\lambda}, \beta) (\vec{x} - \vec{x}_I) \quad (38)$$

In order to obtain the first derivatives of the shape function, it is also necessary to compute  $\nabla N_I^*$

$$\nabla N_I^* = N_I^* \left( \nabla f_I^* - \sum_J^{N_n} N_J^* \nabla f_J^* \right) \quad (39)$$

where

$$f_I^*(\vec{x}, \vec{\lambda}, \beta) = -\beta |\vec{x} - \vec{x}_I|^2 + \vec{\lambda} \cdot (\vec{x} - \vec{x}_I) \quad (40)$$



Employing the chain rule, rearranging and considering  $\beta$  as a constant, Arroyo and Ortiz [9] obtained the following expression for the gradient of the shape function.

$$\nabla N_I^* = -N_I^* (\mathbf{J}^*)^{-1} (\vec{x} - \vec{x}_I) \quad (41)$$

The regularization parameter  $\beta$  of LME shape functions may be controlled by adjusting a dimensionless parameter,  $\gamma = \beta h^2$  [9], where  $h$  is defined as a measure of the nodal spacing. Since  $N_I$  is defined in the entire domain, in practice, the function  $\exp(-\beta \vec{r})$  truncated by a given tolerance,  $10^{-6}$  **in this research**, would ensure a reasonable range of neighbours (see [9] for details). This tolerance defines the limit values of the influence radius and is used thereafter to find the neighbour nodes of a given integration point. An additional remark is that, analogous to alternative non-polynomial meshfree basis functions, the LME approximation scheme requires more than  $d + 1$  nodes to determine the values of the shape functions as well as their derivatives at any point in the convex hull of the nodal set, where  $d$  is the dimension of the problem.

This interpolation technique avoids important shortcomings when using GIMP or B-Spline MPM regarding the computational domain boundaries (see Steffen *et al.* (2008)[35]), which are related to the additional considerations in the application of the boundary conditions. Motivated by their increased extents, particles may share an influence radius that lies outside of the simulation domain. Some researchers have solved this problem with the so called “extra” or “ghost” nodes. These nodes require especial treatment, similar to those employed in the Smoothed Particle Hydrodynamics (SPH), for further details see Liu & Liu (2003)[36]. The approach here described does not requires the employment of this artifices. Due to the fem-compatibility, the LME shape function is degenerated to linear finite element shape function if  $d + 1$  neighbouring nodes are chosen as the support, where  $d$  is the number of dimensions in the problem. Furthermore, with a conveniently adopted *regularization* parameter it is possible to get a GIMP-like shape function. **Finally sph-like behaviour can be obtained for lower values of  $\gamma$  since the support of the shape function is drastically increase, and therefore smoother solutions are obtained. See [14] for an application of this capability, in this publication authors smooth oscillations due to excess of pore water pressure in consolidation problems.** A proof of this statements is observed in figure 4. In this research and in [9],  $\gamma$  is a scalar as the influence area of the shape function is controlled by the Euclidean norm, therefore the search area is geometrically a circle in 2D, or a sphere in 3D. Building upon the idea of anisotropic shape functions, [37] introduced an enhanced version of the

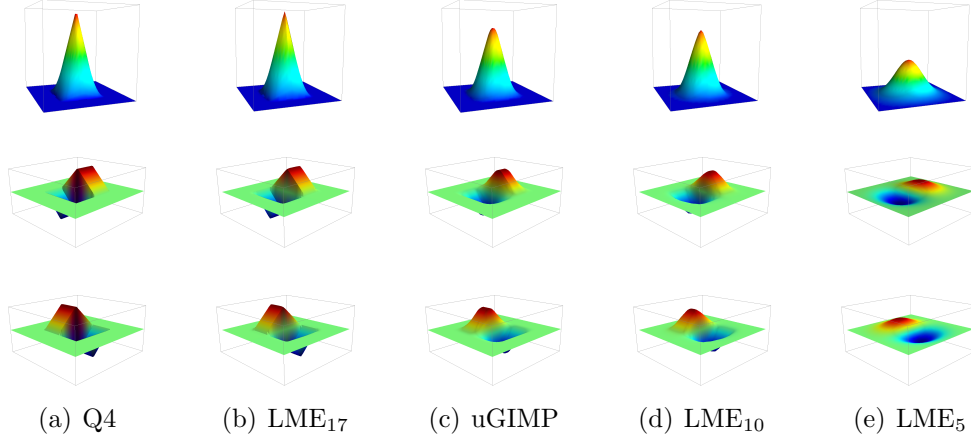


Figure 4: Comparative of linear piecewise shape functions (Q4) and uGIMP shape functions *versus* LME approximation for a two-dimensional arrangement of nodes, and spatial derivatives for several values of  $\gamma = \beta/h^2$ .

original LME scheme, which uses an anisotropic support to deal with tensile instability. **This is another benefit of the proposed methodology, that, although is out of the scope of the present document, will be incorporated in future research.**

### 3. Application to linear elasticity dynamic problems.

This section is devoted to test the ability of both predictor-corrector time integration scheme and the Local *Max-Ent* approximants to overcome spurious oscillations due to the grid crossing and high frequency loads under the context of MPM. Two different test have been adopted for this purpose: the benchmark proposed by Dyka & Ingel (1995)[38] and the test proposed in the PhD thesis of Andersen (2009)[28]. On the one hand, the accuracy of the Newmark Predictor-Corrector (NPC) scheme is compared to the standard Forward Euler (FE) in order to assess the performance of the time integration schemes. On the other hand, LME solutions are compared with those provided by Uniform GIMP (uGIMP) and Q4 shape functions. To avoid some mesh-dependent issues, in all calculations a regular background mesh was set. All simulations were performed with in-house software.

