

# A framework to automate the design of digitally-fabricated timber plate structures



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## ABSTRACT

The current study uses knowledge from digital architecture, computer science, engineering informatics, and structural engineering to formulate an algorithmic framework for integrated Computer-Aided Design (CAD) and Computer-Aided Engineering (CAE) of Integrally-Attached Timber Plate (IATP) structures. The algorithm is designed to take the CAD 3D geometry of an IATP structure as input and automates the construction and analysis of the corresponding CAE model using a macroscopic element, which is an alternative to continuum Finite Element (FE) models. Each component of the macro model is assigned a unique tag that is linked to the relevant geometric and structural parameters. The CAE model integrity is maintained through the use of the common data model (CDM) concept and object-oriented programming. The relevant algorithms are implemented in Rhinoceros 3D using RhinoCommon, a .NET software development kit. Once the CAE macro model is generated, it is introduced to the OpenSees computational platform for structural analysis. The algorithmic framework is demonstrated using two case structures: a prefabricated timber beam with standard geometry and a free-form timber plate arch. The results are verified with measurements from physical experiments and FE models, where the time needed to convert thousands of CAD assemblies to the corresponding CAE models for response simulation is considerably reduced.

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## 1. Introduction

Spurred by the proliferation of interactive modeling tools and visual programming languages, the Computer-Aided Design (CAD) framework and digital geometry processing have facilitated the design of spatial timber plate structures with complex geometries. To understand the structural performance of such systems, especially in terms of their functionality, numerical simulations are generally employed to simulate the structural response. Numerical models that include the geometrical and mechanical properties of the system under consideration are generally described as Computer-Aided Engineering (CAE) models. By and large, CAD and CAE models for spatial timber plate structures are constructed in different environments. As such, investigations have

sought to develop methodologies to convert CAD data into CAE models, which are discussed in Section 1.1. The scope and motivation of the study are then outlined in Section 1.2.

### 1.1. Spatial timber plate structures and available CAD-to-CAE data exchange tools

Data exchange tools are useful for timber plate structures with a large number of elements because of their ability to considerably reduce the time required to convert thousands of CAD assemblies to the corresponding CAE models. These tools also enable the idealization of CAD models so that the numerical simulations can be readily performed in a CAE platform. The CAD-to-CAE exchange for spatial free-form timber plates was first introduced by the Institute for Computational Design [1] and the Institute of Building Structures and Structural Design [2]. In particular, Li and Knippers [3], Krieg et al. [4], and La Magna et al. [5] developed CAD-to-FE exchange tools to simulate the behavior of segmented timber plate shells. The CAD model, which is constructed in Rhinoceros 3D [6]

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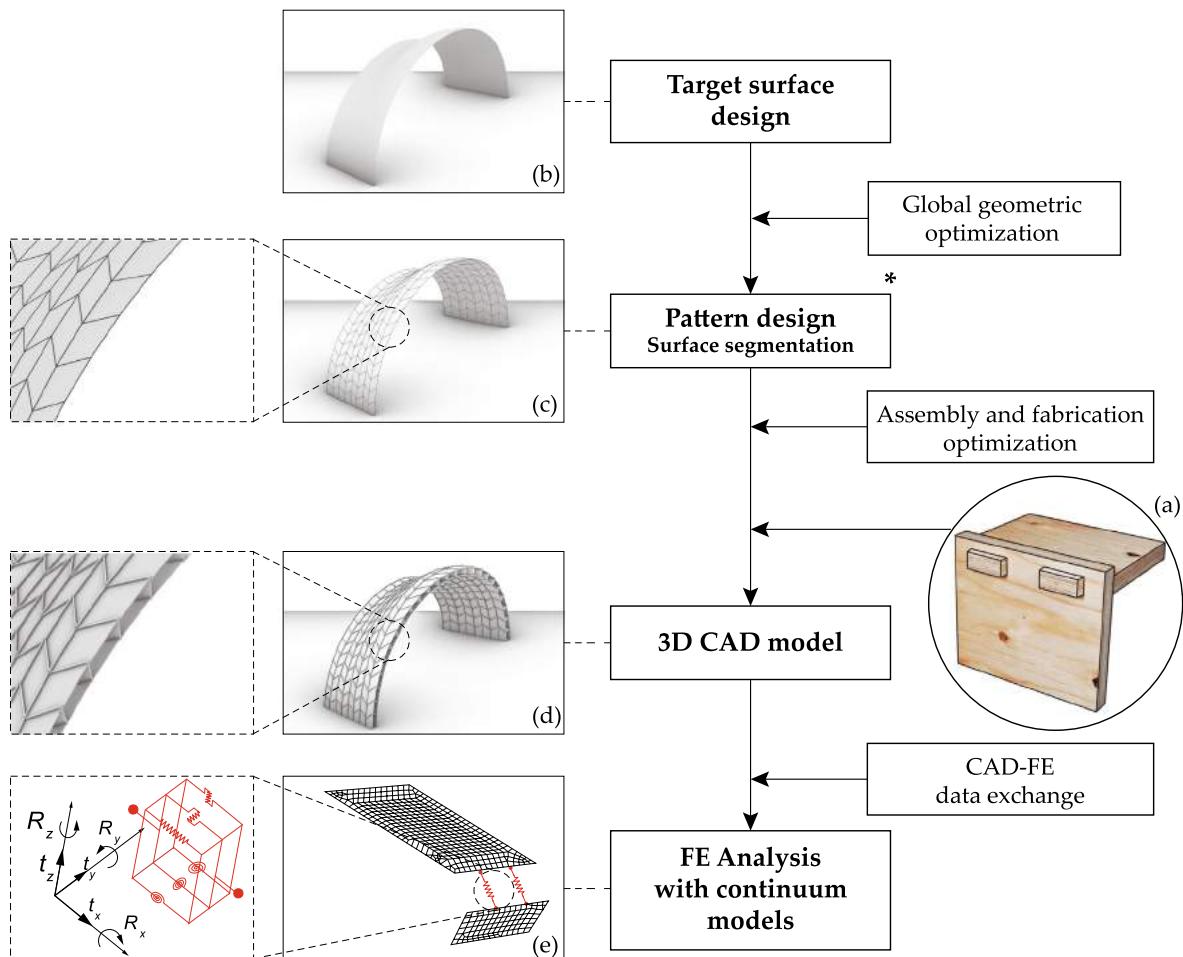
URL: <http://www.dfab.ch/> (N. Rogeau).

using the visual programming environment, Grasshopper 3D [7], consists of three-dimensional (3D) brick elements to represent the timber plates. The data transfer from the CAD to CAE platforms was done either statically via import/export of the corresponding dwg file or dynamically through a parametric modeling interface. A centralized repository is generated to encompass the datastructure associated with the CAE geometry, mechanical properties, and analysis handlers. The generated data is then mapped to an appropriate FE platform(s) such as SOFiSTiK [8] and/or ANSYS® [9], where two-dimensional (2D) plane shell elements and one-dimensional (1D) springs are used to simulate the behavior of the timber panels and joints, respectively.

Recently, the Laboratory for Timber Construction (IBOIS) at École Polytechnique Fédérale de Lausanne (EPFL) [10] has designed free-form Integrally-Attached Timber Plate (IATP) structures [11,12], where elements are joined using the Integral Mechanical Attachment (IMA) technique (Fig. 1a). The defining feature of IMAs is that the connection between timber plates is established solely through their geometry without additional connectors such as nails, screws, dowels, and adhesives [11,12]. The design process for IATP structures begins with the definition of the target surface established in a CAD environment (Fig. 1b). Using ShapeOp [13] along with Grasshopper 3D [7], Robeller et al. [14] introduced an iterative IATP optimization framework to develop and refine the design object, design the pattern, and establish the global form (Fig. 1c). The algorithm was formulated to introduce a new assembly-constrained tiling, and identify the pattern of timber

plates, mesh discretization, the density of planar meshes, the curvature and segmentation of the design surface, the angle between adjacent plates, the position of IMAs, and the assembly pattern between the elements. ShapeOp finds the optimum geometry that satisfies fabrication and assembly constraints but does not consider the structural design objectives. The pseudocode associated with the NURBS subdivision used to obtain the quad mesh is provided in Appendix 1.1. The basis of the NURB subdivision algorithm was developed by Robeller et al. [14]. The CAD model was then completed by converting the segmented planes to 3D brick elements (Fig. 1d). The term 3D is used because the plate thickness is explicitly represented in the model. A data exchange algorithm was then introduced to convert the CAD model to the Computer-Aided Manufacturing (CAM) model [14]. The CAD-to-CAM algorithm provided the information about the orientation of the fabrication tool, which was described with tool center points and two Euler angles. Generating the upper and lower contours for each plate, the exchange algorithm outputs the corresponding G-code using a loft-like offset of the contours. The G-code is then used for 5-axis digital fabrication. The pseudocode associated with the generation of the CAD 3D model and the fabrication outlines is provided in Appendix 1.2. The CAD-to-CAM algorithm was developed by Robeller et al. [14].

Recently, Nguyen et al. [15] developed an automated numerical tool that uses a parametric modeling framework to convert the CAD data associated with IATP structures to the corresponding FE numerical model (Fig. 1e). The CAD model was idealized using pla-



**Fig. 1.** (a) Integral Mechanical Attachment (IMA), (b) target surface, (c) Quad pattern, (d) CAD 3D Integrally-Attached Timber Plate (IATP) structure, (e) FE model.

nar surfaces to represent timber plates, and line elements for the IMAs. The idealization process includes transformation algorithms such as detail removal [16], dimensional reduction [17], and non-manifold topological modeling [18]. The heterogeneous data exchange methodology [19] was employed to export the data associated with the model geometry from the CAD environment. Using the RhinoCommon library [20], plane-plane and line-plane intersection methods were used to generate the idealized geometry. The Initial Graphics Exchange Specification (IGES), consisting of only mathematical descriptions of the model geometry, was used as the neutral data format. The idealized geometry was then introduced to the FE modeling software ABAQUS™ using the scripting interface. Finite strain S4R shell elements were employed to simulate the behavior of planar surfaces and springs were used to simulate the behavior of the IMAs (Fig. 1e).

## 1.2. Motivation and scope of the current study

The structural design of large-scale IATP structures requires several steps: (1) Understanding the behavior of IMAs under different load cases. For this step, physical experiments have been conducted by Rezaei Rad et al. [12,11,21] to gain insight into the behavior of joints under tensile, edgewise, and flatwise loads, respectively. (2) Introducing a computationally-efficient mechanical model, where the level of detail associated with the design object is adapted to simplify the design process. For this step, a macroscopic model that employs only beam and spring elements was introduced by Rezaei Rad et al. [22] to simulate the behavior of timber shells with IMAs. (3) Introducing an interface to automatically convert the design CAD geometry associated with hundreds of timber plates with thousands of IMAs to the corresponding macro model. The details of this step have not been formulated and is not available in typical building information modeling (BIM) tools and is a major challenge in the design and performance assessment of large-scale IATP structures. Therefore, the primary objective of the current study is to develop a CAD-to-CAE algorithmic data exchange framework that translates the CAD data associated with a custom-defined IATP structure to the corresponding CAE macro model. The proposed framework is demonstrated by evaluating the performance of large-scale IATP prototypes that have recently been constructed.

Establishing an automatic link between the CAD and CAE macro models has multiple advantages. It considerably reduces the time required to prepare a CAE model. Furthermore, repetitive manual operations are removed from the design process and human errors are minimized. Moreover, instead of working with three or more software packages, the model integrity is maintained in one platform by using object-oriented programming. In addition, the proposed framework enhances the design flexibility of IATPs since multiple variants can be considered in the design process while the associated structural performance can be evaluated with minimal computational expense. The modeling and computational simulation of complex structures is also improved through the use of an open-source CAE platform.

The parametric CAD-to-CAE exchange integrates data analysis and transformation, geometrical simplification, and structural analysis of timber plates with edgewise IMAs. Furthermore, while the concept and methodology of the proposed CAD-to-CAE data exchange algorithm are independent of the computational platform, the current study employs (1) the Rhinoceros .NET Software Development Kit (SDK) called RhinoCommon [20] and Rhino. Python [23] to construct the plugins and components for the CAD-to-CAE data transformation, and (2) The Open System for Earthquake Engineering Simulation, OpenSees [24], to perform

the structural analysis. The data exchange framework is designed in the CAD environment while the output data generated by the framework is formulated such that it follows the syntax of OpenSees. The overall workflow of the current study is shown in Fig. 2.

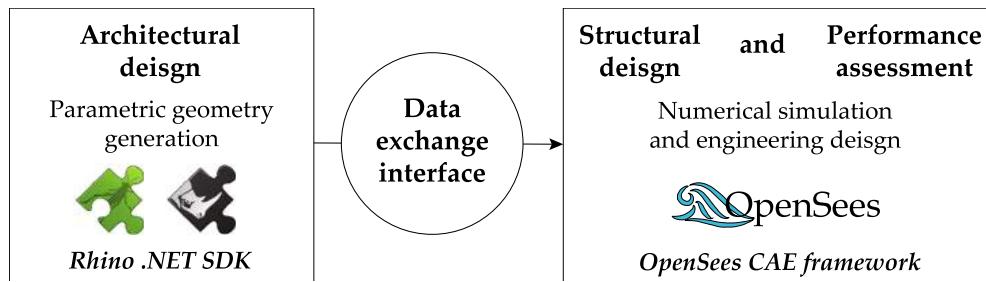
The details of the macroscopic mechanical model for the structural analysis of spatial timber plates are reviewed in Section 2. The algorithmic CAD-to-CAE data exchange methodology and design workflow are outlined in Section 3. The integrative design approach, which encompasses the geometry generation and structural analysis, is also explained. Details of the exchange framework and the indexing algorithm used to define the macro model components are also presented. Section 4 applies the proposed framework to two design cases: prefabricated timber beams with standard geometries and a free-form IATP structure. The conclusions are summarized in Section 5. The pseudocode associated with the IATP design modules is provided in Appendix 1.3.

