

# How High's The Water, Mama?

## Statistical Methods For Flood Frequency Distributions

Philip Yates

Department of Mathematics  
Saint Michael's College  
[pyates@smcvt.edu](mailto:pyates@smcvt.edu)

July 1, 2010



# Congaree River: Columbia, SC



## Congaree River: Columbia, SC



# Flood Frequency Data

## Annual Peak Flows

The maximum momentary peak discharge in each year of record

- The year of record begins the previous October and ends in September of the current year
- Peak flows are measured in cubic feet per second (cfs)

# Flood Frequency Data

## Annual Peak Flows

The maximum momentary peak discharge in each year of record

- The year of record begins the previous October and ends in September of the current year
- Peak flows are measured in cubic feet per second (cfs)

## Historical Flood Data

Any observation of flood stage or conditions made before actual flood data were collected systematically

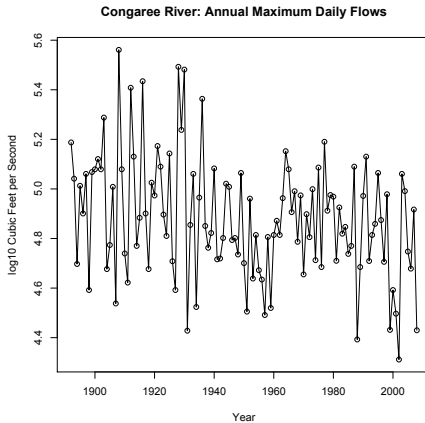
- Data collected from old newspapers, diaries, museums, libraries, etc.

# River Gage

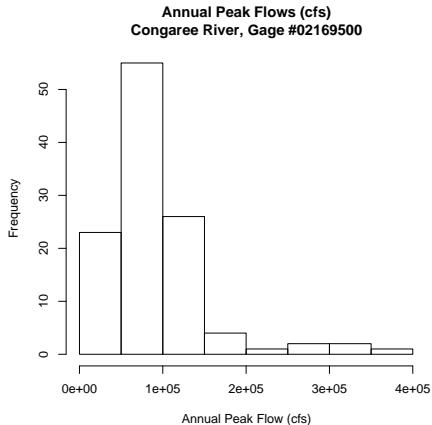


Gage #12041200: Hoh River at U.S. Highway 101 near Forks, WA

# Congaree River, Gage #02169500



# Congaree River, Gage #02169500





# 1865: Columbia, SC



- Ruins, as seen from the State House, 1865.
- General Sherman's Union troops were slowed entering Columbia by a major flood on the Congaree River.

# Modeling Flood Frequency Data

Annual peak flows are assumed to be independent and identically distributed.

Let  $Y = \log_{10} X$ , where  $X$  is the annual peak flow.

$$f_Y(y) = \frac{(y - \gamma)^{\alpha-1} \exp[-(y - \gamma)/\beta]}{\beta^\alpha \Gamma(\alpha)},$$

$\alpha > 0, \beta > 0, y > \gamma$ , where:

$\alpha$  — the shape parameter

$\beta$  — the scale parameter

$\gamma$  — the shift or location parameter

# Modeling Flood Frequency Data

## Bulletin #17B (1981)

- FEMA uses it as the trade standard

# Modeling Flood Frequency Data

## Bulletin #17B (1981)

- FEMA uses it as the trade standard
- Method of moments used to estimate parameters of log-Pearson type III distribution

# Modeling Flood Frequency Data

## Bulletin #17B (1981)

- FEMA uses it as the trade standard
- Method of moments used to estimate parameters of log-Pearson type III distribution
- When dealing with mixed populations, splits data into number of groups and fits a separate curve for each group

# Modeling Flood Frequency Data

## Bulletin #17B (1981)

- FEMA uses it as the trade standard
- Method of moments used to estimate parameters of log-Pearson type III distribution
- When dealing with mixed populations, splits data into number of groups and fits a separate curve for each group
- When collecting annual peak flow data, estimates the 99th percentile of the log-Pearson type III distribution; this is used as an estimate for the 1 percent chance — FEMA uses this in regulatory policy for floodplains

## Other Works

### American River flood frequency analyses (1999)

- Used both systematic annual peak flows and one historical flood observation from 1862

## Other Works

### American River flood frequency analyses (1999)

- Used both systematic annual peak flows and one historical flood observation from 1862
- Used EMA, the Expected Moments Algorithm (Cohn, et al., 1997) to fit the data to a log-Pearson type III distribution



## Other Works

### American River flood frequency analyses (1999)

- Used both systematic annual peak flows and one historical flood observation from 1862
- Used EMA, the Expected Moments Algorithm (Cohn, et al., 1997) to fit the data to a log-Pearson type III distribution
- Recommended the independent identically distributed approach to flood estimation

## Other Works

### American River flood frequency analyses (1999)

- Used both systematic annual peak flows and one historical flood observation from 1862
- Used EMA, the Expected Moments Algorithm (Cohn, et al., 1997) to fit the data to a log-Pearson type III distribution
- Recommended the independent identically distributed approach to flood estimation
- Tentative use of mixed models as remedy for certain types of non-stationarity in the flood data