#### 3.1. Dyka's bar [38]

This benchmark was proposed by Dyka [38] since allows so study easily the capability of the proposed time integration algorithm to avoid velocity field instabilities. It consists of a one-dimensional bar of a length of 0.1333

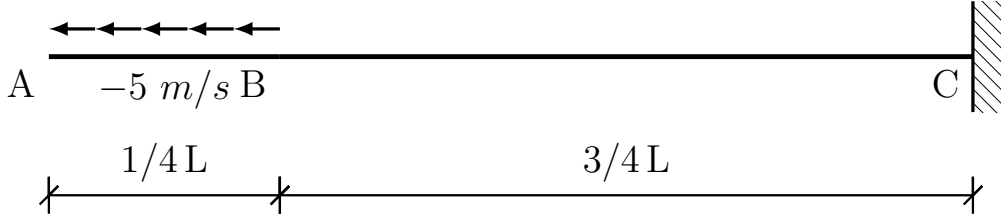


Figure 5: Geometrical description of the Dyka [38] bar.

270 meters, sketched in the figure 5. The boundary conditions are: displacements are constrained ( $\mathbf{v}|_{x=L} = 0$ ) in the right border, letting free in the rest of the body and ( $\sigma|_{x=0} = 0$ ). An initial velocity of  $\mathbf{v}_o = -5 \text{ m/s}$  is given to the left quarter of the bar. Finally, the elastic parameters considered for this test are:

- Density :  $7833 \text{ kg/m}^3$
- Poisson ratio : 0
- 275 • Elastic modulus :  $200 \cdot 10^9 \text{ Pa}$

In this case, 0.0001 seconds **are considered** in the simulation. Therefore, the elastic wave generated travel through the bar (from A to C and back to A) at least two times. For the spatial discretization, a set of seven nodal mesh sizes (0.1, 0.3325, 0.5, 1.0, 3.3325, 6.665, 10.0 millimeters) are considered. For each element a number of four particles was selected. In the initial layout, particles are occupying the exact quadrature points of a linear quadrilateral, with the exception of the uGIMP simulation, where gaps or overlap between voxels of each particle are not allowed. In those cases, each particle occupies the center of each cell quarter. For all simulations, time step is controlled by a Courant-Friedrichs-Levy condition of 0.1, were the adopted celerity is computed as:

$$Cel = \max\{\max_{p \in \Omega_p}\{\mathbf{v}_p\}, \max_{p \in \Omega_p}\{\sqrt{\frac{E_p}{\rho_p}}\}\}. \quad (42)$$

280 An important consideration regarding modellization concerns to the background mesh. Notice that free border of the bar has a maximum horizontal displacement of 0.03 millimeters, therefore a computational domain with an extra gap of 0.03 millimeters is required in order to accommodate the unconstrained displacement of the particles in the left border of the bar. Naturally this problem arises when the mesh size is small enough that relative displacement of the particles is larger then the distance to the border, so grid crossing phenomena could appear even in those cases with infinitesimal displacements.

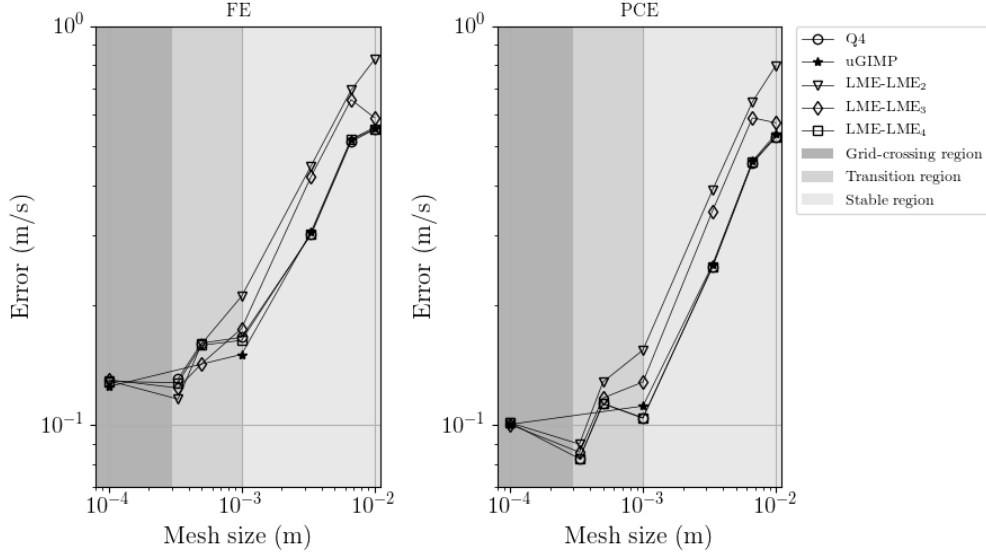


Figure 6: Velocity error evolution at the point A in the Dyka's bar , convergence plots for FE and NPC. The plot is subdivided with colours, the darker part of the diagram shows coincides when the relative movement of the particles is large enough to produce the grid crossing phenomena. The lightest part of the diagram coincides when the relative movement of the particles in negligible in comparison with the mesh size. And in the middle region a transition behaviour take place.