## 2. Macro modeling technique for timber plate structures

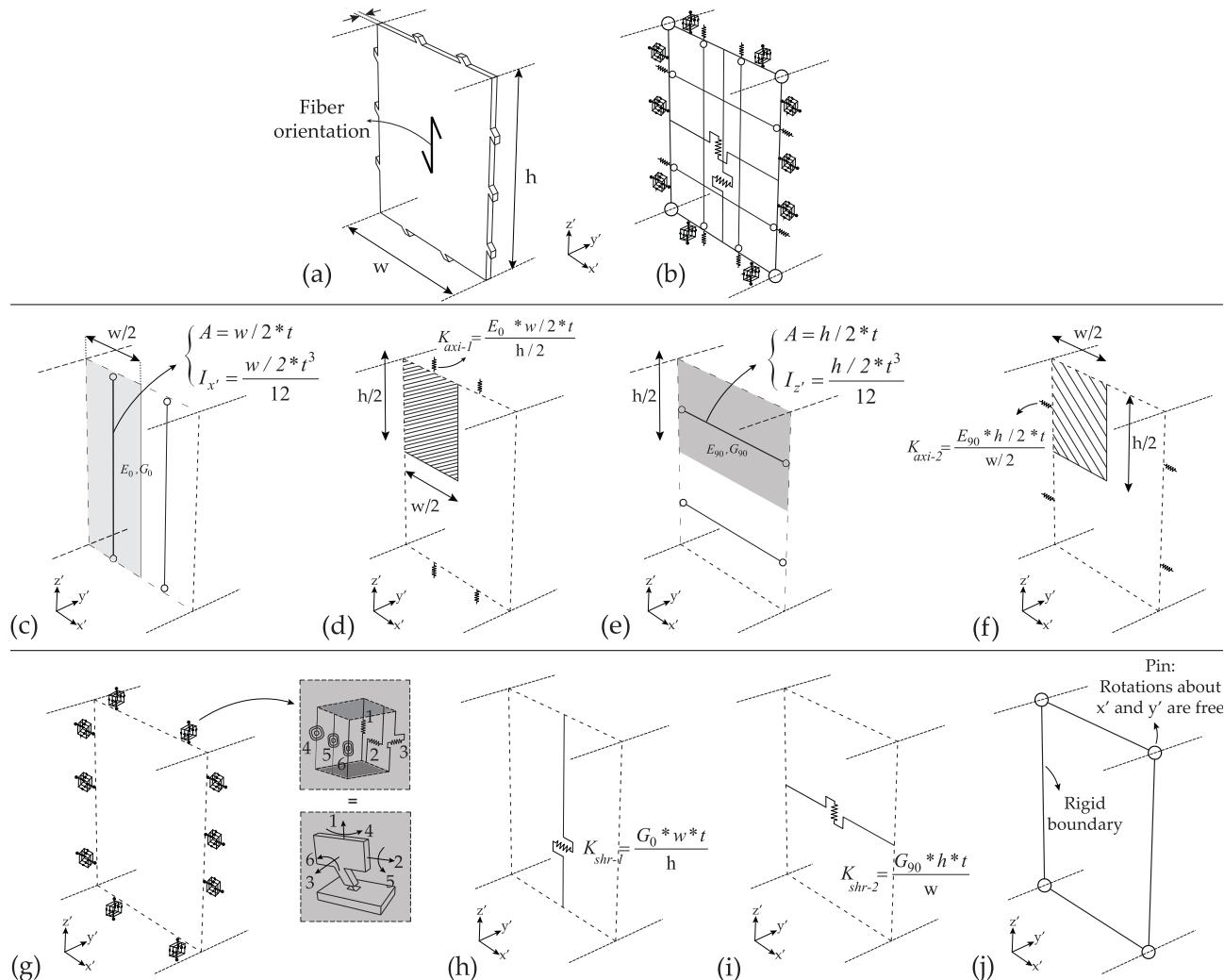
The analysis of structural systems governed by plate/shell action generally requires model simplifications such as dimensional reduction and detail removal to reduce the computational time and complexity of the model [25,26,27]. For IATP structures, a macroscopic modeling technique has been accordingly introduced by Rezaei Rad et al. [22], where a series of one-dimensional elements (i.e. springs and beam elements) are adopted as an alternative to the detailed continuum FE models (i.e. shell or brick elements).

Three key techniques are used to develop the macro model: feature-based simplification, topology-based modification, and virtual topology adaptation. Geometric reduction, which is a part of feature-based simplification, is used to convert the 3D timber plate elements (Fig. 3a) to 1D macro elements (Fig. 3b). The macro model is valid for quadrilateral timber plates with edgewise connections subjected to linear elastic behavior. The quadrilateral timber plates with non-orthonormal edges can be also simulated using the macro model. The model consists of inner beams, boundary elements, two shear springs, two-node link elements, and uniaxial tension-compression springs distributed along the boundary elements.

The inner beams, which are oriented parallel (Fig. 3c) and perpendicular to the fiber orientation of the plate (Fig. 3e), provide out-of-plane flexural stiffness. A set of distributed uniaxial tension-compression springs is used along the edges of the quadrilateral macro model to simulate direct axial and in-plane flexural loads (Fig. 3d, f). Using a pair of uniaxial springs in the fiber-parallel (Fig. 3h) and fiber-perpendicular directions (Fig. 3i), the shear behavior of the timber plate is simulated. As part of the topology-based modification, the connection area is replaced with a two-node link element. Six springs are embedded in the link element, which is used to simulate the kinematic response of the joint (Fig. 3g). Because the connection area is simulated using the two-node link elements, the primary role of the boundary beams is to stabilize the macro model, and therefore, are modeled as rigid (Fig. 3j). These rigid boundary elements are pin-ended, enabling rotation about their in-plane and out-of-plane axes. The boundary elements also facilitate the distribution of force among the inner beams. Consequently, the behavior of the macro model is independent of the cross-section dimensions of the boundary element. It is worth noting that introducing the zero-length rotation-free springs at the end of the boundary elements is an example of the virtual topology adaptation.



**Fig. 2.** CAD-to-CAE data transformation using RhinoCommon .NET SDK and OpenSees, and performance assessment of IATP structures.



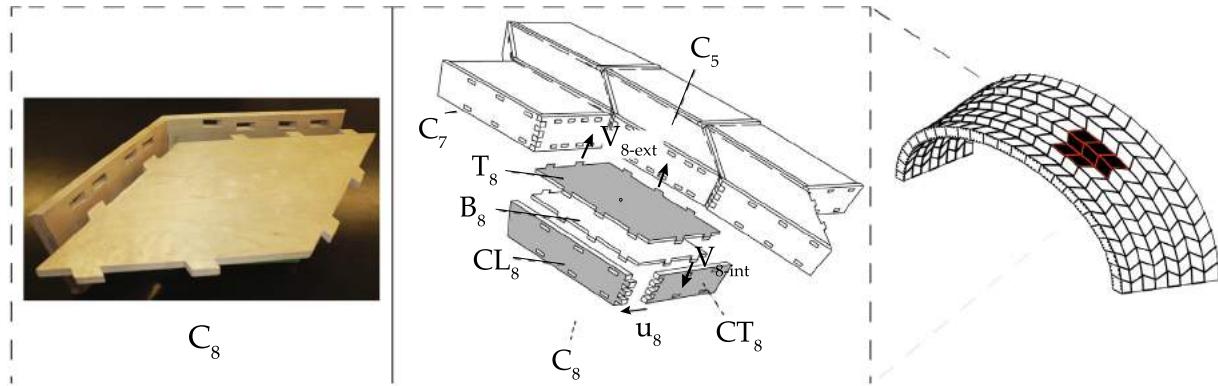
**Fig. 3.** Isometric view of (a) the real geometry, (b) the macro model of an IATP element. (c) fiber-parallel beam, (d) uniaxial fiber-parallel spring, (e) fiber-perpendicular beam, (f) uniaxial fiber-perpendicular spring, (g) IMAs, (h) fiber-parallel shear spring element, (i) fiber-perpendicular shear spring, (j) boundary elements (credit: Rezaei Rad et al. [22]).

### 3. Methodology: automatic CAD-to-CAE data exchange framework

#### 3.1. Description of the IATP system

An IATP system includes the assembly of multiple boxes denoted as  $C_i$  in Fig. 4, where  $i = 1, 2, \dots, 8$ . Each box consists of top, bottom, cross longitudinal, and cross transverse plates, which are labeled as  $T_i$ ,  $B_i$ ,  $CL_i$ , and  $CT_i$  in Fig. 4, respectively. The tenons are located along the perimeter of the  $T_i$  and  $B_i$

plates and the slots are located in the perimeter of the  $CL_i$  and  $CT_i$  plates. The cross plates are first connected via dovetail shaped IMAs and along the vector  $\mathbf{u}_i$  ( $i = 8$  in Fig. 4). The  $T_i$  and  $B_i$  plates are then simultaneously connected to the corresponding cross plates along the internal assembly vector  $\mathbf{V}_{i,int}$  to form a 4-sided box. This is illustrated in Fig. 4, which shows  $T_8$  and  $B_8$  being connected to  $CL_8$  and  $CT_8$  along the vector  $\mathbf{V}_{8,int}$ . Each box is then connected to its neighbor with only the TT joints and along the external assembly vector,  $\mathbf{V}_{i,ext}$  ( $i = 8$  in Fig. 4).



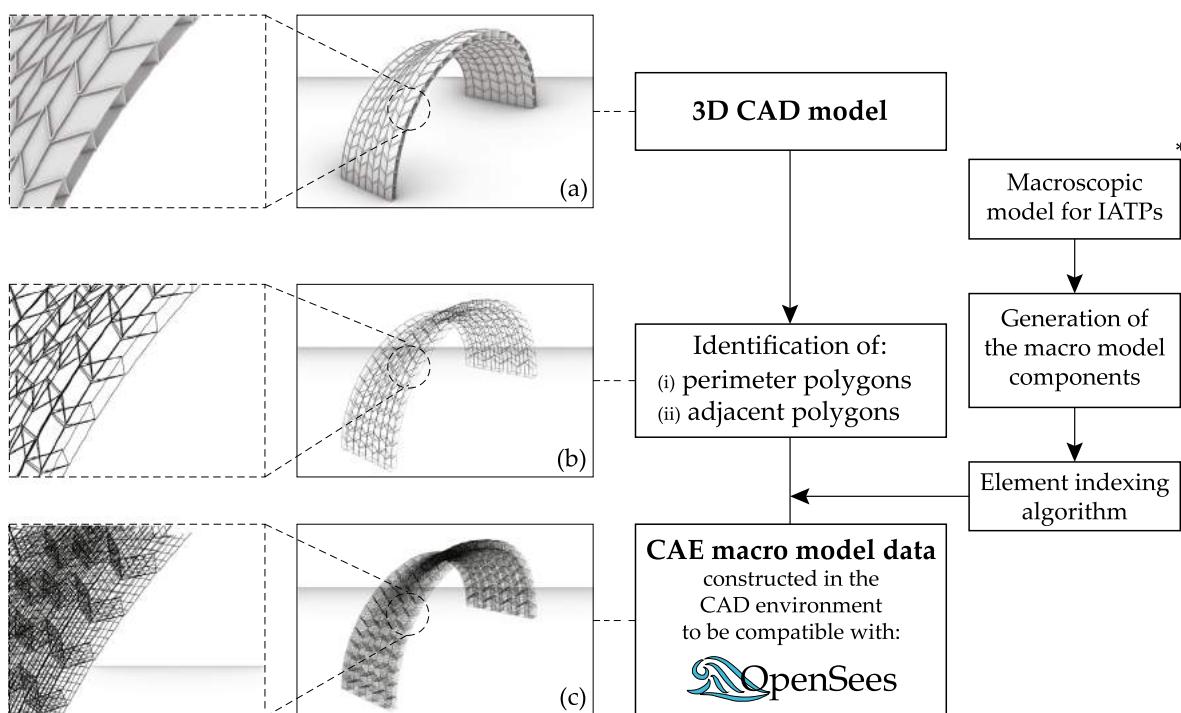
**Fig. 4.** Description of the Integrally-Attached Timber Plate (IATP) system.

### 3.2. CAD-to-CAE macro model exchange

The current investigation aims to add another layer to the design of IATPs by incorporating the macroscopic models in the structural analysis of large-scale structures. Toward this goal, an algorithmic framework for generating the CAE macro model from a custom-defined CAD 3D model is introduced. The framework consists of multiple steps. First, an algorithm is introduced to compute the perimeter polygon associated with the mid-surface of each quadrilateral 3D timber plate element, convert the plate from a 3D solid to a 1D polygon, and assign a unique tag to each polygon (Fig. 5a-to-b). The details of this step are presented in Section 3.2.1. Next, by retrieving the neighboring polygons from the mesh, 1D line elements are defined and used to represent the connection between adjacent polygons. Section 3.2.2 discusses the details of this step. The data associated with the polygons and connections and their ID tags are then stored as a list. In the next step, a new algorithm is formulated to generate the components of the macro model from the list of data generated in the previous steps (Fig. 5b-

to-c). The formulation of this algorithm is detailed in Section 3.2.3. Since a typical IATP structure consists of hundreds of timber plates with thousands of joints, assigning the relevant mechanical properties to each beam/spring element of the macro model would not be possible without a systematic identification protocol. Therefore, Section 3.2.4 formulates an indexing algorithm to assign a unique tag to each component of the macro model.

According to the proposed workflow, and given the CAD 3D geometry of an IATP structure, a data structure is designed to encapsulate the geometry of the macro model components with the corresponding ID tags, and their mechanical properties. In other words, the data structure consists of the coordinates of beam and spring nodes, each of which is assigned a unique index. The material and cross-sectional properties associated with each beam and spring are also included in this data structure. The output provided for the CAE macro model (Fig. 5c) follows the syntax of OpenSees, an object-oriented open-source software for structural modeling and response simulation. As such, the data structure formulated within the CAD environment can generate the data



**Fig. 5.** An overview of the automatic CAD-to-CAE data exchange framework for IATPs (Figure note: \* The macroscopic model for IATPs is originally developed by Rezaei Rad et al. [22]).

required for structural analysis. This, in particular, enables the user to work in the CAD modeling environment while generating the data required for a CAE platform.

The identification of the perimeter polygons and IMAs and associated indexing algorithms (Fig. 5b), as well as the automatic generation of the macro model components and associated indexing (Fig. 5c), are introduced to Rhinoceros 3D [6] as plug-ins using Application Programming Interfaces (APIs). While APIs within Rhinoceros 3D are available in different programming languages such as C++, C#, VB, Python, and VBScript, they use the same .NET RhinoCommon library [20]. Grasshopper 3D [7], which is a graphical algorithm editor within Rhinoceros 3D that enables visual programming, was used to edit the plug-ins and components.