## Other Works (Mixtures)

Singh (1987a,b) used a mixture of two normal distributions and two lognormal distributions in order to consider an observed flood that is composed of two component distributions

## Other Works (Mixtures)

- Singh (1987a,b) used a mixture of two normal distributions and two lognormal distributions in order to consider an observed flood that is composed of two component distributions
- Hirschboeck (1987) & Diehl and Potter (1987) discussed various reasons to believe that there may be two or more components to the observed flood's distribution

## Other Works (Mixtures)

- Singh (1987a,b) used a mixture of two normal distributions and two lognormal distributions in order to consider an observed flood that is composed of two component distributions
- Hirschboeck (1987) & Diehl and Potter (1987) discussed various reasons to believe that there may be two or more components to the observed flood's distribution
- in the southeastern region of US, tropical storms

## Other Works (Mixtures)

Singh (1987a,b) used a mixture of two normal distributions and two lognormal distributions in order to consider an observed flood that is composed of two component distributions

Hirschboeck (1987) & Diehl and Potter (1987) discussed various reasons to believe that there may be two or more components to the observed flood's distribution

- in the southeastern region of US, tropical storms
- in the western, and even midwestern and northeastern, regions of US, snowmelt

# Planned Work

- Investigate methods to find parameter estimates for a mixture of normals

# Planned Work

- Investigate methods to find parameter estimates for a mixture of normals
- Develop an estimate of the standard error for the 99th percentile of a finite mixture model with two components



## Planned Work

- Investigate methods to find parameter estimates for a mixture of normals
- Develop an estimate of the standard error for the 99th percentile of a finite mixture model with two components
- Use these methods on the Congaree flood data in order to find the 1 percent chance flood estimate (the 99th percentile of the fitted distribution)

## Planned Work

- Investigate methods to find parameter estimates for a mixture of normals
- Develop an estimate of the standard error for the 99th percentile of a finite mixture model with two components
- Use these methods on the Congaree flood data in order to find the 1 percent chance flood estimate (the 99th percentile of the fitted distribution)
- Compare these results to methods of standard error estimation in Bulletin #17B and Cox et. al. (2002)

## Planned Work

- Investigate methods to find parameter estimates for a mixture of normals
- Develop an estimate of the standard error for the 99th percentile of a finite mixture model with two components
- Use these methods on the Congaree flood data in order to find the 1 percent chance flood estimate (the 99th percentile of the fitted distribution)
- Compare these results to methods of standard error estimation in Bulletin #17B and Cox et. al. (2002)
- Future work and research projects

## Finite Mixture Model

Suppose  $Y$  is a random variable or vector that takes values from sample space  $\mathcal{Y}$ .

$$p(y) = \pi_1 f_1(y) + \dots + \pi_k f_k(y) \quad (y \in \mathcal{Y}),$$

where

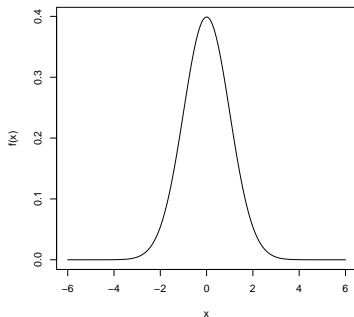
$$\pi_j > 0, \quad j = 1, \dots, k; \quad \pi_1 + \dots + \pi_k = 1$$

and

$$f_j(\cdot) \geq 0, \quad \int_{\mathcal{Y}} f_j(y) dx = 1, \quad j = 1, \dots, k,$$

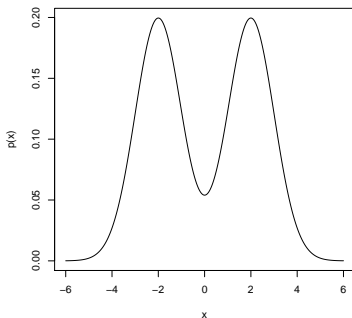
then  $Y$  has a finite mixture distribution

# Finite Mixture Model



- Standard Normal p.d.f.:  $f(x) = \frac{1}{\sqrt{2\pi}} e^{-\frac{1}{2}x^2}$

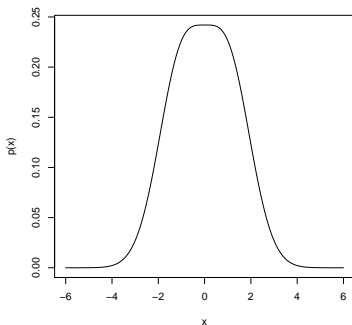
# Finite Mixture Model



- Finite Mixture Distribution:

$$p(x) = 0.5 * \frac{1}{\sqrt{2\pi}} e^{-\frac{1}{2}(x+2)^2} + 0.5 * \frac{1}{\sqrt{2\pi}} e^{-\frac{1}{2}(x-2)^2}$$

# Finite Mixture Model



- Finite Mixture Distribution:

$$p(x) = 0.5 * \frac{1}{\sqrt{2\pi}} e^{-\frac{1}{2}(x+1)^2} + 0.5 * \frac{1}{\sqrt{2\pi}} e^{-\frac{1}{2}(x-1)^2}$$

## Finite Mixture Model – Basic Concepts

### Likelihood Function

If the set of random variables  $Y_1, Y_2, \dots, Y_n$  denotes a random sample from a population with probability density (or mass) function  $f(y|\theta)$ , then

$$\begin{aligned} L(y_1, y_2, \dots, y_n|\theta) &= f(y_1, y_2, \dots, y_n|\theta) \\ &= f(y_1|\theta) \times f(y_2|\theta) \times \dots \times f(y_n|\theta), \end{aligned}$$

is the likelihood function. This will typically be denoted as  $L(\theta)$ .