In this case, an analytical solution can be obtained through the characteristics method, described in the appendix Appendix A. To measure the convergence of the solutions for the different time integration and approximation schemes the root-mean-square (RMS) error in the velocity field is computed. RMS error is defined as

$$RMS = \sqrt{\frac{1}{N} \sum_p^N (\mathbf{v}_p - \hat{\mathbf{v}}_p)^2}, \quad (43)$$

where  $\mathbf{v}_p$  and  $\hat{\mathbf{v}}_p$  are respectively the analytical and numerical solutions evaluated in the final time step in the position of each particle. In Fig. 6 the evolution of the RMS is obtained for both time integration schemes. The right figure, with the results, shows lower values of the estimated error, denoting the higher performance of this methodology. About the spatial discretization, the schemes show an error comparable to the obtained with the , being even lower close to the grid-crossing region.

A first comparative between the velocity results of both time integration schemes is plotted in figure 7. It demonstrates the superior performance of the NPC *versus* the FE. In the NPC the spurious oscillations are quickly

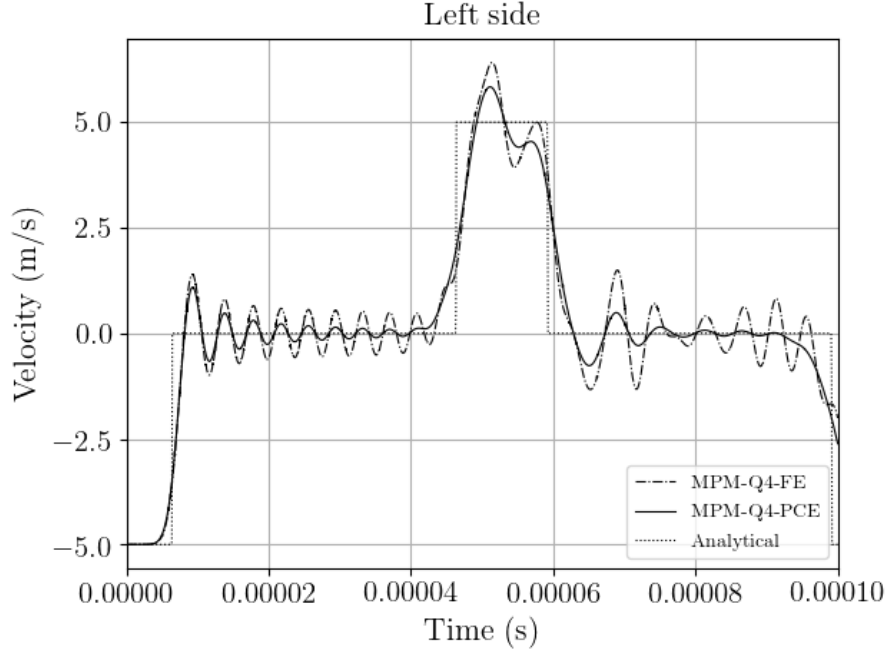


Figure 7: Comparative of the NPC *versus* the FE. In the picture the velocity evolution at the point in the bar left side is plotted.

mitigated in the first time steps, and the error is not propagated in time,  
 295 opposite to the results of the FE scheme, where the simulation becomes  
 unstable after  $6E^5$  seconds.

Figure (8) shows the sensibility of the LME approximation scheme to  
 variations in the parameter  $\gamma$ , which is the one that controls the value of  
 the regularization parameter  $\beta$  together with of the mesh size. Notice how  
 300 lower values of gamma exhibits a behaviour with a soft decay in some parts  
 of the simulation due to the increase of nodes adopted to regularize the so-  
 lutions. This capability could be useful in simulations where extremely noise  
 oscillations could damage the solutions like memory materials. On the other  
 hand larger values of the parameter  $\beta$  makes the solution tend to the linear  
 305 FEM solution as the athermal limit is reached [9]. Intermediate values of the  
 regularization parameters give us a compromise between the both scenarios  
 here described. An additional observation, concerning to the solution sensi-  
 bility depending on the regularization parameters, **is the behaviour of the  
 solution depending on the decreasing of mesh size** . For larger mesh sizes,  
 310 where the relative particle displacement is negligible in comparison with the  
 cell size, the global behaviour is FEM-like; therefore, larger values of  $\gamma$  **may  
 offer better results** . On the other hand, when mesh size is small enough to

produce grid-crossing meshfree behaviour is required to ensure the convergence of the solution and tiny values of  $\gamma$  may lead to better performances . Convergence plot in figure (6) shows how the slope, for the larger values of  $\gamma$ , decreases monotonically with the value of the mesh size, in contrast with larger values of  $\gamma$ , where the performance is punished with significant movement of the particles as far as the mesh size is reduced. The performance of the uGIMP [3] shape function *versus* the LME approximation scheme with a dimensionless regularization parameter  $\gamma$  of 4.0 is compared in Figure 9. Although remarkable differences are not observed, more robust behaviour is exhibited by the LME approximants than the uGIMP shape functions. Regarding this, notice the absence of uGIMP values for a mesh size of 0.3325 and 0.5 millimeters. The reason is due to an unstable increasing of the error suffered during uGIMP simulations of these mesh sizes which yield unacceptable results . A feasible explanation for this phenomena could be the presence of numerical cancellation which could produce gaps between voxels. Further research should be done in this direction for getting a better comprehension of this phenomenon. Conversely, this shortcoming is not suffered by LME, independently of regular or with irregular nodal layout.

