Nonstationary Flood Frequency Analysis: A Mixed and Pooled Approach

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Columbia, SC: October 2015





Garners Ferry Road in Columbia, SC before and during the October 2015 flood.

Economists estimated the floods caused about \$12 billion damage on the state of South Carolina

Source: Sean Rayford/Getty Images

Congaree River: Columbia, SC



Gervais Street Bridge, Columbia, SC

First version: 1827 to 1865 Second version: 1870 to 1928 Third version: 1928 to current

Flood gage about 0.2 miles south of current Gervais Street Bridge

1865: Columbia, SC



- Ruins, as seen from the State House, 1865.
- General Sherman's Union troops were slowed entering Columbia by a major flood on the Congaree River.

Flood Frequency Data

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The maximum momentary peak discharge in each year of record

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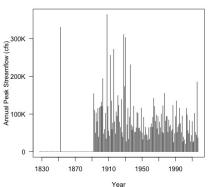
Historical Flood Data

Any observation of flood stage or conditions made before actual flood data were collected systematically

• Data collected from old newspapers, diaries, museums, libraries, etc.

Congaree River: Columbia, SC (USGS #02169500)





Historic Floods: 1852, Possible historic floods: 1886, 1888

Flood Record: 1892 to 2017

Modeling Flood Frequency Data

A generalized extreme value distribution approach can be used on the annual peak streamflows from 1892 to 2017:

$$F(y) = P(Y \le y) = \exp \left\{ -\left[1 + \xi\left(\frac{y - \mu}{\sigma}\right)\right]^{-1/\xi}\right\}$$

 μ (location parameter), σ (scale parameter), ξ (shape parameter) can be estimated by maximum likelihood estimation

Using fevd in R's extRemes package, the MLE's are:

$$\hat{\mu} = 60,473.887, \quad \hat{\sigma} = 31,807.200, \quad \hat{\xi} = 0.264$$

Using ci.fevd in R's extRemes package, the point estimate and 95% confidence interval for the 1% chance flood from this series are:

- Point Estimate for 1% Chance Flood: 344,999 cfs
- 95% CI for 1% Chance Flood: 208,897 cfs to 483,101 cfs

To estimate the 1% chance flood from this flood series:

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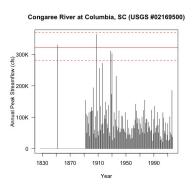
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 - 1% chance flood is the 99th percentile of the fitted finite mixture model
 - Due to the complexity of the likelihood function, the standard error of the estimate of the 1% chance flood is produced solely from score functions (first partial derivatives)

Initial Results: 1827 to 2017



Parameter Estimates:

$$\hat{\boldsymbol{\mu}}^{T} = (4.854, 5.480), \hat{\boldsymbol{\sigma}}^{T} = (0.228, 0.0498), \hat{\tau} = 0.0278$$

Point Estimate for 1% Chance Flood: 321,972 cfs 95% CI for 1% Chance Flood: 280,568 cfs to 369,486 cfs

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POLICYFORUM

CLIMATE CHANGE

Stationarity Is Dead: Whither Water Management?

P. C. D. Milly, 1* Julio Betancourt, 2 Malin Falkenmark, 3 Robert M. Hirsch, 4 Zbigniew W. Kundzewicz, 5 Dennis P. Lettenmaier, 6 Ronald J. Stouffer, 7

Climate change undermines a basic assumption that historically has facilitated management of water supplies, demands, and risks.

Source: Science, Vol. 319, pp. 573-574, 1 February 2008, doi: 10.1126/science.1151915

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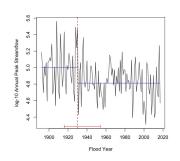
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 - This is estimated by the finite mixture models.

Congaree River: 1892 to 2017



Results from strucchange package in R: two pooled components

- estimated change point is 1930 (95% CI: 1916 to 1954)
- 1930 Dreher Shoals Dam completed about 10 miles west of flood gage/Columbia, SC
- 1892 to 1930 1st pooled component of annual peak streamflows
 - The annual peak streamflow of the 1930 flood year occurred on October 3, 1929.
- 1931 to 2017 2nd pooled component of annual peak streamflows

Proposed Model

Assume that there are K-1 change points in the flood series. The pdf of the annual peak streamflows can be written as a linear combination of K finite mixture models.

$$p(y) = \sum_{k=1}^{K} \pi_k \left[\sum_{j=1}^{J_k} \tau_{jk} f(y|\theta_{jk}) \right],$$

where

$$\sum_{k=1}^K \pi_k = 1, \sum_{j=1}^{J_k} au_{jk} = 1, \ ext{and} \int_{\mathcal{Y}} f\left(y | oldsymbol{ heta}_{jk}
ight) \ dy = 1$$

The likelihood function is

$$L(\boldsymbol{\eta}) = \prod_{i=1}^{n} p(y_i) = \prod_{i=1}^{n} \left\{ \sum_{k=1}^{K} \pi_k \left[\sum_{j=1}^{J_k} \tau_{jk} f(y_i | \boldsymbol{\theta}_{jk}) \right] \right\},$$

where

$$\eta = (\theta_i, \tau_i), i = 1, ..., J_k, k = 1, ..., K$$

Complete Data Likelihood & Log-Likelihood

The complete data, x_i , i = 1, ..., n, are observed indirectly via:

$$x_i = (y_i, \mathbf{z}_i, \zeta_i), i = 1, \ldots, n$$

where each $\mathbf{z}_i = (z_{i11}, \dots, z_{ijk})^\mathsf{T}$ is an indicator vector of length jk with 1 in the position corresponding to the appropriate mixing component and zeroes elsewhere and $\zeta_i = (\zeta_{i1}, \dots, \zeta_{ik})^\mathsf{T}$ is an indicator vector of length k with 1 in the position corresponding to the appropriate pooling component and zeroes elsewhere.