## Finite Mixture Model

### Likelihood Function for Finite Mixture Model

$$L(\psi) = \prod_{i=1}^n p(y_i|\psi) = \prod_{i=1}^n \left[ \sum_{j=1}^k \pi_j f(y_i|\theta_j) \right]$$

$\pi_j f(y_i|\theta_j)$  component keeps track of  $y_i$ 's contribution to entire mixture density

If it was known which component of the mixture the observation was from, the data would be fully categorized:

## Complete Data Likelihood

$$\{x_i, i = 1, \dots, n\} = \{(y_i, \mathbf{z}_i); i = 1, \dots, n\},$$

where

$\mathbf{z}_i = (z_{ij} = 1, \dots, k)$  — indicator vector of length  $k$  with 1 in the position corresponding to the appropriate component and zeros elsewhere

$\mathbf{x}$  — complete data (not observed directly)

$\mathbf{y}$  — incomplete data ( $\mathbf{x}$  is indirectly observed via  $\mathbf{y}$ )

# Complete Data Likelihood

## Complete Data Likelihood

$$g(x_1, \dots, x_n | \psi) = \prod_{i=1}^n \prod_{j=1}^k \pi_j^{z_{ij}} f_j(y_i | \theta_j)^{z_{ij}}$$

# Complete Data Likelihood

## Complete Data Likelihood

$$g(x_1, \dots, x_n | \psi) = \prod_{i=1}^n \prod_{j=1}^k \pi_j^{z_{ij}} f_j(y_i | \theta_j)^{z_{ij}}$$

## Complete Data Log-likelihood

$$l(\psi) = \sum_{i=1}^n \mathbf{z}_i^T \mathbf{V}(\boldsymbol{\pi}) + \sum_{i=1}^n \mathbf{z}_i^T \mathbf{U}_i(\boldsymbol{\theta}),$$

where  $\log \pi_j$  is the  $j^{th}$  component of  $\mathbf{V}(\boldsymbol{\pi})$  and  $\log f_j(y_i | \theta_j)$  is the  $j^{th}$  component of  $\mathbf{U}_i(\boldsymbol{\theta})$

# Observable Log-Likelihood

Define for each  $i$  and  $j$

$$w_i = w_{ij} = E(z_i | y_i, \psi') = \frac{\pi'_j f_j(y_i | \theta'_j)}{p(y_i | \psi')}$$

## Observable Log-Likelihood

Define for each  $i$  and  $j$

$$\mathbf{w}_i = w_{ij} = E(\mathbf{z}_i | y_i, \boldsymbol{\psi}') = \frac{\pi'_j f_j(y_i | \boldsymbol{\theta}'_j)}{p(y_i | \boldsymbol{\psi}')}$$

### Observable Log-Likelihood Function

$$Q(\boldsymbol{\psi} | \boldsymbol{\psi}') = E(\log g(\mathbf{x} | \boldsymbol{\psi}) | \mathbf{y}, \boldsymbol{\psi}') = \sum_{i=1}^n \mathbf{w}_i^T \mathbf{V}(\boldsymbol{\pi}) + \sum_{i=1}^n \mathbf{w}_i^T \mathbf{U}_i(\boldsymbol{\theta})$$

## Maximum Likelihood Estimation

**Example:** Suppose  $Y_1, Y_2, \dots, Y_n$  denote a random sample from the Poisson distribution with mean  $\lambda$ .

$$f(y_i|\lambda) = \frac{\lambda^{y_i} e^{-\lambda}}{y_i!}, \quad i = 1, \dots, n$$

## Maximum Likelihood Estimation

**Example:** Suppose  $Y_1, Y_2, \dots, Y_n$  denote a random sample from the Poisson distribution with mean  $\lambda$ .

$$f(y_i|\lambda) = \frac{\lambda^{y_i} e^{-\lambda}}{y_i!}, \quad i = 1, \dots, n$$

$$L(\lambda) = \frac{\lambda^{\sum_{i=1}^n y_i} e^{-n\lambda}}{\prod_{i=1}^n y_i!}$$



## Maximum Likelihood Estimation

**Example:** Suppose  $Y_1, Y_2, \dots, Y_n$  denote a random sample from the Poisson distribution with mean  $\lambda$ .