Finally, the OTM [39] *versus* obtained solution is compared against the one obtained with MPM, both with same time integration scheme and spatial discretization, and results are shown in figure 10. In this case, the performance of MPM presents better robustness and stability than the OTM. During the first half of the simulation both method seem to perform in a similar way, but during the second half of the simulation after the elastic wave has travel from the free border to the fixed one and back, in OTM the solution becomes more noisy than the one performed by MPM.

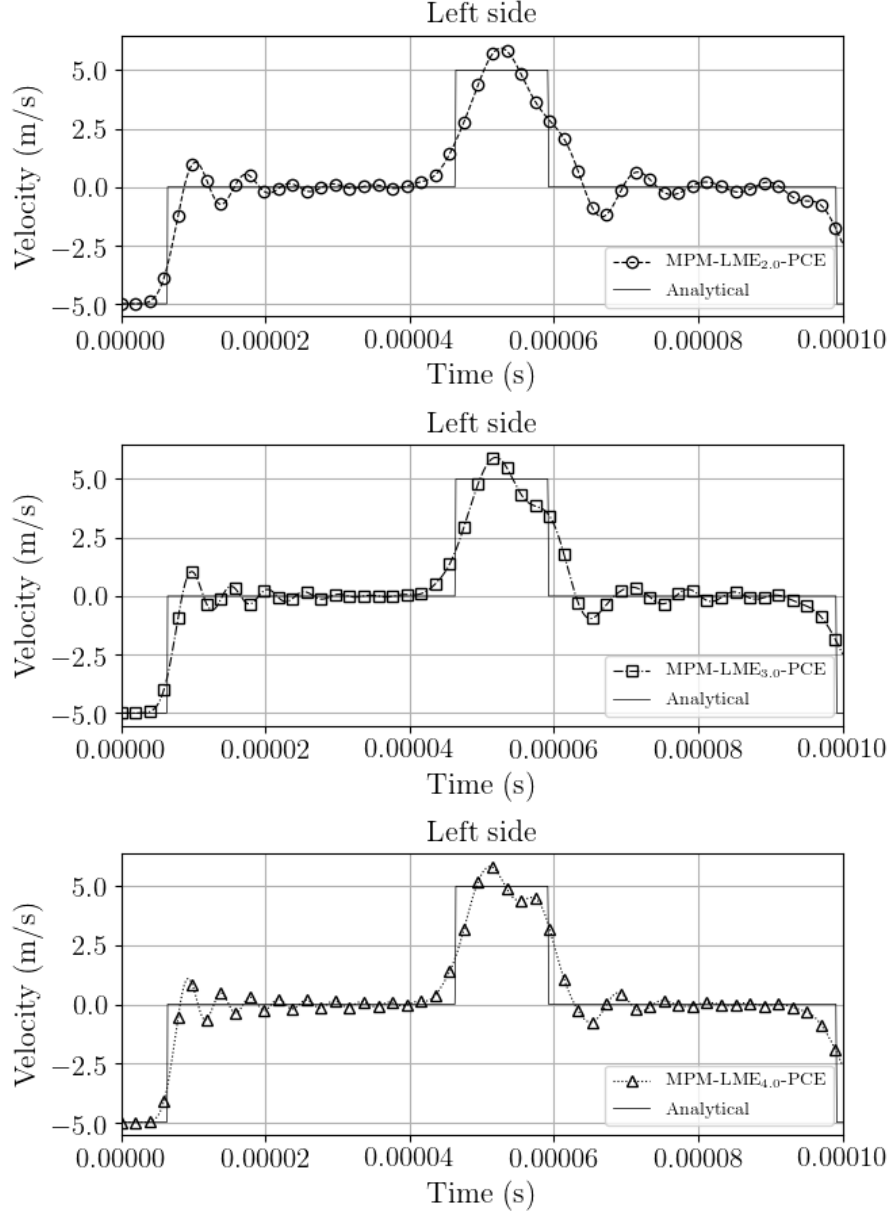


Figure 8: Sensitive of LME approximants performance to changes in the dimensionless regularization parameter  $\gamma = \beta/h^2$ . To illustrate it, the velocity evolution at the point in the bar left side is plotted.