### 3.2.1. CAD-to-CAE Exchange, Module 1: Identification of perimeter polygons and associated indexing

The identification of the perimeter polygon associated with each timber plate is the primary step in the CAD-to-CAE data exchange framework. This step is central to the CAE macro model development because the inner beams are generated by dividing the perimeter polygons into equal segments along the fiber-parallel and fiber-perpendicular directions and adding inner lines at the segment interfaces. From the CAD 3D model (Fig. 6a), the mid-surface plane of each timber plate is recalled (Fig. 6b). Next, using fabrication contours available from the CAD 3D model, the basic mesh edges are retrieved (Fig. 6c). Using the *Intersection.Line-Plane* method available in the RhinoCommon library [20], the edges are intersected with the mid-surface planes. The four corner nodes of the quadrilateral mid-surface are identified (Fig. 6d). Accordingly, a polygon element crossing at these four nodes is generated (Fig. 6e). The polygon element is represented by segmented lines.

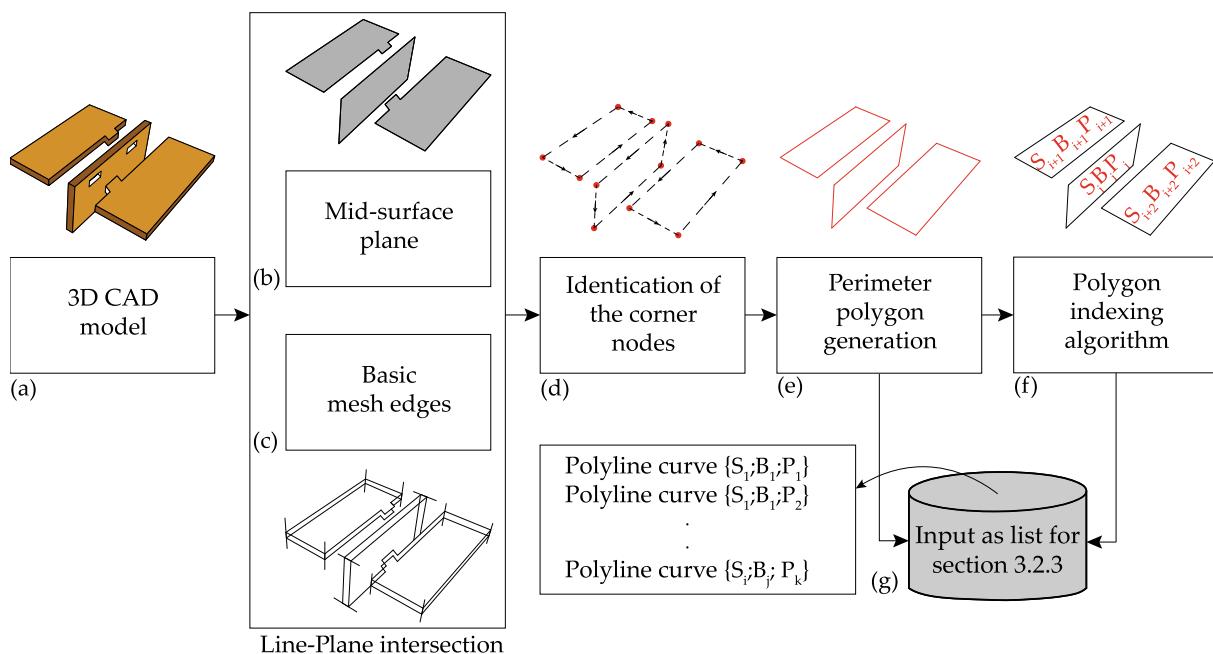
The 3D CAD model assigns a unique identifier index to the following components: strips, boxes, and plates, which are schematically shown in Fig. 7. The most complex IATP structures are those with free-form geometries (Fig. 7a). In these situations, the structure is constructed by assembling multiple strips (Fig. 7b). Each strip is an assembly of multiple boxes (Fig. 7c) and each box consists of four IMA-connected timber plates (Fig. 7d). This data-

structure enables the user to readily find the position of a timber plate by concatenating three numbers. Therefore, each polygon is recognized in 3D space with the following concatenated three number sequences:  $S_iB_jP_k$ , where  $S_i$ ,  $B_j$ , and  $P_k$ , correspond to  $i^{\text{th}}$  strip,  $j^{\text{th}}$  box, and  $k^{\text{th}}$  plate. The algorithm can also accommodate simpler geometries such as beams with hollow cross sections, in which case, the first two types of components (strips and boxes) are not used and only the plate identifier is employed. This will be discussed in Section 4.1.

After identifying the perimeter polygon corresponding to each timber plate, the associated identifier is retrieved from the CAD model (Fig. 6f), and both the polygon geometry and the identifier are collected in a sorted list format using Grasshopper DataTree [28] (Fig. 6g). The list is then used in the module described in Section 3.2.3 to construct the macro model components. The pseudocode associated with this module is provided in Appendices 1.2 and 1.3.

### 3.2.2. CAD-to-CAE exchange, Module 2: Identification of IMAs and associated indexing

The second module of the CAD-to-CAE exchange uses 1D line elements to represent the connection between neighboring polygons. These lines serve as mechanical joints in the macro model according to Fig. 3g. To simplify the IMA geometry, each connection is represented by two points, one in the plate containing the tenon and the other in the plate containing the slot. To compute these points, the mid-surface associated with timber plates is recalled from the CAD 3D model (Fig. 8a, b). Also, the data associated with the neighboring plates are retrieved from the mesh. Next, a plane perpendicular to the mid-surface of each plate is introduced to the middle of the tenon, and the intersection point is identified (Fig. 8c). This point represents the tenon of the IMA. Then, by using the *Line.ClosestPoint* method available in the RhinoCommon library [20], the tenon's mate, which is located in the neighboring polygon, is identified (Fig. 8d). Next, the line element is defined by connecting the two polygons (Fig. 8e). This modeling technique is in accordance with the assumption made for the macroscopic model where each IMA is simulated using a two-node link.



**Fig. 6.** Perimeter polygon identification algorithm.

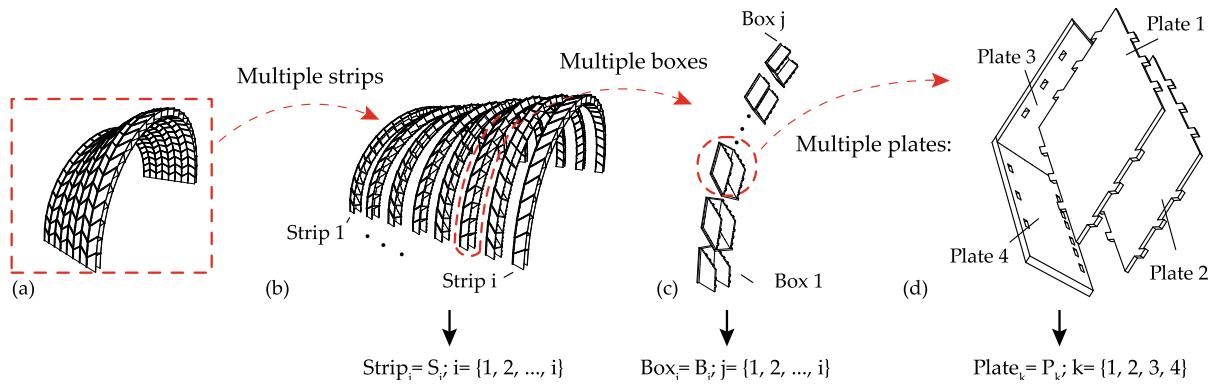


Fig. 7. A schematic representation of the polygon indexing algorithm.

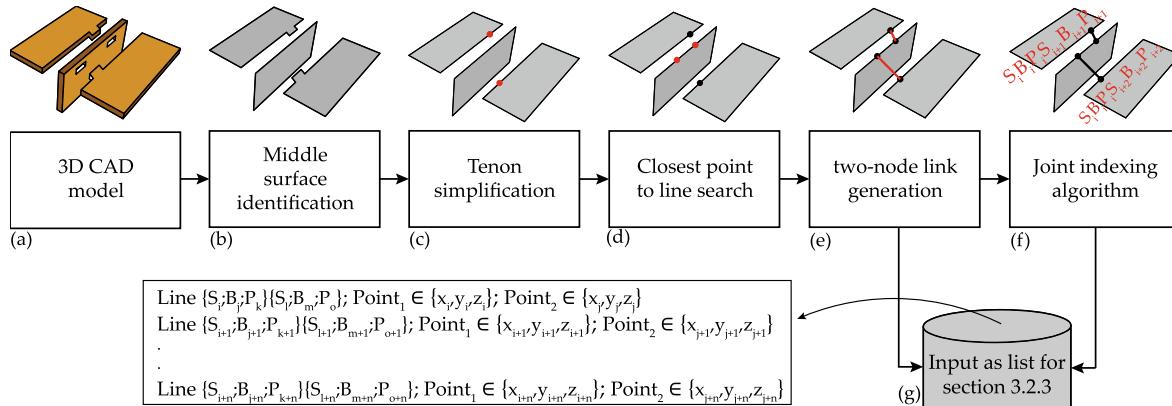


Fig. 8. Identification of wood-wood connections and the corresponding simplification process using 1D line elements.

After identifying the perimeter polygon, an indexing algorithm is introduced to assign a unique identifier to each 1D line element (Fig. 8f). Thousands of joints exist in a typical IATP structure and each joint has specific mechanical characteristics based on its geometry and fiber orientation. Therefore, assigning a unique ID to each joint is central to the construction of the corresponding numerical model.

The most convenient way to assign a unique identifier to each 1D line element is to build a new layer of information using the existing perimeter polygon IDs. Given that a joint primarily serves to connect two adjacent polygons, the indexing algorithm is formulated to include the indices of the two connected polygons. Additionally, since two neighboring polygons can be associated with more than one joint, another counter is added to the joint identifier. In other words, the indexing algorithm includes a seven number sequence where the first six identify the strip, box, and plate number of the first and second polygons and the 7th number,  $Jnt_m$ , identifies the  $m^{\text{th}}$  joint. Accordingly, the index of the  $m^{\text{th}}$  joint element which connects two adjacent polygons in 3D space becomes  $S_i;B_j;P_k;S_x;B_y;P_z;Jnt_m$ . Fig. 9 shows the logic of the indexing algorithm.

Finally, the geometry of the generated lines and associated indices are collected in a list format using Grasshopper DataTree (Fig. 8g) and used in the module described in Section 3.2.3. The pseudocode associated with the generation of the 1D lines is provided in Appendix 1.4.

Generally, the orientation of timber fibers is parallel to that of the length of each polygon (vector  $e_1$  in Fig. 10). The local coordinate system for each joint is shown in Fig. 10. Given the  $i^{\text{th}}$  timber plate, the direction of the plate edge and the vector normal to the

plate are defined as  $e_1$  and  $n_i$  in Fig. 10, respectively. Accordingly, the orientation of each joint is determined by computing the cross product of the vectors  $e_1$  and  $n_i$  ( $e_1 \times n_i$ ).

### 3.2.3. CAD-to-CAE exchange, Module 3: Generation of the macro model components

A generic algorithm, shown in Fig. 11, is introduced in this section to generate the components of the macro model associated with a custom-defined timber plate within an IATP structure (Fig. 3). The main steps are as follows:

- (1) Using the list of input data containing the perimeter polygons and associated indices derived from Section 3.2.1, the four corner nodes are retrieved (Fig. 11b in green) and two additional nodes are generated at each corner (Fig. 11b in red). These additional nodes have the same coordinates as the corresponding corner node, and they will be used to construct zero-length rotationally-free elements in the macro model. This is in accordance with the modeling requirement of the boundary elements in the macro model (Fig. 3j).
- (2) The inner beams of the macro model are constructed by introducing lines with equal distances along the fiber-parallel (Fig. 11c) and fiber-perpendicular directions (Fig. 11d). A virtual polyline connecting the four corner nodes is defined and the length and width of each timber plate are determined. Recalling that the fiber-parallel and fiber-perpendicular directions correspond to the length and width of the timber plate, respectively, these two directions are divided into equal segments. Line elements are then added at the segment interfaces to serve as the inner

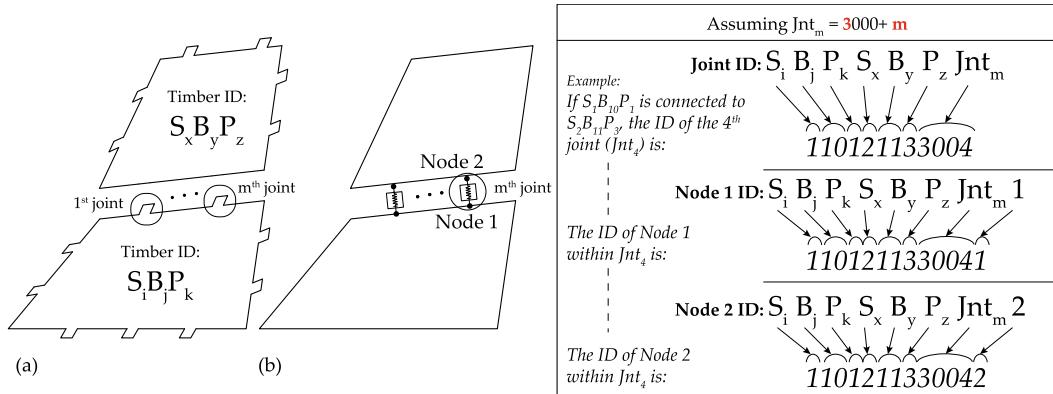


Fig. 9. Joint indexing algorithm.

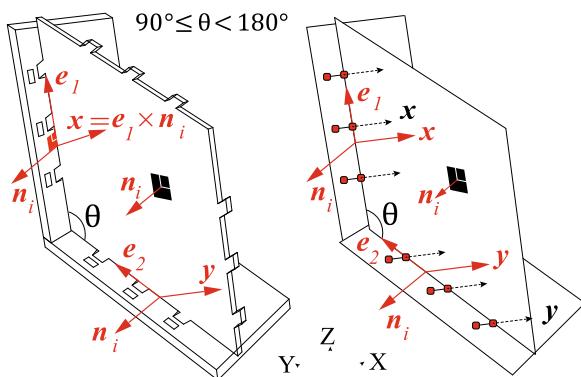


Fig. 10. Identifying the local coordinate system for timber joints (IMAs).

beams for the macro model, each with a unique local coordinate system. The cross-sectional properties of each inner beam element are computed based on its tributary area, and the material properties are assigned based on its fiber orientation.