$$f(y_i|\lambda) = \frac{\lambda^{y_i} e^{-\lambda}}{y_i!}, \quad i = 1, \dots, n$$

$$L(\lambda) = \frac{\lambda^{\sum_{i=1}^n y_i} e^{-n\lambda}}{\prod_{i=1}^n y_i!}$$

$$\log L(\lambda) = \left( \sum_{i=1}^n y_i \right) \log \lambda - n\lambda - \log \left( \prod_{i=1}^n y_i! \right)$$

## Maximum Likelihood Estimation

**Example:** Suppose  $Y_1, Y_2, \dots, Y_n$  denote a random sample from the Poisson distribution with mean  $\lambda$ .

$$f(y_i|\lambda) = \frac{\lambda^{y_i} e^{-\lambda}}{y_i!}, \quad i = 1, \dots, n$$

$$L(\lambda) = \frac{\lambda^{\sum_{i=1}^n y_i} e^{-n\lambda}}{\prod_{i=1}^n y_i!}$$

$$\log L(\lambda) = \left( \sum_{i=1}^n y_i \right) \log \lambda - n\lambda - \log \left( \prod_{i=1}^n y_i! \right)$$

$$\frac{\partial \log L(\lambda)}{\partial \lambda} = \frac{\sum_{i=1}^n y_i}{\lambda} - n$$

## Maximum Likelihood Estimation

**Example:** Suppose  $Y_1, Y_2, \dots, Y_n$  denote a random sample from the Poisson distribution with mean  $\lambda$ .

$$f(y_i|\lambda) = \frac{\lambda^{y_i} e^{-\lambda}}{y_i!}, \quad i = 1, \dots, n$$

$$L(\lambda) = \frac{\lambda^{\sum_{i=1}^n y_i} e^{-n\lambda}}{\prod_{i=1}^n y_i!}$$

$$\log L(\lambda) = \left( \sum_{i=1}^n y_i \right) \log \lambda - n\lambda - \log \left( \prod_{i=1}^n y_i! \right)$$

$$\frac{\partial \log L(\lambda)}{\partial \lambda} = \frac{\sum_{i=1}^n y_i}{\lambda} - n$$

$$\hat{\lambda} = \frac{\sum_{i=1}^n y_i}{n} = \bar{y}$$

## Mixture of Two Normals

$$f(y_i|\mu_0, \sigma_0^2, \mu_1, \sigma_1^2, \pi) = \\ (1 - \pi)\phi\left(\frac{y_i - \mu_0}{\sqrt{\sigma_0^2}}\right) + \pi\phi\left(\frac{y_i - \mu_1}{\sqrt{\sigma_1^2}}\right),$$

To implement ECM (Meng & Rubin, 1993) algorithm, let

$$w_i = \frac{\pi\phi\left(\frac{y_i - \mu_1}{\sqrt{\sigma_1^2}}\right)}{(1 - \pi)\phi\left(\frac{y_i - \mu_0}{\sqrt{\sigma_0^2}}\right) + \pi\phi\left(\frac{y_i - \mu_1}{\sqrt{\sigma_1^2}}\right)}.$$

# Mixture of Two Normals

## E-step

$$\begin{aligned} Q(\psi|\psi^{(p)}) = & \sum_{i=1}^n w_i^{(p)} \log(\pi) + \\ & (1 - w_i^{(p)}) \log(1 - \pi) + w_i^{(p)} \log \phi \left( \frac{y_i - \mu_1}{\sqrt{\sigma_1^2}} \right) + \\ & (1 - w_i^{(p)}) \log \phi \left( \frac{y_i - \mu_0}{\sqrt{\sigma_0^2}} \right). \end{aligned}$$

# Mixture of Two Normals

## CM-steps

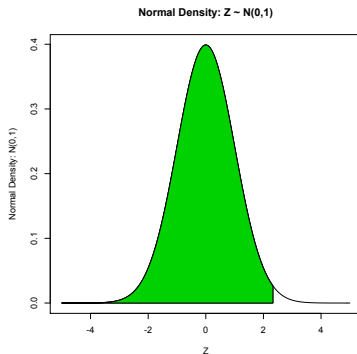
$$\begin{aligned}\pi^{(p+1)} &= \frac{1}{n} \sum_{i=1}^n w_i^{(p)} \\ \mu_0^{(p+1)} &= \frac{\sum_{i=1}^n (1 - w_i^{(p)}) y_i}{\sum_{i=1}^n (1 - w_i^{(p)})} \\ \mu_1^{(p+1)} &= \frac{\sum_{i=1}^n w_i^{(p)} y_i}{\sum_{i=1}^n w_i^{(p)}}\end{aligned}$$

# Mixture of Two Normals

## CM-steps

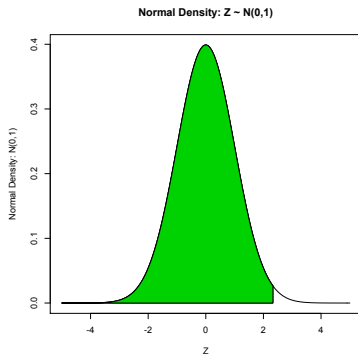
$$\sigma_0^{2(p+1)} = \frac{\sum_{i=1}^n (1 - w_i^{(p)}) (y_i - \mu_0^{(p+1)})^2}{\sum_{i=1}^n (1 - w_i^{(p)})}$$
$$\sigma_1^{2(p+1)} = \frac{\sum_{i=1}^n w_i^{(p)} (y_i - \mu_1^{(p+1)})^2}{\sum_{i=1}^n w_i^{(p)}}$$

# 1% Chance Flood Estimation





# 1% Chance Flood Estimation



The 99th percentile of the standard normal distribution is  
 $\Phi^{-1}(0.99) = 2.326$ .