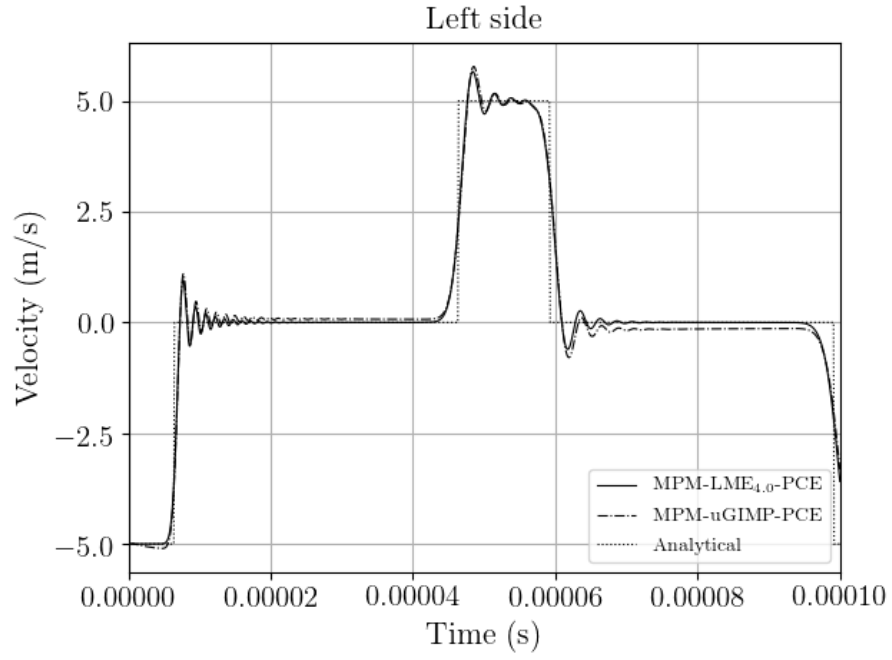


Figure 9: Velocity evolution at the point in the bar left side.

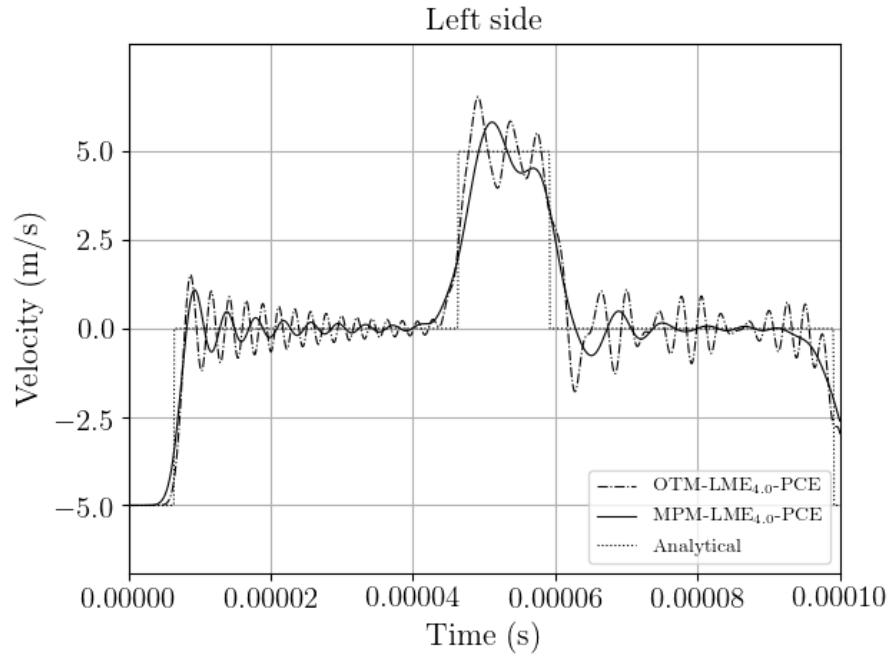


Figure 10: Velocity evolution at the point in the bar left side.



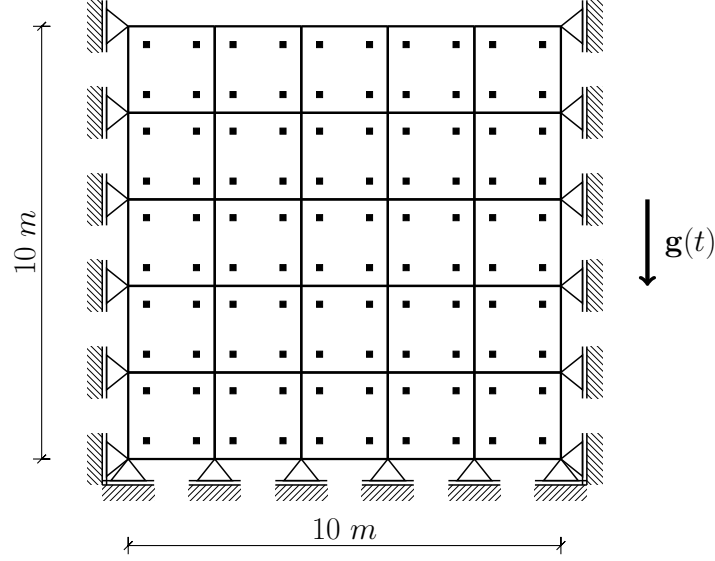


Figure 11: Geometrical description of a soil block

### 3.2. Rigid block

340 The following test was proposed to measure proof the ability of this interpolation technique to deal with grid crossing instabilities. It consists in the simulation of a square block of soil incrementally loaded by a body force. Details of the problem are sketched in figure 11 This test was taken from PhD thesis of Andersen (2009)[28]. The elastic parameters consider for this  
345 test are:

- Initial density :  $6 \cdot 10^3 \text{ kg/m}^3$
- Poisson ratio : 0
- Elastic modulus :  $5 \text{ MPa}$