- (3) Computing the geometric transformation of the elements (Fig. 11e) and using the wood orthotropic properties (Fig. 11f), the data associated with the fiber-parallel (Fig. 11g) and fiber-perpendicular beam elements and nodes (Fig. 11h) is stored in a text file with .tcl format using the OpenSees syntax.
- (4) While defining the beams and springs using the OpenSees syntax, the algorithm assigns each element a unique identification index. The logic behind the indexing algorithm is explained in Section 3.2.4.
- (5) Using the list of input data containing the joints and associated indices derived from Section 3.2.2 (Fig. 11i), the end nodes of the joints are retrieved (Fig. 11j). The mechanical properties of the IMAs derived from physical experiments [11,12,21] are assigned to the joints (Fig. 11m). Using the OpenSees element library, the *two-node link element* with six discrete uncoupled springs is employed to simulate the behavior of the IMAs (Fig. 11n). The elements are stored in a text file with .tcl format and the OpenSees syntax (Fig. 11o), each with the unique identification tag explained in Section 3.2.2.
- (6) The boundary elements of the macro model are generated by connecting the corner nodes, corner release nodes, end nodes of the fiber-parallel and fiber-perpendicular inner beams, and end nodes of the joints (Fig. 11k). A uniaxial elastic material with infinite stiffness is assigned to the bound-

ary elements. These elements are then stored in a text file with .tcl format using the OpenSees syntax (Fig. 11l). While constructing the boundary elements, the algorithm assigns each element the unique identification index explained in Section 3.2.4.

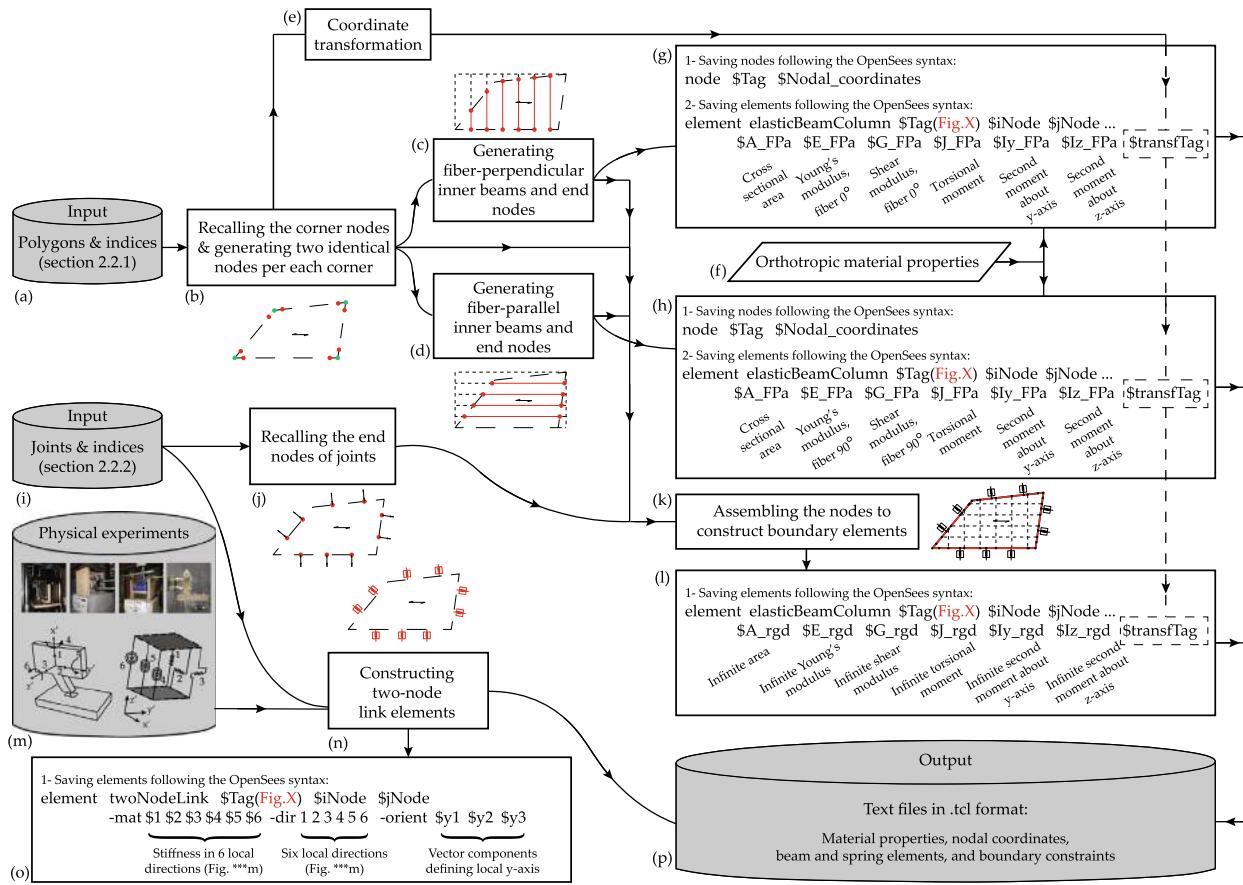
- (7) The .tcl files generated in the previous steps are compiled, and the CAE macro model is generated. Given that a systematic identification protocol is established, the elements that are subjected to loads are parametrically defined. Analysis handlers are specified, and the CAE model is used for structural analysis in OpenSees (Fig. 11p).

### 3.2.4. CAD-to-CAE exchange, Module 4: Nodes and elements indexing

The algorithm in this module automatically generates the components of the macro model associated with an IATP element and assigns them a unique identification index. This is central to the CAD-to-CAE data transformation framework because to construct the OpenSees numerical model, each element of the macro model and the corresponding nodes should be indexed with an identifier. In the algorithm, (1) Crn<sub>m</sub> is the tag that corresponds to the m<sup>th</sup> corner node shown in Fig. 11b in green, (2) Pcrn<sub>m</sub> is the tag that corresponds to the m<sup>th</sup> corner node used to construct the rotationally-free zero-length element (Fig. 11b in red), (3) Trs<sub>m</sub> is the tag that corresponds to the m<sup>th</sup> fiber-perpendicular inner beam and its associated nodes (Fig. 11c), (4) Lng<sub>m</sub> is the tag that corresponds to the m<sup>th</sup> fiber-parallel inner beam and its associated nodes (Fig. 11d), and (5) Cnt<sub>m</sub> is the tag that corresponds to the m<sup>th</sup> boundary element. These tags are then linked to the ones developed for each polygon.

For each plate, the first three numbers are associated with the polygon tag, described in Section 3.2.1, and the fourth number represents the macro model component type, as discussed in the previous paragraph. Accordingly, the index of a corner node and the nodes used for the rotationally-free zero-length element becomes S<sub>i</sub>B<sub>j</sub>P<sub>k</sub>Crn<sub>m</sub> (Fig. 12a) and S<sub>i</sub>B<sub>j</sub>P<sub>k</sub>Pcrn<sub>m</sub> (Fig. 12b), respectively. The index of the fiber-parallel inner beam, fiber-perpendicular inner beam, and boundary element of the macro model becomes S<sub>i</sub>B<sub>j</sub>P<sub>k</sub>-Lng<sub>m</sub> (Fig. 12c), S<sub>i</sub>B<sub>j</sub>P<sub>k</sub>Trs<sub>m</sub> (Fig. 12d), and S<sub>i</sub>B<sub>j</sub>P<sub>k</sub>Cnt<sub>m</sub> (Fig. 12e), respectively. The pseudocode associated with the generation of the macro model components and indexing algorithm is provided in Appendix 1.4. The algorithm is written in the Rhino3D CAD environment using the *RhinoScriptSyntax* library [29] which allows the user to create, access, and manipulate a list of points, vectors, lines, surfaces, and non-uniform rational basis spline (NURBS).

Assigning a unique tag to each component of the macro model within a timber plate has multiple advantages. First, it minimizes the number of manual operations and allows the rapid identification of each component. Given that a typical IATP structure con-



**Fig. 11.** Generation of the macro model components and data storage.

sists of hundreds of timber plates with thousands of joints, assigning the relevant mechanical properties to each beam/spring element of the macro model would not be possible without a systematic identification protocol. Furthermore, using the indexing algorithm, the compatibility between the CAD and CAE data structures is maintained in a single platform. Moreover, after assigning the indices to the elements and nodes, the loaded elements within a structure and boundary conditions are readily identified.

#### 4. Verification and case studies

In this section, the algorithm is used to construct the CAD-to-CAE macro model and analyze prefabricated IATP structures with standard (Section 4.1) and free-form (Section 4.2) geometries. FE models as well as the results from recent experiments performed on full-scale prototypes are used to verify the proposed framework.

##### 4.1. Standard prefabricated IATPs

Using the IMA technique, prefabricated timber plates have been used to construct structural components with standard shapes. Gamarro et al. [30] introduced the construction of large span standard beams with hollow cross sections using segmented timber elements. The structural element consists of either one or two layers of timber plates for each web or flange. Fig. 13a, b shows single- and double-layer assemblies with hollow cross sections, respectively.

##### 4.1.1. Generation of the CAD/CAE macro model

The algorithmic CAD-to-CAE data exchange framework is used to generate the CAE macro model corresponding to an assembly of timber plates with standard geometry, which is then verified against the results of a four-point bending test performed on a full-scale assembly. Three replicates of the assembly, shown in Fig. 14a, were tested by Gamarro et al. [30]. Similar to Fig. 13b, the assembly consists of two layers of timber plates for each flange and web. Oriented Strand Board (OSB) is used for the two webs. For the flanges, the inner and outer layers consist of Kerto Laminated Veneer Lumber (LVL) and OSB boards, respectively. The dimensions of the timber plates and the test setup designed by Gamarro et al. [30] are shown in Fig. 14a-f. The geometry of the CAD model is regenerated in this study using the details provided in Figure A1 in [30], however, the discontinuity between the timber plates within each flange or web is not considered to simplify the CAD model. Furthermore, the small transverse plates used in the reference model [30] were not included in the re-generated CAD model.

Within the context of the CAE macro model generation framework shown in Fig. 5, the assembly shown in Fig. 14a is analogous to a box component in the previously described polygon identification algorithm. As per Sections 3.2.1 and 3.2.2, the perimeter polygons associated with the rectangular plates are first identified, a unique tag is assigned to each one, and the connections between the timber plates are then defined. To simplify the modeling process, the two webs are merged into a single layer with equivalent cross-section properties. Next, and according to Sections 3.2.3 and 3.2.4, the components of the macro model are generated for each polygon, a unique tag is defined for each component, and the associated mechanical properties are computed. Fig. 14g-n shows the corresponding CAE macro model.

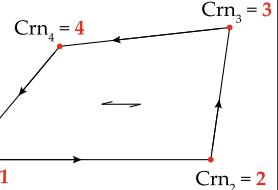
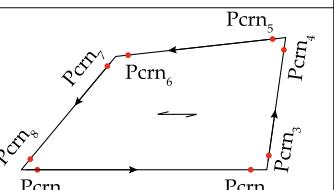
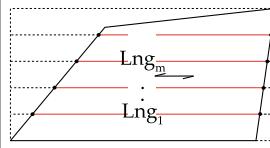
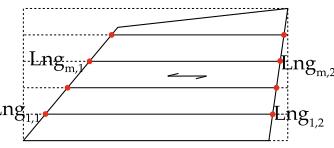
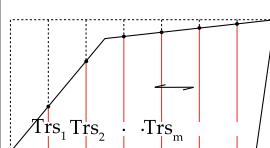
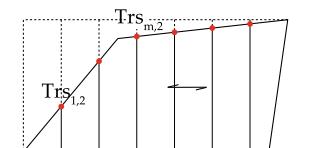
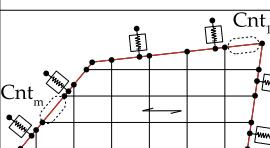
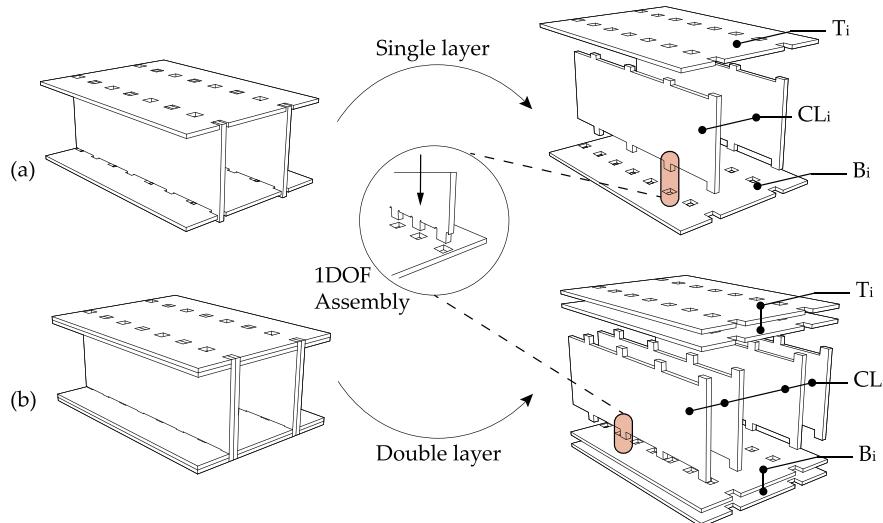
Property	Indicator	Assumption	Element	Element ID	Node	Nodes ID
(a) Corner nodes	$\text{Crn}_i$ $i \in \{1,2,3,4\}$	$\text{Crn}_m = m$	Not applicable	Not applicable		$S_i P_k \text{Crn}_m$
<i>Example:</i> For Strip #1, Box #12, and Plate #3: Nodes ID: 11231, 11232, 11233, 11234						
(b) Pins at the four corners	$\text{Pcrn}_i$ $i \in \{1,2,\dots,8\}$	$\text{Pcrn}_m = 5000 + m$	Not applicable	Not applicable		$S_i P_k \text{Pcrn}_m$
<i>Example:</i> For Strip #1, Box #12, and Plate #3: Nodes ID: 11235001, 11235002, 11235003, 11235004, 11235005, 11235006, 11235007, 11235008						
(c) Fiber-parallel inner beam	$\text{Lng}_i$ $i \in \{1,2,\dots,n\}$	$\text{Lng}_m = 1000 + m$		$S_i P_k \text{Lng}_m$		Node 1: $S_i P_k \text{Lng}_m 1$ Node 2: $S_i P_k \text{Lng}_m 2$
<i>Example:</i> For the 4 <sup>th</sup> fiber-parallel inner beam element within Strip #1, Box #12, and Plate #3: Element ID: 11231004, Node 1 ID: 112310041, Node 2 ID: 112310042						
(d) Fiber-perpendicular inner beam	$\text{Trs}_i$ $i \in \{1,2,\dots,n\}$	$\text{Trs}_m = 2000 + m$		$S_i P_k \text{Trs}_m$		Node 1: $S_i P_k \text{Trs}_m 1$ Node 2: $S_i P_k \text{Trs}_m 2$
<i>Example:</i> For the 4 <sup>th</sup> fiber-perpendicular inner beam element within Strip #1, Box #12, and Plate #3: Element ID: 11232004, Node 1 ID: 112320041, Node 2 ID: 112320042						
(e) Boundary elements	$\text{Cnt}_i$ $i \in \{1,\dots,n\}$	$\text{Cnt}_m = 4000 + m$		$S_i P_k \text{Cnt}_m$	Not applicable	Not applicable
<i>Example:</i> For the 4 <sup>th</sup> boundary element within Strip #1, Box #12, and Plate #3: Element ID: 11234004 Note: the node IDs associated with the boundary elements have been determined in the previous steps.						