## 1% Chance Flood Estimation

To find the 1 percent chance flood when the distribution is from a finite mixture model, one needs to find the 99th percentile of the finite mixture distribution. For example, if the distribution was a finite mixture model of normal densities, then the 100-year flood,  $Q$ , is

$$\int_{-\infty}^{\log_{10} Q} (1 - \pi)f_0(y|\mu_0, \sigma_0^2) + \pi f_1(y|\mu_1, \sigma_1^2) dy = 0.99,$$

where  $f_j(y) \sim N(\mu_j, \sigma_j^2)$  and  $\log_{10} Q$  is the logged value of the 1 percent chance flood.

To find  $\log_{10} \hat{Q}$ , first obtain the 99th percentile from each component of the mixing distribution.

## 1% Chance Flood Estimation

These percentiles from each mixing component are used as end points to search for the root of

$$\int_{-\infty}^{\log_{10} \hat{Q}} (1 - \pi) f_0(y | \mu_0, \sigma_0^2) + \pi f_1(y | \mu_1, \sigma_1^2) dy - 0.99 = 0.$$

In order to obtain a  $100(1 - \alpha)\%$  confidence interval for  $\log_{10} Q$  in this example, a delta method argument can be made to find the standard error of  $\log_{10} Q$

# 1% Chance Flood Estimation – Basic Terms

## Example: Total Derivative of a Function

The total derivative of  $f(t, x, y)$  with respect to  $t$  is

$$\frac{df}{dt} = \frac{\partial f}{\partial t} + \frac{\partial f}{\partial x} \frac{dx}{dt} + \frac{\partial f}{\partial y} \frac{dy}{dt}.$$

# 1% Chance Flood Estimation – Basic Terms

## Example: Total Derivative of a Function

The total derivative of  $f(t, x, y)$  with respect to  $t$  is

$$\frac{df}{dt} = \frac{\partial f}{\partial t} + \frac{\partial f}{\partial x} \frac{dx}{dt} + \frac{\partial f}{\partial y} \frac{dy}{dt}.$$

## Delta Method

For some sequence of random variables  $X_n$  satisfying

$$\sqrt{n}[X_n - \theta] \xrightarrow{\mathcal{D}} N(0, \sigma^2)$$

then

$$\sqrt{n}[g(X_n) - g(\theta)] \xrightarrow{\mathcal{D}} N(0, \sigma^2[g'(\theta)]^2).$$

## 1% Chance Flood Estimation

Define the quantile function for a finite mixture model with two normal densities as:

$$\begin{aligned}\Gamma(\boldsymbol{\theta}, x(\boldsymbol{\theta}, 0.99)) &= \int_{-\infty}^x \left\{ \tau \frac{1}{\sqrt{2\pi}\sigma_1} e^{-\frac{1}{2\sigma_1^2}(y-\mu_1)^2} + (1-\tau) \frac{1}{\sqrt{2\pi}\sigma_0} e^{-\frac{1}{2\sigma_0^2}(y-\mu_0)^2} \right\} dy \\ &= 0.99,\end{aligned}$$

with the chain rule yielding

$$\nabla_{\boldsymbol{\tau}} x(\boldsymbol{\theta}, 0.99) = - \frac{\nabla_{\boldsymbol{\tau}} \Gamma(\boldsymbol{\theta}, x)|_{x=x(\boldsymbol{\theta}, 0.99)}}{\nabla_x \Gamma(\boldsymbol{\theta}, x)|_{x=x(\boldsymbol{\theta}, 0.99)}} \quad \text{Uryasev (2000),}$$

where  $\boldsymbol{\theta}^T = (\mu_0, \sigma_0^2, \mu_1, \sigma_1^2, \tau)^T$ .

## 1% Chance Flood Estimation

The numerator of  $\nabla_{\tau} x(\boldsymbol{\theta}, 0.99)$  is,

$$\begin{aligned}\nabla_{\tau} \Gamma(\boldsymbol{\theta}, x)|_{x=x(\boldsymbol{\theta}, 0.99)} &= \int_{-\infty}^x \left\{ \frac{1}{\sqrt{2\pi}\sigma_1} e^{-\frac{1}{2\sigma_1^2}(y-\mu_1)^2} - \frac{1}{\sqrt{2\pi}\sigma_0} e^{-\frac{1}{2\sigma_0^2}(y-\mu_0)^2} \right\} dy \\ &= \int_{-\infty}^x \frac{1}{\sqrt{2\pi}\sigma_1} e^{-\frac{1}{2\sigma_1^2}(y-\mu_1)^2} dy - \int_{-\infty}^x \frac{1}{\sqrt{2\pi}\sigma_0} e^{-\frac{1}{2\sigma_0^2}(y-\mu_0)^2} dy \\ &= \Phi\left(\frac{x(\boldsymbol{\theta}, 0.99) - \mu_1}{\sigma_1}\right) - \Phi\left(\frac{x(\boldsymbol{\theta}, 0.99) - \mu_0}{\sigma_0}\right),\end{aligned}$$

where  $\Phi(\cdot)$  is the cumulative distribution function for a standard normal probability distribution.