The gravity force is a apply as an external force according to the equations (10), (15). Using a total time period of  $T$  (20 seconds) to apply the gravity, it is increased from 0 to  $9.81 \text{ m/s}$  with a sinus function until  $T/2$  and then maintained constant until  $T$  in order to arrive to a state of equilibrium,

$$\mathbf{g}(t) = \begin{cases} 0.5\mathbf{g}(\sin(\frac{2t\pi}{T} - \frac{\pi}{2}) + 1) & \text{if } t \leq T/2 \\ \mathbf{g} & \text{if } t > T/2 \end{cases} \quad (44)$$

350 In order to get a stable solution, time step was conducted by a Courant number of 0.1. On the other hand, the explicit predictor-corrector scheme is here employed looking forward getting better results. For the initial spatial

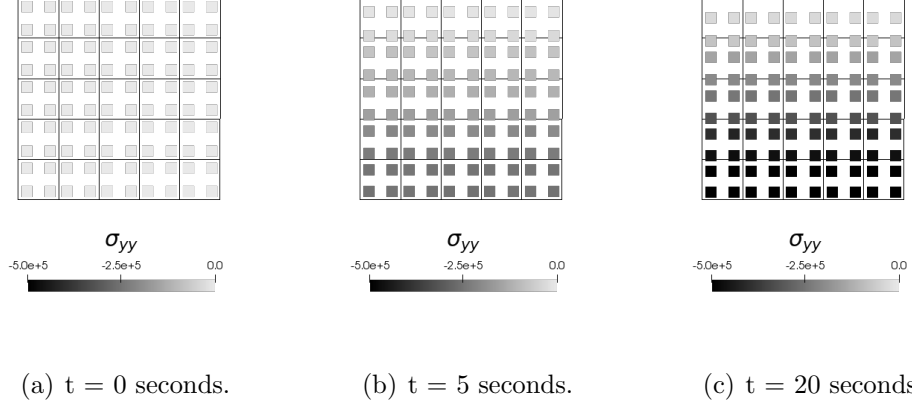


Figure 12: Vertical normal stress and position of material points during the loading process for a soft soil ( $E = 5 \text{ MPa}$ ,  $\rho_0 = 6 \cdot 10^3 \text{ kg/m}^3$ ). Numerical parameters considered for the simulation are : Local *max-ent* shape function  $\gamma = 3$  and explicit PC scheme with CFL 0.1.

discretization four particles per cell ( $\Delta x = 2 \text{ m}$ ) were adopted. The initial layout of particles inside of the cell changes according to the approximation technique adopted. For the bi-linear shape functions and the LME approx-  
 355 imants, the initial position corresponds to the location of the gauss-points in a standard quadratic finite element. For the uGIMP shape function the initial position of each particle is located in the center of each voxel, due to the fact that in the initial situation, the voxel domain should not overlap each others.

360 Figure 12 shows the evolution of the vertical stress during the loading process. The result is physically realistic as stress increments linearly from the top to the bottom of the specimen, and the value of the vertical stress in a material point located in the bottom of the specimen oscillates centered in  $5.2 \text{ MPa}$ , which is the analytic value given by  $\sigma_{yy} = \rho g h_y$ . Figure 13  
 365 shows the vertical displacement evolution of a point in the free surface of the block. This figure shows how simulations performed with a bi-linear interpolation technique (Q4) turns out to be unstable during cell-crossing and consequently fails. The uGIMP simulation is more stable than the one performed by the Q4. Despite this is still unstable and could trigger severe  
 370 oscillations in simulation with non-linear materials. The LME simulation was performed using two kinds of shape functions, one with a low value of the dimensionless parameter,  $\gamma = 0.8$ , and other with a larger value of it,  $\gamma = 3.0$ . Notice that the results are both stable, but the larger values of  $\gamma$  give us a very stable solution. This is due to the fact that with larger