Complete Data Likelihood:

$$g(\mathbf{x}|\boldsymbol{\eta}) = \prod_{i=1}^{n} \prod_{k=1}^{K} \prod_{j=1}^{J_k} \pi_k^{\zeta_{ik}} \tau_{jk}^{z_{ijk}\zeta_{ik}} f(y_i|\boldsymbol{\theta}_{jk})^{z_{ijk}\zeta_{ik}}$$

Complete Data Log-Likelihood:

$$\log g(\mathbf{x}|\boldsymbol{\eta}) = \sum_{i=1}^{n} \sum_{k=1}^{K} \zeta_{ik} \log \pi_k + \sum_{i=1}^{n} \sum_{k=1}^{K} \sum_{i=1}^{J_k} [z_{ijk} \zeta_{ik} \log \tau_{jk} + z_{ijk} \zeta_{ik} \log f(y_i|\boldsymbol{\theta}_{jk})]$$

A Simple Scenario...

The complete data log-likelihood can be "simplified" when K=2 and $J_1=J_2=2$:

$$\begin{split} \log g\left(\mathsf{x}|\eta\right) &= \sum_{i=1}^{n} \left[\zeta_{i} \log \pi + (1-\zeta_{i}) \log \left(1-\pi\right)\right] \\ &+ \sum_{i=1}^{n} \left[z_{i1}\zeta_{i} \log \tau_{1} + (1-z_{i1}) \zeta_{i} \log \left(1-\tau_{1}\right) + z_{i0} \left(1-\zeta_{i}\right) \log \tau_{0} + (1-z_{i0}) \left(1-\zeta_{i}\right) \log \left(1-\tau_{0}\right) \right] \\ &+ z_{i1}\zeta_{i} \log f\left(y_{i}|\theta_{11}\right) + (1-z_{i1}) \zeta_{i} \log f\left(y_{i}|\theta_{01}\right) + z_{i0} \left(1-\zeta_{i}\right) \log f\left(y_{i}|\theta_{10}\right) \\ &+ (1-z_{i0}) \left(1-\zeta_{i}\right) \log f\left(y_{i}|\theta_{00}\right) \end{split}$$

The parameters, $\eta = (\tau_1, \tau_0, \theta_{11}, \theta_{10}, \theta_{01}, \theta_{00})$, can be estimated using an ECM algorithm.

In fact, a separate ECM algorithm can be performed on each known pooling component of finite mixture models!

Pooling of Finite Mixture Models: Congaree River Example

Analysis based on:

- observed annual peak flows: 1892 to 2017
- two known pooled components: 1827 to 1930, 1931 to 2017

Pooling Weights (based off of 1892 to 2017 flood years):

$$\pi^{T} = \left(\frac{39}{126} = 0.310, \frac{87}{126} = 0.690\right)$$

Parameter Estimates (first pooling component):

$$\hat{\boldsymbol{\mu}}^{T} = (4.945, 5.476), \hat{\boldsymbol{\sigma}}^{T} = (0.222, 0.0440), \hat{\tau} = 0.0804$$

Parameter Estimates (second pooling component):

$$\hat{\boldsymbol{\mu}}^{T} = (4.803, 4.970), \hat{\boldsymbol{\sigma}}^{T} = (0.217, 0.00554), \hat{\tau} = 0.0595$$

Point Estimate for 1% Chance Flood: 314,424 cfs 95% CI for 1% Chance Flood: 279,633 cfs to 353,543 cfs

Reference

Eric Gilleland, Richard W. Katz (2016). "extRemes 2.0: An Extreme Value Analysis Package in R," *Journal of Statistical Software*, 72(8), 1–39. doi:10.18637/jss.v072.i08

John M. Grego, Philip A. Yates, Kaiwen Mai (2015). "Standard error estimation for mixed flood distributions with historic maxima," *Environmetrics*, 26, 229–242. doi:10.1002/env.2333

Achim Zeileis, Friedrich Leisch, Kurt Hornik, Christian Kleiber (2002). "strucchange: An R Package for Testing for Structural Change in Linear Regression Models," *Journal of Statistical Software*, 7(2), 1–38. http://www.jstatsoft.org/v07/i02/

Achim Zeileis, Christian Kleiber, Walter Kraemer, Kurt Hornik (2003). "Testing and Dating of Structural Changes in Practice," *Computational Statistics & Data Analysis*, 44, 109–123.