## 1% Chance Flood Estimation

The numerator for the gradient of this quantile function with respect to  $\mu_0$  and  $\mu_1$  is

$$\begin{aligned}\nabla_{\mu_0} \Gamma(\boldsymbol{\theta}, x) \big|_{x=x(\boldsymbol{\theta}, 0.99)} &= \int_{-\infty}^x \frac{(1 - \tau)(y - \mu_0)}{\sqrt{2\pi}\sigma_0^3} e^{-\frac{1}{2\sigma_0^2}(y - \mu_0)^2} dy \\ \nabla_{\mu_1} \Gamma(\boldsymbol{\theta}, x) \big|_{x=x(\boldsymbol{\theta}, 0.99)} &= \int_{-\infty}^x \frac{\tau(y - \mu_1)}{\sqrt{2\pi}\sigma_1^3} e^{-\frac{1}{2\sigma_1^2}(y - \mu_1)^2} dy\end{aligned}$$



# 1% Chance Flood Estimation

The numerator for the gradient of this quantile function with respect to  $\sigma_0^2$  and  $\sigma_1^2$  is

$$\begin{aligned}\nabla_{\sigma_0^2} \Gamma(\boldsymbol{\theta}, x)|_{x=x(\boldsymbol{\theta})} &= \int_{-\infty}^x \left\{ \frac{1-\tau}{2\sqrt{2\pi}\sigma_0^3} e^{-\frac{1}{2\sigma_0^2}(y-\mu_0)^2} \left( \frac{(y-\mu_0)^2}{\sigma_0^2} - 1 \right) \right\} dy \\ \nabla_{\sigma_1^2} \Gamma(\boldsymbol{\theta}, x)|_{x=x(\boldsymbol{\theta})} &= \int_{-\infty}^x \left\{ \frac{\tau}{2\sqrt{2\pi}\sigma_1^3} e^{-\frac{1}{2\sigma_1^2}(y-\mu_1)^2} \left( \frac{(y-\mu_1)^2}{\sigma_1^2} - 1 \right) \right\} dy.\end{aligned}$$

## 1% Chance Flood Estimation

The denominator for the gradient of this quantile function with respect to any of the parameters in  $\theta$  is,

$$\begin{aligned}\nabla_x \Gamma(\theta, x)|_{x=x(\theta, 0.99)} &= p(x(\tau, 0.99)) \\ &= \tau \frac{1}{\sqrt{2\pi}\sigma_1} e^{-\frac{1}{2\sigma_1^2}(x(\theta, 0.99) - \mu_1)^2} \\ &\quad + (1 - \tau) \frac{1}{\sqrt{2\pi}\sigma_0} e^{-\frac{1}{2\sigma_0^2}(x(\theta, 0.99) - \mu_0)^2}.\end{aligned}$$

# 1% Chance Flood Estimation

## Confidence Interval

Estimate  $\pm$  Confidence Level  $\times$  Standard Error

# 1% Chance Flood Estimation

## Confidence Interval

Estimate  $\pm$  Confidence Level  $\times$  Standard Error

## 100(1 - $\alpha$ )% Confidence Interval for $\log_{10} Q$

$$\log_{10} \hat{Q} \pm z_{\frac{\alpha}{2}} \left( \nabla \log_{10} \hat{Q}^T I_Y(\hat{\psi})^{-1} \nabla \log_{10} \hat{Q} \right)^{1/2},$$

where  $I_Y$  is the observed information matrix.

# Observed Information Matrix

## Observed Information Matrix

$$\begin{aligned} I_Y(\psi) = & E(\mathbf{B}(\mathbf{x}|\psi)|\mathbf{y}, \psi) \\ & - E(\mathbf{S}(\mathbf{x}|\psi)\mathbf{S}(\mathbf{x}|\psi)^T|\mathbf{y}, \psi) + \mathbf{S}^*(\mathbf{y}|\psi)\mathbf{S}^*(\mathbf{y}|\psi)^T. \end{aligned}$$

## Observed Information Matrix

### Observed Information Matrix

$$\mathbf{I}_Y(\psi) = E(\mathbf{B}(\mathbf{x}|\psi)|\mathbf{y}, \psi) \\ - E(\mathbf{S}(\mathbf{x}|\psi)\mathbf{S}(\mathbf{x}|\psi)^T|\mathbf{y}, \psi) + \mathbf{S}^*(\mathbf{y}|\psi)\mathbf{S}^*(\mathbf{y}|\psi)^T.$$

This is the conditional expected complete data information matrix minus the expected information matrix for the conditional distribution of the complete data,  $\mathbf{x}$ , given the incomplete data,  $\mathbf{y}$ , and  $\psi$ .