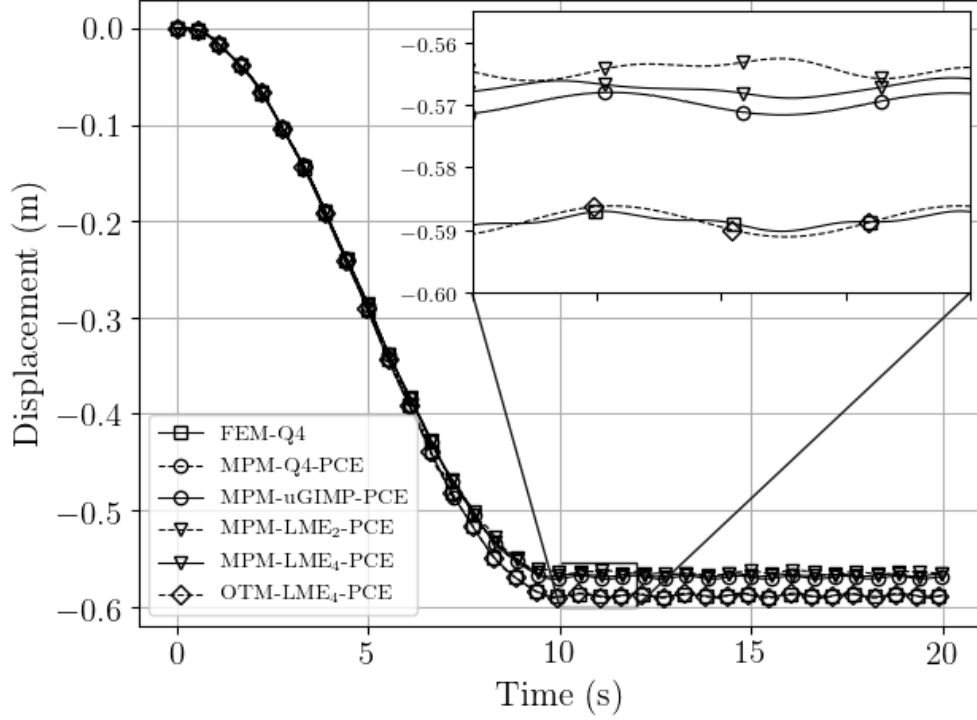


Figure 13: Comparative of the vertical displacement evolution in a point located in the free surface employing different interpolation schemes and numerical techniques.

value of  $\gamma$ , the shape functions behaves in a similar way to the FEM, which performs very accurate in those cases with a reasonable mesh distortion, and with a lower value it behaves in a similar way to the uGIMP. This behaviour was noticed previously by [9], where authors highlight how by adjusting the spatial variation of  $\beta(\mathbf{x})$ , it is possible to select regions of the domain of analysis which are treated by finite elements and regions that are treated in the style of meshfree methods, with seamless transitions between those regions.

#### 4. Conclusions

We have developed a novel time integration scheme for MPM, and proved how local *max-ent* approximation scheme could be employed as an useful technique in MPM. The NPC arise as a highly efficient alternative for challenging dynamic problems like coupled  $u - p_w$  without appeal to expensive implicit time integration algorithms. Also the procedure employed to design the NPC algorithm open the door to revisit a huge variety of time integration schemes developed originally for FEM, which can be rearranged to

MPM framework with some modifications. Anyway, further research should be done to improve the formal comprehension of the algorithm good performance. This paper also enhances the suitability of the LME approximation as a general promising alternative to the wide range of approximation. techniques developed for the MPM to overcome grid crossing limitations and to avoid the constriction of the uGIMP of a regular mesh or a high density of particles per cell. Future research of the group will be on the employ of this scheme to improve the localization capabilities of MPM for viscoplastic materials. Finally we remark on the possibility of adapting the function  $\beta(\mathbf{x})$  as a second order tensor with the aim of adapt the shape function with the strain field which improves the performance of it in the aforementioned localization capabilities. Other possibility is to adapt the value of  $\beta$  to solve the equations FEM-like of meshfree-like depending of how behaves the region, this could be extremely useful in simulating all together initialization and propagation of fast landslides.

If you'd like to thank anyone, place your comments here and remove the percent signs.

### Conflict of interest

The authors declare that they have no conflict of interest.

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## Appendix A. The analytical solution of the 1D Dyka benchmark

For the derivation of this analytical solution, a 1D elastic bar is consider. Henceforth for convenience the governing equations will be written in terms of stress and velocity. The balance of linear momentum,

$$\rho \frac{\partial v}{\partial t} = \frac{\partial \sigma}{\partial x}, \quad (\text{A.1})$$

Secondly the constitutive equation is the well known linear elastic one,

$$\frac{\partial \sigma}{\partial t} = E \frac{\partial \varepsilon}{\partial t}, \quad (\text{A.2})$$

where  $E$  is the elastic modulus. And finally the compatibility equation,

$$\frac{\partial \varepsilon}{\partial t} = \frac{\partial v}{\partial x}. \quad (\text{A.3})$$

Next for simplicity, we will introduce (A.3) in (A.2), it yield to,

$$\frac{\partial v}{\partial t} = \frac{1}{\rho} \frac{\partial \sigma}{\partial x}, \quad (\text{A.4})$$

$$\frac{\partial \sigma}{\partial t} = E \frac{\partial v}{\partial x}. \quad (\text{A.5})$$

Introducing (A.5) in (A.4) and expressing the remaining equation in terms of the displacement, results the wave equation for linear elastic materials,

$$\frac{\partial^2 u}{\partial t^2} = \frac{E}{\rho} \frac{\partial^2 u}{\partial x^2} = c^2 \frac{\partial^2 u}{\partial x^2} \quad (\text{A.6})$$

where  $c = \sqrt{\frac{E}{\rho}}$  is the material celerity. Alternative, rearranging both equations (A.4) and (A.5) it is possible to join them in a single system of equations as,

$$\frac{\partial}{\partial t} \begin{bmatrix} \sigma \\ v \end{bmatrix} + \begin{bmatrix} 0 & -E \\ -1/\rho & 0 \end{bmatrix} \begin{bmatrix} \frac{\partial \sigma}{\partial x} \\ \frac{\partial v}{\partial x} \end{bmatrix} = \mathbf{0}. \quad (\text{A.7})$$

Or in a more compact format,

$$\frac{\partial \phi}{\partial t} + \mathbf{A} \frac{\partial \phi}{\partial x} = \mathbf{0}. \quad (\text{A.8})$$

In (A.9) stress and velocity are joined in to a single structure  $\phi$  and  $\mathbf{A}$  in coupling matrix between both equations,

$$\phi = \begin{bmatrix} \sigma \\ v \end{bmatrix}, \quad \mathbf{A} = \begin{bmatrix} 0 & -E \\ -1/\rho & 0 \end{bmatrix}.$$