Fig. 12. Automatic indexing algorithm for the macro model components.

For each timber plate used in the flanges, the inner beam elements of the corresponding macro model are spaced at 400.0 mm and 150.0 mm along the fiber-parallel and fiber-perpendicular directions, respectively. For the web plates, the

inner beams of the corresponding macro model are spaced at 150.0 mm along both the fiber-parallel and fiber-perpendicular directions. After computing the tributary area associated with each inner beam, the cross-sectional properties are determined. The



**Fig. 13.** Standard beams using single and double-layer timber plates with IMAs.

mechanical properties of the timber material ([Table 1](#)) are assigned to each element based on its fiber orientation. Additionally, the properties of the shear and tension-compression springs are computed and assigned to the respective elements. Moreover, the load-deformation response of the IMAs, investigated by Gamarro et al. [[31](#)], is assigned to the two-node link elements. [Fig. 14k-l](#) shows the section size of the inner beam elements, as well as the stiffness values for the uniaxial tension-compression, shear, and the IMA springs.

Using the proposed CAD-to-CAE data exchange, the mean force values and the corresponding stresses of a given cross section can be computed. Given the timber plate cross-section (i.e. section D-D in [Fig. 14l](#)), the corresponding mean force is computed by averaging the forces of the inner beams. Additionally, the corresponding mean stress is computed by averaging the stress values of each inner beam element. This is detailed in [Fig. 14m](#).

#### 4.1.2. Results and discussions

The assembly shown in [Fig. 14a](#) was subjected to a static load at its mid-span [[30](#)]. The vertical mid-span displacement of the assembly was recorded using a Linear Variable Differential Transducer (LVDT). The corresponding CAE macro model is generated and subjected to the same loading protocol used in the physical experiment. The load-displacement curves obtained from the macro model and the physical specimen are shown in [Fig. 15](#). The performance of the assembly using the macro model is evaluated up to the peak strength. This corresponds to the maximum strength of the physical assembly, and according to [Fig. 15](#), it is equal to 33.2 kN.

The results in [Fig. 15](#) show that the responses obtained from the macro model are similar to the experimental load-deflection behavior. The initial stiffness obtained from the macro model and the physical specimen is computed from the associated load-deflection curves in [Fig. 15](#). The average difference between the stiffness from the macro model ( $K_{Macro} = 730.46\text{N/mm}$ ) and the physical experiment ( $K_{Test} = 822.4\text{N/mm}$ ) is 11.1%, which represents an acceptable degree of accuracy. The adequate performance of the macro-model is attributed to (i) the symmetry of the beam assembly and the regular configuration of the constituent timber plates (which are rectangular), and (ii) the fact that the overall performance of the system is primarily governed by the IMAs. More specifically, since the mechanical properties of the IMAs have been carefully characterized in Gamarro et al. [[30](#)][[31](#)] and used in the

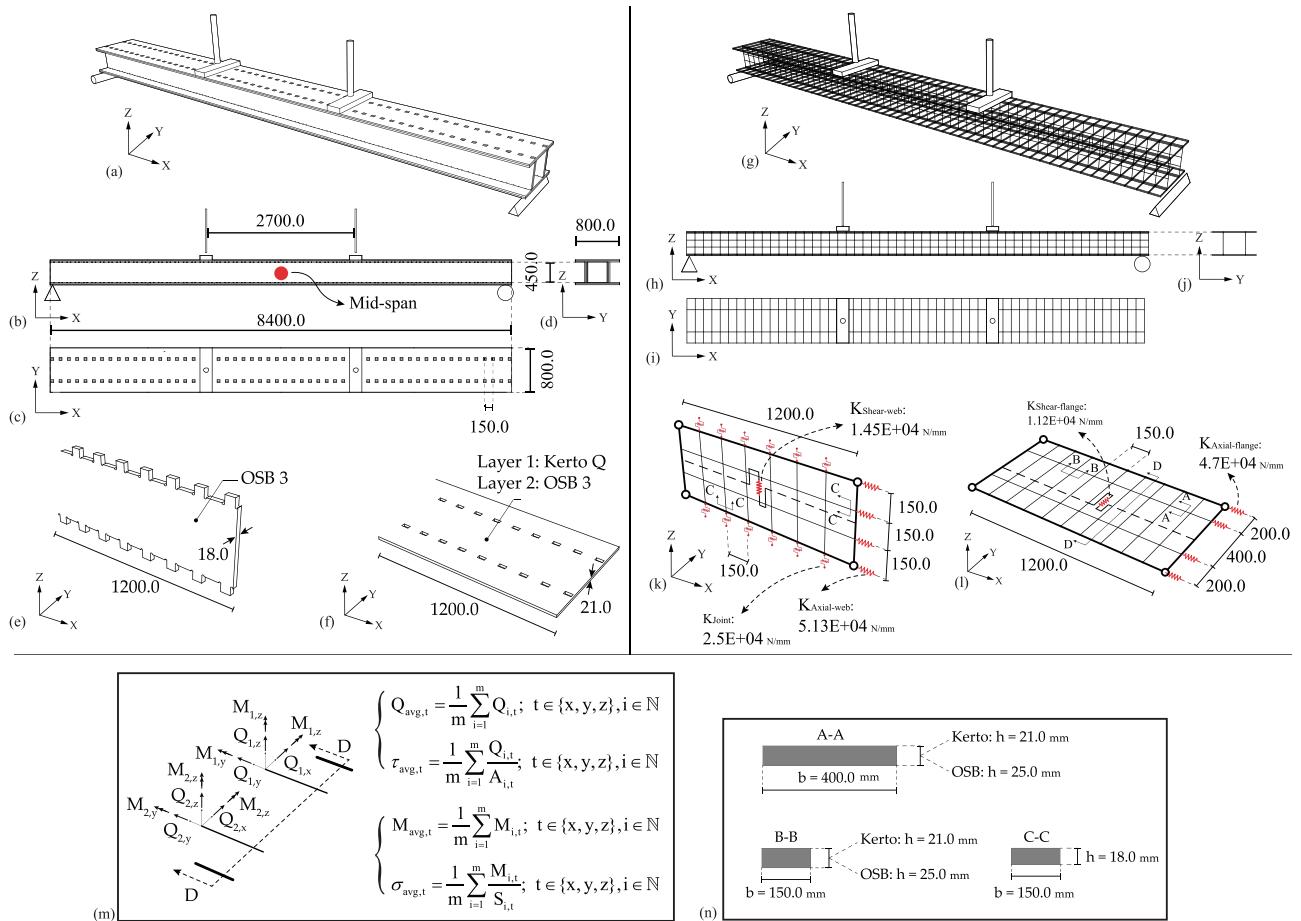
macro model, the predicted load-deformation behavior is comparable to the measurements from the physical experiment. The accuracy of the model can be improved by including the compression and embedment behavior of the IMAs in the macroscopic behavior. Moreover, according to Table 4 in Gamarro et al. [[30](#)], a brittle tensile failure was observed in the outer layer of the bottom flange, where the maximum tensile stress was 5.36 MPa. According to the results obtained from the macro model, the mean tensile stress at the maximum strength, which corresponds to the failure state, is 4.93 MPa in the same outer layer of the bottom flange. This represents an acceptable degree of accuracy in computing the state of stress in the timber elements using the macro model.

Using the proposed CAD-to-CAE data exchange, the CAD geometry is translated to the corresponding CAE macro model with an Intel Core i7-4800MQ CPU @2.7 GHz and 16 GB of RAM machine. The time required to generate the CAE macro model is approximately 3 s with 15% of the CPU and 280 MB of memory utilized. The CAE model analysis runtime is approximately 2 s, using 0.4% of the CPU and 7.8 MB of memory.

## 4.2. Spatial free-form IATPs

### 4.2.1. Description of the system and generation of the CAD/CAE macro model

Using the IMA technique, prefabricated timber plates are assembled to construct free-form large-scale IATP structures. Detailed documentation of the assembly and construction processes and geometry generation can be found in Rezaei Rad et al. [[11](#),[12](#)], Nguyen et al. [[15](#)], and Robeller et al. [[14](#)]. In this section, the proposed algorithmic framework is used to generate and analyze the CAE macro model associated with a double-layered double-curved prototype that was constructed by ANNEN SA in Manternach, Luxembourg [[34](#)]. The entire structure consists of 23 free-form and separate IATP arches, each with a specific and custom-defined geometry. The span and height of the arches range between 22.5 to 53.7 m, and 6.0 to 9.0 m, respectively. [Fig. 16a](#) shows the 23 arches along with a schematic representation of the design target surface (denoted as "A" and shown for arches 1 to 5), and the design pattern and the surface discretization (denoted as "B" and shown for arches 6 to 10). The CAD 3D model (denoted as "C" and shown for arches 11 to 15), the CAD 2D model with mid-surface elements (denoted as "D" and shown for arches 16 to 19), and the CAE macro model associated with arches 20 to



**Fig. 14.** Prefabricated beams: (a) 3D perspective view of the CAD model (re-generated from [30]), (b-d) side and top views, (e) dimensions of a typical web, (f) dimension of a typical flange, (g) 3D perspective view of the CAE macro model, (h-j) side and top videws of the macro model, (k) macro model for the web, (l) macro model for the flange, (m) computation of force and stress in section D-D, (n) dimensions of sections A-A, B-B, and C-C.

23 (denoted as "E") are also shown in Fig. 16. The overall design process, from the target surface generation to the construction of the CAE macro model, is schematically illustrated for arch #22 Fig. 16b.

The performance of arch #22 (Fig. 17a) was experimentally evaluated by Nguyen and Weinand [35]. This arch consists of eight strips, each made from 25 boxes and each box is constructed through the assembly of four timber plates (Fig. 17b). The top and bottom plates have non-orthonormal edges while the cross plates are rectangular. Portions of the structure were loaded with 25 kg cement bags. Fig. 17a shows the top view of arch #22 and the timber boxes that were loaded by the cement bags. The number of the cement bags used for each box is also shown in Fig. 17a. A total load of 55 kN was applied to the structure. Based on the area loaded by the cement bags, the average surface load was 1.5 kN/m<sup>2</sup>. An electronic total station was used to measure the displacement at 16 different points on the structure. A 3D terrestrial laser scanning technique using FARO® [36] was also used to measure structural displacements.

The vertical displacements measured by the laser scanner and the location of the 16 points measured by the electronic total station are shown in Fig. 18a and also documented in Table 3. Nguyen and Weinand [35] observed that there is considerable difference between the displacements recorded by the total station device and the 3D laser scanner. They concluded that the cloud-cloud distance computation used in the 3D laser scanner depends on the density of the reference cloud. Furthermore, the accuracy of the point clouds depends on the computational method used to cali-

brate (register) the laser scanner. Using five spherical targets, the point cloud method leads to errors of 10 mm when computing the displacements. The vertical displacements obtained from a FE model is also shown in Fig. 18b and documented in Table 3. The FE continuum model, constructed in ABAQUS™, utilizes shell elements to simulate the behavior of the timber plates, and springs to simulate the behavior of the IMAs [15,35].

Using the algorithmic framework, the CAE macro model of arch #22 is generated and analyzed. The top view of the CAE macro model geometry is shown in Fig. 18c. The inner beam elements of the macro model are generated by dividing the plates into four equal segments along the fiber-parallel and fiber-perpendicular directions. The tributary area associated with each inner beam is automatically computed and the cross-section properties are accordingly defined. The mechanical properties of the timber material, which are given in Table 2, are assigned to each inner beam element based on its fiber orientation. The macro model is subjected to the same load used in the physical experiment. The surface loads applied by the cement bags are divided into uniform loads and applied to the beam elements of the macro models.

#### 4.2.2. Results and discussions

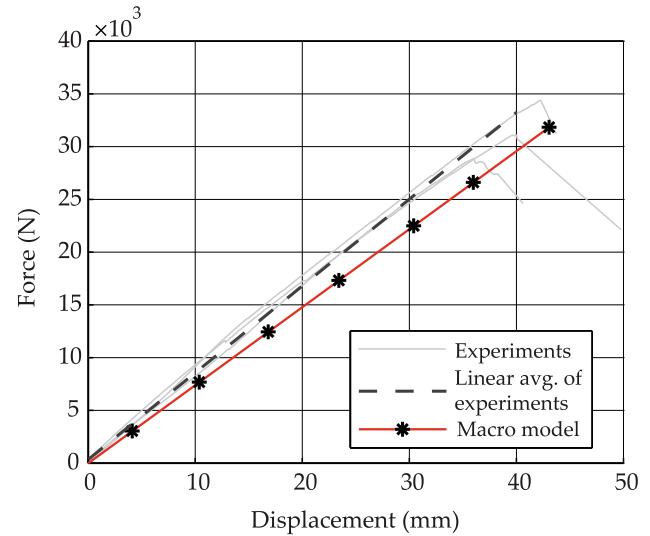
Table 3 compares the results obtained from the macro and FE models, as well as the measurements from the physical specimen. The macro model is 21.56%, 30.92%, and 22.31% different when compared to the total station measurements, laser scanning measurements, and the continuum FE model, respectively. Considering the scale of the structure and the differences in responses from the

**Table 1**

Material Properties of Kerto-Q LVL [32] and OSB 3 [33,30].