## Congaree River

### Bulletin #17B

1 percent chance flood is estimated by

$$\log_{10} \hat{Q} = \bar{y} + K_{.01}s$$

where  $K_{.01}$  is a Pearson Type III deviate

For the Congaree River, Gage #02169500:

$$\log_{10} Q = \bar{y} + K_{.01}s = 4.884468 + 2.541867 \times 0.241563 = 5.498489$$

This gives a point estimate for the 1 percent chance flood,  $Q$ , of 315,129 cfs

## Congaree River

Confidence intervals in Bulletin #17B approximate a noncentral  $t$ -distribution. For ease of computation, the following formulas are suggested as a large sample approximation to the noncentral  $t$ -distribution:

$$K_{.01, \frac{\alpha}{2}}^U = \frac{K_{.01} + \sqrt{K_{.01}^2 - ab}}{a}$$
$$K_{.01, \frac{\alpha}{2}}^L = \frac{K_{.01} - \sqrt{K_{.01}^2 - ab}}{a}$$

where

$$a = 1 - \frac{z_{\frac{\alpha}{2}}^2}{2(n-1)}$$
$$b = K_{.01}^2 - \frac{z_{\frac{\alpha}{2}}^2}{n}$$



## Congaree River

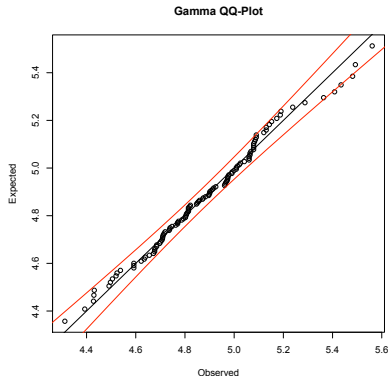
To obtain the 90% confidence interval of the 1 percent chance flood for the Congaree River at gage #02169500, first use  $z_{0.05} = 1.645$  to get  $K_{.01,.05}^L = 2.252476$  and  $K_{.01,.05}^U = 2.892320$ .

$$\log_{10} Q^L = \bar{y} + K_{.01,.05}^L s = 5.428582$$

$$\log_{10} Q^U = \bar{y} + K_{.01,.05}^U s = 5.583145.$$

With a confidence level of 90%, the 1 percent chance flood for the Congaree River at gage #02169500 is between 268,276 cfs and 382,953 cfs.

## Congaree River



Toward the upper tail of the distribution, the MOMs seem to be fitting the  $\log_{10}$  of the Congaree River's annual peak flows well

# Congaree River

## Gumbel Probability Density Function

$$f_Y(y|\mu, \beta) = \frac{e^{-(y-\mu)/\beta} e^{-e^{-(y-\mu)/\beta}}}{\beta},$$

$$-\infty < \mu < \infty; \beta > 0; -\infty < y < \infty$$

$\mu$  is the location parameter;  $\beta$  is the scale parameter.

The Gumbel distribution is used to find the maximum (or minimum) of a number of samples of various distribution.

## Congaree River

Using a Gumbel distribution on the non-transformed annual peak flood flows:

Gumbel MLE's  $\hat{\mu} = 67050.89$ ,  $\hat{\beta} = 35747.7$

Point Estimate The 99th percentile is 231,496 cfs.

To find the standard error for a quantile from a Gumbel pdf, Cox, Isham, & Northrop (2002) suggest:

$$nV(\hat{Q}) = \beta^2 \left\{ 1 + \frac{(r_p + \psi(2))^2}{1 + \psi'(2)} \right\},$$

where  $r_p = -\ln\{-\ln(1-p)\}$  and  $\psi(\cdot)$  is the digamma function.

## Congaree River

Using a Gumbel distribution on the non-transformed annual peak flood flows:

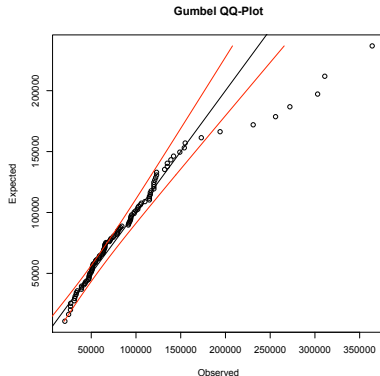
Gumbel MLE's  $\hat{\mu} = 66,517.37$ ,  $\hat{\beta} = 35,715.94$

Point Estimate The 99th percentile is 230,816 cfs.

Standard Error Estimate 13,347 cfs.

90% CI Q: 208,861 to 252,771 cfs.

## Congaree River



The QQ-plot indicates that a Gumbel does not seem to handle the large flood events very well.

## Congaree River

### Generalized Extreme Value (GEV) Distribution

The cumulative distribution function for a GEV distribution is

$$P(X \leq x) = F(x) = \exp \left\{ - \left[ 1 + \xi \left( \frac{x - \mu}{\sigma} \right) \right]^{-1/\xi} \right\},$$

where  $\mu$  is the location parameter,  $\sigma$  is the scale parameter, and  $\xi$  is the shape parameter.