Despite of this manipulation, the nature of is still hyperbolic. A proof of this can be easily obtained computing the zeros of the hypersurface defined by (A.6). And later the eigenvalues of  $\mathbf{A}$  in (A.9). In both cases, eigenvalues are real and distinct ( $\lambda = \pm \sqrt{\frac{E}{\rho}}$ ), therefore the system is called strictly hyperbolic. Assuming that  $\mathbf{A}$  has  $n$  different eigenvalues  $\{\lambda_1, \dots, \lambda_i, \dots, \lambda_n\}$

and  $n$  eigenvectors  $\{\vec{x}^1, \dots, \vec{x}^i, \dots, \vec{x}^n\}$  satisfying that  $\mathbf{A}\vec{x} = \lambda\vec{x}$ . Now we introduce the matrix  $\mathbf{P}$  whose columns are the  $n$  eigenvalues  $\mathbf{x}$

$$\mathbf{P} = \{\vec{x}^1, \vec{x}^2, \vec{x}^3, \dots, \vec{x}^n\}. \quad (\text{A.9})$$

Diagonalizing  $\mathbf{A}$  using  $\mathbf{P}$  yields,

$$\Lambda = \mathbf{P}^{-1} \mathbf{A} \mathbf{P}, \quad (\text{A.10})$$

where  $\Lambda_{ii} = \lambda_i$ . Now, lets define a vector  $\mathfrak{R}$  as

$$\phi = \mathbf{P} \mathfrak{R}. \quad (\text{A.11})$$

Expanding the above expression with the chain rule and passing the matrix  $\mathbf{P}$  to left hand side of the equality we get,

$$d\vec{\mathfrak{R}} = \frac{\partial \mathfrak{R}}{\partial t} dt + \frac{\partial \mathfrak{R}}{\partial x} dx = \mathbf{P}^{-1} \left( \frac{\partial \phi}{\partial t} dt + \frac{\partial \phi}{\partial x} dx \right) \quad (\text{A.12})$$

and setting the terms we get,

$$\frac{\partial \mathfrak{R}}{\partial t} = \mathbf{P}^{-1} \frac{\partial \phi}{\partial t}, \quad \frac{\partial \mathfrak{R}}{\partial x} = \mathbf{P}^{-1} \frac{\partial \phi}{\partial x} \quad (\text{A.13})$$

Next, if we multiply (A.9) by  $\mathbf{P}^{-1}$  we get:

$$\mathbf{P}^{-1} \frac{\partial \phi}{\partial t} + (\mathbf{P}^{-1} \mathbf{A} \mathbf{P}) \mathbf{P}^{-1} \frac{\partial \phi}{\partial x} = \mathbf{0} \quad (\text{A.14})$$

finally introducing the expressions (A.14) we reach to

$$\frac{\partial \mathfrak{R}}{\partial t} + \Lambda \frac{\partial \mathfrak{R}}{\partial x} = \mathbf{0} \quad (\text{A.15})$$

which consists of  $n$  uncoupled equations as  $\Lambda$  is diagonal matrix as we can see in (A.11). Each of this equations are 1D scalar convective transport equations, with solutions of the form:

$$\mathfrak{R}^{(i)} = F^{(i)}(x - \lambda^{(i)}t) \quad (\text{A.16})$$

This uncoupled system, has a set of  $n$  characteristics. These magnitudes  $\mathfrak{R}_i$  which propagate along characteristics are known as *Riemann invariants* of the problem. For the closure of the problem it is required “n” initial conditions of the form  $\mathfrak{R}_i(x, t = 0) = h_i(x)$ , and “n” boundary conditions.

Particularizing the previous equations for the 1D elastic bar described in [38],  $\mathbf{P}$  can be computed as,

$$\mathbf{P} = \begin{bmatrix} -\sqrt{E\rho} & \sqrt{E\rho} \\ 1 & 1 \end{bmatrix}$$

With the value of the inverse matrix  $\mathbf{P}^{-1}$  in the Riemann definition (A.12) we get the following system of equations,

$$\mathfrak{R}^I = \frac{1}{2\sqrt{\rho E}} \left( -\sigma + v \sqrt{\rho E} \right) \quad (\text{A.17})$$

$$\mathfrak{R}^{II} = \frac{1}{2\sqrt{\rho E}} \left( \sigma + v \sqrt{\rho E} \right) \quad (\text{A.18})$$

From (A.18) and (A.19) we can obtain the values of the stress and the velocity as:

$$v = \mathfrak{R}^I + \mathfrak{R}^{II} \quad , \quad \sigma = \sqrt{E\rho} (\mathfrak{R}^{II} - \mathfrak{R}^I) \quad (\text{A.19})$$

The boundary conditions are in both cases of radiation as there is not wave in-going from the exterior. So for the right side (fixed boundary) we get the following conditions:

$$\mathfrak{R}^{II} = 0 \quad \text{and} \quad v_{x=L} = 0$$

Therefore  $\sigma_{x=L} = -2\sqrt{\rho E} \mathfrak{R}^I$ . And in the left side (free boundary) we get the following conditions:

$$\mathfrak{R}^I = 0 \quad \text{and} \quad \sigma_{x=0} = 0$$

Therefore  $v_{x=0} = 2\mathfrak{R}^{II}$ . By imposing this boundary conditions the problem is fully defined as in [38].

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