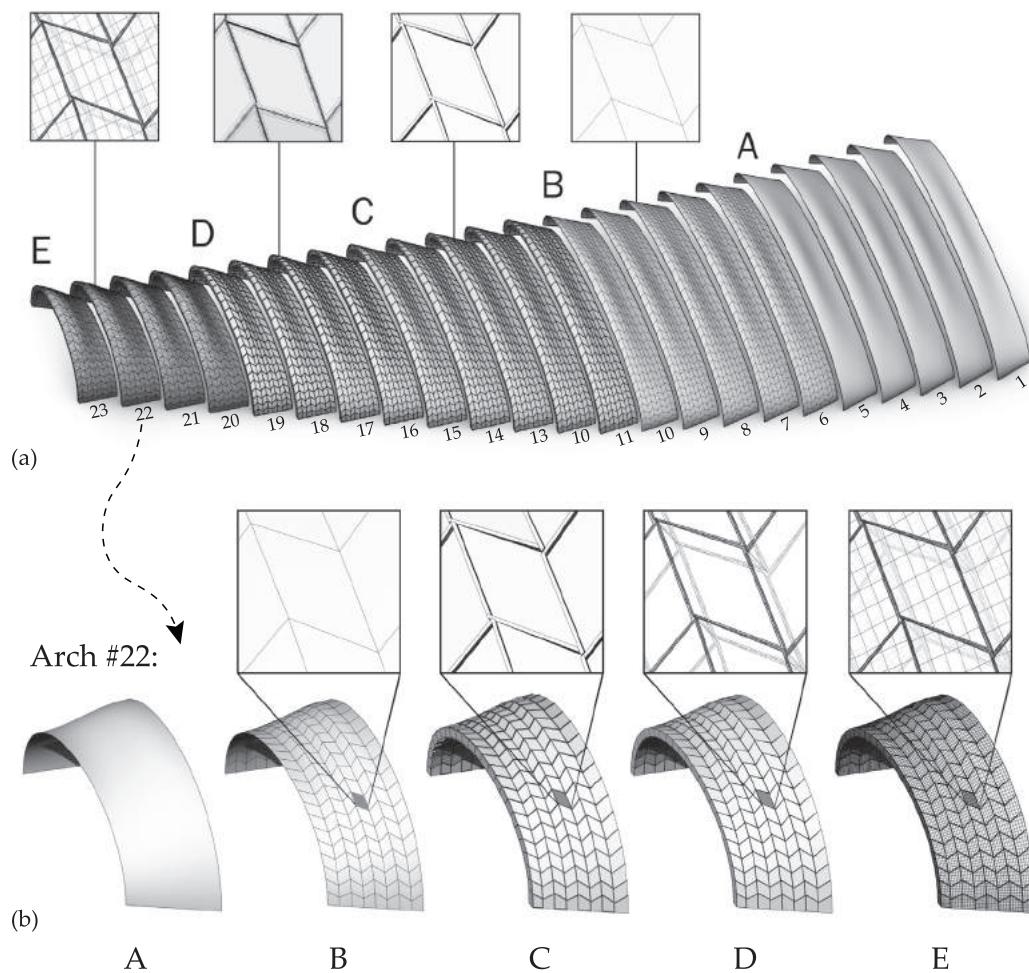
Thickness (mm)	OSB 3		Kerto Q - LVL
	18	25	21
Position	Flatwise	Edgewise	Flatwise
Density (kg/m <sup>3</sup> )	576	600	481
$E_0, \text{mean}$ (N/mm <sup>2</sup> )	4930	3800	10,000
$E_{90}, \text{mean}$ (N/mm <sup>2</sup> )	1980	3000	3300
$G_0, \text{mean}$ (N/mm <sup>2</sup> )	50	1080	60

two types of physical measurements, the performance of the macro model is deemed reasonable. Furthermore, it is observed that the responses from the macro model are closer to that of the experimental results when compared to the FE model. There are several possible reasons for this observation. First, the FE model is mesh sensitive. More specifically, the size of meshes, especially near the joints, can affect the response. Furthermore, because the 1D wire elements used to simulate the TT joints are connected to the 2D shell elements with high mesh density, the continuum FE model is susceptible to numerical instabilities. Also, depending on the element type, adopted integration scheme and mesh size, the FE models were susceptible to variations in the response. It has also been shown that detailed FE simulations do not always provide more accurate responses when compared to simpler models, especially in large-scale structures [38,39]. In fact, while large number of input parameters are used to construct a continuum FE model, the influence of each parameter on the global behavior is

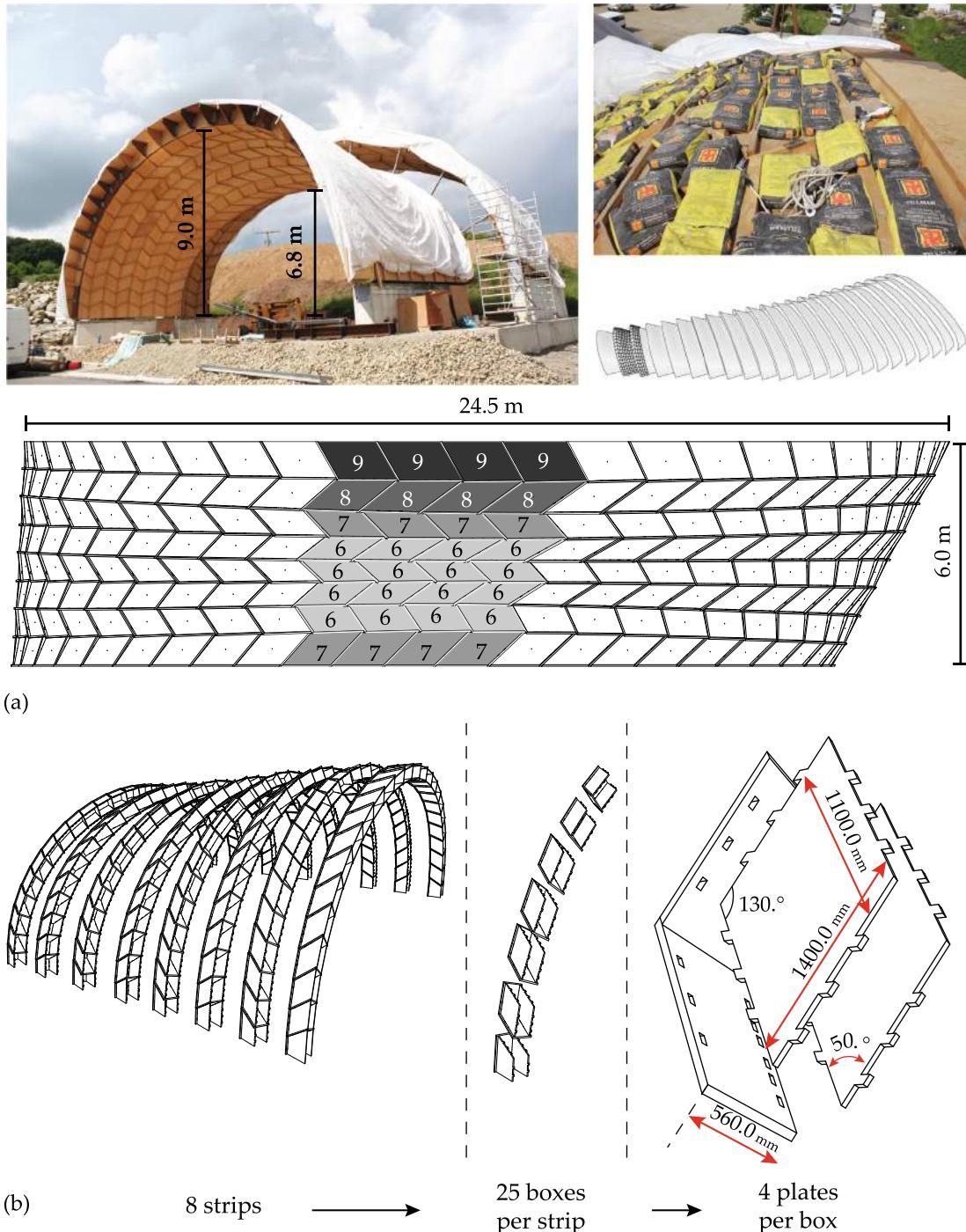


**Fig. 15.** Force-displacement response from the physical experiment [30] and the macroscopic model developed by the proposed CAD-to-CAE framework.

not clearly understood. On the other hand, the macro model uses a relatively small number of input parameters and their role in the global behavior is much better understood. However, it is



**Fig. 16.** (a) The geometry of the 23 IATP free-form arches, (b) schematic representation of the target surface (A), design pattern and mesh discretization (B), CAD 3D model (C), CAD 2D model (D), and CAD/CAE macro model (E).

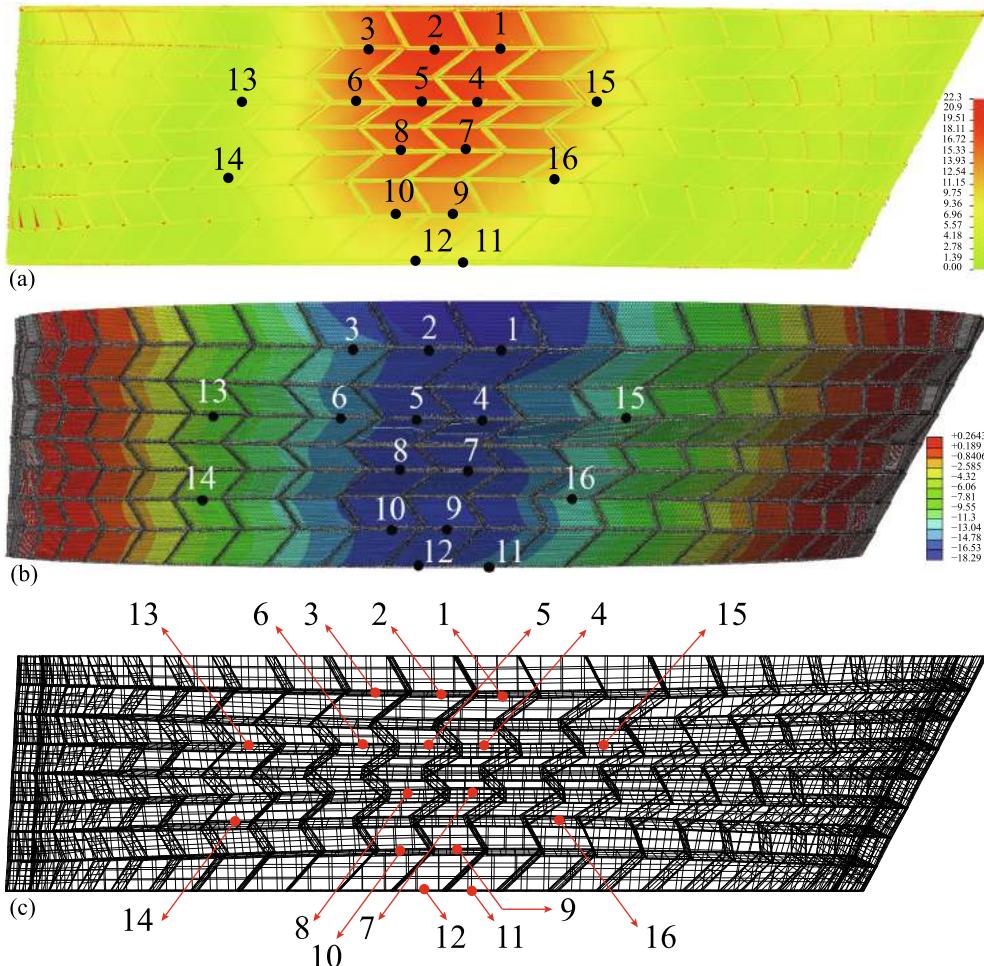


**Fig. 17.** (a) On-site experimental evaluation of the performance of arch #22 of the IATP structure [35], (b) the process used to assemble the plates, boxes, and strips.

important to note that the accuracy of the macro model in predicting the response of timber plates in the regions that are not directly loaded (i.e. stations 13 and 14) is lower than that of the directly loaded regions. This is attributed to two main factors. First, the scale of the structure is relatively large ( $25 \text{ m} \times 6 \text{ m} \times 9 \text{ m}$ ) and the global geometry of the structure is asymmetric in plan and elevation. Second, the element formulation in the macro model is such that the rigidity is concentrated in a specific number of discrete and uncoupled beam and springs elements [22], which leads to errors in large-scale IATP structures.

One of the major benefits of using the macro modeling approach for large-scale structures is that the resource use, memory footprint, and computational time are considerably reduced. Using the pro-

posed CAD-to-CAE data exchange, the CAD geometry is translated to the corresponding CAE macro model using a machine with an Intel Core i7-4800MQ CPU @2.7 GHz and 16 GB of RAM. The time required to generate the CAE macro model is approximately 1 min and 3 s while using 22% of the CPU and 781 MB of memory. The structural analysis runtime is 11 min and 27 s, using 17.9% of the CPU and 261 MB of memory. This is compared to the computational time and resource use required for the continuum FE analysis of the arch using the same machine. Nguyen et al. [15] reported that the time required to generate the FE model is 64 min and 12 s using 42% of the CPU and 1560 MB of memory. Moreover, the structural analysis took 1864 min and 2 s while using 22% of the CPU and 4482 MB of memory.



**Fig. 18.** Top view of Arch #22: The vertical displacements: (a) measured by the laser scanner and the location of the points used in the electronic total station measurement [35], (b) obtained from the FE simulation [35], and (c) the CAE macro model generated by the proposed algorithmic framework for arch #22.

**Table 2**  
Material Properties of beech BauBuche [37].

Symbol	Description	Value (N/mm <sup>2</sup> )
$E_0$	Modulus of elasticity, fiber-parallel	13200.
$E_{90}$	Modulus of elasticity, fiber-perpendicular	2200.
$G_0$	Shear modulus, fiber-parallel	820.
$G_{90}$	Shear modulus, fiber-perpendicular	430.

## 5. Conclusions

An algorithmic modular framework was developed to take the 3D geometry of a custom-defined Integrally-Attached Timber Plate (IATP) structure as input, compute the mechanical properties and other non-geometric information, and construct and analyze the associated CAE macro model. The key techniques used to convert a 3D CAD model to the macro model include feature-based simplification, topology-based modification, and virtual topology adaptation. An algorithm for identifying the perimeter polygons associated with 3D timber plates was first developed. A simplified line element was used to establish the connection between neighboring elements. An indexing algorithm was then introduced to assign a unique tag to the polygons and connections based on the following three types of IATP components: strip, box, and plate. Next, an algorithm to generate the components of the macro model

(inner beams, boundary beams, and spring elements) for each polygon was developed. Another algorithm was formulated to assign a unique tag to the macro model components. The cross-section properties of each inner beam element were determined based on the associated tributary area. Furthermore, the material properties were assigned to the macro model components based on the associated fiber orientation. The CAD-to-CAE data exchange framework was written and compiled in a CAD environment (Rhino 3D) using the RhinoCommon library. The output of the data exchange was designed to follow the syntax of OpenSees, which was used as the structural analysis platform.