## Congaree River

GEV MLE's  $\hat{\mu} = 61,924.87$ ,  $\hat{\sigma} = 31,198.16$ ,  $\hat{\xi} = 0.2482148$

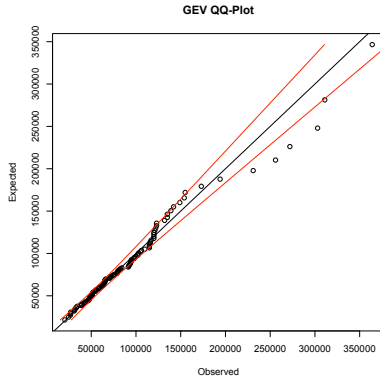
Point Estimate The 99th percentile is 329,957 cfs

Standard Error Estimate From a delta method argument in Grundstein, Lu, and Lund (2006): 17,283.91 cfs

90% CI Q: 301,525 to 358,369 cfs



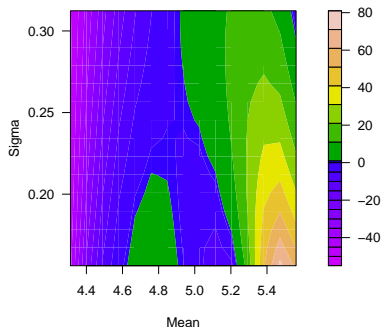
## Congaree River



The QQ-plot indicates that a GEV does not handle some of the larger flood events very well.

## Congaree River

Contour plot of gradient function



The results here suggest two mixing components are needed for the  $\log_{10}$  of the Congaree River's annual peak flows

## Congaree River

Mixture of Normals  $\hat{\mu}_0 = 4.859969$ ,  $\hat{\sigma}_0^2 = 0.0451328$ ,  $\hat{\mu}_1 = 5.471577$ ,  
 $\hat{\sigma}_1^2 = 0.00341794$ ,  $\hat{\pi} = 0.04005613$

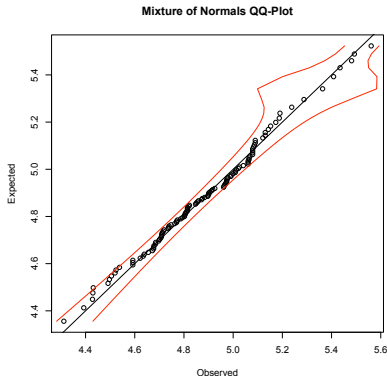
Point Estimate  $\log_{10} \hat{Q} = 5.515678$ ;  $\hat{Q} = 327,852$  cfs

Standard Error Estimate 0.03543783

90% CI  $Q$ : 286,671 cfs to 374,950 cfs

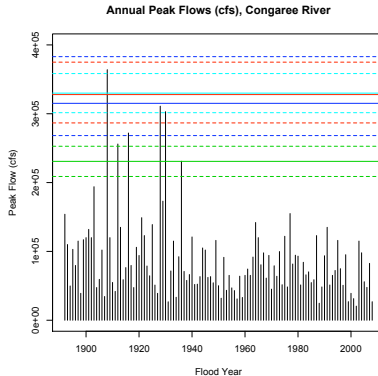
This interval for the 1 percent chance flood is smaller than the one produced by Bulletin #17B.

## Congaree River



The QQ-plot indicates that a mixture of normals handles observations of the lower and upper tails of the flood distribution better than Bulletin #17B

# Congaree River



The point estimate and 90% CI of  $Q$  using the finite mixture model (red), Gumble (green), Bulletin #17B (blue), and GEV (light blue).

## Recommendations

Use the gradient contour plot developed by Lindsay (1983) to decide whether or not additional components need to be added to a finite mixing distribution

If the plot indicates that additional components are needed, then fit the  $\log_{10}$  of the annual peak flood flows to a mixture of two normal distributions

If additional components are not needed, proceed with Bulletin #17B in order to obtain that point estimate and confidence interval for the annual peak flood flows

## Future Research

How would short-term dependence affect the estimation of the 1 percent chance flood? The 1929 (March 1, 1929) and 1930 (October 3, 1929) flood years could pose a problem for the Congaree River.

Incorporating historical floods into the flood distribution as censored observations, and use these historical floods in an EM or ECM algorithm to obtain MLEs of the annual peak flood flows. Extend the use of historical floods into the finite mixture model.

Pre-dam analysis versus post-dam analysis: Dreher Shoals Dam (a.k.a. Lake Murray Dam) built in 1930. Built on the Saluda River, it lies about approximately 10 miles west of where the Broad River and Saluda River meet to form the Congaree River in Columbia.

## Future Collaboration – Undergraduate

### Possible Undergraduate Research Projects:

- Flood frequency analysis on the Winooski River – Two gauges still in use
  - Gage #04282850 – Montpelier, VT (1912 – 2008)
  - Gage #04290500 – near Essex Junction, VT (1927 – 2008)
- Fit a finite mixture of Gumbel distributions and find the standard error for the 1% chance flood
- Fit a finite mixture model where the components are from two different distributions?
  - Gamma and Normal?