The proposed CAD-to-CAE data exchange framework was applied to the following two structures, where the macro models associated with the CAD 3D models were generated: (i) a standard beam element with a hollow cross section, and (ii) a large-scale free-form IATP arch. The proposed data exchange framework was verified against the responses obtained from the physical experiments and continuum Finite Element (FE) models. It is concluded that the centralized repository, which includes the model inputs, analysis options, data management structure, and CAD and macro model databases, is necessary for analyzing complex large-scale IATP structures. Furthermore, since a typical IATP structure consists of hundreds of timber plates with thousands of joints, the development of the CAD-to-CAE exchange framework facilitates a parametric data storage methodology and systematic identifica-

**Table 3**

Vertical displacements at the identified locations of arch #22.

Station ID	Total station [35]	Laser scanner [35]	Finite Element [35]	Macro	Unit
1	20.5	15	16.58	18.74	mm
2	22.3	17	16.71	20.54	mm
3	18.4	15	14.18	18.41	mm
4	19.8	17	14.81	16.81	mm
5	19.8	18	15.19	18.35	mm
6	15.9	15	13.42	16.47	mm
7	16.5	16	15.32	16.26	mm
8	17.5	15	15.44	17.6	mm
9	12.9	12	15.06	15.14	mm
10	12.0	12	15.0	16.18	mm
11	11.3	7	12.91	13.37	mm
12	11.1	7	14.43	14.63	mm
13	7.6	3	6.96	1.19	mm
14	5.6	1	6.33	1.11	mm
15	10.1	8	9.24	6.78	mm
16	11.8	8	10.76	11.75	mm

tion protocol. The CAD-to-CAE data exchange was shown to minimize the manual operations, and the generation of the CAE macro model becomes deceptively straightforward. Moreover, the proposed framework enables design flexibility in IATPs since it can be applied to different geometries.

Applying the proposed CAD-to-CAE data exchange framework to the prototypes with standard and free-form geometries, it is concluded that the results obtained from the macro model compare well with that of the physical experiments. Moreover, the interpretation of the results is straightforward when the macro modeling technique is used for structural analysis. This, in particular, facilitates seamless global performance evaluation of IATP structures. The total computational time for the macro model, including the creation of the model database and runtime, is significantly less than that of the continuum FE model. As the main conclusion, the proposed CAD-to-CAE data exchange framework can be used by practitioners to design IATP structures. Accordingly, the framework can be adapted by typical BIM tools that support parametric modeling in digital architecture and macro modeling

in structural analysis to create, manage, and share the 3D model associated with an IATP structure.

With the proposed CAD-to-CAE data exchange framework, structural design parameters can be included in the design process of IATPs during the architectural design and CAD model development. This would lead to a master model that aims to optimize the design of IATP structures by satisfying not only the assembly and fabrication constraints but also the structural engineering design requirements. Furthermore, the proposed CAD-to-CAE data exchange can facilitate design optimization of large-scale IATP structures by including the non-linear behavior of IMAs. The non-linear behavior, ultimate limit state, and the failure criteria can be easily embedded in the spring elements of the macro model and applied to the thousands of connections in the structure under consideration.

### Declaration of Competing Interest

The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

### Acknowledgments

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### Appendix 1

#### 1.1. NURBS subdivision and quad mesh generation



```

// S - NURBS, UDiv - divisions of rows, VDist - distance of quad in V-direction, T - translation value
// Scale - Scaling towards center, Thres - threshold value to stop NURBS divisions towards centre
(Surface S, double UDiv, double VDist, double T, double Scale, double Thres)

List<MeshFace> quads
List<Point3d> vertices
Initialize division parameters: Surface Domain, divisionStep, baseStep, maxStep, shiftParameter, etc.

for (int i = 0; i < UDiv; i++)
    Initialize strip parameters: divisionThreshold, stepV1, ctV
    Initialize points Point3d to form quad face
    while (iteration is less than the threshold)
        Get surface points to construct mesh faces for the first half of the arch
        Get surface points to construct mesh faces for the second half of the arch

Construct mesh from vertices and quads
return quadMesh
    
```

```

// NURBS division to create
// Iterate strips of longitudinal
// Strip is constructed from 2
// Iterate until the middle of
// Mesh output
    
```

## 1.2. CAD 3D model generation of IATP structures

```

// QuadMesh – Mesh from SkewedQuadMesh method, Divisions – double strip division,
// PlateThickness – Thickness of the plate, TenonDivisions – Distance to Divide Tenons,
// ShiftLinear – Box shift along longitudinal axis, ShiftRotation – rotation of side plates to obtain
reciprocal like pattern,
// MEdgesIDFront, MEdgesIDBottom, MEdgesIDBack – meshes edges for boundary conditions,
// CutYCoord0, CutYCoord1, CutZCoord – Coordinates to cut timber plates at gives mesh id
// OSDivisions – joint division for OpenSees, OSPlateOffset – Top and bottom plate offset
(Mesh QuadMesh, int Divisions, double Offset, double PlateThickness, double TenonDivisions,
double ShiftLinear, double ShiftRotation,
int MEdgesIDFront, int MEdgesIDBottom, int MEdgesIDBack,
double CutYCoord0, double CutYCoord1, double CutZCoord,
int OSDivisions, int OSPlateOffset)

MultiData
DLTPScomp2 :
GH_Component

GH_InputParamManager pManager)
RegisterOutputParams (GH_OutputParamManager pManager)
override Bitmap Icon
override Guid ComponentGuid

SolveInstance(IGH_DataAccess DA)
Fetch all component inputs
Fetch mesh data, including mesh face normal and sorting mesh edges

Decide edge divisions
Create default planes for each edge
Initialize BoxPlate objects for each mesh face

foreach(BoxPlate box in boxes)
if box is the even row
    box.RotateVertCW()
    box.RotateEdgesCW()
    box.ProjectBoxPoints()

foreach(BoxPlate box in boxes)
    box.SetBoxNeighbours()

foreach(BoxPlate box in boxes)
    box.SetDivisionAxes()

foreach(BoxPlate box in boxes)
    box.CreateNewMidContours()

foreach(BoxPlate box in boxes)
    box.CorrectEdgeMidPts()

foreach(BoxPlate box in boxes)
    box.CreateNewPlateContours()

foreach(BoxPlate box in boxes)
    box.CreateTopPlateSlots()
    box.CreateBottomPlateSlots()
    box.AssignSlotsToNeighbours()

foreach(BoxPlate box in boxes)
    Retrieve Fabrication Outlines

foreach(BoxPlate box in boxes)
    Retrieve data as a DataTree<Polyline>
    Retrieve data as a DataTree<Point3d> 4Corners,
    DataTree<Point3d> JointPoints, DataTree<int> currentJointStartIndex
    DataTree<int> nextJointStartIndex

    Output the data to Grasshopper Component Interface

MultiData: Datatree<Polyline> Fabrication, Datatree<Point3d> 4Corners,
Datatree<Point3d> JointPoints, Datatree<int> currentJointStartIndex
Datatree<int> nextJointStartIndex

```

// Grasshopper Components  
data for Fabrication and St  
Calculation

//Generic template methods  
//Register Input Parameters  
//Register Output Parameters  
//Create a Component Icon  
//Give a unique index to the

//Main method  
//Component input

//Initialize Box objects from

//Iterate boxes  
//Shift boxes (even rows)

//Set box and plate adjacencies

//Series of methods for fabrication

//Component output

//Fabrication

//Perimeter polygons  
//Connections (IMAs)

//Output: fabrication contours of the plates, joint points for each plate, indexing for current and next joint id following n (strip, box, plate; side)

### 1.3. Identification of the perimeter polygons and simplified IMAs

```

// BoxPlate class contains 6 plate objects and main geometry processing method to obtain the overall
geometry
BoxPlate Class
class BoxPlate

int SID
int ID
int Neighbor_ab_ID, Neighbor_bc_ID, Neighbor_cd_ID, Neighbor_da_ID
int EdgeIndex_AB, EdgeIndex_BC, EdgeIndex_CD, EdgeIndex_DA
double div_ab, div_bc, div_cd, div_da
double planarity
double h_top, h_bot,
double L_tab
Plate PT, PB, W0, W1
Vector3d v
Vector3d ad, ba, ab, bc, cd, da
Vector3d n_ab, n_bc, n_cd, n_da
Line line_A, line_B, line_C, line_D, line_E, line_F
Line line_A_offs, line_B_offs, line_C_offs, line_D_offs, line_E_offs, line_F_offs
Line line_A_mid, line_B_mid, line_C_mid, line_D_mid
Point3d A, B, C, D
Plane pS
Plane p_AB, p_BC, p_CD, p_DA
Plane p_AB_mid, p_BC_mid, p_CD_mid, p_DA_mid
Plane p_AB_offs, p_BC_offs, p_CD_offs, p_DA_offs
Plane PTT2, PTT, PTM, PTB
Plane PBT, PBM, PBB, PBB2
Plane TP_da, TP_ab
string info
bool DoubleUp_Wall_BC, DoubleUp_Wall_CD
bool HasNeighbor_ab, HasNeighbor_bc, HasNeighbor_cd, HasNeighbor_da
bool HasNeighborPlane_ab, HasNeighborPlane_da
bool IsBoundary_ab, IsBoundary_bc, IsBoundary_cd, IsBoundary_da
bool HasPolylines, HasBreps

void SetNeighbors(MeshTopologyEdgeList mte)
GetConnectedFaces(FaceEdgeIndex)
if(the face has two neighbours)
    Set Neighbor_ab_ID, HasNeighborPlane_ab, HasNeighbor_ab
else
    Set IsBoundary_ab
Repeat the same step for each edge

void SetDivAxes (List<BoxPlate> boxes, List<int> elds1, List<int> elds2, List<int> elds3,
double r, double s, double Yb, double Yf, double Zg, int id)
Create planes for cutting sides and base of the shell, because edges are curved by vertex normals
Set cut plates per each box by user defined mesh index numbering
Set edge planes for each mesh edge, considering normal of adjacent mesh faces
Shift and rotate edges considering user input
Repeat the same process for each edge
Get lines for mesh edge plane intersections
Offset planes and obtain plate line segments from PlanePlane intersection
Create line segments as well for structural calculation considering specifications of Macro model
List<Point3d> CreateNewPlateMidContourTop(Plane plane, Line lA, Line lB, Line lC, Line lD,
Plate Plate)
Intersect lines obtained from previous method to plate top plane
Collect intersection result – Point3d to construct an outline
Also call Plate.CreateEdgeMidPts method to get middle points of the polygons
Return list of corner points

List<Point3d> CreateNewPlateMidContourTop2(Plane plane, Line lA, Line lB, Line lC, Line
ID,
Plate Plate)
Repeat same process as in previous method but for bottom outline

List<Point3d> CreateNewPlateMidContourSides(Plane plane0, Plane plane1, Line l0, Line l1,
Plate Plate)
Repeat previous method for side plates

void CreateNewBoxMidContour(Plane plane, Line lA, Line lB, Line lC, Line lD)
Intersect lines from mesh edge plane intersection with mesh faces planes
Get polygon from lines points
Get edge vectors from pair of corner points A,B,C,D
Computer insertion vector from two edges v = -(ba+da), where ba = B-A and da = D-A

```



```

void CreateNewMidContours(double femScaleTopPlate)
    Index the plates (PT.name, PB.name, W0.name, W1.name)
    Index StripID of each plate
    Index BoxID of each plate
    Index PlateID of each plate
    Compute planes for top and bottom sides of the plates
    Compute outlines from these planes - CreateNewBoxMidContour()
    Create offset outlines for top and bottom plates – CreateNewPlateMidContourTop()
    Create outlines for top and bottom plates – CreateNewPlateMidContourTop2()
    Create side outlines
    Create middle outlines
                                                //Get plate polygons
                                                //Fabrication
                                                //Fabrication
                                                //Fabrication
                                                //Perimeter polygon for Ope

void CorrectEdgeMidPts(List<BoxPlate> boxes)
    bool bollNeighbour
    int NeighborID
    for(iterate 4 times)
        check edge neighbours
        if(edge is not boundary)
            foreach (iterate edge mid points)
                if(when the distance between current and neighbor point is less than the user distance)
                    Assign the corrected edge middle point by mean value of both points
                repeat the same process for bottom plate
                                                //Correct top, bottom edge m

void CreateNewPlateContours ()
    Create top and bottom plate contours – CreateNewPContour()
    Create side plates outlines – CreateNewWContour()
    Assign middle and base planes for the plates
    Create side outlines
    Call List<List<Point3d>>Dovetail() method for the box
    Add corner points to dovetail nested point list
    Assign side plate the middle and base planes
                                                //Assign plate planes and co
                                                //dovetail and tenon-mortise

Outline CreateNewPContour(Plane plane, Line lA, Line lB, Line lC, Line lD, Plate Plate)
    Get corner points per plate side (top or bottom)
    Create tenon-mortise joint if edge is not boundary CreateTenons()
    Add joint point collection in correct order of the plate contour
    Add joint sequence to keep track of how many tenons does the plate have
    Return the outline including tenon joints
                                                //Create tenons for each con

Outline CreateNewWContour(Plane plane, Line lA, Line lB, Line lC, Line lD)
    Get corner points by line plane intersections
    Create an outline and assign empty joint sequence
    Return the outline
                                                //Create side outlines witho

List<Point3d> CreateTenons(Point3d p0, Point3d p1, Vector3d v, Plane pT, Point3d CorrMidPt,
double t_plate, double L_tab, double div)
    Create tenons following the insertion vector
    Move tenon geometry from the corners considering the plate thickness and the insertion angle
    Enumerate points following a modular of 2
    Add tenon at every second pin
                                                //Slots are computed separa
                                                //outline geometry

AssignSlots(List<BoxPlate> Boxes)
    Iterate top and bottom plate and add slots from PB and PT to side plates W0,W1
    Slots data is also used for the CAD-to-CAE joinery and indexing
    Append slots also from neighbor plates since side plates are connected to four plates in total
                                                //Creates hole for side plate
                                                //and bottom plate of tenon j

List<List<>> Dovetail()
    Get corner points of the side plate edge, where the joints have to be added
    Create 4 lines for side plate edges
    for (iterate 8 times)
        Create plane for two neighbor mesh edge directions
        for the first plane add not rotation and for the rest turn plane by 15 degrees
    foreach (infinite line in lines)
        intersect with 6 planes to get the zigzag outline of dovetail
        Sort outlines to match the orientation
        Return nested point lists for each edge of side plates
                                                //Create edge outlines repre
                                                //Dovetail joints

void RotateVertCW()
    Copy corner points
    Assign corner point shifted to the right
                                                //Shift corner points

void RotateEdgesCW()
    Get division values
    Get edge indices
    Assign both division and edge values by shifting by one index
                                                //Shift edge values

void ProjectToPlane()
    Project corner point to plates contour base plane using, by line plane intersection
                                                //Planarize corner points

```



```

void FitPlane()
    Get corner points
    Fit corner points to plane
    Move origin of plate to corner points centre
    Check direction of the newly created plane by user define normal
    Flip plane if angle* 180.0 / Math.PI is more than 90 deg

List<Point3d> Sort(List<Point3d> a, List<Point3d> b)           //Fit corner points
    Create and empty Point3d list
    for(iterate 8 times)
        Add first point to the list
        if(i%2!=0)
            add a[i] and b[i] points to the empty list
        else add in reverse order
        Add last point to the list
    Return sorted list

// Plate class that represents top and bottom outlines of the timber plate including slots
Plate Class
class Plate                                         // Class representing timber

    Outline ContourTop = new Outline(new Polyline());
    Outline ContourBot = new Outline(new Polyline());
    Plane Base = new Plane();
    Plane MidPlane = new Plane();
    List<Point3d> MidCornerPts = new List<Point3d>();           //Properties of the Plate class
    List<Point3d> MidCornerPts2 = new List<Point3d>();           //Top and bottom polygons
    Polyline outline2 = new Polyline();
    Brep MidSurface4pts;                                         //Corner points
    List<Point3d> MidSrfEdgeMidPts = new List<Point3d>();         //Mid edge points
    Point3d[] MidSrfEdgeMidPtsCorr = new Point3d[4];
    Polyline outline = new Polyline();

    List<List<Polyline>> Slots = new List<List<Polyline>>();           //connection Properties use
    List<List<Point3d>> SlotsSimplified = new List<List<Point3d>>();   macro model
    List<List<GH_Path>> SlotsSimplifiedNeiID = new List<List<GH_Path>>();
    List<List<List<Polyline>>> Slots_Out = new List<List<List<Polyline>>>();
    List<List<List<Point3d>>> Slots_InSimplified= new List<List<List<Point3d>>>();           //indexing
    List<List<List<Point3d>>> Slots_OutSimplified = new List<List<List<Point3d>>>();
    List<List<List<GH_Path>>> Slots_InSimplifiedID = new List<List<List<GH_Path>>>();

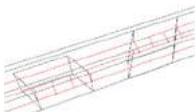
    int StripID;
    int BoxID;
    int PlateID;
    int hCount;
    string name = null;
    bool HasSlot = false;
    double Thickness = 0;

    void CreateEdgeMidPts()
        Get mid-points of corner polygons
        Define polygon from points

    void CreateSlots(int FEMDivisions = 1)           //Iterate joint sequence to g
        Get top and bottom contours polygons as Point3D arrays           //of the plates (mortise outline
        Declare slots for nest polygon array for the plate slots as List<List<List<Polyline>>>
        Declare collection for OpenSees joint representations as List<List<List<Point3d>>>
        Declare collection for tracking joint indices List<List<List<GH_Path>>>

    foreach(int num in ContourTop.jointSeq)           //Iterate joint sequence
        Create local nested collections to store polygons and OpenSees point data for joints           //that defines how many joint
        for (int i = 0; i < num; i++)                  //plate edge
            Create local collections to store polygons and joint data           //iterate each tenon
            Create rectangle polygon for each tenon and store the data
            Get middle points of the tenons and mortises of current and adjacent plates
            Find closest-point to four corner polygons since they differ from fabrication model
            The tenons and mortises has an angle while the macro model contains perpendicular edge joints
            Two lines are created per each joint
            Line segments are interpolated depending on the user needs
            Joint data is stored in a point list together with indexing of StripID, BoxID, PlateID, EdgeID

```



```

// pline - polygon, jointSeq - joint order
Outline Class
class Outline
    Polyline pline
    List<int> jointSeq
    Outline(Polyline outline)
    pline = new Polyline(outline)

// Static helper for geometry processing
Outline Utilities
static class Utilities
    Line TweenLine(Line l0, Line l1)
    List<Point3d> TweenPolylines(List<Point3d> p0, List<Point3d> p1, double t = 0.5)
    double GetDist(Point3d p0, Vector3d v, Plane P)
    List<Point3d> CP(List<Point3d> points, Polyline outline)
    List<List<Point3d>> CP(List<List<Point3d>> points, Polyline outline)
    void ShiftPoints(List<Point3d> A, int n)
    void ShiftPolyline(Polyline A, int n)
    Polyline TwoPlaneTwoLines(Plane plane0, Plane plane1, Line l0, Line l1)
    Line ScaleLn(Line line, double scale = 10)
    bool IsClockwisePolygon(Polyline polygon)
    InsertPolyline(Polyline x, IEnumerable<Polyline> y)
    Point3d LinePlane(Line line, Plane plane)
    Line IntPtPln(Point3d origin, Vector3d axis, Plane planeBot, Plane planeTop)
    double Lerp(double value1, double value2, double amount)
    Point3d[] InterpolatePoints(Point3d from, Point3d to, int Steps)
    Point3d PlanarPoint(Point3d A0, Plane newplane)
    bool MeasurePlanarity(Point3d A, Point3d B, Point3d C, Point3d D)
    // Class representing polygo
    // of timber plate i.e. top or

```

#### 1.4. Generation of the macro model components

```

Using : rhinoscriptsyntax library // basic geometry fu
Input : C (list of all Cells as polylines), S, B and P (associated Strip, Box and Plate identifiers as list of integers)
Input : J (list of all Joints as segments), Js and Je (associated list of tags (Strip, Box, and Plate) for the connecting polygons)
Output: N, E, G (lists of strings for nodes, elements, and geometric transformation) // following OpenSe
Initializing: nodesInfo = [] // temporary storage

Tag
for i = 0 to i = length of C -1
    tag = 1000*S[i] + 10*B[i] + P[i] // Calling S,B,Pk to e

Corner nodes
1.2     points = corners of C[i]
        for j = 0 to 3
            Add to N: 'node' + (100000*tag + j+1) + x,y,z coordinates of points[j] // Calling the corner
            Add to nodesInfo: ((100000*tag + j+1), points[j], (100000*tag + j+1)) // indexing corner n
        1.3.1
        1.3.2     Add to nodesInfo: ((100000*tag + j+1), points[j], (100000*tag + j+1)) // Adding the nodes

Pins at the
corners
hinge1 = copy of points[0] translated 1mm in direction of points[1]
Add to N: 'node' + (100000*tag + 5001) + x,y,z coordinates of hinge1 // id format: SBP50
Add to nodesInfo: ((100000*tag + 5001), hinge1, (100000*tag + 5001))

hinge2 = copy of points[1] translated 1mm in direction of points[0]
Add to N: 'node' + (100000*tag + 5002) + x,y,z coordinates of hinge2 // id format: SBP50
Add to nodesInfo: ((100000*tag + 5002), hinge2, (100000*tag + 5002))

hinge3 = copy of points[1] translated 1mm in direction of points[2]
Add to N: 'node' + (100000*tag + 5003) + x,y,z coordinates of hinge3 // id format: SBP50
Add to nodesInfo: ((100000*tag + 5003), hinge3, (100000*tag + 5003))

hinge4 = copy of points[2] translated 1mm in direction of points[1]
Add to N: 'node' + (100000*tag + 5004) + x,y,z coordinates of hinge4 // id format: SBP50
Add to nodesInfo: ((100000*tag + 5004), hinge4, (100000*tag + 5004))

hinge5 = copy of points[2] translated 1mm in direction of points[3]
Add to N: 'node' + (100000*tag + 5005) + x,y,z coordinates of hinge5 // id format: SBP50
Add to nodesInfo: ((100000*tag + 5005), hinge5, (100000*tag + 5005))

hinge6 = copy of points[3] translated 1mm in direction of points[2]
Add to N: 'node' + (100000*tag + 5006) + x,y,z coordinates of hinge6 // id format: SBP50
Add to nodesInfo: ((100000*tag + 5006), hinge6, (100000*tag + 5006))

```

```

hinge7 = copy of points[3] translated 1mm in direction of points[0]
Add to N: 'node' + (100000*tag + 5007) + x,y,z coordinates of hinge7
Add to nodesInfo: ((100000*tag + 5007), hinge7, (100000*tag + 5007)) // id format: SBP50

hinge8 = copy of points[0] translated 1mm in direction of points[3]
Add to N: 'node' + (100000*tag + 5008) + x,y,z coordinates of hinge8
Add to nodesInfo: ((100000*tag + 5008), hinge8, (100000*tag + 5008)) // id format: SBP50

Geom.
Transf.

Fiber Parallel
beams

vector1 = vector from points[0] to points[1]
vector2 = vector from points[0] to points[3]
normal = cross product between vector 1 and vector 2
Add to G: 'geomTransf Linear' + tag + x,y,z coordinates of normal

Fiber
perpendicular
beams

if P[i] == 1 or P[i] == 2
    mat = '$A_P12_FPa $E_P12_FPa $G_P12_FPa $J_P12_FPa $Iy_P12_FPa $Iz_P12_FPa' + tag
    // assign material pr parallel beams of pl (OpenSees format)
if P[i] == 3
    mat = '$A_P3_FPa $E_P3_FPa $G_P3_FPa $J_P3_FPa $Iy_P3_FPa $Iz_P3_FPa' + tag
    // assign material pr parallel beams of pl format
if P[i] == 4
    mat = '$A_P4_FPa $E_P4_FPa $G_P4_FPa $J_P4_FPa $Iy_P4_FPa $Iz_P4_FPa' + tag
    // assign material pr parallel beams of pl format
Create an array of 4 hatches parallel to the longitudinal direction (between points[0] and points[1])
for j = 0 to 2
    FPa_tag = 10000*tag + 1000 + j+1
    Add to N: 'node' + (FPa_tag + '1') + x,y,z coordinates of FPa[j] start point
    Add to nodesInfo: (FPa_tag + '1'), FPa start point, (FPa_tag + '1')
    Add to N: 'node' + (FPa_tag + '2') + x,y,z coordinates of FPa[j] end point
    Add to nodesInfo: (FPa_tag + '2'), FPa end point, (FPa_tag + '2')
    Add to E: 'element elasticBeamColumn' + FPa_tag + (FPa_tag + '1') + (FPa_tag + '2') + mat
    // Fig. X
    // Assigning indices beams
    // id format: SBPLn
    // id format: SBPLn
    // id format: SBPLn
Create an array of 6 hatches perpendicular to the longitudinal direction (between points[0] and points[2])
for j = 0 to 4
    FPe_tag = 10000*tag + 2000 + j+1
    Add to N: 'node' + (FPe_tag + '1') + x,y,z coordinates of FPe[j] start point
    Add to nodesInfo: (FPe_tag + '1'), FPe start point, (FPe_tag + '1')
    Add to N: 'node' + (FPe_tag + '2') + x,y,z coordinates of FPe[j] end point
    Add to nodesInfo: (FPe_tag + '2'), FPe end point, (FPe_tag + '2')
    Add to E: 'element elasticBeamColumn' + FPe_tag + (FPe_tag + '1') + (FPe_tag + '2') + mat
    // Fig. X
    // Assigning indices perpendicular beam
    // id format: SBPTrs
    // id format: SBPTrs
    // id format: SBPTrs
Joints

for i = 0 to i = length of J -1
    J_num = i+1
    J_tag = Js[i] + Je[i] + J_num
    Add to E: 'element twoNodeLink' + J_num + (J_num+'19') + (J_num+'29') +
        '-mat $Stiffness_Tension $Stiffness_IP $Stiffness_OOP $Stiffness_about_Tension $Stiffness_about_OOP
        $Stiffness_about_IP -dir 1 2 3 4 5 6 -orient 1 0.0 0.0'
    Add to N: 'node' + (J_num+'1') + x,y,z coordinates of J[i] start point
    Add to nodesInfo: (J_tag + '1'), J[i] start point, (J_num+'1')
    Add to N: 'node' + (J_num+'2') + x,y,z coordinates of J[i] end point
    Add to nodesInfo: (J_tag + '2'), J[i] end point, (J_num+'2')
    // Assigning indices according to the Op
    // id format: SjBjPs
    // id format: SjBjPs
    // id format: SjBjPs

```

## Boundary elements

```

for i = 0 to i = length of C -1
    Initialize PointsOnPline = []
    tag = 1000*S[i] + 10*B[i] + P[i]
    normal = cross product between vector from points[0] to points[1] and vector from points[0] to points[3]
    mat = '$A_cnt $E_cnt $G_cnt $J_cnt $Iy_cnt $Iz_cnt' + tag
    // list for sorting all points on

    for j = 0 to j = length of nodesInfo -1
        if length of nodesInfo[j][0] < 10
            if the four first digits of nodesInfo[j][0] correspond to the tag of the cell
                Add to PointsOnPline: nodesInfo[j][2], curve parameter at nodesInfo[j][1]
            else
                if the four first digits of nodesInfo[j][0] correspond to the tag of the cell and the last one is 1
                    Add to PointsOnPline: nodesInfo[j][2], curve parameter at nodesInfo[j][1]
                if the next four digits of nodesInfo[j][0] correspond to the tag of the cell and the last one is 2
                    Add to PointsOnPline: nodesInfo[j][2], curve parameter at nodesInfo[j][1]
                Sort PointsOnPline by curve parameters
                for j = 0 to j = length of PointsOnPline -1
                    cont_tag = 10000*tag + 4000 + j + 1
                    string = 'element elasticBeamColumn' + cont_tag + PointsOnPline [j][0] + PointsOnPline [j+1][0] +
    mat
        if (the element connects corner nodes to corner pins)
            Replace 'elasticBeamColumn' by 'twoNodeLink' in string
        Add string to E
    
```

## Export

Export N, E, G, D as text in .tcl format

## References

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